# CONSTRUCTION CALCULATIONS MANUAL 



SIDNEY M. LEVY

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## Introduction

Construction Calculations provides the construction, engineering, and project owner community with a single source guide for many of the formulas and conversion factors that are frequently encountered during the design and construction phase of a project.

The geometry and trigonometry lessons learned years ago sometimes need refreshing. Construction Calculations provides a refresher course on some of the formulas and concepts that tend to crop up from time to time.

A book divided into sections devoted to most of the common components of construction makes it easier to determine how to achieve a Sound Transmission Coefficient (STC) rating of 50, for example, or how to equate the amperage capacity of copper and aluminum cable of the same wire size.

A detailed index preceding each section makes it easy to locate the answer to one's question or at least points the way to its solution.

This one-source volume can prove invaluable for office- or field-based designers and contractors and will come in handy at project and design development meetings as well as provide assistance in specifying and purchasing materials and equipment.

I have selected material that in my 40-some years in the construction business appears relevant to the many situations where answers to questions are required, and required "yesterday."

I hope you will find Construction Calculations a worthy addition to your professional library.

Sidney M. Levy

# The National Institute of Standards and Testing (NIST) 

NIST Handbook 44-2007 Edition

1. Introduction
2. Units and Systems of Measurement
3. Standards of Length, Mass, and Capacity or Volume
4. Specialized Use of the Terms "Ton" and "Tonnage"

2 1. Tables of Metric Units of Measurement 14
3 2. Tables of U.S. Units of Measurement 16
4 3. Notes on British Units of Measurement 18
4. Tables of Units of Measurement 19
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A book on construction calculations that includes references to material dimensions, weight, volume, and conversion factors should introduce the reader to the National Institute of Standards and Testing, generally referred to simply as NIST.

Founded in 1901 under the U.S. Department of Agriculture, NIST is a nonregulatory federal agency whose mission is to "promote U.S. innovation and industrial competitiveness by advancing measurement science, standards and technology in ways that enhance economic security and improve our quality of life."

NIST maintains four cooperative programs to carry out its mission:

- NIST Laboratories, headquartered in Gaithersburg, Maryland, and a campus in Boulder, Colorado, to research and advance U.S. technology infrastructure.
- The Baldrige National Quality Program to promote excellence in the performance of manufacturing, service, educational and health care industries recognizing excellence in those organizations with its highly prized annual Malcolm Baldrige Award.
- The Hollings Manufacturing Extension Partnership consisting of a nationwide network of local centers that offer technical and business assistance to small manufacturers
- The Technology Innovations Program providing cost-shared awards to industry, academia, and key organizations that meet national and societal needs.

The NIST Handbook 44 was first published in 1949 and is issued yearly at the Annual Meeting of the National Conference on Weights and Measures.

The Table of Contents of Handbook 44 reflects the type of information contained in this volume, the contents of which hold much value for the design and construction industry. Copies of the entire handbook can be downloaded from the NIST website.

I have chosen to include only Appendix B: Units and Systems of Measurement—Their Origin, Development, and Present Status, and Appendix C: General Tables of Units of Measurement, which seem to be a fitting start to a book on construction calculations.

## NIST Handbook 44-2007 Edition

## Specifications, Tolerances, and Other Technical Requirements for Weighing and Measuring Devices

as adopted by the 91st National Conference on Weights and Measures 2006

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## APPENDIX B: UNITS AND SYSTEMS OF MEASUREMENT: THEIR ORIGIN, DEVELOPMENT, AND PRESENT STATUS

## 1. Introduction

The National Institute of Standards and Technology (NIST) (formerly the National Bureau of Standards) was established by Act of Congress in 1901 to serve as a national scientific laboratory in the physical sciences, and to provide fundamental measurement standards for science and industry. In carrying out these related functions, the Institute conducts research and development in many fields of physics, mathematics, chemistry, and engineering. At the time of its founding, the Institute had custody of two primary standards-the meter bar for length and the kilogram cylinder for mass. With the phenomenal growth of science and technology over the past century, the Institute has become a major research institution concerned not only with everyday weights and measures, but also with hundreds of other scientific and engineering standards that are necessary to the industrial progress of the nation. Nevertheless, the country still looks to NIST for information on the units of measurement, particularly their definitions and equivalents.

The subject of measurement systems and units can be treated from several different standpoints. Scientists and engineers are interested in the methods by which precision measurements are made. State weights and measures officials are concerned with laws and regulations that assure equity in the marketplace, protect public health and safety, and with methods for verifying commercial weighing and measuring devices. But a vastly larger group of people is interested in some general knowledge of the origin and development of measurement systems, of the present status of units and standards, and of miscellaneous facts that will be useful in everyday life.

This material has been prepared to supply that information on measurement systems and units that experience has shown to be the common subject of inquiry.

## 2. Units and Systems of Measurement

The expression "weights and measures" is often used to refer to measurements of length, mass, and capacity or volume, thus excluding such quantities as electrical and time measurements and thermometry. This section on units and measurement systems presents some fundamental information to clarify the concepts of this subject and to eliminate erroneous and misleading use of terms.

It is essential that the distinction between the terms "units" and "standards" be established and kept in mind.
A unit is a special quantity in terms of which other quantities are expressed. In general, a unit is fixed by definition and is independent of such physical conditions as temperature. Examples: the meter, the liter, the gram, the yard, the pound, the gallon.

A standard is a physical realization or representation of a unit. In general, it is not entirely independent of physical conditions, and it is a representation of the unit only under specified conditions. For example, a meter standard has a length of one meter when at some definite temperature and supported in a certain manner. If supported in a different manner, it might have to be at a different temperature to have a length of one meter.

### 2.1 Origin and Early History of Units and Standards

### 2.1.1 General Survey of Early History of Measurement Systems

Weights and measures were among the earliest tools invented by man. Primitive societies needed rudimentary measures for many tasks: constructing dwellings of an appropriate size and shape, fashioning clothing, or bartering food or raw materials.

Man understandably turned first to parts of the body and the natural surroundings for measuring instruments. Early Babylonian and Egyptian records and the Bible indicate that length was first measured with the forearm, hand, or finger and that time was measured by the periods of the sun, moon, and other heavenly bodies. When it was necessary to compare the capacities of containers such as gourds or clay or metal vessels, they were filled with plant seeds which were then counted to measure the volumes. When means for weighing were invented, seeds and stones served as standards. For instance, the "carat," still used as a unit for gems, was derived from the carob seed.

Our present knowledge of early weights and measures comes from many sources. Archaeologists have recovered some rather early standards and preserved them in museums. The comparison of the dimensions of buildings with the descriptions of contemporary writers is another source of information. An interesting example of this is the comparison of the dimensions of the Greek Parthenon with the description given by Plutarch from which a fairly accurate idea of the size of the Attic foot is obtained. In some cases, we have only plausible theories and we must sometimes select the interpretation to be given to the evidence.

For example, does the fact that the length of the double-cubit of early Babylonia was equal (within two parts per thousand) to the length of the seconds pendulum at Babylon suggest a scientific knowledge of the pendulum at a very early date, or do we merely have a curious coincidence? By studying the evidence given by all available sources, and by correlating the relevant facts, we obtain some idea of the origin and development of the units. We find that they have changed more or less gradually with the passing of time in a complex manner because of a great variety of modifying influences. We find the units modified and grouped into measurement systems: The Babylonian system, the Egyptian system, the Phileterian system of the Ptolemaic age, the Olympic system of Greece, the Roman system, and the British system, to mention only a few.

### 2.1.2 Origin and Development of Some Common Customary Units

The origin and development of units of measurement has been investigated in considerable detail and a number of books have been written on the subject. It is only possible to give here, somewhat sketchily, the story about a few units.

Units of length: The cubit was the first recorded unit used by ancient peoples to measure length. There were several cubits of different magnitudes that were used. The common cubit was the length of the forearm from the elbow to the tip of the middle finger. It was divided into the span of the hand (one-half cubit), the palm or width of the hand (one sixth), and the digit or width of a finger (one twenty-fourth). The Royal or Sacred Cubit, which was 7 palms or 28 digits long, was used in constructing buildings and monuments and in surveying. The inch, foot, and yard evolved from these units through a complicated transformation not yet fully understood. Some believe they evolved from cubic measures; others believe they were simple proportions or multiples of the cubit. In any case, the Greeks and Romans inherited the foot from the Egyptians. The Roman foot was divided into both 12 unciae (inches) and 16 digits. The Romans also introduced the mile of 1000 paces or double steps, the pace being equal to five Roman feet. The Roman mile of 5000 feet was introduced into England during the occupation. Queen Elizabeth, who reigned from 1558 to 1603 , changed, by statute, the mile to 5280 feet or 8 furlongs, a furlong being 40 rods of $5 \frac{1}{2}$ yards each.

The introduction of the yard as a unit of length came later, but its origin is not definitely known. Some believe the origin was the double cubit, others believe that it originated from cubic measure. Whatever its origin, the early yard was divided by the binary method into $2,4,8$, and 16 parts called the half-yard, span, finger, and nail. The association of the yard with the "gird" or circumference of a person's waist or with the distance from the tip of the nose to the end of the thumb of Henry I are probably standardizing actions, since several yards were in use in Great Britain.

The point, which is a unit for measuring print type, is recent. It originated with Pierre Simon Fournier in 1737. It was modified and developed by the Didot brothers, Francois Ambroise and Pierre Francois, in 1755. The point was first used in the United States in 1878 by a Chicago type foundry (Marder, Luse, and Company). Since 1886, a point has been exactly 0.3514598 millimeters, or about $1 / 22$ inch.

Units of mass: The grain was the earliest unit of mass and is the smallest unit in the apothecary, avoirdupois, Tower, and Troy systems. The early unit was a grain of wheat or barleycorn used to weigh the precious metals silver and gold. Larger units preserved in stone standards were developed that were used as both units of mass and of monetary currency. The pound was derived from the mina used by ancient civilizations. A smaller unit was the shekel, and a larger unit was the talent. The magnitude of these units varied from place to place. The Babylonians and Sumerians had a system in which there were 60 shekels in a mina and 60 minas in a talent. The Roman talent consisted of 100 libra (pound) which were smaller in magnitude than the mina. The Troy pound used in England and the United States for monetary purposes, like the Roman pound, was divided into 12 ounces, but the Roman uncia (ounce) was smaller. The carat is a unit for measuring gemstones that had its origin in the carob seed, which later was standardized at $1 / 44$ ounce and then 0.2 gram.

Goods of commerce were originally traded by number or volume. When weighing of goods began, units of mass based on a volume of grain or water were developed. For example, the talent in some places was approximately equal to the mass of one cubic foot of water. Was this a coincidence or by design? The diverse magnitudes of units having the same name, which still appear today in our dry and liquid measures, could have arisen from the various commodities traded. The larger avoirdupois pound for goods of commerce might have been based on volume of water which has a higher bulk density than grain. For example, the Egyptian hon was a volume unit about $11 \%$ larger than a cubic palm and corresponded to one mina of water. It was almost identical in volume to the present U.S. pint.

The stone, quarter, hundredweight, and ton were larger units of mass used in Great Britain. Today only the stone continues in customary use for measuring personal body weight. The present stone is 14 pounds, but an earlier unit appears to have been 16 pounds. The other units were multiples of 2,8 , and 160 times the stone, or 28,

112, and 2240 pounds, respectively. The hundredweight was approximately equal to two talents. In the United States the ton of 2240 pounds is called the "long ton." The "short ton" is equal to 2000 pounds.

Units of time and angle: We can trace the division of the circle into 360 degrees and the day into hours, minutes, and seconds to the Babylonians who had a sexagesimal system of numbers. The 360 degrees may have been related to a year of 360 days.

### 2.2 The Metric System

### 2.2.1 Definition, Origin, and Development

Metric systems of units have evolved since the adoption of the first well defined system in France in 1791. During this evolution the use of these systems spread throughout the world, first to the non-English speaking countries, and more recently to the English speaking countries. The first metric system was based on the centimeter, gram, and second (cgs), and these units were particularly convenient in science and technology. Later metric systems were based on the meter, kilogram, and second (mks) to improve the value of the units for practical applications. The present metric system is the International System of Units (SI). It is also based on the meter, kilogram, and second as well as additional base units for temperature, electric current, luminous intensity, and amount of substance. The International System of Units is referred to as the modern metric system.

The adoption of the metric system in France was slow, but its desirability as an international system was recognized by geodesists and others. On May 20, 1875, an international treaty known as the International Metric Convention or the Treaty of the Meter was signed by seventeen countries including the United States. This treaty established the following organizations to conduct international activities relating to a uniform system for measurements:
(1) The General Conference on Weights and Measures (French initials: CGPM), an intergovernmental conference of official delegates of member nations and the supreme authority for all actions;
(2) The International Committee of Weights and Measures (French initials: CIPM), consisting of selected scientists and metrologists, which prepares and executes the decisions of the CGPM and is responsible for the supervision of the International Bureau of Weights and Measures;
(3) The International Bureau of Weights and Measures (French initials: BIPM), a permanent laboratory and world center of scientific metrology, the activities of which include the establishment of the basic standards and scales of the principal physical quantities and maintenance of the international prototype standards.

The National Institute of Standards and Technology provides official United States representation in these organizations. The CGPM, the CIPM, and the BIPM have been major factors in the continuing refinement of the metric system on a scientific basis and in the evolution of the International System of Units.

Multiples and submultiples of metric units are related by powers of ten. This relationship is compatible with the decimal system of numbers, and it contributes greatly to the convenience of metric units.

### 2.2.2 International System of Units

At the end of World War II, a number of different systems of measurement still existed throughout the world. Some of these systems were variations of the metric system, and others were based on the customary inch-pound system of the English-speaking countries. It was recognized that additional steps were needed to promote a worldwide measurement system. As a result the 9th GCPM, in 1948, asked the ICPM to conduct an international study of the measurement needs of the scientific, technical, and educational communities. Based on the findings of this study, the 10th General Conference in 1954 decided that an international system should be derived from six base units to provide for the measurement of temperature and optical radiation in addition to mechanical and
electromagnetic quantities. The six base units recommended were the meter, kilogram, second, ampere, Kelvin degree (later renamed the kelvin), and the candela.

In 1960, the 11th General Conference of Weights and Measures named the system based on the six base quantities of the International System of Units, abbreviated SI from the French name: Le Système International d'Unités. The SI metric system is now either obligatory or permissible throughout the world.

### 2.2.3 Units and Standards of the Metric System

In the early metric system there were two fundamental or base units, the meter and the kilogram, for length and mass. The other units of length and mass, and all units of area, volume, and compound units such as density were derived from these two fundamental units.

The meter was originally intended to be one ten-millionth part of a meridional quadrant of the earth. The Meter of the Archives, the platinum length standard which was the standard for most of the 19th century, at first was supposed to be exactly this fractional part of the quadrant. More refined measurements over the earth's surface showed that this supposition was not correct. In 1889, a new international metric standard of length, the International Prototype Meter, a graduated line standard of platinum-iridium, was selected from a group of bars because precise measurements found it to have the same length as the Meter of the Archives. The meter was then defined as the distance, under specified conditions, between the lines on the International Prototype Meter without reference to any measurements of the earth or to the Meter of the Archives, which it superseded. Advances in science and technology have made it possible to improve the definition of the meter and reduce the uncertainties associated with artifacts. From 1960 to 1983, the meter was defined as the length equal to 1650763.73 wavelengths in a vacuum of the radiation corresponding to the transition between the specified energy levels of the krypton 86 atom. Since 1983 the meter has been defined as the length of the path traveled by light in a vacuum during an interval of $1 / 29979458$ of a second.

The kilogram, originally defined as the mass of one cubic decimeter of water at the temperature of maximum density, was known as the Kilogram of the Archives. It was replaced after the International Metric Convention in 1875 by the International Prototype Kilogram which became the unit of mass without reference to the mass of a cubic decimeter of water or to the Kilogram of the Archives. Each country that subscribed to the International Metric Convention was assigned one or more copies of the international standards; these are known as National Prototype Meters and Kilograms.

The liter is a unit of capacity or volume. In 1964, the 12th GCPM redefined the liter as being one cubic decimeter. By its previous definition-the volume occupied, under standard conditions, by a quantity of pure water having a mass of one kilogram-the liter was larger than the cubic decimeter by 28 parts per 1000000 . Except for determinations of high precision, this difference is so small as to be of no consequence.

The modern metric system (SI) includes two classes of units:
base units for length, mass, time, temperature, electric current, luminous intensity, and amount of substance; and
derived units for all other quantities (e.g., work, force, power) expressed in terms of the seven base units.
For details, see NIST Special Publication 330 (2001), The International System of Units (SI) and NIST Special Publication 811 (1995), Guide for the Use of the International System of Units.

### 2.2.4 International Bureau of Weights and Measures

The International Bureau of Weights and Measures (BIPM) was established at Sèvres, a suburb of Paris, France, by the International Metric Convention of May 20, 1875. The BIPM maintains the International Prototype Kilogram, many secondary standards, and equipment for comparing standards and making precision measurements. The Bureau, funded by assessment of the signatory governments, is truly international. In recent years the scope of the work at the Bureau has been considerably broadened. It now carries on researches in the fields of
electricity, photometry and radiometry, ionizing radiations, and time and frequency besides its work in mass, length, and thermometry.

### 2.2.5 Status of the Metric System in the United States

The use of the metric system in this country was legalized by Act of Congress in 1866, but was not made obligatory then or since.

Following the signing of the Convention of the Meter in 1875, the United States acquired national prototype standards for the meter and the kilogram. U.S. Prototype Kilogram No. 20 continues to be the primary standard for mass in the United States. It is recalibrated from time to time at the BIPM. The prototype meter has been replaced by modern stabilized lasers following the most recent definition of the meter.

From 1893 until 1959, the yard was defined as equal exactly to ${ }^{3600} / 3937$ meter. In 1959, a small change was made in the definition of the yard to resolve discrepancies both in this country and abroad. Since 1959, we define the yard as equal exactly to 0.9144 meter; the new yard is shorter than the old yard by exactly two parts in a million. At the same time, it was decided that any data expressed in feet derived from geodetic surveys within the United States would continue to bear the relationship as defined in 1893 (one foot equals ${ }^{120 \%} /{ }_{3937}$ meter). We call this foot the U.S. Survey Foot, while the foot defined in 1959 is called the International Foot. Measurements expressed in U.S. statute miles, survey feet, rods, chains, links, or the squares thereof, and acres should be converted to the corresponding metric values by using pre-1959 conversion factors if more than five significant figure accuracy is required.

Since 1970, actions have been taken to encourage the use of metric units of measurement in the United States. A brief summary of actions by Congress is provided below as reported in the Federal Register Notice dated July 28, 1998.

Section 403 of Public Law 93-380, the Education Amendment of 1974, states that it is the policy of the United States to encourage educational agencies and institutions to prepare students to use the metric system of measurement as part of the regular education program. Under both this act and the Metric Conversion Act of 1975, the "metric system of measurement" is defined as the International System of Units as established in 1960 by the General Conference on Weights and Measures and interpreted or modified for the United States by the Secretary of Commerce (Sec. 4(4)—Pub. L. 94-168; Sec. 403(a)(3)—Pub. L. 93-380). The Secretary has delegated authority under these subsections to the Director of the National Institute of Standards and Technology.

Section 5164 of Public Law 100-418, the Omnibus Trade and Competitiveness Act of 1988, amends Public Law 94-168, The Metric Conversion Act of 1975. In particular, Section 3 Metric Conversion Act is amended to read as follows:
"Sec. 3. It is therefore the declared policy of the United States-
(1) to designate the metric system of measurement as the preferred system of weights and measures for United States trade and commerce;
(2) to require that each federal agency, by a date certain and to the extent economically feasible by the end of the fiscal year 1992, use the metric system of measurement in its procurements, grants, and other business-related activities, except to the extent that such use is impractical or is likely to cause significant inefficiencies or loss of markets to U.S. firms, such as when foreign competitors are producing competing products in non-metric units;
(3) to seek ways to increase understanding of the metric system of measurement through educational information and guidance and in government publications; and
(4) to permit the continued use of traditional systems of weights and measures in nonbusiness activities."

The Code of Federal Regulations makes the use of metric units mandatory for agencies of the federal government. (Federal Register, Vol. 56, No. 23, Page 160, January 2, 1991.)

### 2.3 British and United States Systems of Measurement

In the past, the customary system of weights and measures in the British Commonwealth countries and that in the United States were very similar; however, the SI metric system is now the official system of units in the United Kingdom, while the customary units are still predominantly used in the United States. Because references to the units of the old British customary system are still found, the following discussion describes the differences between the U.S. and British customary systems of units.

After 1959, the U.S. and the British inches were defined identically for scientific work and were identical in commercial usage. A similar situation existed for the U.S. and the British pounds, and many relationships, such as 12 inches $=1$ foot, 3 feet $=1$ yard, and 1760 yards $=1$ international mile, were the same in both countries; but there were some very important differences.

In the first place, the U.S. customary bushel and the U.S. gallon, and their subdivisions differed from the corresponding British Imperial units. Also the British ton is 2240 pounds, whereas the ton generally used in the United States is the short ton of 2000 pounds. The American colonists adopted the English wine gallon of 231 cubic inches. The English of that period used this wine gallon and they also had another gallon, the ale gallon of 282 cubic inches. In 1824, the British abandoned these two gallons when they adopted the British Imperial gallon, which they defined as the volume of 10 pounds of water, at a temperature of $62^{\circ} \mathrm{F}$, which, by calculation, is equivalent to 277.42 cubic inches. At the same time, they redefined the bushel as 8 gallons.

In the customary British system, the units of dry measure are the same as those of liquid measure. In the United States these two are not the same; the gallon and its subdivisions are used in the measurement of liquids and the bushel, with its subdivisions, is used in the measurement of certain dry commodities. The U.S. gallon is divided into four liquid quarts and the U.S. bushel into 32 dry quarts. All the units of capacity or volume mentioned thus far are larger in the customary British system than in the U.S. system. But the British fluid ounce is smaller than the U.S. fluid ounce, because the British quart is divided into 40 fluid ounces whereas the U.S. quart is divided into 32 fluid ounces.

From this we see that in the customary British system an avoirdupois ounce of water at $62^{\circ} \mathrm{F}$ has a volume of one fluid ounce, because 10 pounds is equivalent to 160 avoirdupois ounces, and 1 gallon is equivalent to 4 quarts, or 160 fluid ounces. This convenient relation does not exist in the U.S. system because a U.S. gallon of water at $62^{\circ} \mathrm{F}$ weighs about $8 \frac{1}{3}$ pounds, or $1331 / 3$ avoirdupois ounces, and the U.S. gallon is equivalent to $4 \times 32$, or 128 fluid ounces.

| 1 U.S. fluid ounce | $=1.041$ British fluid ounces |
| :--- | :--- |
| 1 British fluid ounce | $=0.961$ U.S. fluid ounce |
| 1 U.S. gallon | $=0.833$ British Imperial gallon |
| 1 British Imperial gallon | $=1.201$ U.S. gallons |

Among other differences between the customary British and the United States measurement systems, we should note that they abolished the use of the troy pound in England on January 6, 1879; they retained only the troy ounce and its subdivisions, whereas the troy pound is still legal in the United States, although it is not now greatly used. We can mention again the common use, for body weight, in England of the stone of 14 pounds, this being a unit now unused in the United States, although its influence was shown in the practice until World War II of selling flour by the barrel of 196 pounds ( 14 stone). In the apothecary system of liquid measure the British add a unit, the fluid scruple, equal to one third of a fluid drachm (spelled dram in the United States) between their minim and their fluid drachm. In the United States, the general practice now is to sell dry commodities, such as fruits and vegetables, by their mass.

### 2.4 Subdivision of Units

In general, units are subdivided by one of three methods: (a) decimal, into tenths; (b) duodecimal, into twelfths; or (c) binary, into halves (twos). Usually the subdivision is continued by using the same method. Each method has its advantages for certain purposes, and it cannot properly be said that any one method is "best" unless the use to which the unit and its subdivisions are to be put is known.

For example, if we are concerned only with measurements of length to moderate precision, it is convenient to measure and to express these lengths in feet, inches, and binary fractions of an inch, thus 9 feet, $4 \frac{3}{8}$ inches. However, if these lengths are to be subsequently used to calculate area or volume, that method of subdivision at once becomes extremely inconvenient. For that reason, civil engineers, who are concerned with areas of land, volumes of cuts, fills, excavations, etc., instead of dividing the foot into inches and binary subdivisions of the inch, divide it decimally; that is, into tenths, hundredths, and thousandths of a foot.

The method of subdivision of a unit is thus largely made based on convenience to the user. The fact that units have commonly been subdivided into certain subunits for centuries does not preclude their also having another mode of subdivision in some frequently used cases where convenience indicates the value of such other method. Thus, while we usually subdivide the gallon into quarts and pints, most gasoline-measuring pumps, of the price-computing type, are graduated to show tenths, hundredths, or thousandths of a gallon.

Although the mile has for centuries been divided into rods, yards, feet, and inches, the odometer part of an automobile speedometer shows tenths of a mile. Although we divide our dollar into 100 parts, we habitually use and speak of halves and quarters. An illustration of rather complex subdividing is found on the scales used by draftsmen. These scales are of two types: (a) architects, which are commonly graduated with scales in which $3 / 32,3 / 16,1 / 8,1 / 4,3 / 8,1 / 2,3 / 4,1,1^{1 / 12}$, and 3 inches, respectively, represent 1 foot full scale, and also having a scale graduated in the usual manner to $1 / 16$ inch; and (b) engineers, which are commonly subdivided to $10,20,30,40$, 50 , and 60 parts to the inch.

The dictum of convenience applies not only to subdivisions of a unit but also to multiples of a unit. Land elevations above sea level are given in feet although the height may be several miles; the height of aircraft above sea level as given by an altimeter is likewise given in feet, no matter how high it may be.

On the other hand, machinists, toolmakers, gauge makers, scientists, and others who are engaged in precision measurements of relatively small distances, even though concerned with measurements of length only, find it convenient to use the inch, instead of the tenth of a foot, but to divide the inch decimally to tenths, hundredths, thousandths, etc., even down to millionths of an inch. Verniers, micrometers, and other precision measuring instruments are usually graduated in this manner. Machinist scales are commonly graduated decimally along one edge and are also graduated along another edge to binary fractions as small as $1 / 64$ inch. The scales with binary fractions are used only for relatively rough measurements.

It is seldom convenient or advisable to use binary subdivisions of the inch that are smaller than $1 / 64$. In fact, $1 / 32-, 1 / 16^{-}$, or $1 / 8$-inch subdivisions are usually preferable for use on a scale to be read with the unaided eye.

### 2.5 Arithmetical Systems of Numbers

The subdivision of units of measurement is closely associated with arithmetical systems of numbers. The systems of units used in this country for commercial and scientific work, having many origins as has already been shown, naturally show traces of the various number systems associated with their origins and developments. Thus, (a) the binary subdivision has come down to us from the Hindus, (b) the duodecimal system of fractions from the Romans, (c) the decimal system from the Chinese and Egyptians, some developments having been made by the Hindus, and (d) the sexagesimal system (division by 60 ) now illustrated in the subdivision of units of angle and of time, from the ancient Babylonians. The use of decimal numbers in measurements is becoming the standard practice.

## 3. Standards of Length, Mass, and Capacity or Volume

### 3.1 Standards of Length

The meter, which is defined in terms of the speed of light in a vacuum, is the unit on which all length measurements are based.

The yard is defined ${ }^{1}$ as follows:

$$
1 \text { yard }=0.9144 \text { meter }
$$

and the inch is exactly equal to 25.4 millimeters.

### 3.1.1 Calibration of Length Standards

NIST calibrates standards of length including meter bars, yard bars, miscellaneous precision line standards, steel tapes, invar geodetic tapes, precision gauge blocks, micrometers, and limit gauges. It also measures the linear dimensions of miscellaneous apparatus such as penetration needles, cement sieves, and hemacytometer chambers. In general, NIST accepts for calibration only apparatus of such material, design, and construction as to ensure accuracy and permanence sufficient to justify calibration by the Institute. NIST performs calibrations in accordance with fee schedules, copies of which may be obtained from NIST.

NIST does not calibrate carpenters' rules, machinist scales, draftsman scales, and the like. Such apparatus, if they require calibration, should be submitted to state or local weights and measures officials.

### 3.2 Standards of Mass

The primary standard of mass for this country is United States Prototype Kilogram 20, which is a platinumiridium cylinder kept at NIST. We know the value of this mass standard in terms of the International Prototype Kilogram, a platinum-iridium standard which is kept at the International Bureau of Weights and Measures.

In Colonial Times the British standards were considered the primary standards of the United States. Later, the U.S. avoirdupois pound was defined in terms of the Troy Pound of the Mint, which is a brass standard kept at the United States Mint in Philadelphia. In 1911, the Troy Pound of the Mint was superseded, for coinage purposes, by the Troy Pound of the Institute.

The avoirdupois pound is defined in terms of the kilogram by the relation:

$$
1 \text { avoirdupois pound }=0.45359237 \text { kilogram. }^{2}
$$

These changes in definition have not made any appreciable change in the value of the pound.
The grain is $1 / 000$ of the avoirdupois pound and is identical in the avoirdupois, troy, and apothecary systems. The troy ounce and the apothecary ounce differ from the avoirdupois ounce but are equal to each other, and equal to 480 grains. The avoirdupois ounce is equal to 437.5 grains.

### 3.2.1 Mass and Weight

The mass of a body is a measure of its inertial property or how much matter it contains. The weight of a body is a measure of the force exerted on it by gravity or the force needed to support it. Gravity on earth gives a body a downward acceleration of about $9.8 \mathrm{~m} / \mathrm{s}^{2}$. (In common parlance, weight is often used as a synonym for mass as in weights and measures.) The incorrect use of weight in place of mass should be phased out, and the term mass used when mass is meant.

Standards of mass are ordinarily calibrated by comparison to a reference standard of mass. If two objects are compared on a balance and give the same balance indication, they have the same "mass" (excluding the effect of air buoyancy). The forces of gravity on the two objects are balanced. Even though the value of the acceleration of gravity, g , is different from location to location, because the two objects of equal mass in the same location (where both masses are acted upon by the same g ) will be affected in the same manner and by the same amount by any change in the value of $g$, the two objects will balance each other under any value of $g$.

[^0]However, on a spring balance the mass of a body is not balanced against the mass of another body. Instead, the gravitational force on the body is balanced by the restoring force of a spring. Therefore, if a very sensitive spring balance is used, the indicated mass of the body would be found to change if the spring balance and the body were moved from one locality to another locality with a different acceleration of gravity. But a spring balance is usually used in one locality and is adjusted or calibrated to indicate mass at that locality.

### 3.2.2 Effect of Air Buoyancy

Another point that must be taken into account in the calibration and use of standards of mass is the buoyancy or lifting effect of the air. A body immersed in any fluid is buoyed up by a force equal to the force of gravity on the displaced fluid. Two bodies of equal mass, if placed one on each pan of an equal-arm balance, will balance each other in a vacuum. A comparison in a vacuum against a known mass standard gives "true mass." If compared in air, however, they will not balance each other unless they are of equal volume. If of unequal volume, the larger body will displace the greater volume of air and will be buoyed up by a greater force than will the smaller body, and the larger body will appear to be of less mass than the smaller body.

The greater the difference in volume, and the greater the density of the air in which we make the comparison weighing, the greater will be the apparent difference in mass. For that reason, in assigning a precise numerical value of mass to a standard, it is necessary to base this value on definite values for the air density and the density of the mass standard of reference.

The apparent mass of an object is equal to the mass of just enough reference material of a specified density (at $20^{\circ} \mathrm{C}$ ) that will produce a balance reading equal to that produced by the object if the measurements are done in air with a density of $1.2 \mathrm{mg} / \mathrm{cm}^{3}$ at $20^{\circ} \mathrm{C}$. The original basis for reporting apparent mass is apparent mass versus brass. The apparent mass versus a density of $8.0 \mathrm{~g} / \mathrm{cm}^{3}$ is the more recent definition, and is used extensively throughout the world. The use of apparent mass versus $8.0 \mathrm{~g} / \mathrm{cm}^{3}$ is encouraged over apparent mass versus brass. The difference in these apparent mass systems is insignificant in most commercial weighing applications.

A full discussion of this topic is given in NIST Monograph 133, Mass and Mass Values, by Paul E. Pontius [for sale by the National Technical Information Service, 5285 Port Royal Road, Springfield, VA 22161 (COM 7450309).]

### 3.2.3 Calibrations of Standards of Mass

Standards of mass regularly used in ordinary trade should be tested by state or local weights and measures officials. NIST calibrates mass standards submitted, but it does not manufacture or sell them. Information regarding the mass calibration service of NIST and the regulations governing the submission of standards of mass to NIST for calibration are contained in NIST Special Publication 250, Calibration and Related Measurement Services of NIST, latest edition.

### 3.3 Standards of Capacity

Units of capacity or volume, being derived units, are in this country defined in terms of linear units. Laboratory standards have been constructed and are maintained at NIST. These have validity only by calibration with reference either directly or indirectly to the linear standards. Similarly, NIST has made and distributed standards of capacity to the several states. Other standards of capacity have been verified by calibration for a variety of uses in science, technology, and commerce.

### 3.3.1 Calibrations of Standards of Capacity

NIST makes calibrations on capacity or volume standards that are in the customary units of trade; that is, the gallon, its multiples, and submultiples, or in metric units. Further, NIST calibrates precision-grade volumetric glassware which is normally in metric units. NIST makes calibrations in accordance with fee schedules, copies of which may be obtained from NIST.

### 3.4 Maintenance and Preservation of Fundamental Standard of Mass

It is a statutory responsibility of NIST to maintain and preserve the national standard of mass at NIST and to realize all the other base units. The U.S. Prototype Kilogram maintained at NIST is fully protected by an alarm system. All measurements made with this standard are conducted in special air-conditioned laboratories to which the standard is taken a sufficiently long time before the observations to ensure that the standard will be in a state of equilibrium under standard conditions when the measurements or comparisons are made. Hence, it is not necessary to maintain the standard at standard conditions, but care is taken to prevent large changes of temperature. More important is the care to prevent any damage to the standard because of careless handling.

## 4. Specialized Use of the Terms "Ton" and "Tonnage"

As weighing and measuring are important factors in our everyday lives, it is quite natural that questions arise about the use of various units and terms and about the magnitude of quantities involved. For example, the words "ton" and "tonnage" are used in widely different senses, and a great deal of confusion has arisen regarding the application of these terms.

The ton is used as a unit of measure in two distinct senses: (1) as a unit of mass, and (2) as a unit of capacity or volume. In the first sense, the term has the following meanings:
(a) The short, or net ton of 2000 pounds.
(b) The long, gross, or shipper's ton of 2240 pounds.
(c) The metric ton of 1000 kilograms, or 2204.6 pounds.

In the second sense (capacity), it is usually restricted to uses relating to ships and has the following meaning:
(a) The register ton of 100 cubic feet.
(b) The measurement ton of 40 cubic feet.
(c) The English water ton of 224 British Imperial gallons.

In the United States and Canada the ton (mass) most commonly used is the short ton. In Great Britain, it is the long ton, and in countries using the metric system, it is the metric ton. The register ton and the measurement ton are capacity or volume units used in expressing the tonnage of ships. The English water ton is used, chiefly in Great Britain, in statistics dealing with petroleum products.

There have been many other uses of the term ton such as the timber ton of 40 cubic feet and the wheat ton of 20 bushels, but their uses have been local and the meanings have not been consistent from one place to another.

Properly, the word "tonnage" is used as a noun only in respect to the capacity or volume and dimensions of ships, and to the amount of the ship's cargo. There are two distinct kinds of tonnage; namely, vessel tonnage and cargo tonnage and each of these is used in various meanings. The several kinds of vessel tonnage are as follows:

Gross tonnage, or gross register tonnage, is the total cubical capacity or volume of a ship expressed in register tons of 100 cubic feet, or 2.83 cubic meters, less such space as hatchways, bakeries, galleys, etc., as are exempted from measurement by different governments. There is some lack of uniformity in the gross tonnages as given by different nations due to lack of agreement on the spaces that are to be exempted. Official merchant marine statistics of most countries are published in terms of the gross register tonnage. Press references to ship tonnage are usually to the gross tonnage.

The net tonnage, or net register tonnage, is the gross tonnage less the different spaces specified by maritime nations in their measurement rules and laws. The spaces deducted are those totally unavailable for carrying cargo, such as the engine room, coal bunkers, crew quarters, chart and instrument room, etc. The net tonnage is used in computing how much cargo that can be loaded on a ship. It is used as the basis for wharfage and other similar charges.

The register under-deck tonnage is the cubical capacity of a ship under her tonnage deck expressed in register tons. In a vessel having more than one deck, the tonnage deck is the second from the keel.

There are several variations of displacement tonnage.
The dead weight tonnage is the difference between the "loaded" and "light" displacement tonnages of a vessel. It is expressed in terms of the long ton of 2240 pounds, or the metric ton of 2204.6 pounds, and is the weight of fuel, passengers, and cargo that a vessel can carry when loaded to its maximum draft.

The second variety of tonnage, cargo tonnage, refers to the weight of the particular items making up the cargo. In overseas traffic it is usually expressed in long tons of 2240 pounds or metric tons of 2204.6 pounds. The short ton is only occasionally used. Therefore, the cargo tonnage is very distinct from vessel tonnage.

## APPENDIX C: GENERAL TABLES OF UNITS OF MEASUREMENT

These tables have been prepared for the benefit of those requiring tables of units for occasional ready reference. In Section 4 of this Appendix, the tables are carried out to a large number of decimal places and exact values are indicated by underlining. In most of the other tables, only a limited number of decimal places are given, therefore making the tables better adopted to the average user.

## 1. Tables of Metric Units of Measurement

In the metric system of measurement, designations of multiples and subdivisions of any unit may be arrived at by combining with the name of the unit the prefixes deka, hecto, and kilo meaning, respectively, 10, 100, and 1000, and deci, centi, and milli, meaning, respectively, one-tenth, one-hundredth, and one-thousandth. In some of the following metric tables, some such multiples and subdivisions have not been included for the reason that these have little, if any currency in actual usage.

In certain cases, particularly in scientific usage, it becomes convenient to provide for multiples larger than 1000 and for subdivisions smaller than one-thousandth. Accordingly, the following prefixes have been introduced and these are now generally recognized:

| yotta, | (Y) | meaning $10^{24}$ | deci, | (d), | meaning $10^{-1}$ |
| :---: | :---: | :---: | :---: | :---: | :---: |
| zetta, | (Z), | meaning $10^{21}$ | centi, | (c), | meaning $10^{-2}$ |
| exa, | (E), | meaning $10^{18}$ | milli, | (m), | meaning $10^{-3}$ |
| peta, | (P), | meaning $10^{15}$ | micro, | ( $\mu$ ), | meaning $10^{-6}$ |
| tera, | (T), | meaning $10^{12}$ | nano, | ( n ), | meaning $10^{-9}$ |
| giga, | (G), | meaning $10^{9}$ | pico, | (p), | meaning $10^{-12}$ |
| mega, | (M), | meaning $10^{6}$ | femto, | (f), | meaning $10^{-15}$ |
| kilo, | (k), | meaning $10^{3}$ | atto, | (a), | meaning $10^{-18}$ |
| hecto, | (h), | meaning $10^{2}$ | zepto, | (z), | meaning $10^{-21}$ |
| deka, | (da), | meaning $10^{1}$ | yocto, | (y), | meaning $10^{-24}$ |

Thus a kilometer is 1000 meters and a millimeter is 0.001 meter.

## Units of Length

| 10 millimeters $(\mathrm{mm})$ | $=1$ centimeter $(\mathrm{cm})$ |
| :--- | :--- |
| 10 centimeters | $=1$ decimeter $(\mathrm{dm})=100$ millimeters |
| 10 decimeters | $=1$ meter $(\mathrm{m})=1000$ millimeters |
| 10 meters | $=1$ dekameter $(\mathrm{dam})$ |
| 10 dekameters | $=1$ hectometer $(\mathrm{hm})=100$ meters |
| 10 hectometers |  |

## Units of Area

| 100 square millimeters $\left(\mathrm{mm}^{2}\right)$ | $=1$ square centimeter $\left(\mathrm{cm}^{2}\right)$ |
| :--- | :--- |
| 100 square centimeters | $=1$ square decimeter $\left(\mathrm{dm}^{2}\right)$ |
| 100 square decimeters | $=1$ square meter $\left(\mathrm{m}^{2}\right)$ |
| 100 square meters | $=1$ square dekameter $\left(\mathrm{dam}^{2}\right)=1$ are |
| 100 square dekameters | $=1$ square hectometer $\left(\mathrm{hm}^{2}\right)=1$ hectare (ha) |
| 100 square hectometers | $=1$ square kilometer $\left(\mathrm{km}^{2}\right)$ |

## Units of Liquid Volume

| 10 milliliters $(\mathrm{mL})$ | $=1$ centiliter $(\mathrm{cL})$ |
| :--- | :--- |
| 10 centiliters | $=1$ deciliter $(\mathrm{dL})=100$ milliliters |
| 10 deciliters | $=1$ liter $^{1}=1000$ milliliters |
| 10 liters | $=1$ dekaliter $(\mathrm{daL})$ |
| 10 dekaliters | $=1$ hectoliter $(\mathrm{hL})=100$ liters |
| 10 hectoliters | $=1$ kiloliter $(\mathrm{kL})=1000$ liters |

## Units of Volume

| 1000 cubic millimeters $\left(\mathrm{mm}^{3}\right)$ |  |
| ---: | :--- |
| 1000 cubic centimeters | $=1$ cubic centimeter $\left(\mathrm{cm}^{3}\right)$ |
|  | $=1000000$ cubic decimeter millimeters |
| 1000 cubic decimeters | $=1$ cubic meter $\left(\mathrm{m}^{3}\right)$ |
|  | $=1000000$ cubic centimeters |
|  | $=1000000000$ cubic millimeters |

## Units of Mass

| 10 milligrams $(\mathrm{mg})$ | $=1$ centigram $(\mathrm{cg})$ |
| :--- | :--- |
| 10 centigrams | $=1 \operatorname{decigram}(\mathrm{dg})=100$ milligrams |
| 10 decigrams | $=1 \operatorname{gram}(\mathrm{~g})=1000$ milligrams |
| 10 grams | $=1$ dekagram $(\mathrm{dag})$ |
| 10 dekagrams | $=1$ hectogram $(\mathrm{hg})=100$ grams |
| 10 hectograms | $=1$ kilogram $(\mathrm{kg})=1000$ grams |
| 1000 kilograms |  |

[^1]
## 2. Tables of U.S. Units of Measurement ${ }^{2}$

In these tables where foot or mile is underlined, it is survey foot or U.S. statute mile rather than international foot or mile that is meant.

## Units of Length

| 12 inches (in) | $=1$ foot $(\mathrm{ft})$ |
| :--- | :--- |
| 3 feet | $=1$ yard $(\mathrm{yd})$ |
| $161 / 2$ feet |  |
| 40 rods |  |
| 8 furlongs | $=1$ furlong $($ fur $)=660$ feet |
| 1852 meters $(\mathrm{m})$ |  |
|  |  |
|  |  |
|  |  |
|  |  |
|  |  |
|  | $=1$ U.S. statute mile $(\mathrm{mi})=5280$ feet |
|  |  |

Units of Area ${ }^{3}$

| 144 square inches ( $\mathrm{in}^{2}$ ) | $=1$ square foot ( $\mathrm{ft}^{2}$ ) |
| :---: | :---: |
| 9 square feet | $=1$ square yard ( $\mathrm{yd}^{2}$ ) |
|  | $=1296$ square inches |
| $272^{1 / 4}$ square feet | $=1$ square rod ( $\mathrm{rd}^{2}$ ) |
| 160 square rods | $=1$ acre $=43560$ square feet |
| 640 acres | $=1$ square mile ( $\mathrm{mi}^{2}$ ) |
| 1 mile square | $=1$ section of land |
| 6 miles square | $=1$ township |
|  | $=36$ sections $=36$ square $\underline{\text { miles }}$ |

Units of Volume ${ }^{3}$

| 1728 cubic inches $\left(\mathrm{in}^{3}\right)$ | $=1$ cubic foot $\left(\mathrm{ft}^{3}\right)$ |
| :--- | :--- |
| 27 cubic feet | $=1$ cubic yard $\left(\mathrm{yd}^{3}\right)$ |

Gunter's or Surveyors Chain Units of Measurement

| 0.66 foot $(\mathrm{ft})$ | $=1$ link (li) |
| :--- | :--- |
| 100 links |  |
|  | $=1$ chain $(\mathrm{ch})$ |
| 80 chains |  |
|  | $=1 \mathrm{rods}=66$ feet |
|  |  |
|  | $=320$ rods $=5280$ feet |

[^2]
## Units of Liquid Volume ${ }^{4}$

| 4 gills (gi) | $=1$ pint $(\mathrm{pt})=28.875$ cubic inches $\left(\mathrm{in}^{3}\right)$ |
| :--- | :--- |
| 2 pints | $=1$ quart $(\mathrm{qt})=57.75$ cubic inches |
| 4 quarts | $=1$ gallon (gal) $=231$ cubic inches |
|  |  |
|  | $=8$ pints $=32$ gills |

Apothecaries Units of Liquid Volume

| 60 minims | $=1$ fluid dram $(\mathrm{fl} \mathrm{dr}$ or $f \mathbf{Z})$ |
| :--- | :--- |
|  | $=0.2256$ cubic inch $\left(\mathrm{in}^{3}\right)$ |
| 8 fluid drams | $=1$ fluid ounce $(\mathrm{fl}$ oz or $f$ З |
|  | $=1.8047$ cubic inches |
| 16 fluid ounces | $=1$ pint (pt) |
|  | $=28.875$ cubic inches |
| 2 pints | $=128$ fluid drams |
| 4 quarts | $=1$ quart (qt) $=57.75$ cubic inches |
|  | $=32$ fluid ounces $=256$ fluid drams |
|  | $=1$ gallon (gal) $=231$ cubic inches |
|  | $=128$ fluid ounces $=1024$ fluid drams |

## Units of Dry Volume ${ }^{5}$

| 2 pints $(\mathrm{pt})$ | $=1$ quart $(\mathrm{qt})=67.2006$ cubic inches $\left(\mathrm{in}^{3}\right)$ |
| :--- | :--- |
| 8 quarts | $=1$ peck $(\mathrm{pk})=537.605$ cubic inches |
| 4 pecks | $=16$ pints |
|  | $=1$ bushel $($ bu $)=2150.42$ cubic inches |
|  | $=32$ quarts |
|  | Avoirdupois Units of Mass ${ }^{6}$ |

[The "grain" is the same in avoirdupois, troy, and apothecaries units of mass.]

| $27^{11 / 32}$ grains $(\mathrm{gr})$ | $=1$ dram $(\mathrm{dr})$ |
| :--- | :--- |
| 16 drams | $=1$ ounce $(\mathrm{oz})$ |
| 16 ounces | $=4377^{1 / 2}$ grains |
|  | $=1$ pound $(\mathrm{lb})$ |
|  | $=256$ drams |
| 100 pounds | $=7000$ grains |
| 20 hundredweights | $=1$ hundredweight $(\mathrm{cwt})^{7}$ |
|  | $=1$ ton $(\mathrm{t})$ |
|  | $=2000$ pounds $^{7}$ |

4. When necessary to distinguish the liquid pint or quart from the dry pint or quart, the word "liquid" or the abbreviation "liq" should be used in combination with the name or abbreviation of the liquid unit.
5. When necessary to distinguish dry pint or quart from the liquid pint or quart, the word "dry" should be used in combination with the name or abbreviation of the dry unit.
6. When necessary to distinguish the avoirdupois dram from the apothecaries dram, or to distinguish the avoirdupois dram or ounce from the fluid dram or ounce, or to distinguish the avoirdupois ounce or pound from the troy or apothecaries ounce or pound, the word "avoirdupois" or the abbreviation "avdp" should be used in combination with the name or abbreviation of the avoirdupois unit.
7. When the terms "hundredweight" and "ton" are used unmodified, they are commonly understood to mean the 100 -pound hundredweight and the 2000-pound ton, respectively; these units may be designated "net" or "short" when necessary to distinguish them from the corresponding units in gross or long measure.

In "gross" or "long" measure, the following values are recognized:

| 112 pounds $(\mathrm{lb})$ | $=1$ gross or long hundredweight $(\mathrm{cwt})^{7}$ |
| ---: | :--- |
| 20 gross or long hundredweights | $=1$ gross or long ton |
|  | $=2240$ pounds |

## Troy Units of Mass

[The "grain" is the same in avoirdupois, troy, and apothecaries units of mass.]

| 24 grains (gr) | $=1$ pennyweight $(\mathrm{dwt})$ |
| :--- | :--- |
| 20 pennyweights | $=1$ ounce troy $(\mathrm{oz} \mathrm{t})=480$ grains |
| 12 ounces troy | $=1$ pound troy $(\mathrm{lb} \mathrm{t})$ |
|  | $=240$ pennyweights $=5760$ grains |

Apothecaries Units of Mass
[The "grain" is the same in avoirdupois, troy, and apothecaries units of mass.]
20 grains (gr) $\quad=1$ scruple ( s ap or $\bigoplus$ )
3 scruples $\quad=1$ dram apothecaries (dr ap or 3)
$=60$ grains
8 drams apothecaries $\quad=1$ ounce apothecaries (oz ap or $\mathbf{3}$ )
12 ounces apothecaries $=1$ pound apothecaries ( lb ap )
$=96$ drams apothecaries
$=288$ scruples $=5760$ grains

## 3. Notes on British Units of Measurement

In Great Britain, the yard, the avoirdupois pound, the troy pound, and the apothecaries pound are identical with the units of the same names used in the United States. The tables of British linear measure, troy mass, and apothecaries mass are the same as the corresponding United States tables, except for the British spelling "drachm" in the table of apothecaries mass. The table of British avoirdupois mass is the same as the United States table up to 1 pound; above that point the table reads:

| 14 pounds | $=1$ stone |
| :--- | :--- |
| 2 stones | $=1$ quarter $=28$ pounds |
| 4 quarters | $=1$ hundredweight $=112$ pounds |
| 20 hundredweight | $=1$ ton $=2240$ pounds |

The present British gallon and bushel-known as the "Imperial gallon" and "Imperial bushel"-are, respectively, about $20 \%$ and $3 \%$ larger than the United States gallon and bushel. The Imperial gallon is defined as the volume of 10 avoirdupois pounds of water under specified conditions, and the Imperial bushel is defined as 8 Imperial gallons. Also, the subdivision of the Imperial gallon as presented in the table of British apothecaries fluid measure differs in two important respects from the corresponding United States subdivision, in that the

Imperial gallon is divided into 160 fluid ounces (whereas the United States gallon is divided into 128 fluid ounces), and a "fluid scruple" is included. The full table of British measures of capacity (which are used alike for liquid and for dry commodities) is as follows:

| 4 gills | $=1$ pint |
| :--- | :--- |
| 2 pints | $=1$ quart |
| 4 quarts | $=1$ gallon |
| 2 gallons | $=1$ peck |
| 8 gallons ( 4 pecks) | $=1$ bushel |
| 8 bushels | $=1$ quarter |

The full table of British apothecaries measure is as follows:

| 20 minims | $=1$ fluid scruple |
| :--- | :--- |
| 3 fluid scruples | $=1$ fluid drachm |
|  | $=60$ minims |
| 8 fluid drachms | $=1$ fluid ounce |
| 20 fluid ounces | $=1$ pint |
| 8 pints | $=1$ gallon ( 160 fluid ounces) |

## 4. Tables of Units of Measurement (all underlined figures are exact)

| Units | Inches | Feet | Yards | Miles | Centimeters | Meters |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 inch = | 1 | $\begin{aligned} & 0.083 \\ & 33333 \end{aligned}$ | $\begin{aligned} & 0.027 \\ & 77778 \end{aligned}$ | $\begin{aligned} & 0.000015 \\ & 78283 \end{aligned}$ | 2.54 | $\underline{0.0254}$ |
| 1 foot $=$ | 12 | 1 | $\begin{aligned} & 0.333 \\ & 3333 \end{aligned}$ | $\begin{aligned} & 0.000189 \\ & 3939 \end{aligned}$ | 30.48 | $\underline{0.3048}$ |
| 1 yard $=$ | 36 | 3 | 1 | $\begin{aligned} & 0.000568 \\ & 1818 \end{aligned}$ | $\underline{91.44}$ | $\underline{0.9144}$ |
| 1 mile $=$ | $\underline{63360}$ | $\underline{5280}$ | $\underline{1760}$ | 1 | $\underline{160934.4}$ | $\underline{1609.344}$ |
| 1 <br> centimeter $=$ | $\begin{aligned} & 0.393 \\ & 7008 \end{aligned}$ | $\begin{aligned} & 0.032 \\ & 80840 \end{aligned}$ | $\begin{aligned} & 0.010 \\ & 93613 \end{aligned}$ | $\begin{aligned} & 0.000006 \\ & 213712 \end{aligned}$ | 1 | 0.01 |
| 1 meter $=$ | $\begin{aligned} & 39.370 \\ & 08 \end{aligned}$ | $\begin{aligned} & 3.280 \\ & 840 \end{aligned}$ | $\begin{aligned} & 1.093 \\ & 613 \end{aligned}$ | $\begin{aligned} & 0.000621 \\ & 3712 \end{aligned}$ | 100 | $\underline{1}$ |

[^3]Units of Length - Survey Measure ${ }^{8}$

| Units | Links | Feet | Rods | Chains | Miles | Meters |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| 1 link $=$ | $\underline{1}$ | $\underline{0.66}$ | $\underline{0.04}$ | $\underline{0.01}$ | $\underline{0.000125}$ | 0.2011684 |
| 1 foot $=$ | 1.515152 | $\underline{1}$ | $\underline{0.060} 60606$ | 0.01515152 | 0.0001893939 | 0.3048006 |
| 1 rod $=$ | $\underline{25}$ | $\underline{16.5}$ | $\underline{1}$ | $\underline{0.25}$ | $\underline{0.003125}$ | 5.029210 |
| 1 chain $=$ | $\underline{100}$ | $\underline{66}$ | $\underline{4}$ | $\underline{1}$ | $\underline{0.0125}$ | 20.11684 |
| 1 mile $=$ | $\underline{8000}$ | $\underline{5280}$ | $\underline{320}$ | $\underline{80}$ | $\underline{1}$ | 1609.347 |
| 1 meter $=$ | 4.970960 | 3.280833 | 0.1988384 | 0.04970960 | 0.0006213699 | $\underline{1}$ |

## Units of Area-International Measure ${ }^{9}$



| Units | Square Miles | Square Centimeters | Square Meters |
| :--- | :--- | :--- | :--- |
| 1 square inch $=$ | 0.0000000002490977 | $\underline{6.4516}$ | $\underline{0.00064516}$ |
| 1 square foot $=$ | 0.00000003587006 | $\underline{929.0304}$ | $\underline{0.09290304}$ |
| 1 square yard $=$ | 0.0000003228306 | $\underline{2589.2736}$ | $\underline{0.83612736}$ |
| 1 square mile $=$ | $\underline{1}$ | $\underline{1} 5899988.110336$ |  |
| 1 square centimeter $=$ | 0.00000000003861022 | 10000 | $\underline{0.0001}$ |
| 1 square meter $=$ | 0.0000003861022 |  | 1 |

[^4]Units of Area-Survey Measure ${ }^{9}$

| Units | Square Feet | Square Rods | Square Chains | Acres |
| :---: | :---: | :---: | :---: | :---: |
| 1 square foot $=$ | 1 | 0.003673095 | 0.0002295684 | 0.00002295684 |
| 1 square rod = | $\underline{272.25}$ | 1 | $\underline{0.0625}$ | $\underline{0.00625}$ |
| 1 square chain $=$ | $\underline{4356}$ | 16 | 1 | 0.1 |
| 1 acre $=$ | 43560 | 160 | 10 | 1 |
| 1 square mile = | $\underline{27878400}$ | $\underline{102400}$ | $\underline{6400}$ | $\underline{640}$ |
| 1 square meter $=$ | 10.76387 | 0.03953670 | 0.002471044 | 0.0002471044 |
| 1 hectare $=$ | 107638.7 | 395.3670 | 24.71044 | 2.471044 |


| Units | Square Miles | Square Meters | Hectares |
| :--- | :--- | :--- | :--- |
| 1 square foot $=$ | 0.00000003587006 | 0.09290341 | 0.000009290341 |
| 1 square rod $=$ | $\underline{0.000009765625}$ | 25.29295 | 0.002529295 |
| 1 square chain $=$ | $\underline{0.00015625}$ | 404.6873 | 0.04046873 |
| 1 acre $=$ | $\underline{0.0015625}$ | 4046.873 | 0.4046873 |
| 1 square mile $=$ | $\underline{1}$ | 2589998 | 258.9998 |
| 1 square meter $=$ | 0.0000003861006 | $\underline{1}$ | $\underline{0.0001}$ |
| 1 hectare $=$ | 0.003861006 | $\underline{10000}$ |  |

Units of Volume

| Units | Cubic Inches | Cubic Feet | Cubic Yards |
| :--- | :--- | :--- | :--- |
| 1 cubic inch $=$ | $\frac{1}{1728}$ | 0.0005787037 | 0.00002143347 |
| 1 cubic foot $=$ | $\underline{46656}$ | $\frac{1}{27}$ | 0.03703704 |
| 1 cubic yard $=$ | 0.06102374 | 0.00003531467 | $\underline{1}$ |
| 1 cubic centimeter $=$ | 61.02374 | 0.03531467 | 0.000001307951 |
| 1 cubic decimeter $=$ | 61023.74 | 35.31467 | 0.001307951 |
| 1 cubic meter $=$ |  |  | 1.307951 |


| Units | Milliliters (Cubic Centimeters) | Liters (Cubic Decimeters) | Cubic Meters |
| :--- | :--- | :--- | :--- |
| 1 cubic inch $=$ | $\underline{16.387064} 5$ | $\underline{0.016387064}$ | 0.000016387064 |
| 1 cubic foot $=$ | $\underline{28316.846592}$ | $\underline{\underline{28.316 .846592}} 7$ | 0.028316846592 |
| 1 cubic yard $=$ | $\underline{764.554857984}$ | 0.764554857984 |  |

(Continued)

[^5]| Units | Milliliters (Cubic Centimeters) | Liters (Cubic Decimeters) | Cubic Meters |
| :--- | :--- | :--- | :--- |
| 1 cubic centimeter $=$ | $\frac{1}{1000}$ | $\underline{0.001}$ | 0.000001 |
| 1 cubic decimeter $=$ | $\frac{1}{1000} 000$ | $\frac{1}{1000}$ | 0.001 |
| 1 cubic meter $=$ |  | 1 |  |

Units of Capacity or Volume-Dry Volume Measure

| Units | Dry Pints | Dry Quarts | Pecks | Bushels |
| :--- | :--- | :--- | :--- | :--- |
| 1 dry pint $=$ | $\underline{1}$ | $\underline{0.5}$ | $\underline{0.062 .5}$ | $\underline{0.015625}$ |
| 1 dry quart $=$ | $\underline{1}$ | $\underline{1}$ | $\underline{0.125}$ | $\underline{0.03125}$ |
| 1 peck $=$ | $\underline{16}$ | $\underline{1}$ | $\underline{0.25}$ |  |
| 1 bushel $=$ | $\underline{64}$ | $\underline{32}$ | $\underline{1}$ |  |
| 1 cubic inch $=$ | 0.0297616 | 0.0148808 | 0.00186010 | $\underline{0.000465025}$ |
| 1 cubic foot $=$ | 51.42809 | 25.71405 | 3.214256 | 0.80356395 |
| 1 liter $=$ | 1.816166 | 0.9080830 | 0.1135104 | 0.02837759 |
| 1 cubic meter $=$ | 1816.166 | 908.0830 | 113.5104 | 28.37759 |


| Units | Cubic Inches | Cubic Feet | Liters | Cubic Meters |
| :---: | :---: | :---: | :---: | :---: |
| 1 dry pint $=$ | $\underline{33.6003125}$ | 0.01944463 | 0.5506105 | 0.0005506105 |
| 1 dry quart = | 67.200625 | 0.03888925 | 1.101221 | 0.001101221 |
| 1 peck $=$ | 537.605 | 0.311114 | 8.809768 | 0.008809768 |
| 1 bushel = | $\underline{150.42}$ | 1.244456 | 35.23907 | 0.03523907 |
| 1 cubic inch = | 1 | 0.0005787037 | 0.01638706 | 0.00001638706 |
| 1 cubic foot $=$ | 1728 | 1 | 28.31685 | 0.02831685 |
| 1 liter = | 61.02374 | 0.03531467 | 1 | $\underline{0.001}$ |
| 1 cubic meter $=$ | 61023.74 | 35.31467 | 1000 | 1 |

Units of Capacity or Volume-Liquid Volume Measure

| Units | Minims | Fluid Drams | Fluid Ounces | Gills |
| :---: | :---: | :---: | :---: | :---: |
| 1 minim = | 1 | $\underline{0.01666667}$ | 0.002083333 | 0.0005208333 |
| 1 fluid dram = | $\underline{60}$ | 1 | $\underline{0.125}$ | $\underline{0.03125}$ |
| 1 fluid ounce $=$ | 480 | 8 | 1 | 0.25 |
| 1 gill = | 1920 | $\underline{32}$ | $\underline{4}$ | 1 |
| 1 liquid pint $=$ | 7680 | $\underline{128}$ | 16 | $\underline{4}$ |
| 1 liquid quart = | 15360 | $\underline{256}$ | $\underline{32}$ | $\underline{8}$ |
| 1 gallon = | 61440 | $\underline{1024}$ | 128 | 32 |


| Units | Minims | Fluid Drams | Fluid Ounces | Gills |
| :--- | :--- | :--- | :--- | :--- |
| 1 cubic inch $=$ | 265.9740 | 4.432900 | 0.5541126 | 0.1385281 |
| 1 cubic foot $=$ | 459603.1 | 7660.052 | 957.5065 | 239.3766 |
| 1 milliliter $=$ | 16.23073 | 0.2705122 | 0.03381402 | 0.008453506 |
| 1 liter $=$ | 16230.73 | 270.5122 | 33.81402 | 8.453506 |


| Units | Liquid Pints | Liquid Quarts | Gallons | Cubic Inches |
| :---: | :---: | :---: | :---: | :---: |
| 1 minim $=$ | 0.0001302083 | 0.00006510417 | 0.00001627604 | 0.003759766 |
| 1 fluid dram = | $\underline{0.0078125}$ | $\underline{0.00390625}$ | 0.0009765625 | 0.22558594 |
| 1 fluid ounce = | $\underline{0.0625}$ | $\underline{0.03125}$ | $\underline{0.0078125}$ | 1.8046875 |
| 1 gill = | $\underline{0.25}$ | $\underline{0.125}$ | $\underline{0.03125}$ | $\underline{7.21875}$ |
| 1 liquid pint $=$ | 1 | $\underline{0.5}$ | $\underline{0.125}$ | $\underline{28.875}$ |
| 1 liquid quart = | $\underline{2}$ | 1 | 0.25 | $\underline{57.75}$ |
| 1 gallon = | $\underline{8}$ | $\underline{4}$ | 1 | $\underline{231}$ |
| 1 cubic inch = | 0.03463203 | 0.01731602 | 0.004329004 | 1 |
| 1 cubic foot $=$ | 59.84416 | 29.92208 | 7.480519 | 1728 |
| 1 milliliter = | 0.002113376 | 0.001056688 | 0.0002641721 | 0.06102374 |
| 1 liter = | 2.113376 | 1.056688 | 0.2641721 | 61.02374 |


| Units | Cubic Feet | Milliliters | Liters |
| :--- | :--- | :--- | :--- |
| 1 minim $=$ | 0.000002175790 | 0.06161152 | 0.00006161152 |
| 1 fluid dram $=$ | 0.0001305474 | 3.696691 | 0.003696691 |
| 1 fluid ounce $=$ | 0.001044379 | 29.57353 | 0.02957353 |
| 1 gill $=$ | 0.004177517 | 118.2941 | 0.1182941 |
| 1 liquid pint $=$ | 0.01671007 | 473.1765 | 0.4731765 |
| 1 liquid quart $=$ | 0.03342014 | 946.3529 | 0.9463529 |
| 1 gallon $=$ | 0.1336806 | 3785.412 | 3.785412 |
| 1 cubic inch $=$ | 0.0005787037 | 16.38706 | 0.01638706 |
| 1 cubic foot $=$ | $\underline{1}$ | 28316.85 | 28.31685 |
| 1 milliliter $=$ | 0.00003531467 | $\underline{1}$ | 0.001 |
| 1 liter $=$ | 0.03531467 | $\underline{1000}$ | 1 |

Units of Mass Not Less Than Avoirdupois Ounces

| Units | Avoirdupois Ounces | Avoirdupois Pounds | Short Hundred-weights | Short tons |
| :--- | :--- | :--- | :--- | :--- |
| 1 avoirdupois ounce $=$ | $\underline{1}$ | $\underline{0.0625}$ | $\underline{0.000625}$ | 0.00003125 |
| 1 avoirdupois pound $=$ | $\underline{16}$ | $\underline{1}$ | $\underline{0.01}$ | 0.0005 |
| 1 short hundredweight $=$ | $\underline{1600}$ | $\underline{100}$ | $\underline{1}$ | 0.05 |
| 1 short ton $=$ | $\underline{32000}$ | $\underline{2000}$ | $\underline{20}$ | 1 |
| 1 long ton $=$ | $\underline{35840}$ | $\underline{2240}$ | 0.022046 | 1.12 |
| 1 kilogram $=$ | 35.27396 | 2204.623 | 22.04623 | 0.001102311 |
| 1 metric ton $=$ | 35273.96 |  |  | 1.102311 |


| Units | Long Tons | Kilograms | Metric Tons |
| :---: | :---: | :---: | :---: |
| 1 avoirdupois ounce $=$ | 0.00002790179 | 0.028349523125 | $\underline{0.000028349523125 ~}$ |
| 1 avoirdupois pound = | 0.0004464286 | $\underline{0.45359237}$ | $\underline{0.00045359237}$ |
| 1 short hundredweight = | 0.04464286 | 45.359237 | $\underline{0.045359237}$ |
| 1 short ton = | 0.8928571 | 907.18474 | $\underline{0.90718474}$ |
| 1 long ton = | 1 | 1016.0469088 | $\underline{1.0160469088}$ |
| 1 kilogram = | 0.0009842065 | 1 | 0.001 |
| 1 metric ton $=$ | 0.9842065 | 1000 | 1 |

Units of Mass Not Greater Than Pounds and Kilograms

| Units | Grains | Apothecaries Scruples | Pennyweights | Avoirdupois Drams |
| :---: | :---: | :---: | :---: | :---: |
| 1 grain = | 1 | $\underline{0.05}$ | 0.04166667 | 0.03657143 |
| 1 apoth. scruple = | $\underline{20}$ | 1 | 0.8333333 | 0.7314286 |
| 1 penny weight = | $\underline{24}$ | 1.2 | 1 | 0.8777143 |
| 1 avdp. dram = | $\underline{27.34375}$ | 1.3671875 | 1.139323 | 1 |
| 1 apoth. dram = | $\underline{60}$ | $\underline{3}$ | $\underline{2.5}$ | 2.194286 |
| 1 avdp. ounce $=$ | 437.5 | $\underline{21.875}$ | 18.22917 | 16 |
| 1 apoth. or troy oz. $=$ | $\underline{480}$ | $\underline{24}$ | $\underline{20}$ | 17.55429 |
| 1 apoth. or troy pound $=$ | $\underline{5760}$ | $\underline{288}$ | $\underline{240}$ | 210.6514 |
| 1 avdp. pound $=$ | $\underline{7000}$ | 350 | 291.6667 | $\underline{256}$ |
| 1 milligram = | 0.01543236 | 0.0007716179 | 0.0006430149 | 0.0005643834 |
| 1 gram = | 15.43236 | 0.7716179 | 0.6430149 | 0.5643834 |
| 1 kilogram = | 15432.36 | 771.6179 | 643.0149 | 564.3834 |


| Units | Apothecaries <br> Drams | Avoirdupois Ounces | Apothecaries or Troy Ounces | Apothecaries or Troy Pounds |
| :---: | :---: | :---: | :---: | :---: |
| 1 grain $=$ | 0.01666667 | 0.002285714 | 0.002083333 | 0.0001736111 |
| 1 apoth. scruple $=$ | 0.3333333 | 0.04571429 | 0.04166667 | 0.003472222 |
| 1 pennyweight $=$ | 0.4 | 0.05485714 | 0.05 | 0.004166667 |
| 1 avdp. dram $=$ | 0.4557292 | $\underline{0.0625}$ | 0.5696615 | 0.004747179 |
| 1 apoth. dram $=$ | 1 | 0.1371429 | 0.125 | 0.01041667 |
| 1 avdp. ounce $=$ | 7.291667 | 1 | 0.9114583 | 0.07595486 |
| 1 apoth. or troy ounce $=$ | 8 | 1.097143 | 1 | 0.083333333 |
| 1 apoth. or troy pound $=$ | 96 | 13.16571 | 12 | 1 |
| 1 avdp. pound $=$ | 116.6667 | 16 | 14.58333 | 1.215278 |
| 1 milligram $=$ | 0.0002572060 | 0.00003527396 | 0.00003215075 | 0.000002679229 |
| 1 gram = | 0.2572060 | 0.03527396 | 0.03215075 | 0.002679229 |
| 1 kilogram = | 257.2060 | 35.27396 | 32.15075 | 2.679229 |


| Units | Avoirdupois Pounds | Milligrams | Grams | Kilograms |
| :---: | :---: | :---: | :---: | :---: |
| 1 grain = | 0.0001428571 | 64.79891 | $\underline{0.06479891}$ | 0.00006479891 |
| 1 apoth. scruple $=$ | 0.002857143 | 1295.9782 | 1.2959782 | 0.0012959782 |
| 1 penny weight = | 0.003428571 | 1555.17384 | 1.55517384 | 0.00155517384 |
| 1 avdp. dram = | 0.00390625 | $\frac{1771.845195312}{5}$ | 1.7718451953125 | 0.0017718451953125 |
| 1 apoth. dram = | 0.008571429 | 3887.9346 | 3.8879346 | 0.0038879346 |
| 1 avdp. ounce = | $\underline{0.0625}$ | $\underline{28349.523125}$ | $\underline{28.349523125}$ | 0.028349523125 |
| 1 apoth. or troy ounce $=$ | 0.06857143 | 31103.4768 | $\underline{31.1034768}$ | 0.0311034768 |
| 1 apoth. or troy pound = | 0.8228571 | 373241.7216 | 373.2417216 | $\underline{0.3732417216}$ |
| 1 avdp. pound $=$ | 1 | 453592.37 | 453.59237 | $\underline{0.45359237}$ |
| 1 milligram $=$ | 0.000002204623 | 1 | 0.001 | $\underline{0.000} 001$ |
| 1 gram = | 0.002204623 | 1000 | 1 | 0.001 |
| 1 kilogram = | 2.204623 | 1000000 | 1000 | 1 |

## 5. Tables of Equivalents

In these tables it is necessary to differentiate between the "international foot" and the "survey foot." Therefore, the survey foot is underlined.

When the name of a unit is enclosed in brackets (thus, [1 hand] . . .), this indicates (1) that the unit is not in general current use in the United States, or (2) that the unit is believed to be based on "custom and usage" rather than on formal authoritative definition.

Equivalents involving decimals are, in most instances, rounded off to the third decimal place except where they are exact, in which cases these exact equivalents are so designated. The equivalents of the imprecise units "tablespoon" and "teaspoon" are rounded to the nearest milliliter.

| Units of Length |  |
| :--- | :--- |
| angstrom $(\Delta)^{10}$ | 0.1 nanometer (exactly) |
|  | 0.0001 micrometer (exactly) |
| 1 cable's length | 0.0000001 millimeter (exactly) |
|  | 0.000000004 inch |
|  | 120 fathoms (exactly) |
| 1 centimeter (cm) | 720 feet (exactly) |
| 1 chain (ch) (Gunter's or surveyors) | 219 meters |
| 1 decimeter (dm) | 0.3937 inch |
| 1 dekameter (dam) | 66 feet (exactly) |
| 1 fathom | 20.1168 meters |
| 1 foot (ft) | 3.937 inches |
|  | 32.808 feet |

Units of Length

| 1 furlong (fur) | 10 chains (surveyors) (exactly) |
| :--- | :--- |
|  | 660 feet (exactly) |
|  | $1 / 8 \cup . S$. statute mile (exactly) |
| $[1$ hand] | 201.168 meters |
| 1 inch (in) | 4 inches |
| 1 kilometer (km) | 2.54 centimeters (exactly) |
| 1 league (land) | 0.621 mile |
| 1 link (li) (Gunter's or surveyors) | $3 \cup . S$. statute miles (exactly) |
|  | 4.828 kilometers |
| 1 meter (m) | 0.66 foot (exactly) |
|  | 0.201168 meter |
| 1 micrometer | 39.37 inches |
|  | 1.094 yards |
| 1 mil | 0.001 millimeter (exactly) |
|  | 0.000 039 37 inch |

10. The angstrom is basically defined as $10^{-10}$ meter.

| 1 mile (mi) (U.S. statute) ${ }^{11}$ | 5280 feet survey (exactly) 1.609 kilometers |
| :---: | :---: |
| 1 mile (mi) (international) | 5280 feet international (exactly) |
| 1 mile (mi) (international nautical) ${ }^{12}$ | 1.852 kilometers (exactly) <br> 1.151 survey miles |
| 1 millimeter (mm) | 0.03937 inch 0.001 meter (exactly) |
| 1 nanometer (nm) | 0.00000003937 inch 0.013837 inch (exactly) |
| 1 point (typography) | 1/72 inch (approximately) 0.351 millimeter |
| $1 \mathrm{rod}(\mathrm{rd})$, pole, or perch | $161 / 2$ feet (exactly) 5.0292 meters |
| 1 yard (yd) | 0.9144 meter (exactly) |
| Units of Area 1 acre $^{13}$ | 43560 square feet (exactly) 0.405 hectare |
| 1 are | 119.599 square yards 0.025 acre |
| 1 hectare | 2.471 acres |
| [1 square (building)] | 100 square feet |
| 1 square centimeter ( $\mathrm{cm}^{2}$ ) | 0.155 square inch |
| 1 square decimeter ( $\mathrm{dm}^{2}$ ) | 15.500 square inches |
| 1 square foot ( $\mathrm{ft}^{2}$ ) | 929.030 square centimeters |
| 1 square inch (in ${ }^{2}$ ) | 6.4516 square centimeters (exactly) |
| 1 square kilometer ( $\mathrm{km}^{2}$ ) | 247.104 acres 0.386 square mile |
| 1 square meter ( $\mathrm{m}^{2}$ ) | 1.196 square yards 10.764 square feet |
| 1 square mile ( $\mathrm{mi}^{2}$ ) | 258.999 hectares |
| 1 square millimeter ( $\mathrm{mm}^{2}$ ) | 0.002 square inch |
| 1 square rod ( $\mathrm{rd}^{2}$ ), sq pole, or sq perch | 25.293 square meters |
| 1 square yard ( $\mathrm{yd}^{2}$ ) | 0.836 square meter |

11. The term "statute mile" originated with Queen Elizabeth I who changed the definition of the mile from the Roman mile of 5000 feet to the statute mile of 5280 feet. The international mile and the U.S. statute mile differ by about 3 millimeters, although both are defined as being equal to 5280 feet. The international mile is based on the international foot ( 0.3048 meter) whereas the U.S. statute mile is based on the survey foot (1200/3937 meter).
12. The international nautical mile of 1852 meters ( 6076.11549 . . feet) was adopted effective July 1, 1954, for use in the United States. The value formerly used in the United States was 6080.20 feet $=1$ nautical (geographical or sea) mile.
13. The question is often asked as to the length of a side of an acre of ground. An acre is a unit of area containing 43560 square feet. It is not necessarily square, or even rectangular. But, if it is square, then the length of a side is equal to $\sqrt{43560 \mathrm{ft}^{2}}=208.710 \mathrm{ft}$ (not exact).

## Units of Capacity or Volume

| 1 barrel (bbl), liquid | 31 to 42 gallons ${ }^{14}$ |
| :--- | :--- |
| 1 barrel (bbl), standard for fruits, vegetables, and other dry commodities, except | 7056 cubic inches |
| cranberries | 105 dry quarts |
|  | 3.281 bushels, struck measure |
| 1 barrel (bbl), standard, cranberry | 5826 cubic inches |
|  | $86^{45 / 64}$ dry quarts <br> 2.709 bushels, struck measure |
| 1 bushel (bu) (U.S.) struck measure | 2150.42 cubic inches (exactly) |
|  | 35.238 liters |
| $[1$ bushel, heaped (U.S.)] | 2747.715 cubic inches |
|  | 1.278 bushels, struck measure ${ }^{15}$ |
| $[1$ bushel (bu) (British Imperial) (struck measure)] | 1.032 U.S. bushels, struck |
|  | measure |
| 1 cord (cd) (firewood) | 2219.36 cubic inches |
| 1 cubic centimeter (cm ${ }^{3}$ ) | 128 cubic feet (exactly) |
| 1 cubic decimeter (dm ${ }^{3}$ ) | 0.061 cubic inch |
| 1 cubic foot (ft $\left.{ }^{3}\right)$ | 61.024 cubic inches |

## Units of Capacity or Volume

| 1 cubic inch $\left(\right.$ in $\left.^{3}\right)$ | 0.554 fluid ounce |
| :--- | :--- |
|  | 4.433 fluid drams |
|  | 16.387 cubic centimeters |
| 1 cubic meter $\left(\mathrm{m}^{3}\right)$ | 1.308 cubic yards |
| 1 cubic yard $\left(\mathrm{yd}^{3}\right)$ | 0.765 cubic meter |
| 1 cup, measuring | 8 fluid ounces (exactly) |
|  | 237 milliliters |
|  | $1 / 2$ liquid pint (exactly) |
| 1 dekaliter $($ daL) | 2.642 gallons |
|  | 1.135 pecks |
|  | $1 / 8$ fluid ounce (exactly) |

14. There are a variety of "barrels" established by law or usage. For example, federal taxes on fermented liquors are based on a barrel of 31 gallons; many state laws fix the "barrel for liquids" as $31 \frac{1}{2}$ gallons; one state fixes a 36 -gallon barrel for cistern measurement; federal law recognizes a 40 -gallon barrel for "proof spirits"; by custom, 42 gallons comprise a barrel of crude oil or petroleum products for statistical purposes, and this equivalent is recognized "for liquids" by four states.
15. Frequently recognized as $1 \frac{1}{4}$ bushels, struck measure.

| 1 dram, fluid (or liquid) (fl dr) or $f \mathbf{3}$ ) (U.S.) | 0.226 cubic inch <br> 3.697 milliliters <br> 1.041 British fluid drachms |
| :---: | :---: |
| [1 drachm, fluid (fl dr) (British)] | 0.961 U.S. fluid dram 0.217 cubic inch 3.552 milliliters |
| 1 gallon (gal) (U.S.) | 231 cubic inches (exactly) <br> 3.785 liters <br> 0.833 British gallon <br> 128 U.S. fluid ounces (exactly) |
| [1 gallon (gal) (British Imperial)] | 277.42 cubic inches <br> 1201 U.S. gallons <br> 4.546 liters <br> 160 British fluid ounces (exactly) |
| 1 gill (gi) | 7.219 cubic inches 4 fluid ounces (exactly) 0.118 liter |
| 1 hectoliter (hL) | 26.418 gallons 2.838 bushels |
| 1 liter (1 cubic decimeter exactly) | 1.057 liquid quarts 0.908 dry quart <br> 61.025 cubic inches |
| 1 milliliter (mL) | 0.271 fluid dram 16.231 minims 0.061 cubic inch |
| 1 ounce, fluid (or liquid) (fl oz) or $f \mathbf{Z}$ ) (U.S.) | 1.805 cubic inches 29.573 milliliters <br> 1.041 British fluid ounces |
| [1 ounce, fluid (fl oz) (British)] | 0.961 U.S. fluid ounce 1.734 cubic inches 28.412 milliliters |
| 1 peck (pk) | 8.810 liters |
| 1 pint (pt), dry | 33.600 cubic inches 0.551 liter |
| 1 pint (pt), liquid | 28.875 cubic inches exactly 0.473 liter |
| 1 quart (qt), dry (U.S.) | 67.201 cubic inches <br> 1.101 liters <br> 0.969 British quart |
| 1 quart (qt), liquid (U.S.) | $\begin{aligned} & \text { 57.75 cubic inches (exactly) } \\ & \text { 0.946 liter } \\ & \text { 0.833 British quart } \\ & \hline \end{aligned}$ |

## Units of Capacity or Volume

| [1 quart (qt) (British)] | 69.354 cubic inches <br> 1.032 U.S. dry quarts <br> 1.201 U.S. liquid quarts |
| :---: | :---: |
| 1 tablespoon, measuring | 3 teaspoons (exactly) <br> 15 milliliters <br> 4 fluid drams <br> $1 / 2$ fluid ounce (exactly) |
| 1 teaspoon, measuring | ```1/3 tablespoon (exactly) 5 milliliters 1/3 fluid drams }\mp@subsup{}{}{16``` |
| 1 water ton (English) | 270.91 U.S. gallons 224 British Imperial gallons (exactly) |
| Units of Mass 1 assay ton ${ }^{17}$ (AT) | 29.167 grams |
| 1 carat (c) | 200 milligrams (exactly) <br> 3.086 grains |
| 1 dram apothecaries (dr ap or $\mathbf{3}$ ) | 60 grains (exactly). <br> 3.888 grams |
| 1 dram avoirdupois (dr avdp) | $27^{11} / 32(=27.344)$ grains 1.772 grams |
| 1 gamma ( $\gamma$ ) | 1 microgram (exactly) |
| 1 grain | 64.79891 milligrams (exactly) |
| 1 gram (g) | 15.432 grains 0.035 ounce, avoirdupois |
| 1 hundredweight, gross or long ${ }^{18}$ (gross cwt) | 112 pounds (exactly) 50.802 kilograms |
| 1 hundredweight, gross or short (cwt or net cwt) | 100 pounds (exactly) 45.359 kilograms |
| 1 kilogram (kg) | 2.205 pounds |
| 1 microgram ( $\mu \mathrm{g}$ ) | 0.000001 gram (exactly) |
| 1 milligram (mg) | 0.015 grain |
| 1 ounce, avoirdupois (oz avdp) | 437.5 grains (exactly) <br> 0.911 troy or apothecaries ounce 28.350 grams |
| 1 ounce, troy, or apothecaries (oz t or oz ap or $\mathbf{3}$ ) | 480 grains (exactly) <br> 1.097 avoirdupois ounces <br> 31.103 grams |
| 1 pennyweight (dwt) | 1.555 grams |

16. The equivalent " 1 teaspoon $=1 / 1 / 3$ fluid drams" has been found by the Bureau to correspond more closely with the actual capacities of "measuring" and silver teaspoons than the equivalent " 1 teaspoon $=1$ fluid dram," which is given by a number of dictionaries.
17. Used in assaying. The assay ton bears the same relation to the milligram that a ton of 2000 pounds avoirdupois bears to the ounce troy; hence the mass in milligrams of precious metal obtained from one assay ton of ore gives directly the number of troy ounces to the net ton. 18. The gross or long ton and hundredweight are used commercially in the United States to a limited extent only, usually in restricted industrial fields. These units are the same as the British "ton" and "hundredweight."

Units of Mass

| 1 point | 0.01 carat <br> 2 milligrams |
| :--- | :--- |
| 1 pound, avoirdupois (lb avdp) | 7000 grains (exactly) |
|  | 1.215 troy or apothecaries pounds |
|  | 453.59237 grams (exactly) |
| 1 pound, troy or apothecaries (lb t or lb ap) | 5760 grains (exactly) |
|  | 0.823 avoirdupois pound |
| 1 scruple (s ap or $Э$ ) | 373.242 grams |
| 1 ton, gross or long ${ }^{19}$ | 20 grains (exactly) |
|  | 1.296 grams |
| 1 ton, metric (t) | 2240 pounds (exactly) |
|  | 1.12 net tons (exactly) |
| 1 ton, net or short | 1.016 metric tons |
|  | 2204.623 pounds |
|  | 0.984 gross ton |
|  | 1.102 net tons |

19. The gross or long ton and hundredweight are used commercially in the United States to a limited extent only, usually in restricted industrial fields. These units are the same as the British "ton" and "hundredweight."

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### 2.0.0 Executive Order 12770—The Metric Conversion Law

On July 25, 1991, President George H.W.Bush signed Executive Order 12770, referred to as the Metric Conversion Law, directing U.S. businesses and professionals to use the SI (metric) system as the "preferred" system for weights and measures.

The designation SI is derived from Systems International d' unites, the current international designation for the standard metric system of weights and measures.

Although engineering, design, and construction professionals in the United States have not fully adopted conversion to the metric system, the increased globalization of these professions requires familiarization with metric conversion units.

Avoirdupois units of mass (weight) differ from the Troy system, which is now the accepted weight denomination for precious metals. The Troy pound contains 12 ounces, whereas the Avoirdupois system in current use today for almost everything except precious metals contains 16 ounces to the pound.

Imperial units such as inches, feet, yards, and miles are similar to customary U.S. units, but there are some exceptions. The Imperial system uses the term stone instead of pounds-a stone is the equivalent of 14 U.S. pounds.

The Imperial system also uses long hundredweight, which is equal to 112 U.S. pounds, and it also uses the long ton, equivalent to 2240 U.S. pounds.

Metric to U.S. and U.S. to metric conversion tables in this section provide easy conversion for:

1. Length, width, and thickness
2. Area-Volume
3. Weight
4. Temperature

### 2.0.1 Inches to Feet Conversion Table

| Inches | Feet | Inches | Feet | Inches | Feet | Inches | Feet |
| :---: | :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| 0 | 0.00 | 13 | 1.08 | 35 | 2.91 | 57 | 4.75 |
| 0.1 | 0.008 | 14 | 1.16 | 36 | 3.00 | 58 | 4.83 |
| 0.2 | 0.016 | 15 | 1.25 | 37 | 3.08 | 59 | 4.91 |
| 0.3 | 0.025 | 16 | 1.33 | 38 | 3.16 | 60 | 5.00 |
| 0.4 | 0.033 | 17 | 1.41 | 39 | 3.25 | 61 | 5.08 |
| 0.5 | 0.041 | 18 | 1.50 | 40 | 3.33 | 62 | 5.16 |
| 0.6 | 0.05 | 19 | 1.58 | 41 | 3.41 | 63 | 5.25 |
| 0.7 | 0.058 | 20 | 1.66 | 42 | 3.50 | 64 | 5.33 |
| 0.8 | 0.066 | 21 | 1.75 | 43 | 3.58 | 65 | 5.41 |
| 0.9 | 0.075 | 22 | 1.83 | 44 | 3.66 | 66 | 5.50 |
| 1 | 0.08 | 23 | 1.91 | 45 | 3.75 | 67 | 5.58 |
| 2 | 0.16 | 24 | 2.00 | 46 | 3.83 | 68 | 5.66 |
| 3 | 0.25 | 25 | 2.08 | 47 | 3.91 | 69 | 5.75 |
| 4 | 0.33 | 26 | 2.16 | 48 | 4.00 | 70 | 5.83 |
| 5 | 0.41 | 27 | 2.25 | 49 | 4.08 | 71 | 5.91 |
| 6 | 0.5 | 28 | 2.33 | 50 | 4.16 | 72 | 6.00 |
| 7 | 0.58 | 29 | 2.41 | 51 | 4.25 | 73 | 6.08 |
| 8 | 0.66 | 30 | 2.50 | 52 | 4.33 | 74 | 6.16 |
| 9 | 0.75 | 31 | 2.58 | 53 | 4.41 | 75 | 6.25 |
| 10 | 0.83 | 32 | 2.66 | 54 | 4.50 | 76 | 6.33 |
| 11 | 0.91 | 33 | 2.75 | 55 | 4.58 | 77 | 6.41 |
| 12 | 1.00 | 34 | 2.83 | 56 | 4.66 | 78 | 6.50 |


| Inches | Feet | Inches | Feet | Inches | Feet | Inches | Feet |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 79 | 6.58 | 87 | 7.25 | 95 | 7.91 | 400 | 33.30 |
| 80 | 6.66 | 88 | 7.33 | 96 | 8.00 | 500 | 41.60 |
| 81 | 6.75 | 89 | 7.41 | 97 | 8.08 | 600 | 50.00 |
| 82 | 6.83 | 90 | 7.50 | 98 | 8.16 | 700 | 58.30 |
| 83 | 6.91 | 91 | 7.58 | 99 | 8.25 | 800 | 66.60 |
| 84 | 7.00 | 92 | 7.66 | 100 | 8.30 | 900 | 75.00 |
| 85 | 7.08 | 93 | 7.75 | 200 | 16.60 |  |  |
| 86 | 7.16 | 94 | 7.83 | 300 | 25.00 |  |  |

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### 2.0.2 Inches to Centimeter Conversion

| Inches | Centimeters | Inches | Centimeters | Inches | Centimeters | Inches | Centimeters |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0 | 0.00 | 28 | 71.12 | 65 | 165.1 | 102 | 259.08 |
| 0.1 | 0.254 | 29 | 73.66 | 66 | 167.64 | 103 | 261.62 |
| 0.2 | 0.508 | 30 | 76.20 | 67 | 170.18 | 104 | 264.16 |
| 0.3 | 0.762 | 31 | 78.74 | 68 | 172.72 | 105 | 266.7 |
| 0.4 | 1.016 | 32 | 81.28 | 69 | 175.26 | 106 | 269.24 |
| 0.5 | 1.27 | 33 | 83.82 | 70 | 177.8 | 107 | 271.78 |
| 0.6 | 1.524 | 34 | 86.36 | 71 | 180.34 | 108 | 274.32 |
| 0.7 | 1.778 | 35 | 88.90 | 72 | 182.88 | 109 | 276.86 |
| 0.8 | 2.032 | 36 | 91.44 | 73 | 185.42 | 110 | 279.4 |
| 0.9 | 2.286 | 37 | 93.98 | 74 | 187.96 | 111 | 281.94 |
| 1 | 2.54 | 38 | 96.52 | 75 | 190.5 | 112 | 284.48 |
| 2 | 5.08 | 39 | 99.06 | 76 | 193.04 | 113 | 287.02 |
| 3 | 7.62 | 40 | 101.6 | 77 | 195.58 | 114 | 289.56 |
| 4 | 10.16 | 41 | 104.14 | 78 | 198.12 | 115 | 292.1 |
| 5 | 12.70 | 42 | 106.68 | 79 | 200.66 | 116 | 294.64 |
| 6 | 15.24 | 43 | 109.22 | 80 | 203.2 | 117 | 297.18 |
| 7 | 17.78 | 44 | 111.76 | 81 | 205.74 | 118 | 299.72 |
| 8 | 20.32 | 45 | 114.3 | 82 | 208.28 | 119 | 302.26 |
| 9 | 22.86 | 46 | 116.84 | 83 | 210.82 | 120 | 304.8 |
| 10 | 25.40 | 47 | 119.38 | 84 | 213.36 | 121 | 307.34 |
| 11 | 27.94 | 48 | 121.92 | 85 | 215.9 | 122 | 309.88 |
| 12 | 30.48 | 49 | 124.46 | 86 | 218.44 | 123 | 312.42 |
| 13 | 33.02 | 50 | 127.0 | 87 | 220.98 | 124 | 314.96 |
| 14 | 35.56 | 51 | 129.54 | 88 | 223.52 | 125 | 317.5 |
| 15 | 38.10 | 52 | 132.08 | 89 | 226.06 | 126 | 320.04 |
| 16 | 40.64 | 53 | 134.62 | 90 | 228.6 | 127 | 322.58 |
| 17 | 43.18 | 54 | 137.16 | 91 | 231.14 | 128 | 325.12 |
| 18 | 45.72 | 55 | 139.7 | 92 | 233.68 | 129 | 327.66 |
| 19 | 48.26 | 56 | 142.24 | 93 | 236.22 | 130 | 330.2 |
| 20 | 50.80 | 57 | 144.78 | 94 | 238.76 | 131 | 332.74 |
| 21 | 53.34 | 58 | 147.32 | 95 | 241.3 | 132 | 335.28 |
| 22 | 55.88 | 59 | 149.86 | 96 | 243.84 | 133 | 337.82 |
| 23 | 58.42 | 60 | 152.4 | 97 | 246.38 | 134 | 340.36 |
| 24 | 60.96 | 61 | 154.94 | 98 | 248.92 | 135 | 342.9 |
| 25 | 63.50 | 62 | 157.48 | 99 | 251.46 | 136 | 345.44 |
| 26 | 66.04 | 63 | 160.02 | 100 | 254.0 | 137 | 347.98 |
| 27 | 68.58 | 64 | 162.56 | 101 | 256.54 | 138 | 350.52 |


| Inches | Centimeters | Inches | Centimeters | Inches | Centimeters |
| :--- | :---: | :---: | :---: | :---: | :---: |
| 139 | 353.06 | 150 | 381.0 | 300 | 762.0 |
| 140 | 355.6 | 151 | 383.54 | 400 | 1016 |
| 141 | 358.14 | 152 | 386.08 | 500 | 1270 |
| 142 | 360.68 | 153 | 388.62 | 600 | 1524 |
| 143 | 363.22 | 154 | 391.16 | 700 | 1778 |
| 144 | 365.76 | 155 | 393.7 | 800 | 2032 |
| 145 | 368.3 | 156 | 396.24 | 900 | 2286 |
| 146 | 370.84 | 157 | 398.78 |  |  |
| 147 | 373.38 | 158 | 401.32 |  |  |
| 148 | 375.92 | 159 | 403.86 |  |  |
| 149 | 378.46 | 200 | 508.0 |  |  |

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### 2.0.3 Centimeter to Inches Conversion

| Centimeters | Inches | Centimeters | Inches | Centimeters | Inches | Centimeters | Inches |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0 | 0.00 | 24 | 9.44 | 57 | 22.440 | 90 | 35.433 |
| 0.1 | 0.039 | 25 | 9.84 | 58 | 22.834 | 91 | 35.826 |
| 0.2 | 0.078 | 26 | 10.23 | 59 | 23.228 | 92 | 36.220 |
| 0.3 | 0.118 | 27 | 10.62 | 60 | 23.622 | 93 | 36.614 |
| 0.4 | 0.157 | 28 | 11.02 | 61 | 24.015 | 94 | 37.007 |
| 0.5 | 0.196 | 29 | 11.41 | 62 | 24.409 | 95 | 37.401 |
| 0.6 | 0.236 | 30 | 11.81 | 63 | 24.803 | 96 | 37.795 |
| 0.7 | 0.275 | 31 | 12.20 | 64 | 25.196 | 97 | 38.188 |
| 0.8 | 0.314 | 32 | 12.59 | 65 | 25.590 | 98 | 38.582 |
| 0.9 | 0.354 | 33 | 12.99 | 66 | 25.984 | 99 | 38.976 |
| 1 | 0.39 | 34 | 13.38 | 67 | 26.377 | 100 | 39.370 |
| 2 | 0.78 | 35 | 13.77 | 68 | 26.771 | 101 | 39.763 |
| 3 | 1.18 | 36 | 14.17 | 69 | 27.165 | 102 | 40.157 |
| 4 | 1.57 | 37 | 14.56 | 70 | 27.559 | 103 | 40.551 |
| 5 | 1.96 | 38 | 14.96 | 71 | 27.952 | 104 | 40.944 |
| 6 | 2.36 | 39 | 15.35 | 72 | 28.346 | 105 | 41.338 |
| 7 | 2.75 | 40 | 15.748 | 73 | 28.740 | 106 | 41.732 |
| 8 | 3.14 | 41 | 16.141 | 74 | 29.133 | 107 | 42.125 |
| 9 | 3.54 | 42 | 16.535 | 75 | 29.527 | 108 | 42.519 |
| 10 | 3.93 | 43 | 16.929 | 76 | 29.921 | 109 | 42.913 |
| 11 | 4.33 | 44 | 17.322 | 77 | 30.314 | 110 | 43.307 |
| 12 | 4.72 | 45 | 17.716 | 78 | 30.708 | 111 | 43.700 |
| 13 | 5.11 | 46 | 18.110 | 79 | 31.102 | 112 | 44.094 |
| 14 | 5.51 | 47 | 18.503 | 80 | 31.496 | 113 | 44.488 |
| 15 | 5.90 | 48 | 18.897 | 81 | 31.889 | 114 | 44.881 |
| 16 | 6.29 | 49 | 19.291 | 82 | 32.283 | 115 | 45.275 |
| 17 | 6.69 | 50 | 19.685 | 83 | 32.677 | 116 | 45.669 |
| 18 | 7.08 | 51 | 20.078 | 84 | 33.070 | 117 | 46.062 |
| 19 | 7.48 | 52 | 20.472 | 85 | 33.464 | 118 | 46.456 |
| 20 | 7.87 | 53 | 20.866 | 86 | 33.858 | 119 | 46.850 |
| 21 | 8.26 | 54 | 21.259 | 87 | 34.251 | 120 | 47.244 |
| 22 | 8.66 | 55 | 21.653 | 88 | 34.645 | 121 | 47.637 |
| 23 | 9.05 | 56 | 22.047 | 89 | 35.039 | 122 | 48.031 |


| Centimeters | Inches | Centimeters | Inches | Centimeters | Inches | Centimeters | Inches |
| :---: | :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| 123 | 48.425 | 135 | 53.149 | 147 | 57.874 | 159 | 62.598 |
| 124 | 48.818 | 136 | 53.543 | 148 | 58.267 | 200 | 78.70 |
| 125 | 49.212 | 137 | 53.937 | 149 | 58.661 | 300 | 118.1 |
| 126 | 49.606 | 138 | 54.330 | 150 | 59.055 | 400 | 157.4 |
| 127 | 50.00 | 139 | 54.724 | 151 | 59.448 | 500 | 196.8 |
| 128 | 50.393 | 140 | 55.118 | 152 | 59.842 | 600 | 236.2 |
| 129 | 50.787 | 141 | 55.511 | 153 | 60.236 | 700 | 275.5 |
| 130 | 51.181 | 142 | 55.905 | 154 | 60.629 | 800 | 314.9 |
| 131 | 51.574 | 143 | 56.299 | 155 | 61.023 | 900 | 354.3 |
| 132 | 51.968 | 144 | 56.692 | 156 | 61.417 |  |  |
| 133 | 52.362 | 145 | 57.086 | 157 | 61.811 |  |  |
| 134 | 52.755 | 146 | 57.480 | 158 | 62.204 |  |  |

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### 2.0.4 Feet to Meters Conversion

| Feet | Meters | Feet | Meters | Feet | Meters | Feet | Meters |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0 | 0.00 | 25 | 7.62 | 59 | 17.983 | 93 | 28.346 |
| 0.1 | 0.030 | 26 | 7.92 | 60 | 18.288 | 94 | 28.651 |
| 0.2 | 0.060 | 27 | 8.22 | 61 | 18.592 | 95 | 28.958 |
| 0.3 | 0.091 | 28 | 8.53 | 62 | 18.897 | 96 | 29.260 |
| 0.4 | 0.121 | 29 | 8.83 | 63 | 19.202 | 97 | 29.565 |
| 0.5 | 0.152 | 30 | 9.14 | 64 | 19.507 | 98 | 29.870 |
| 0.6 | 0.182 | 31 | 9.44 | 65 | 19.812 | 99 | 30.175 |
| 0.7 | 0.213 | 32 | 9.75 | 66 | 20.116 | 100 | 30.48 |
| 0.8 | 0.243 | 33 | 10.05 | 67 | 20.421 | 101 | 30.784 |
| 0.9 | 0.274 | 34 | 10.36 | 68 | 20.726 | 102 | 31.089 |
| 1 | 0.30 | 35 | 10.66 | 69 | 21.031 | 103 | 31.394 |
| 2 | 0.60 | 36 | 10.97 | 70 | 21.336 | 104 | 31.699 |
| 3 | 0.91 | 37 | 11.27 | 71 | 21.640 | 105 | 32.004 |
| 4 | 1.21 | 38 | 11.58 | 72 | 21.945 | 106 | 32.308 |
| 5 | 1.52 | 39 | 11.88 | 73 | 22.250 | 107 | 32.613 |
| 6 | 1.82 | 40 | 12.192 | 74 | 22.555 | 108 | 32.918 |
| 7 | 2.13 | 41 | 12.496 | 75 | 22.86 | 109 | 33.223 |
| 8 | 2.43 | 42 | 12.801 | 76 | 23.164 | 110 | 33.528 |
| 9 | 2.74 | 43 | 13.106 | 77 | 23.469 | 111 | 33.832 |
| 10 | 3.04 | 44 | 13.411 | 78 | 23.774 | 112 | 34.137 |
| 11 | 3.35 | 45 | 13.716 | 79 | 24.079 | 113 | 34.442 |
| 12 | 3.65 | 46 | 14.020 | 80 | 24.384 | 114 | 34.747 |
| 13 | 3.96 | 47 | 14.325 | 81 | 24.688 | 115 | 35.052 |
| 14 | 4.26 | 48 | 14.630 | 82 | 24.993 | 116 | 35.356 |
| 15 | 4.57 | 49 | 14.935 | 83 | 25.298 | 117 | 35.661 |
| 16 | 4.87 | 50 | 15.24 | 84 | 25.603 | 118 | 35.966 |
| 17 | 5.18 | 51 | 15.544 | 85 | 25.908 | 119 | 36.271 |
| 18 | 5.48 | 52 | 15.849 | 86 | 26.212 | 120 | 36.576 |
| 19 | 5.79 | 53 | 16.154 | 87 | 26.517 | 121 | 36.880 |
| 20 | 6.09 | 54 | 16.459 | 88 | 26.822 | 122 | 37.185 |
| 21 | 6.40 | 55 | 16.764 | 89 | 27.127 | 123 | 37.490 |
| 22 | 6.70 | 56 | 17.068 | 90 | 27.432 | 124 | 37.795 |
| 23 | 7.01 | 57 | 17.373 | 91 | 27.736 | 125 | 38.10 |
| 24 | 7.31 | 58 | 17.678 | 92 | 28.041 | 126 | 38.404 |


| Feet | Meters | Feet | Meters | Feet | Meters | Feet | Meters |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 127 | 38.709 | 138 |  | 149 |  | 200 | 60.90 |
| 128 | 39.014 | 139 |  | 150 |  | 300 | 91.40 |
| 129 | 39.319 | 140 |  | 151 |  | 400 | 121.9 |
| 130 |  | 141 |  | 152 |  | 500 | 152.4 |
| 131 |  | 142 |  | 153 |  | 600 | 182.8 |
| 132 |  | 143 |  | 154 |  | 700 | 213.3 |
| 133 |  | 144 |  | 155 |  | 800 | 243.8 |
| 134 |  | 145 |  | 156 |  | 900 | 274.3 |
| 135 |  | 146 |  | 157 |  |  |  |
| 136 |  | 147 |  | 158 |  |  |  |
| 137 |  | 148 |  | 159 |  |  |  |

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### 2.0.5 Meters to Feet Conversion

| Meters | Feet | Meters | Feet | Meters | Feet | Meters | Feet |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0 | 0.00 | 27 | 88.58 | 63 | 206.692 | 99 | 324.803 |
| 0.1 | 0.328 | 28 | 91.86 | 64 | 209.973 | 100 | 328.083 |
| 0.2 | 0.656 | 29 | 95.14 | 65 | 213.254 | 101 | 331.364 |
| 0.3 | 0.984 | 30 | 98.42 | 66 | 216.535 | 102 | 334.645 |
| 0.4 | 1.312 | 31 | 101.70 | 67 | 219.816 | 103 | 337.926 |
| 0.5 | 1.640 | 32 | 104.98 | 68 | 223.097 | 104 | 341.207 |
| 0.6 | 1.968 | 33 | 108.26 | 69 | 226.377 | 105 | 344.488 |
| 0.7 | 2.296 | 34 | 111.54 | 70 | 229.658 | 106 | 347.769 |
| 0.8 | 2.624 | 35 | 114.82 | 71 | 232.939 | 107 | 351.049 |
| 0.9 | 2.952 | 36 | 118.11 | 72 | 236.220 | 108 | 354.330 |
| 1 | 3.28 | 37 | 121.39 | 73 | 239.501 | 109 | 357.611 |
| 2 | 6.56 | 38 | 124.67 | 74 | 242.782 | 110 | 360.892 |
| 3 | 9.84 | 39 | 127.95 | 75 | 246.062 | 111 | 364.173 |
| 4 | 13.12 | 40 | 131.233 | 76 | 249.343 | 112 | 367.454 |
| 5 | 16.40 | 41 | 134.514 | 77 | 252.624 | 113 | 370.734 |
| 6 | 19.68 | 42 | 137.795 | 78 | 255.905 | 114 | 374.015 |
| 7 | 22.96 | 43 | 141.076 | 79 | 259.186 | 115 | 377.296 |
| 8 | 26.24 | 44 | 144.356 | 80 | 262.467 | 116 | 380.577 |
| 9 | 29.52 | 45 | 147.637 | 81 | 265.748 | 117 | 383.858 |
| 10 | 32.80 | 46 | 150.918 | 82 | 269.028 | 118 | 387.139 |
| 11 | 36.08 | 47 | 154.199 | 83 | 272.309 | 119 | 390.419 |
| 12 | 39.37 | 48 | 157.480 | 84 | 275.590 | 120 | 393.700 |
| 13 | 42.65 | 49 | 160.761 | 85 | 278.871 | 121 | 396.981 |
| 14 | 45.93 | 50 | 164.041 | 86 | 282.152 | 122 | 400.262 |
| 15 | 49.21 | 51 | 167.322 | 87 | 285.433 | 123 | 403.543 |
| 16 | 52.49 | 52 | 170.603 | 88 | 288.713 | 124 | 406.824 |
| 17 | 55.77 | 53 | 173.884 | 89 | 291.994 | 125 | 410.104 |
| 18 | 59.05 | 54 | 177.165 | 90 | 295.275 | 126 | 413.385 |
| 19 | 62.33 | 55 | 180.446 | 91 | 298.556 | 127 | 416.666 |
| 20 | 65.61 | 56 | 183.727 | 92 | 301.837 | 128 | 419.947 |
| 21 | 68.89 | 57 | 187.007 | 93 | 305.118 | 129 | 423.228 |
| 22 | 72.17 | 58 | 190.288 | 94 | 308.398 | 130 | 426.508 |
| 23 | 75.45 | 59 | 193.569 | 95 | 311.679 | 131 | 429.79 |
| 24 | 78.74 | 60 | 196.850 | 96 | 314.960 | 132 | 433.07 |
| 25 | 82.02 | 61 | 200.131 | 97 | 318.241 | 133 | 436.351 |
| 26 | 85.30 | 62 | 203.412 | 98 | 321.522 | 134 | 439.632 |


| Meters | Feet | Meters | Feet | Meters | Feet | Meters | Feet |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 135 | 442.913 | 144 | 472.440 | 153 | 501.968 | 400 | 1312.3 |
| 136 | 444.194 | 145 | 475.721 | 154 | 505.249 | 500 | 1640.4 |
| 137 | 449.475 | 146 | 479.002 | 155 | 508.530 | 600 | 1968.5 |
| 138 | 452.755 | 147 | 482.283 | 156 | 511.811 | 700 | 2296.5 |
| 139 | 456.036 | 148 | 485.564 | 157 | 515.091 | 800 | 2624.6 |
| 140 | 459.317 | 149 | 488.845 | 158 | 518.372 | 900 | 2952.7 |
| 141 | 462.598 | 150 | 492.125 | 159 | 521.653 |  |  |
| 142 | 465.879 | 151 | 495.406 | 200 | 656.1 |  |  |
| 143 | 469.160 | 152 | 498.687 | 300 | 984.2 |  |  |

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### 2.0.6 Acres to Hectares Conversion

| Acres | Hectares | Acres | Hectares | Acres | Hectares | Acres | Hectares |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0 | 0.00 | 21 | 8.49 | 51 | 20.63 | 81 | 32.77 |
| 0.1 | 0.040 | 22 | 8.90 | 52 | 21.04 | 82 | 33.18 |
| 0.2 | 0.080 | 23 | 9.30 | 53 | 21.44 | 83 | 33.58 |
| 0.3 | 0.121 | 24 | 9.71 | 54 | 21.85 | 84 | 33.99 |
| 0.4 | 0.161 | 25 | 10.11 | 55 | 22.25 | 85 | 34.39 |
| 0.5 | 0.202 | 26 | 10.52 | 56 | 22.66 | 86 | 34.80 |
| 0.6 | 0.242 | 27 | 10.92 | 57 | 23.06 | 87 | 35.20 |
| 0.7 | 0.283 | 28 | 11.33 | 58 | 23.47 | 88 | 35.61 |
| 0.8 | 0.323 | 29 | 11.73 | 59 | 23.87 | 89 | 36.01 |
| 0.9 | 0.364 | 30 | 12.14 | 60 | 24.28 | 90 | 36.42 |
| 1 | 0.40 | 31 | 12.54 | 61 | 24.68 | 91 | 36.82 |
| 2 | 0.80 | 32 | 12.94 | 62 | 25.09 | 92 | 37.23 |
| 3 | 1.21 | 33 | 13.35 | 63 | 25.49 | 93 | 37.63 |
| 4 | 1.61 | 34 | 13.75 | 64 | 25.89 | 94 | 38.04 |
| 5 | 2.02 | 35 | 14.16 | 65 | 26.30 | 95 | 38.44 |
| 6 | 2.42 | 36 | 14.56 | 66 | 26.70 | 96 | 38.84 |
| 7 | 2.83 | 37 | 14.97 | 67 | 27.11 | 97 | 39.25 |
| 8 | 3.23 | 38 | 15.37 | 68 | 27.51 | 98 | 39.65 |
| 9 | 3.64 | 39 | 15.78 | 69 | 27.92 | 99 | 40.06 |
| 10 | 4.04 | 40 | 16.18 | 70 | 28.32 | 100 | 40.40 |
| 11 | 4.45 | 41 | 16.59 | 71 | 28.73 | 200 | 80.90 |
| 12 | 4.85 | 42 | 16.99 | 72 | 29.13 | 300 | 121.4 |
| 13 | 5.26 | 43 | 17.40 | 73 | 29.54 | 400 | 161.8 |
| 14 | 5.66 | 44 | 77.80 | 74 | 29.94 | 500 | 202.3 |
| 15 | 6.07 | 45 | 18.21 | 75 | 30.35 | 600 | 242.8 |
| 16 | 6.47 | 46 | 18.61 | 76 | 30.75 | 700 | 283.2 |
| 17 | 6.87 | 47 | 19.02 | 77 | 31.16 | 800 | 323.7 |
| 18 | 7.28 | 48 | 19.42 | 78 | 31.56 | 900 | 364.2 |
| 19 | 7.68 | 49 | 79.82 | 79 | 31.97 |  |  |
| 20 | 80 |  |  |  |  |  |  |

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2.0.7 Hectares to Acres Conversion

| Hectares | Acres | Hectares | Acres | Hectares | Acres | Hectares | Acres |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0 | 0.00 | 21 | 51.89 | 51 | 126.02 | 81 | 200.15 |
| 0.1 | 0.247 | 22 | 54.36 | 52 | 128.49 | 82 | 202.62 |
| 0.2 | 0.494 | 23 | 56.83 | 53 | 130.96 | 83 | 205.09 |
| 0.3 | 0.741 | 24 | 59.30 | 54 | 133.43 | 84 | 207.56 |
| 0.4 | 0.988 | 25 | 61.77 | 55 | 135.90 | 85 | 210.03 |
| 0.5 | 1.235 | 26 | 64.24 | 56 | 138.37 | 86 | 212.51 |
| 0.6 | 1.482 | 27 | 66.71 | 57 | 140.85 | 87 | 214.98 |
| 0.7 | 1.729 | 28 | 69.18 | 58 | 143.32 | 88 | 217.45 |
| 0.8 | 1.976 | 29 | 71.66 | 59 | 145.79 | 89 | 219.92 |
| 0.9 | 2.223 | 30 | 74.13 | 60 | 148.26 | 90 | 222.39 |
| 1 | 2.47 | 31 | 76.60 | 61 | 150.73 | 91 | 224.86 |
| 2 | 4.94 | 32 | 79.07 | 62 | 153.20 | 92 | 227.33 |
| 3 | 7.41 | 33 | 81.54 | 63 | 155.67 | 93 | 229.80 |
| 4 | 9.88 | 34 | 84.01 | 64 | 158.14 | 94 | 232.27 |
| 5 | 12.35 | 35 | 86.48 | 65 | 160.61 | 95 | 234.75 |
| 6 | 14.82 | 36 | 88.95 | 66 | 163.08 | 96 | 237.22 |
| 7 | 17.29 | 37 | 91.42 | 67 | 165.56 | 97 | 239.69 |
| 8 | 19.76 | 38 | 93.90 | 68 | 168.03 | 98 | 242.16 |
| 9 | 22.23 | 39 | 96.37 | 69 | 170.50 | 99 | 244.63 |
| 10 | 24.71 | 40 | 98.84 | 70 | 172.97 | 100 | 247.1 |
| 11 | 27.18 | 41 | 101.31 | 71 | 175.44 | 200 | 494.2 |
| 12 | 29.65 | 42 | 103.78 | 72 | 177.91 | 300 | 741.3 |
| 13 | 32.12 | 43 | 106.25 | 73 | 180.38 | 400 | 988.4 |
| 14 | 34.59 | 44 | 108.72 | 74 | 182.85 | 500 | 1235.5 |
| 15 | 37.06 | 45 | 111.19 | 75 | 185.32 | 600 | 1482.6 |
| 16 | 39.53 | 46 | 113.66 | 76 | 187.80 | 700 | 1729.7 |
| 17 | 42.00 | 47.47 | 47 | 116.13 | 77 | 190.27 | 800 |
| 18 | 49 | 118.61 | 78 | 192.74 | 900 | 2223.9 |  |
| 19 | 46.95 | 49 | 121.08 | 79 | 195.21 |  |  |
| 20 | 49.42 | 50 | 123.55 | 80 | 197.68 |  |  |

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### 2.0.8 Square Inch to Square Feet Conversion

| Square <br> Inches | Square <br> Feet | Square <br> Inches | Square <br> Feet | Square <br> Inches | Square <br> Feet | Square <br> Inches | Square <br> Feet |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0 | 0.00 | 11 | 0.076 | 22 | 0.152 | 33 | 0.229 |
| 1 | 0.006 | 12 | 0.083 | 23 | 0.159 | 34 | 0.236 |
| 2 | 0.013 | 13 | 0.090 | 24 | 0.166 | 35 | 0.243 |
| 3 | 0.020 | 14 | 0.097 | 25 | 0.173 | 36 | 0.25 |
| 4 | 0.027 | 15 | 0.104 | 26 | 0.180 | 37 | 0.256 |
| 5 | 0.034 | 16 | 0.111 | 27 | 0.187 | 38 | 0.263 |
| 6 | 0.041 | 17 | 0.118 | 28 | 0.194 | 39 | 0.270 |
| 7 | 0.048 | 18 | 0.125 | 29 | 0.201 | 40 | 0.277 |
| 8 | 0.055 | 19 | 0.131 | 30 | 0.208 | 41 | 0.284 |
| 9 | 0.062 | 20 | 0.138 | 31 | 0.215 | 42 | 0.291 |
| 10 | 0.069 | 21 | 0.145 | 32 | 0.222 | 43 | 0.298 |


| Square <br> Inches | Square <br> Feet | Square <br> Inches | Square <br> Feet | Square <br> Inches | Square <br> Feet | Square <br> Inches | Square <br> Feet |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 44 | 0.305 | 63 | 0.437 | 82 | 0.569 | 101 | 0.701 |
| 45 | 0.312 | 64 | 0.444 | 83 | 0.576 | 102 | 0.708 |
| 46 | 0.319 | 65 | 0.451 | 84 | 0.583 | 103 | 0.715 |
| 47 | 0.326 | 66 | 0.458 | 85 | 0.590 | 104 | 0.722 |
| 48 | 0.333 | 67 | 0.465 | 86 | 0.597 | 105 | 0.729 |
| 49 | 0.340 | 68 | 0.472 | 87 | 0.604 | 106 | 0.736 |
| 50 | 0.347 | 69 | 0.479 | 88 | 0.611 | 107 | 0.743 |
| 51 | 0.354 | 70 | 0.486 | 89 | 0.618 | 108 | 0.75 |
| 52 | 0.361 | 71 | 0.493 | 90 | 0.625 | 109 | 0.756 |
| 53 | 0.368 | 72 | 0.5 | 91 | 0.631 | 110 | 0.763 |
| 54 | 0.375 | 73 | 0.506 | 92 | 0.638 | 111 | 0.770 |
| 55 | 0.381 | 74 | 0.513 | 93 | 0.645 | 112 | 0.777 |
| 56 | 0.388 | 75 | 0.520 | 94 | 0.652 | 113 | 0.784 |
| 57 | 0.395 | 76 | 0.527 | 95 | 0.659 | 114 | 0.791 |
| 58 | 0.402 | 77 | 0.534 | 96 | 0.666 | 115 | 0.798 |
| 59 | 0.409 | 78 | 0.541 | 97 | 0.673 | 116 | 0.805 |
| 60 | 0.416 | 79 | 0.548 | 98 | 0.680 | 117 | 0.812 |
| 61 | 0.423 | 80 | 0.555 | 99 | 0.687 | 118 | 0.819 |
| 62 | 0.430 | 81 | 0.562 | 100 | 0.694 | 119 | 0.826 |

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2.0.9 Square Feet to Square Inch Conversion

| Square <br> Feet | Square <br> Inches | Square <br> Feet | Square <br> Inches | Square <br> Feet | Square <br> Inches | Square <br> Feet | Square <br> Inches |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0 | 0.00 | 13 | 1872 | 35 | 5040 | 57 | 8208 |
| 0.1 | 14.40 | 14 | 2016 | 36 | 5184 | 58 | 8352 |
| 0.2 | 28.80 | 15 | 2160 | 37 | 5328 | 59 | 8496 |
| 0.3 | 43.20 | 16 | 2304 | 38 | 5472 | 60 | 8640 |
| 0.4 | 57.60 | 17 | 2448 | 39 | 5616 | 61 | 8784 |
| 0.5 | 72.00 | 18 | 2592 | 40 | 5760 | 62 | 8928 |
| 0.6 | 86.40 | 19 | 2736 | 41 | 5904 | 63 | 9072 |
| 0.7 | 100.8 | 20 | 2880 | 42 | 6048 | 64 | 9216 |
| 0.8 | 115.2 | 21 | 3024 | 43 | 6192 | 65 | 9360 |
| 0.9 | 129.6 | 22 | 3168 | 44 | 6336 | 66 | 9504 |
| 1 | 144.0 | 23 | 3312 | 45 | 6480 | 67 | 9648 |
| 2 | 288.0 | 24 | 3456 | 46 | 6624 | 68 | 9792 |
| 3 | 432.0 | 25 | 3600 | 47 | 6768 | 69 | 9936 |
| 4 | 576.0 | 26 | 3744 | 48 | 6912 | 70 | 10080 |
| 5 | 720.0 | 27 | 3888 | 49 | 7056 | 71 | 10224 |
| 6 | 864.0 | 28 | 4032 | 50 | 7200 | 72 | 10368 |
| 7 | 1008 | 29 | 4176 | 51 | 7344 | 73 | 10512 |
| 8 | 1152 | 30 | 4320 | 52 | 7488 | 74 | 10656 |
| 9 | 1296 | 31 | 4464 | 53 | 7632 | 75 | 10800 |
| 10 | 1440 | 32 | 4608 | 54 | 7776 | 76 | 10944 |
| 11 | 1584 | 33 | 4752 | 55 | 7920 | 77 | 11088 |
| 12 | 1728 | 34 | 4896 | 56 | 8064 | 78 | 11232 |


| Square <br> Feet | Square <br> Inches | Square <br> Feet | Square <br> Inches | Square <br> Feet | Square <br> Inches | Square <br> Feet | Square <br> Inches |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 79 | 11376 | 87 | 12528 | 95 | 13680 | 400 | 57600 |
| 80 | 11520 | 88 | 12672 | 96 | 13824 | 500 | 72000 |
| 81 | 11664 | 89 | 12816 | 97 | 13968 | 600 | 86400 |
| 82 | 11808 | 90 | 12960 | 98 | 14112 | 700 | 100800 |
| 83 | 11952 | 91 | 13104 | 99 | 14256 | 800 | 115200 |
| 84 | 12096 | 92 | 13248 | 100 | 14400 | 900 | 129600 |
| 85 | 12240 | 93 | 13392 | 200 | 28800 |  |  |
| 86 | 123845 | 94 | 13536 | 300 | 43200 |  |  |

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### 2.0.10 Square Feet to Square Mile Conversion

| Square Feet | Square <br> Miles | Square <br> Feet | Square <br> Miles | Square <br> Feet | Square <br> Miles | Square <br> Feet | Square <br> Miles |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0 | 0.00 | 21 | 7.53 | 51 | 1.82 | 81 | 2.90 |
| 0.1 | 3.587 | 22 | 7.89 | 52 | 1.86 | 82 | 2.94 |
| 0.2 | 7.174 | 23 | 8.25 | 53 | 1.90 | 83 | 2.97 |
| 0.3 | 1.076 | 24 | 8.60 | 54 | 1.93 | 84 | 3.01 |
| 0.4 | 1.434 | 25 | 8.96 | 55 | 1.97 | 85 | 3.04 |
| 0.5 | 1.793 | 26 | 9.32 | 56 | 2.00 | 86 | 3.08 |
| 0.6 | 2.152 | 27 | 9.68 | 57 | 2.04 | 87 | 3.12 |
| 0.7 | 2.510 | 28 | 1.00 | 58 | 2.08 | 88 | 3.15 |
| 0.8 | 2.669 | 29 | 1.04 | 59 | 2.11 | 89 | 3.19 |
| 0.9 | 3.228 | 30 | 1.07 | 60 | 2.15 | 90 | 3.22 |
| 1 | 3.58 | 31 | 1.11 | 61 | 2.18 | 91 | 3.26 |
| 2 | 7.17 | 32 | 1.14 | 62 | 2.22 | 92 | 3.30 |
| 3 | 1.07 | 33 | 1.18 | 63 | 2.25 | 93 | 3.33 |
| 4 | 1.43 | 34 | 1.21 | 64 | 2.29 | 94 | 3.37 |
| 5 | 1.79 | 35 | 1.25 | 65 | 2.33 | 95 | 3.40 |
| 6 | 2.15 | 36 | 1.29 | 66 | 2.36 | 96 | 3.44 |
| 7 | 2.51 | 37 | 1.32 | 67 | 2.40 | 97 | 3.47 |
| 8 | 2.86 | 38 | 1.36 | 68 | 2.43 | 98 | 3.51 |
| 9 | 3.22 | 39 | 1.39 | 69 | 2.47 | 99 | 3.55 |
| 10 | 3.58 | 40 | 1.43 | 70 | 2.51 | 100 | 3.50 |
| 11 | 3.94 | 41 | 1.47 | 71 | 2.54 | 200 | 7.10 |
| 12 | 4.30 | 42 | 1.50 | 72 | 2.58 | 300 | 1.00 |
| 13 | 4.66 | 43 | 1.54 | 73 | 2.61 | 400 | 1.40 |
| 14 | 5.02 | 44 | 1.57 | 74 | 2.65 | 500 | 1.70 |
| 15 | 5.38 | 45 | 1.61 | 75 | 2.69 | 600 | 2.10 |
| 16 | 5.73 | 46 | 1.65 | 76 | 2.72 | 700 | 2.60 |
| 17 | 6.09 | 47 | 1.68 | 77 | 2.76 | 800 | 2.80 |
| 18 | 5.45 | 48 | 1.72 | 78 | 2.79 | 900 | 3.20 |
| 19 | 6.81 | 49 | 1.75 | 79 | 2.83 |  |  |
| 20 | 7.17 | 50 | 1.79 | 80 | 2.86 |  |  |

[^6]
### 2.0.11 Square Mile to Square Feet Conversion

| Square <br> Miles | Square <br> Feet | Square <br> Miles | Square <br> Feet | Square <br> Miles | Square <br> Feet | Square <br> Miles | Square <br> Feet |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0 | 0.00 | 21 | 585446400 | 51 | 1421798400 | 81 | 2258150400 |
| 0.1 | 2767840 | 22 | 613324800 | 52 | 1449676800 | 82 | 2286028800 |
| 0.2 | 5575660 | 23 | 641203200 | 53 | 1477555200 | 83 | 2313907200 |
| 0.3 | 8363520 | 24 | 669081600 | 54 | 1505433600 | 84 | 2341785600 |
| 0.4 | 11151360 | 25 | 696960000 | 55 | 1533312000 | 85 | 2369664000 |
| 0.5 | 13939200 | 26 | 724838400 | 56 | 1561190400 | 86 | 2397542400 |
| 0.6 | 16727040 | 27 | 752716800 | 57 | 1589068800 | 87 | 2425420800 |
| 0.7 | 19514880 | 28 | 760595200 | 58 | 1616947200 | 88 | 2453299200 |
| 0.8 | 22302720 | 29 | 808473600 | 59 | 1644825600 | 89 | 2481177600 |
| 0.9 | 25090560 | 30 | 836352000 | 60 | 1672704000 | 90 | 2509056000 |
| 1 | 27878400 | 31 | 864230400 | 61 | 1700582400 | 91 | 2536934400 |
| 2 | 55756800 | 32 | 892108800 | 62 | 1728460800 | 92 | 2584812800 |
| 3 | 83635200 | 33 | 919987200 | 63 | 1756339200 | 93 | 2592691200 |
| 4 | 111513600 | 34 | 947865600 | 64 | 1784217600 | 94 | 2620569600 |
| 5 | 139392000 | 35 | 975744000 | 65 | 1812096000 | 95 | 2648448000 |
| 6 | 167270400 | 36 | 1003622400 | 66 | 1839974400 | 96 | 2676326400 |
| 7 | 195148800 | 37 | 1031500800 | 67 | 1867852800 | 97 | 2704204800 |
| 8 | 223027200 | 38 | 1059379200 | 68 | 189531200 | 98 | 2732083200 |
| 9 | 250905600 | 39 | 1067257600 | 69 | 1923609600 | 99 | 2759961600 |
| 10 | 278784000 | 40 | 1115136000 | 70 | 1951488000 | 100 | 2787840000 |
| 11 | 306662400 | 41 | 1143014400 | 71 | 1979366400 | 200 | 5575680000 |
| 12 | 334540800 | 42 | 1170892800 | 72 | 2007244800 | 300 | 8363520000 |
| 13 | 362419200 | 43 | 1198771200 | 73 | 2035123200 | 400 | 11151360000 |
| 14 | 390297600 | 44 | 1226649600 | 74 | 2063001600 | 500 | 13939200000 |
| 15 | 418176000 | 45 | 1254526000 | 75 | 2090880000 | 600 | 16727040000 |
| 16 | 446054400 | 46 | 1282406400 | 76 | 2118758400 | 700 | 19514880000 |
| 17 | 473932800 | 47 | 1310284800 | 77 | 214636800 | 800 | 22302720000 |
| 18 | 501811200 | 48 | 1338153200 | 78 | 2174515200 | 900 | 25090560000 |
| 19 | 529589600 | 49 | 1366041600 | 79 | 2202393600 |  |  |
| 20 | 557568000 | 50 | 1393920000 | 80 | 2230272000 |  |  |
|  |  |  |  |  |  |  |  |

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### 2.0.12 Square Feet to Acres Conversion

| Square Feet | Acres | Square Feet | Acres | Square Feet | Acres | Square Feet | Acres |
| :---: | :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| 0 | 0.00 | 3 | 6.88 | 15 | 0.00 | 27 | 0.00 |
| 0.1 | 2.295 | 4 | 9.18 | 16 | 0.00 | 28 | 0.00 |
| 0.2 | 4.591 | 5 | 0.00 | 17 | 0.00 | 29 | 0.00 |
| 0.3 | 6.887 | 6 | 0.00 | 18 | 0.00 | 30 | 0.00 |
| 0.4 | 9.182 | 7 | 0.00 | 19 | 0.00 | 31 | 0.00 |
| 0.5 | 1.147 | 8 | 0.00 | 20 | 0.00 | 32 | 0.00 |
| 0.6 | 1.377 | 9 | 0.00 | 21 | 0.00 | 33 | 0.00 |
| 0.7 | 1.606 | 10 | 0.00 | 22 | 0.00 | 34 | 0.00 |
| 0.8 | 1.836 | 11 | 0.00 | 23 | 0.00 | 35 | 0.00 |
| 0.9 | 2.066 | 12 | 0.00 | 24 | 0.00 | 36 | 0.00 |
| 1 | 2.29 | 13 | 0.00 | 25 | 0.00 | 37 | 0.00 |
| 2 | 4.59 | 14 | 0.00 | 26 | 0.00 | 38 | 0.00 |


| Square Feet | Acres | Square Feet | Acres | Square Feet | Acres | Square Feet | Acres |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 39 | 0.00 | 57 | 0.00 | 75 | 0.00 | 93 | 0.00 |
| 40 | 0.00 | 58 | 0.00 | 76 | 0.00 | 94 | 0.00 |
| 41 | 0.00 | 59 | 0.00 | 77 | 0.00 | 95 | 0.00 |
| 42 | 0.00 | 60 | 0.00 | 78 | 0.00 | 96 | 0.00 |
| 43 | 0.00 | 61 | 0.00 | 79 | 0.00 | 97 | 0.00 |
| 44 | 0.00 | 62 | 0.00 | 80 | 0.00 | 98 | 0.00 |
| 45 | 0.00 | 63 | 0.00 | 81 | 0.00 | 99 | 0.00 |
| 46 | 0.00 | 64 | 0.00 | 82 | 0.00 | 100 | 0.00 |
| 47 | 0.00 | 65 | 0.00 | 83 | 0.00 | 200 | 0.00 |
| 48 | 0.00 | 66 | 0.00 | 84 | 0.00 | 300 | 0.00 |
| 49 | 0.00 | 67 | 0.00 | 85 | 0.00 | 400 | 0.00 |
| 50 | 0.00 | 68 | 0.00 | 86 | 0.00 | 500 | 0.00 |
| 51 | 0.00 | 69 | 0.00 | 87 | 0.00 | 600 | 0.00 |
| 52 | 0.00 | 70 | 0.00 | 88 | 0.00 | 700 | 0.00 |
| 53 | 0.00 | 71 | 0.00 | 89 | 0.00 | 800 | 0.00 |
| 54 | 0.00 | 72 | 0.00 | 90 | 0.00 | 900 | 0.00 |
| 55 | 0.00 | 73 | 0.00 | 91 | 0.00 |  |  |
| 56 | 0.00 | 74 | 0.00 | 92 | 0.00 |  |  |

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### 2.0.13 Acres to Square Feet Conversion

| Acres | Square Feet | Acres | Square Feet | Acres | Square Feet | Acres | Square Feet |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0 | 0.00 | 21 | 914760 | 51 | 2221560 | 81 | 3528360 |
| 0.1 | 4356 | 22 | 958320 | 52 | 2265120 | 82 | 3571920 |
| 0.2 | 8712 | 23 | 1001880 | 53 | 2308680 | 83 | 3615480 |
| 0.3 | 13068 | 24 | 1045440 | 54 | 2352240 | 84 | 3659040 |
| 0.4 | 17424 | 25 | 1089000 | 55 | 2395800 | 85 | 3702600 |
| 0.5 | 21780 | 26 | 1132560 | 56 | 2439360 | 86 | 3746160 |
| 0.6 | 26136 | 27 | 1176120 | 57 | 2482920 | 87 | 3789720 |
| 0.7 | 30492 | 28 | 1219680 | 58 | 2526480 | 88 | 3833280 |
| 0.8 | 34848 | 29 | 1263240 | 59 | 2570040 | 89 | 3876840 |
| 0.9 | 39204 | 30 | 1306800 | 60 | 2613600 | 90 | 3920400 |
| 1 | 43560 | 31 | 1350360 | 61 | 2657160 | 91 | 3963960 |
| 2 | 87120 | 32 | 1393920 | 62 | 2700720 | 92 | 4007520 |
| 3 | 130680 | 33 | 1437480 | 63 | 2744280 | 93 | 4051080 |
| 4 | 174240 | 34 | 1481040 | 64 | 2787840 | 94 | 4094640 |
| 5 | 217800 | 35 | 1524600 | 65 | 2831400 | 95 | 4138200 |
| 6 | 261360 | 36 | 1568160 | 66 | 2874960 | 96 | 4181760 |
| 7 | 304920 | 37 | 1611720 | 67 | 2918520 | 97 | 4225320 |
| 8 | 348480 | 38 | 1655280 | 68 | 2962080 | 98 | 4268880 |
| 9 | 392040 | 39 | 1698840 | 69 | 3005640 | 99 | 4312440 |
| 10 | 435600 | 40 | 1742400 | 70 | 3049200 | 100 | 4356000 |
| 11 | 479160 | 41 | 1785960 | 71 | 3092760 | 200 | 8712000 |
| 12 | 522720 | 42 | 1829520 | 72 | 3136320 | 300 | 13068000 |
| 13 | 566280 | 43 | 1873080 | 73 | 3179880 | 400 | 17424000 |
| 14 | 609840 | 44 | 1916640 | 74 | 3223440 | 500 | 21780000 |
| 15 | 653400 | 45 | 1960200 | 75 | 3267000 | 600 | 26136000 |
| 16 | 696960 | 46 | 2003760 | 76 | 3310560 | 700 | 30492000 |
| 17 | 740520 | 47 | 2047320 | 77 | 3354120 | 800 | 34848000 |
| 18 | 784080 | 48 | 2090880 | 78 | 3397680 | 900 | 39204000 |
| 19 | 827640 | 49 | 2134440 | 79 | 3441240 |  |  |
| 20 | 871200 | 50 | 2178000 | 80 | 3484800 |  |  |

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2.0.14 Square Yard to Square Meter Conversion Table

| Square <br> Yards | Square <br> Meters | Square <br> Yards | Square <br> Meters | Square <br> Yards | Square <br> Meters | Square <br> Yards | Square <br> Meters |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0 | 0.00 | 21 | 17.55 | 51 | 42.64 | 81 | 67.72 |
| 0.1 | 0.083 | 22 | 18.39 | 52 | 43.47 | 82 | 68.56 |
| 0.2 | 0.167 | 23 | 19.23 | 53 | 44.31 | 83 | 69.39 |
| 0.3 | 0.250 | 24 | 20.06 | 54 | 45.15 | 84 | 70.23 |
| 0.4 | 0.334 | 25 | 20.90 | 55 | 45.98 | 85 | 71.07 |
| 0.5 | 0.418 | 26 | 21.73 | 56 | 46.82 | 86 | 71.90 |
| 0.6 | 0.501 | 27 | 22.57 | 57 | 47.65 | 87 | 72.74 |
| 0.7 | 0.585 | 28 | 23.41 | 58 | 48.49 | 88 | 73.57 |
| 0.8 | 0.668 | 29 | 24.24 | 59 | 49.33 | 89 | 74.41 |
| 0.9 | 0.752 | 30 | 25.08 | 60 | 50.16 | 90 | 75.25 |
| 1 | 0.83 | 31 | 25.91 | 61 | 51.00 | 91 | 76.08 |
| 2 | 1.67 | 32 | 26.75 | 62 | 51.83 | 92 | 76.92 |
| 3 | 2.50 | 33 | 27.59 | 63 | 52.67 | 93 | 77.75 |
| 4 | 3.34 | 34 | 28.42 | 64 | 53.51 | 94 | 78.59 |
| 5 | 4.18 | 35 | 29.26 | 65 | 54.34 | 95 | 79.43 |
| 6 | 5.01 | 36 | 30.10 | 66 | 55.18 | 96 | 80.26 |
| 7 | 5.85 | 37 | 30.93 | 67 | 56.02 | 97 | 81.10 |
| 8 | 6.68 | 38 | 31.77 | 68 | 56.85 | 98 | 81.94 |
| 9 | 7.52 | 39 | 32.60 | 69 | 57.69 | 99 | 82.77 |
| 10 | 8.36 | 40 | 33.44 | 70 | 58.52 | 100 | 83.60 |
| 11 | 9.19 | 41 | 34.28 | 71 | 59.36 | 200 | 167.2 |
| 12 | 10.03 | 42 | 35.11 | 72 | 60.20 | 300 | 250.8 |
| 13 | 10.86 | 43 | 35.95 | 73 | 61.03 | 400 | 334.4 |
| 14 | 11.70 | 44 | 36.78 | 74 | 61.87 | 500 | 418.0 |
| 15 | 12.54 | 45 | 37.62 | 75 | 62.70 | 600 | 501.6 |
| 16 | 13.37 | 46 | 38.46 | 76 | 63.54 | 700 | 585.2 |
| 17 | 14.21 | 47 | 39.29 | 77 | 64.38 | 800 | 668.9 |
| 18 | 15.05 | 48 | 40.13 | 78 | 65.21 | 900 | 752.5 |
| 19 | 15.88 | 49 | 40.97 | 79 | 66.05 |  |  |
| 20 | 16.72 | 50 | 41.80 | 80 | 66.89 |  |  |

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2.0.15 Square Meter to Square Yard Conversion Table

| Square <br> Meters | Square <br> Yards | Square <br> Meters | Square <br> Yards | Square <br> Meters | Square <br> Yards | Square <br> Meters | Square <br> Yards |
| :---: | :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| 0 | 0.00 | 0.119 | 2 | 1.19 | 11 | 13.15 | 21 |
| 0.1 | 0.239 | 3 | 2.39 | 12 | 14.35 | 22 | 25.11 |
| 0.2 | 0.358 | 4 | 3.58 | 13 | 15.54 | 23 | 27.31 |
| 0.3 | 0.478 | 5 | 4.78 | 14 | 16.74 | 24 | 28.70 |
| 0.4 | 0.597 | 6 | 5.97 | 15 | 17.93 | 25 | 29.89 |
| 0.5 | 0.717 | 7 | 7.17 | 16 | 19.13 | 26 | 31.09 |
| 0.6 | 0.837 | 9 | 9.37 | 17 | 20.33 | 27 | 32.29 |
| 0.7 | 0.956 | 10 | 10.76 | 11.95 | 19 | 21.52 | 28 |
| 0.8 |  |  | 20 | 22.72 | 29 | 33.48 |  |
| 0.9 |  |  |  |  |  | 30 | 35.68 |
|  |  |  |  |  |  |  |  |


| Square <br> Meters | Square <br> Yards | Square <br> Meters | Square <br> Yards | Square <br> Meters | Square <br> Yards | Square <br> Meters | Square <br> Yards |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 31 | 37.07 | 51 | 60.99 | 71 | 84.91 | 91 | 108.83 |
| 32 | 38.27 | 52 | 62.19 | 72 | 86.11 | 92 | 110.03 |
| 33 | 39.46 | 53 | 63.38 | 73 | 87.30 | 93 | 111.22 |
| 34 | 40.66 | 54 | 64.58 | 74 | 88.50 | 94 | 112.42 |
| 35 | 41.85 | 55 | 65.77 | 75 | 89.69 | 95 | 113.61 |
| 36 | 43.05 | 56 | 66.97 | 76 | 90.89 | 96 | 114.81 |
| 37 | 44.25 | 57 | 68.17 | 77 | 92.09 | 97 | 116.01 |
| 38 | 45.44 | 58 | 69.36 | 78 | 93.28 | 98 | 117.20 |
| 39 | 46.64 | 59 | 70.56 | 79 | 94.48 | 99 | 118.40 |
| 40 | 47.83 | 60 | 71.75 | 80 | 95.67 | 100 | 119.5 |
| 41 | 49.03 | 61 | 72.95 | 81 | 96.87 | 200 | 239.1 |
| 42 | 50.23 | 62 | 74.15 | 82 | 98.07 | 300 | 358.7 |
| 43 | 51.42 | 63 | 75.34 | 83 | 99.26 | 400 | 478.3 |
| 44 | 52.62 | 64 | 76.54 | 84 | 100.46 | 500 | 597.9 |
| 45 | 53.81 | 65 | 77.73 | 85 | 101.65 | 600 | 717.5 |
| 46 | 55.01 | 66 | 78.93 | 86 | 102.85 | 700 | 837.1 |
| 47 | 56.21 | 67 | 80.13 | 87 | 104.05 | 800 | 956.7 |
| 48 | 57.40 | 68 | 81.32 | 88 | 105.24 | 900 | 1076.3 |
| 49 | 58.60 | 69 | 82.52 | 89 | 106.44 |  |  |
| 50 | 59.79 | 70 | 83.71 | 90 | 107.63 |  |  |

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### 2.0.16 Square Mile to Square Meter Conversion Table

| Square <br> Miles | Square <br> Meters | Square <br> Miles | Square <br> Meters | Square <br> Miles | Square <br> Meters | Square <br> Miles | Square <br> Meters |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0 | 0.00 | 13 | 33669845.43 | 35 | 90649583.86 | 57 | 147629322.28 |
| 0.1 | 258998.811 | 14 | 36259833.54 | 36 | 93239571.97 | 58 | 150219310.39 |
| 0.2 | 517997.622 | 15 | 38849821.65 | 37 | 95829560.08 | 59 | 152809298.50 |
| 0.3 | 776996.433 | 16 | 41439809.76 | 38 | 98419548.19 | 60 | 155399286.62 |
| 0.4 | 1035995.244 | 17 | 44029797.87 | 39 | 101009536.30 | 61 | 157989274.73 |
| 0.5 | 1294994.055 | 18 | 46619785.98 | 40 | 103599524.41 | 62 | 160579262.84 |
| 0.6 | 1553992.866 | 19 | 49209774.09 | 41 | 106189512.52 | 63 | 163169250.95 |
| 0.7 | 1812991.677 | 20 | 51799762.20 | 42 | 108779500.63 | 64 | 165759239.06 |
| 0.8 | 2071990.488 | 21 | 54389750.31 | 43 | 111369488.74 | 65 | 168349227.17 |
| 0.9 | 2330989.299 | 22 | 56979738.42 | 44 | 113959476.85 | 66 | 170939215.28 |
| 1 | 2589988.11 | 23 | 59569726.53 | 45 | 116549464.96 | 67 | 173529203.39 |
| 2 | 5179976.22 | 24 | 62159714.64 | 46 | 119139453.07 | 68 | 176119191.50 |
| 3 | 7769964.33 | 25 | 64749702.75 | 47 | 121729441.18 | 69 | 178709179.61 |
| 4 | 10359952.44 | 26 | 67339690.86 | 48 | 124319429.29 | 70 | 181299167.72 |
| 5 | 12949940.55 | 27 | 69929678.97 | 49 | 126909417.40 | 71 | 183889155.83 |
| 6 | 15539928.66 | 28 | 72519667.08 | 50 | 129499405.51 | 72 | 186479143.94 |
| 7 | 18129916.77 | 29 | 75109655.19 | 51 | 132089393.62 | 73 | 189069132.05 |
| 8 | 20719904.88 | 30 | 77699643.31 | 52 | 134679381.73 | 74 | 191659120.16 |
| 9 | 23309892.99 | 31 | 80289631.42 | 53 | 137269369.84 | 75 | 194249108.27 |
| 10 | 25899881.10 | 32 | 82879619.53 | 54 | 139859357.95 | 76 | 196839096.38 |
| 11 | 28489869.21 | 33 | 85469607.64 | 55 | 142449346.06 | 77 | 199429084.49 |
| 12 | 31079857.32 | 34 | 88059595.75 | 56 | 145039334.17 | 78 | 202019072.60 |


| Square <br> Miles | Square <br> Meters | Square <br> Miles | Square <br> Meters | Square <br> Miles | Square <br> Meters | Square <br> Miles | Square <br> Meters |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 79 | 204609060.71 | 87 | 225328965.59 | 95 | 246048870.48 | 400 | 1035995244.1 |
| 80 | 207199048.82 | 88 | 227918953.70 | 96 | 248638858.59 | 500 | 1294994055.1 |
| 81 | 209789036.93 | 89 | 230508941.81 | 97 | 251228846.70 | 600 | 1553992866.2 |
| 82 | 212379025.04 | 90 | 233098929.93 | 98 | 253818834.81 | 700 | 1812991677.2 |
| 83 | 214969013.15 | 91 | 235688918.04 | 99 | 256408822.92 | 800 | 2071990488.2 |
| 84 | 217559001.26 | 92 | 238278906.15 | 100 | 258998811.0 | 900 | 2330989299.3 |
| 85 | 220148989.37 | 93 | 240868894.26 | 200 | 517997622.0 |  |  |
| 86 | 222738977.48 | 94 | 243458882.37 | 300 | 776996433.1 |  |  |

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### 2.0.17 Square Meter to Square Mile Conversion Table

| Square <br> Meters | Square <br> Miles | Square <br> Meters | Square <br> Miles | Square <br> Meters | Square <br> Miles | Square <br> Meters | Square <br> Miles |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0 | 0.00 | 21 | 8.10 | 51 | 1.96 | 81 | 3.12 |
| 0.1 | 3.861 | 22 | 8.49 | 52 | 2.00 | 82 | 3.16 |
| 0.2 | 7.722 | 23 | 8.88 | 53 | 2.04 | 83 | 3.20 |
| 0.3 | 1.158 | 24 | 9.26 | 54 | 2.08 | 84 | 3.24 |
| 0.4 | 1.544 | 25 | 9.65 | 55 | 2.12 | 85 | 3.28 |
| 0.5 | 1.930 | 26 | 1.00 | 56 | 2.16 | 86 | 3.32 |
| 0.6 | 2.316 | 27 | 1.04 | 57 | 2.20 | 87 | 3.35 |
| 0.7 | 2.702 | 28 | 1.08 | 58 | 2.23 | 88 | 3.39 |
| 0.8 | 3.088 | 29 | 1.11 | 59 | 2.27 | 89 | 3.43 |
| 0.9 | 3.474 | 30 | 1.15 | 60 | 2.31 | 90 | 3.47 |
| 1 | 3.86 | 31 | 1.19 | 61 | 2.35 | 91 | 3.51 |
| 2 | 7.72 | 32 | 1.23 | 62 | 2.39 | 92 | 3.55 |
| 3 | 1.15 | 33 | 1.27 | 63 | 2.43 | 93 | 3.59 |
| 4 | 1.54 | 34 | 1.31 | 64 | 2.47 | 94 | 3.62 |
| 5 | 1.93 | 35 | 1.35 | 65 | 2.50 | 95 | 3.66 |
| 6 | 2.31 | 36 | 1.38 | 66 | 2.54 | 96 | 3.70 |
| 7 | 2.70 | 37 | 1.42 | 67 | 2.58 | 97 | 3.74 |
| 8 | 3.08 | 38 | 1.46 | 68 | 2.62 | 98 | 3.78 |
| 9 | 3.47 | 39 | 1.50 | 69 | 2.66 | 99 | 3.82 |
| 10 | 3.86 | 40 | 1.54 | 70 | 2.70 | 100 | 3.80 |
| 11 | 4.24 | 41 | 1.58 | 71 | 2.74 | 200 | 7.70 |
| 12 | 4.63 | 42 | 1.62 | 72 | 2.77 | 300 | 0.0 |
| 13 | 5.01 | 43 | 1.66 | 73 | 2.81 | 400 | 0.0 |
| 14 | 5.40 | 44 | 1.69 | 74 | 2.85 | 500 | 0.0 |
| 15 | 5.79 | 45 | 1.73 | 75 | 2.89 | 600 | 0.0 |
| 16 | 6.17 | 46 | 1.77 | 76 | 2.93 | 700 | 0.0 |
| 17 | 6.56 | 47 | 1.81 | 77 | 2.97 | 800 | 0.0 |
| 18 | 6.94 | 48 | 1.85 | 78 | 3.01 | 900 | 0.0 |
| 19 | 7.33 | 49 | 1.89 | 79 | 3.05 |  |  |
| 20 | 7.72 | 50 | 1.93 | 80 | 3.08 |  |  |

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2.0.18 Square Mile to Hectare Conversion Table

| Square Miles | Hectares | Square Miles | Hectares | Square Miles | Hectares | Square Miles | Hectares |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | ---: |
| 0 | 0.00 | 21 | 5438.97 | 51 | 13208.93 | 81 | 20978.90 |
| 0.1 | 25.899 | 22 | 5697.97 | 52 | 13467.93 | 82 | 21237.90 |
| 0.2 | 51.799 | 23 | 5956.97 | 53 | 13726.93 | 83 | 21496.90 |
| 0.3 | 77.699 | 24 | 6215.97 | 54 | 13985.93 | 84 | 21755.90 |
| 0.4 | 103.599 | 25 | 6474.97 | 55 | 14244.93 | 85 | 22014.89 |
| 0.5 | 129.499 | 26 | 6733.96 | 56 | 14503.93 | 86 | 22273.89 |
| 0.6 | 155.399 | 27 | 6992.96 | 57 | 14762.93 | 87 | 22532.89 |
| 0.7 | 181.299 | 28 | 7251.96 | 58 | 15021.93 | 88 | 22791.89 |
| 0.8 | 207.199 | 29 | 7510.96 | 59 | 15280.92 | 89 | 23050.89 |
| 0.9 | 233.098 | 30 | 7769.96 | 60 | 15539.92 | 90 | 23309.89 |
| 1 | 258.99 | 31 | 8028.96 | 61 | 15798.92 | 91 | 23568.89 |
| 2 | 517.99 | 32 | 8287.96 | 62 | 16057.92 | 92 | 23827.89 |
| 3 | 776.99 | 33 | 8546.96 | 63 | 16316.92 | 93 | 24086.88 |
| 4 | 1035.99 | 34 | 8805.95 | 64 | 16575.92 | 94 | 24345.88 |
| 5 | 1294.99 | 35 | 9064.95 | 65 | 16834.92 | 95 | 24604.88 |
| 6 | 1553.99 | 36 | 9323.95 | 66 | 17093.92 | 96 | 24863.88 |
| 7 | 1812.99 | 37 | 9582.95 | 67 | 17352.92 | 97 | 25122.88 |
| 8 | 2071.99 | 38 | 9841.95 | 68 | 17611.91 | 98 | 25381.88 |
| 9 | 2330.98 | 39 | 10100.95 | 69 | 17870.91 | 99 | 25640.88 |
| 10 | 2589.98 | 40 | 10359.95 | 70 | 18129.91 | 100 | 25899.8 |
| 11 | 2848.98 | 41 | 10618.95 | 71 | 18388.91 | 200 | 51799.7 |
| 12 | 3107.98 | 42 | 10877.95 | 72 | 18647.91 | 300 | 77699.6 |
| 13 | 3366.98 | 43 | 11136.94 | 73 | 18906.91 | 400 | 103599.5 |
| 14 | 3625.98 | 44 | 11395.94 | 74 | 19165.91 | 500 | 129499.4 |
| 15 | 3884.98 | 45 | 11654.94 | 75 | 19424.91 | 600 | 155399.2 |
| 16 | 4143.98 | 46 | 11913.94 | 76 | 19683.90 | 700 | 181299.1 |
| 17 | 4402.97 | 47 | 12172.94 | 77 | 19942.90 | 800 | 207199.0 |
| 18 | 4661.97 | 48 | 12431.94 | 78 | 20201.90 | 900 | 233098.9 |
| 19 | 4920.97 | 49 | 12690.94 | 79 | 20460.90 |  |  |
| 20 | 5179.97 | 50 | 12949.94 | 80 | 20719.90 |  | 9 |

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### 2.0.19 Hectare to Square Mile Conversion Table

| Hectares | Square Miles | Hectares | Square Miles | Hectares | Square Miles | Hectares | Square Miles |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0 | 0.00 | 5 | 0.01 | 19 | 0.07 | 33 | 0.12 |
| 0.1 | 0.000 | 6 | 0.02 | 20 | 0.07 | 34 | 0.13 |
| 0.2 | 0.000 | 7 | 0.02 | 21 | 0.08 | 35 | 0.13 |
| 0.3 | 0.001 | 8 | 0.03 | 22 | 0.08 | 36 | 0.13 |
| 0.4 | 0.001 | 9 | 0.03 | 23 | 0.08 | 37 | 0.14 |
| 0.5 | 0.001 | 10 | 0.03 | 24 | 0.09 | 38 | 0.14 |
| 0.6 | 0.002 | 11 | 0.04 | 25 | 0.09 | 39 | 0.15 |
| 0.7 | 0.002 | 12 | 0.04 | 26 | 0.10 | 40 | 0.15 |
| 0.8 | 0.003 | 13 | 0.05 | 27 | 0.10 | 41 | 0.15 |
| 0.9 | 0.003 | 14 | 0.05 | 28 | 0.10 | 42 | 0.16 |
| 1 | 0.00 | 15 | 0.05 | 29 | 0.11 | 43 | 0.16 |
| 2 | 0.00 | 16 | 0.06 | 30 | 0.11 | 44 | 0.16 |
| 3 | 0.01 | 17 | 0.06 | 31 | 0.11 | 45 | 0.17 |
| 4 | 0.01 | 18 | 0.06 | 32 | 0.12 | 46 | 0.17 |


| Hectares | Square Miles | Hectares | Square Miles | Hectares | Square Miles | Hectares | Square Miles |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 47 | 0.18 | 63 | 0.24 | 79 | 0.30 | 95 | 0.36 |
| 48 | 0.18 | 64 | 0.24 | 80 | 0.30 | 96 | 0.37 |
| 49 | 0.18 | 65 | 0.25 | 81 | 0.31 | 97 | 0.37 |
| 50 | 0.19 | 66 | 0.25 | 82 | 0.31 | 98 | 0.37 |
| 51 | 0.19 | 67 | 0.25 | 83 | 0.32 | 99 | 0.38 |
| 52 | 0.20 | 68 | 0.26 | 84 | 0.32 | 100 | 0.3 |
| 53 | 0.20 | 69 | 0.26 | 85 | 0.32 | 200 | 0.7 |
| 54 | 0.20 | 70 | 0.27 | 86 | 0.33 | 300 | 1.10 |
| 55 | 0.21 | 71 | 0.27 | 87 | 0.33 | 400 | 1.50 |
| 56 | 0.21 | 72 | 0.27 | 88 | 0.33 | 500 | 1.90 |
| 57 | 0.22 | 73 | 0.28 | 89 | 0.34 | 600 | 2.30 |
| 58 | 0.22 | 74 | 0.28 | 90 | 0.34 | 700 | 2.70 |
| 59 | 0.22 | 75 | 76 | 0.28 | 91 | 0.35 | 800 |
| 60 | 0.23 | 77 | 0.29 | 92 | 0.35 | 900 | 3.00 |
| 61 | 0.23 | 78 | 0.39 | 93 | 0.35 |  | 3.40 |
| 62 | 0.23 |  |  |  | 94 | 0.36 |  |

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### 2.0.20 Miles to Kilometers Conversion

| Miles | Kilometers | Miles | Kilometers | Miles | Kilometers | Miles | Kilometers |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0 | 0.00 | 22 | 35.40 | 53 | 85.295 | 84 | 135.184 |
| 0.1 | 0.160 | 23 | 37.01 | 54 | 86.904 | 85 | 136.794 |
| 0.2 | 0.321 | 24 | 38.62 | 55 | 88.513 | 86 | 138.403 |
| 0.3 | 0.482 | 25 | 40.23 | 56 | 90.123 | 87 | 140.012 |
| 0.4 | 0.643 | 26 | 41.84 | 57 | 91.732 | 88 | 141.622 |
| 0.5 | 0.804 | 27 | 43.45 | 58 | 93.341 | 89 | 143.231 |
| 0.6 | 0.965 | 28 | 45.06 | 59 | 94.951 | 90 | 144.840 |
| 0.7 | 1.126 | 29 | 46.67 | 60 | 96.560 | 91 | 146.450 |
| 0.8 | 1.287 | 30 | 48.28 | 61 | 98.169 | 92 | 148.059 |
| 0.9 | 1.448 | 31 | 49.88 | 62 | 99.779 | 93 | 149.668 |
| 1 | 1.60 | 32 | 51.49 | 63 | 101.388 | 94 | 151.278 |
| 2 | 3.21 | 33 | 53.10 | 64 | 102.998 | 95 | 152.887 |
| 3 | 4.82 | 34 | 54.71 | 65 | 104.607 | 96 | 154.497 |
| 4 | 6.43 | 35 | 56.32 | 66 | 106.218 | 97 | 156.106 |
| 5 | 8.04 | 36 | 57.93 | 67 | 107.826 | 98 | 157.715 |
| 6 | 9.65 | 37 | 59.54 | 68 | 109.435 | 99 | 159.325 |
| 7 | 11.26 | 38 | 61.15 | 69 | 111.044 | 100 | 160.934 |
| 8 | 12.87 | 39 | 62.76 | 70 | 112.654 | 101 | 162.543 |
| 9 | 14.48 | 40 | 64.373 | 71 | 114.263 | 102 | 164.153 |
| 10 | 16.09 | 41 | 65.983 | 72 | 115.872 | 103 | 165.782 |
| 11 | 17.70 | 42 | 67.592 | 73 | 117.482 | 104 | 167.371 |
| 12 | 19.31 | 43 | 69.201 | 74 | 119.091 | 105 | 168.981 |
| 13 | 20.92 | 44 | 70.811 | 75 | 120.700 | 106 | 170.590 |
| 14 | 22.53 | 45 | 72.420 | 76 | 122.310 | 107 | 172.199 |
| 15 | 24.14 | 46 | 74.029 | 77 | 123.919 | 108 | 173.809 |
| 16 | 25.74 | 47 | 75.639 | 78 | 125.528 | 109 | 175.418 |
| 17 | 27.35 | 48 | 77.248 | 79 | 127.138 | 110 | 177.027 |
| 18 | 28.96 | 49 | 78.857 | 80 | 128.747 | 111 | 178.637 |
| 19 | 30.57 | 50 | 80.467 | 81 | 130.356 | 112 | 180.246 |
| 20 | 32.18 | 51 | 82.076 | 82 | 131.966 | 113 | 181.855 |
| 21 | 33.79 | 52 | 83.685 | 83 | 133.575 | 114 | 183.465 |


| Miles | Kilometers | Miles | Kilometers | Miles | Kilometers | Miles | Kilometers |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 115 | 185.074 | 129 | 207.605 | 143 | 230.136 | 157 | 252.667 |
| 116 | 186.683 | 130 | 209.214 | 144 | 231.745 | 158 | 254.276 |
| 117 | 188.293 | 131 | 210.824 | 145 | 233.354 | 159 | 255.885 |
| 118 | 189.902 | 132 | 212.433 | 146 | 234.964 | 200 | 321.8 |
| 119 | 191.511 | 133 | 214.042 | 147 | 236.573 | 300 | 482.8 |
| 120 | 193.121 | 134 | 215.652 | 148 | 238.182 | 400 | 643.7 |
| 121 | 194.730 | 135 | 217.261 | 149 | 239.792 | 500 | 804.6 |
| 122 | 196.339 | 136 | 218.870 | 150 | 241.401 | 600 | 965.6 |
| 123 | 197.949 | 137 | 220.480 | 151 | 243.010 | 700 | 1126.5 |
| 124 | 199.558 | 138 | 222.089 | 152 | 244.620 | 800 | 1287.4 |
| 125 | 201.168 | 139 | 223.698 | 153 | 246.229 | 900 | 1448.4 |
| 126 | 202.777 | 140 | 225.308 | 154 | 247.838 |  |  |
| 127 | 204.386 | 141 | 226.917 | 155 | 249.448 |  |  |
| 128 | 205.996 | 142 | 228.526 | 156 | 251.057 |  |  |

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### 2.0.21 Kilometers to Miles Conversion

| Kilometers | Miles | Kilometers | Miles | Kilometers | Miles | Kilometers | Miles |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0 | 0.00 | 24 | 14.91 | 57 | 35.418 | 90 | 55.923 |
| 0.1 | 0.062 | 25 | 15.53 | 58 | 36.039 | 91 | 56.544 |
| 0.2 | 0.124 | 26 | 16.15 | 59 | 36.660 | 92 | 57.166 |
| 0.3 | 0.186 | 27 | 16.77 | 60 | 37.282 | 93 | 57.787 |
| 0.4 | 0.248 | 28 | 17.39 | 61 | 37.903 | 94 | 58.408 |
| 0.5 | 0.310 | 29 | 18.01 | 62 | 38.525 | 95 | 59.030 |
| 0.6 | 0.372 | 30 | 18.64 | 63 | 39.146 | 96 | 59.551 |
| 0.7 | 0.434 | 31 | 19.26 | 64 | 39.767 | 97 | 60.273 |
| 0.8 | 0.497 | 32 | 19.88 | 65 | 40.389 | 98 | 60.894 |
| 0.9 | 0.559 | 33 | 20.50 | 66 | 41.010 | 99 | 61.515 |
| 1 | 0.62 | 34 | 21.12 | 67 | 41.631 | 100 | 62.137 |
| 2 | 1.24 | 35 | 21.74 | 68 | 42.253 | 101 | 62.758 |
| 3 | 1.86 | 36 | 22.36 | 69 | 42.874 | 102 | 63.379 |
| 4 | 2.48 | 37 | 22.99 | 70 | 43.495 | 103 | 64.001 |
| 5 | 3.10 | 38 | 23.61 | 71 | 44.117 | 104 | 64.622 |
| 6 | 3.72 | 39 | 24.23 | 72 | 44.738 | 105 | 65.243 |
| 7 | 4.34 | 40 | 24.854 | 73 | 45.360 | 106 | 65.865 |
| 8 | 4.97 | 41 | 25.476 | 74 | 45.981 | 107 | 66.486 |
| 9 | 5.59 | 42 | 26.097 | 75 | 46.602 | 108 | 67.108 |
| 10 | 6.21 | 43 | 26.718 | 76 | 47.224 | 109 | 67.729 |
| 11 | 6.83 | 44 | 27.340 | 77 | 47.845 | 110 | 68.350 |
| 12 | 7.45 | 45 | 27.961 | 78 | 48.466 | 111 | 68.972 |
| 13 | 8.07 | 46 | 28.583 | 79 | 49.088 | 112 | 69.593 |
| 14 | 8.69 | 47 | 29.204 | 80 | 49.709 | 113 | 70.214 |
| 15 | 9.32 | 48 | 29.825 | 81 | 50.331 | 114 | 70.836 |
| 16 | 9.94 | 49 | 30.447 | 82 | 50.952 | 115 | 71.457 |
| 17 | 10.56 | 50 | 31.068 | 83 | 51.573 | 116 | 72.079 |
| 18 | 11.18 | 51 | 31.689 | 84 | 52.195 | 117 | 72.700 |
| 19 | 11.80 | 52 | 32.311 | 85 | 52.816 | 118 | 73.321 |
| 20 | 12.42 | 53 | 32.932 | 86 | 53.437 | 119 | 73.943 |
| 21 | 13.04 | 54 | 33.554 | 87 | 54.059 | 120 | 74.564 |
| 22 | 13.67 | 55 | 34.175 | 88 | 54.680 | 121 | 75.185 |
| 23 | 14.29 | 56 | 34.796 | 89 | 55.302 | 122 | 75.807 |
|  |  |  |  |  |  |  |  |
|  |  |  | 35 |  |  |  |  |


| Kilometers | Miles | Kilometers | Miles | Kilometers | Miles | Kilometers | Miles |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 123 | 76.428 | 135 | 83.885 | 147 | 91.341 | 159 | 98.798 |
| 124 | 77.050 | 136 | 84.506 | 148 | 91.962 | 200 | 124.2 |
| 125 | 77.671 | 137 | 85.127 | 149 | 92.584 | 300 | 186.4 |
| 126 | 78.292 | 138 | 85.749 | 150 | 93.205 | 400 | 248.5 |
| 127 | 78.914 | 139 | 86.370 | 151 | 93.827 | 500 | 310.6 |
| 128 | 79.535 | 140 | 86.991 | 152 | 94.448 | 600 | 372.8 |
| 129 | 80.156 | 141 | 87.613 | 153 | 95.069 | 700 | 434.9 |
| 130 | 80.778 | 142 | 88.234 | 154 | 95.691 | 800 | 497.0 |
| 131 | 81.399 | 143 | 88.856 | 155 | 96.312 | 900 | 559.2 |
| 132 | 82.020 | 144 | 89.477 | 156 | 96.933 |  |  |
| 133 | 82.642 | 145 | 90.098 | 157 | 97.555 |  |  |
| 134 | 83.263 | 146 | 90.720 | 158 | 98.176 |  |  |

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### 2.0.22 Pounds to Kilograms Conversion

| Pounds | Kilograms | Pounds | Kilograms | Pounds | Kilograms | Pounds | Kilograms |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0 | 0.00 | 26 | 11.79 | 61 | 27.669 | 96 | 43.544 |
| 0.1 | 0.045 | 27 | 12.24 | 62 | 28.122 | 97 | 43.998 |
| 0.2 | 0.090 | 28 | 12.70 | 63 | 28.576 | 98 | 44.452 |
| 0.3 | 0.136 | 29 | 13.15 | 64 | 29.029 | 99 | 44.905 |
| 0.4 | 0.181 | 30 | 13.60 | 65 | 29.483 | 100 | 45.359 |
| 0.5 | 0.226 | 31 | 14.06 | 66 | 29.937 | 101 | 45.812 |
| 0.6 | 0.272 | 32 | 14.51 | 67 | 30.390 | 102 | 46.266 |
| 0.7 | 0.317 | 33 | 14.96 | 68 | 30.844 | 103 | 46.720 |
| 0.8 | 0.362 | 34 | 15.42 | 69 | 31.297 | 104 | 47.173 |
| 0.9 | 0.408 | 35 | 15.87 | 70 | 31.751 | 105 | 47.627 |
| 1 | 0.45 | 36 | 16.32 | 71 | 32.205 | 106 | 48.080 |
| 2 | 0.90 | 37 | 16.78 | 72 | 32.658 | 107 | 48.534 |
| 3 | 1.36 | 38 | 17.23 | 73 | 33.112 | 108 | 48.987 |
| 4 | 1.81 | 39 | 17.69 | 74 | 33.565 | 109 | 49.441 |
| 5 | 2.26 | 40 | 18.143 | 75 | 34.019 | 110 | 49.895 |
| 6 | 2.72 | 41 | 18.597 | 76 | 34.473 | 111 | 50.348 |
| 7 | 3.17 | 42 | 19.050 | 77 | 34.926 | 112 | 50.802 |
| 8 | 3.62 | 43 | 19.504 | 78 | 35.380 | 113 | 51.255 |
| 9 | 4.08 | 44 | 19.958 | 79 | 35.833 | 114 | 51.709 |
| 10 | 4.53 | 45 | 20.411 | 80 | 36.287 | 115 | 52.163 |
| 11 | 4.98 | 46 | 20.865 | 81 | 36.740 | 116 | 52.616 |
| 12 | 5.44 | 47 | 21.318 | 82 | 37.194 | 117 | 53.070 |
| 13 | 5.89 | 48 | 21.772 | 83 | 37.648 | 118 | 53.523 |
| 14 | 6.35 | 49 | 22.226 | 84 | 38.101 | 119 | 53.977 |
| 15 | 6.80 | 50 | 22.679 | 85 | 38.555 | 120 | 54.431 |
| 16 | 7.25 | 51 | 23.133 | 86 | 39.008 | 121 | 54.884 |
| 17 | 7.71 | 52 | 23.586 | 87 | 39.462 | 122 | 55.338 |
| 18 | 8.16 | 53 | 24.040 | 88 | 39.916 | 123 | 55.791 |
| 19 | 8.61 | 54 | 24.493 | 89 | 40.369 | 124 | 56.245 |
| 20 | 9.07 | 55 | 24.947 | 90 | 40.823 | 125 | 56.699 |
| 21 | 9.52 | 56 | 25.401 | 91 | 41.276 | 126 | 57.152 |
| 22 | 9.97 | 57 | 25.854 | 92 | 41.730 | 127 | 57.606 |
| 23 | 10.43 | 58 | 26.308 | 93 | 42.184 | 128 | 58.059 |
| 24 | 10.88 | 59 | 26.761 | 94 | 42.637 | 129 | 58.513 |
| 25 | 11.33 | 60 | 27.215 | 95 | 43.091 | 130 | 58.967 |


| Pounds | Kilograms | Pounds | Kilograms | Pounds | Kilograms | Pounds | Kilograms |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | ---: |
| 131 | 59.240 | 143 | 64.863 | 155 | 70.306 | 900 | 408.2 |
| 132 | 59.870 | 144 | 65.317 | 156 | 70.760 |  |  |
| 133 | 60.327 | 145 | 65.770 | 157 | 71.214 |  |  |
| 134 | 60.781 | 146 | 66.224 | 158 | 71.667 |  |  |
| 135 | 61.234 | 147 | 66.678 | 159 | 90.121 |  |  |
| 136 | 61.688 | 148 | 67.131 | 200 | 136.0 |  |  |
| 137 | 62.142 | 149 | 67.585 | 300 | 181.4 |  |  |
| 138 | 62.595 | 150 | 68.038 | 400 | 226.7 |  |  |
| 139 | 63.049 | 151 | 68.492 | 500 | 272.1 |  |  |
| 140 | 63.502 | 152 | 68.946 | 600 | 317.5 |  |  |
| 141 | 63.596 | 153 | 69.399 | 700 | 362.8 |  |  |
| 142 | 64.410 | 154 | 69.853 | 800 |  |  |  |

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### 2.0.23 Kilograms to Pounds Conversion

| Kilograms | Pounds | Kilograms | Pounds | Kilograms | Pounds | Kilograms | Pounds |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0 | 0.00 | 26 | 57.32 | 61 | 134.481 | 96 | 211.643 |
| 0.1 | 0.220 | 27 | 59.52 | 62 | 136.686 | 97 | 213.848 |
| 0.2 | 0.440 | 28 | 61.72 | 63 | 138.891 | 98 | 216.053 |
| 0.3 | 0.661 | 29 | 63.93 | 64 | 141.095 | 99 | 218.257 |
| 0.4 | 0.881 | 30 | 66.13 | 65 | 143.300 | 100 | 220.462 |
| 0.5 | 1.102 | 31 | 68.34 | 66 | 145.605 | 101 | 222.666 |
| 0.6 | 1.322 | 32 | 70.54 | 67 | 147.709 | 102 | 224.871 |
| 0.7 | 1.543 | 33 | 72.75 | 68 | 149.914 | 103 | 227.076 |
| 0.8 | 1.763 | 34 | 74.95 | 69 | 152.118 | 104 | 229.280 |
| 0.9 | 1.984 | 35 | 77.16 | 70 | 154.323 | 105 | 231.485 |
| 1 | 2.20 | 36 | 79.36 | 71 | 156.528 | 106 | 233.689 |
| 2 | 4.40 | 37 | 81.57 | 72 | 158.732 | 107 | 235.894 |
| 3 | 6.61 | 38 | 83.77 | 73 | 160.937 | 108 | 238.099 |
| 4 | 8.81 | 39 | 85.98 | 74 | 163.142 | 109 | 240.303 |
| 5 | 11.02 | 40 | 88.184 | 75 | 165.346 | 110 | 242.508 |
| 6 | 13.22 | 41 | 90.389 | 76 | 167.551 | 111 | 244.713 |
| 7 | 15.43 | 42 | 92.594 | 77 | 169.755 | 112 | 246.917 |
| 8 | 17.63 | 43 | 94.798 | 78 | 171.960 | 113 | 249.122 |
| 9 | 19.84 | 44 | 97.003 | 79 | 174.165 | 114 | 251.326 |
| 10 | 22.04 | 45 | 99.208 | 80 | 176.369 | 115 | 253.531 |
| 11 | 24.25 | 46 | 101.412 | 81 | 178.574 | 116 | 255.736 |
| 12 | 26.45 | 47 | 103.617 | 82 | 180.779 | 117 | 257.940 |
| 13 | 28.66 | 48 | 105.821 | 83 | 182.983 | 118 | 260.145 |
| 14 | 30.86 | 49 | 108.026 | 84 | 185.188 | 119 | 262.350 |
| 15 | 33.06 | 50 | 110.231 | 85 | 187.392 | 120 | 264.554 |
| 16 | 35.27 | 51 | 112.435 | 86 | 189.597 | 121 | 266.759 |
| 17 | 37.47 | 52 | 114.640 | 87 | 191.802 | 122 | 268.963 |
| 18 | 39.68 | 53 | 116.844 | 88 | 194.006 | 123 | 271.168 |
| 19 | 41.88 | 54 | 119.049 | 89 | 196.211 | 124 | 273.373 |
| 20 | 44.09 | 55 | 121.254 | 90 | 198.416 | 125 | 275.577 |
| 21 | 46.29 | 56 | 123.458 | 91 | 200.620 | 126 | 277.782 |
| 22 | 48.50 | 57 | 125.663 | 92 | 202.825 | 127 | 279.987 |
| 23 | 50.70 | 58 | 127.868 | 93 | 205.029 | 128 | 282.191 |
| 24 | 52.91 | 59 | 130.072 | 94 | 207.234 | 129 | 284.396 |
| 25 | 55.11 | 60 | 132.277 | 95 | 209.439 | 130 | 286.600 |


| Kilograms | Pounds | Kilograms | Pounds | Kilograms | Pounds | Kilograms | Pounds |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | ---: |
| 131 | 288.805 | 141 | 310.851 | 151 | 332.898 | 300 | 661.3 |
| 132 | 291.010 | 142 | 313.056 | 152 | 335.102 | 400 | 881.8 |
| 133 | 293.214 | 143 | 315.261 | 153 | 337.307 | 500 | 1102.3 |
| 134 | 295.419 | 144 | 317.465 | 154 | 339.511 | 600 | 1322.7 |
| 135 | 297.624 | 145 | 319.670 | 155 | 341.716 | 700 | 1543.2 |
| 136 | 299.828 | 146 | 319.875 | 156 | 343.921 | 800 | 1763.6 |
| 137 | 302.033 | 147 | 324.079 | 157 | 346.125 | 900 | 1984.1 |
| 138 | 304.237 | 148 | 326.284 | 158 | 348.330 |  |  |
| 139 | 306.422 | 149 | 329.488 | 159 | 350.534 |  |  |
| 140 | 308.647 | 150 | 330.693 | 200 | 440.9 |  |  |

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### 2.0.24 Fahrenheit to Celsius Conversion

| Fahrenheit | Celsius | Fahrenheit | Celsius | Fahrenheit | Celsius | Fahrenheit | Celsius |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0 | -17.777 | 21 | -6.11 | 51 | 10.55 | 81 | 27.22 |
| 0.1 | -17.722 | 22 | -5.55 | 52 | 11.11 | 82 | 27.77 |
| 0.2 | -17.666 | 23 | -5.00 | 53 | 11.66 | 83 | 28.33 |
| 0.3 | -17.611 | 24 | -4.44 | 54 | 12.22 | 84 | 28.88 |
| 0.4 | -17.555 | 25 | -3.88 | 55 | 12.77 | 85 | 29.44 |
| 0.5 | -17.5 | 26 | -3.33 | 56 | 13.33 | 86 | 30.00 |
| 0.6 | -17.444 | 27 | -2.77 | 57 | 13.88 | 87 | 30.55 |
| 0.7 | -17.388 | 28 | -2.22 | 58 | 14.44 | 88 | 31.11 |
| 0.8 | -17.333 | 29 | -1.66 | 59 | 15.00 | 89 | 31.66 |
| 0.9 | -17.277 | 30 | -1.11 | 60 | 15.55 | 90 | 32.22 |
| 1 | -17.22 | 31 | -0.55 | 61 | 16.11 | 91 | 32.77 |
| 2 | -16.66 | 32 | 0.00 | 62 | 16.66 | 92 | 33.33 |
| 3 | -16.11 | 33 | 0.55 | 63 | 17.22 | 93 | 33.88 |
| 4 | -15.55 | 34 | 1.11 | 64 | 17.77 | 94 | 34.44 |
| 5 | -15.00 | 35 | 1.66 | 65 | 18.33 | 95 | 35.00 |
| 6 | -14.44 | 36 | 2.22 | 66 | 18.88 | 96 | 35.55 |
| 7 | -13.88 | 37 | 2.77 | 67 | 19.44 | 97 | 36.11 |
| 8 | -13.33 | 38 | 3.33 | 68 | 20.00 | 98 | 36.66 |
| 9 | -12.77 | 39 | 3.88 | 69 | 20.55 | 99 | 37.22 |
| 10 | -12.22 | 40 | 4.44 | 70 | 21.11 | 100 | 37.70 |
| 11 | -11.66 | 41 | 5.00 | 71 | 21.66 | 200 | 93.30 |
| 12 | -11.11 | 42 | 5.55 | 72 | 22.22 | 300 | 148.8 |
| 13 | -10.55 | 43 | 6.11 | 73 | 22.77 | 400 | 204.4 |
| 14 | -10.00 | 44 | 6.66 | 74 | 23.33 | 500 | 260.0 |
| 15 | -9.44 | 45 | 7.22 | 75 | 23.88 | 600 | 315.5 |
| 16 | -8.88 | 46 | 7.77 | 76 | 24.44 | 700 | 371.1 |
| 17 | -8.33 | 47 | 8.33 | 77 | 25.00 | 800 | 426.6 |
| 18 | -7.77 | 48 | 8.88 | 78 | 25.55 | 900 | 482.2 |
| 19 | -7.22 | 49 | 9.44 | 79 | 26.11 |  |  |
| 20 | -6.66 | 50 | 10.00 | 80 | 26.66 |  |  |

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2.0.25 Celsius to Fahrenheit Temperature Conversion

| Celsius | Fahrenheit | Celsius | Fahrenheit | Celsius | Fahrenheit | Celsius | Fahrenheit |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0 | 32.00 | 21 | 69.80 | 51 | 123.8 | 81 | 177.8 |
| 0.1 | 32.18 | 22 | 71.60 | 52 | 125.6 | 82 | 179.6 |
| 0.2 | 32.36 | 23 | 73.40 | 53 | 127.4 | 83 | 181.4 |
| 0.3 | 32.54 | 24 | 75.20 | 54 | 129.2 | 84 | 183.2 |
| 0.4 | 32.72 | 25 | 77.00 | 55 | 131.0 | 85 | 185.0 |
| 0.5 | 32.90 | 26 | 78.80 | 56 | 132.8 | 86 | 186.8 |
| 0.6 | 33.08 | 27 | 80.60 | 57 | 134.6 | 87 | 188.6 |
| 0.7 | 33.26 | 28 | 82.40 | 58 | 136.4 | 88 | 190.4 |
| 0.8 | 33.44 | 29 | 84.20 | 59 | 138.2 | 89 | 192.2 |
| 0.9 | 33.62 | 30 | 86.00 | 60 | 140.0 | 90 | 194.0 |
| 1 | 33.80 | 31 | 87.80 | 61 | 141.8 | 91 | 195.8 |
| 2 | 35.60 | 32 | 89.60 | 62 | 143.6 | 92 | 197.6 |
| 3 | 37.40 | 33 | 91.40 | 63 | 145.4 | 93 | 199.4 |
| 4 | 39.20 | 34 | 93.20 | 64 | 147.2 | 94 | 201.2 |
| 5 | 41.00 | 35 | 95.00 | 65 | 149.0 | 95 | 203.0 |
| 6 | 42.80 | 36 | 96.80 | 66 | 150.8 | 96 | 204.8 |
| 7 | 44.60 | 37 | 98.60 | 67 | 152.6 | 97 | 206.6 |
| 8 | 45.40 | 38 | 100.4 | 68 | 154.4 | 98 | 208.4 |
| 9 | 48.20 | 39 | 102.2 | 69 | 156.2 | 99 | 210.2 |
| 10 | 50.00 | 40 | 104.0 | 70 | 158.0 | 100 | 212.0 |
| 11 | 51.80 | 41 | 105.8 | 71 | 159.8 | 200 | 392.0 |
| 12 | 53.60 | 42 | 107.6 | 72 | 161.6 | 300 | 572.0 |
| 13 | 55.40 | 43 | 109.4 | 73 | 163.4 | 400 | 752.0 |
| 14 | 57.20 | 44 | 111.2 | 74 | 165.2 | 500 | 932.0 |
| 15 | 59.00 | 45 | 113.0 | 75 | 167.0 | 600 | 700 |
| 16 | 60.80 | 46 | 114.8 | 76 | 168.8 | 700 | 800 |
| 17 | 62.60 | 47 | 116.6 | 77 | 170.6 | 172.4 | 900 |
| 18 | 64.40 | 48 | 118.4 | 78 | 174.2 | 176.0 | 1292 |
| 19 | 66.20 | 49 | 120.2 | 79 | 80 | 1652 |  |
| 20 | 68.00 | 50 | 122.0 | 80 |  |  |  |

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### 2.0.26 Fahrenheit to Rankine Temperature Conversion

| Fahrenheit | Rankine | Fahrenheit | Rankine | Fahrenheit | Rankine | Fahrenheit | Rankine |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0 | 460.0 | 7 | 467.0 | 23 | 483.0 | 39 | 499.0 |
| 0.1 | 460.1 | 8 | 468.0 | 24 | 484.0 | 40 | 500.0 |
| 0.2 | 460.2 | 9 | 469.0 | 25 | 485.0 | 41 | 501.0 |
| 0.3 | 460.3 | 10 | 470.0 | 26 | 486.0 | 42 | 502.0 |
| 0.4 | 460.4 | 11 | 471.0 | 27 | 487.0 | 43 | 503.0 |
| 0.5 | 460.5 | 12 | 472.0 | 28 | 488.0 | 44 | 504.0 |
| 0.6 | 460.6 | 13 | 473.0 | 29 | 489.0 | 45 | 505.0 |
| 0.7 | 460.7 | 14 | 474.0 | 30 | 490.0 | 46 | 506.0 |
| 0.8 | 460.8 | 15 | 475.0 | 31 | 491.0 | 47 | 507.0 |
| 0.9 | 460.9 | 16 | 476.0 | 32 | 492.0 | 48 | 508.0 |
| 1 | 461.0 | 17 | 477.0 | 33 | 493.0 | 49 | 509.0 |
| 2 | 462.0 | 18 | 478.0 | 34 | 494.0 | 50 | 510.0 |
| 3 | 463.0 | 19 | 479.0 | 35 | 495.0 | 51 | 511.0 |
| 4 | 464.0 | 20 | 480.0 | 36 | 496.0 | 52 | 512.0 |
| 5 | 465.0 | 21 | 481.0 | 37 | 497.0 | 53 | 513.0 |
| 6 | 466.0 | 22 | 482.0 | 38 | 498.0 | 54 | 514.0 |


| Fahrenheit | Rankine | Fahrenheit | Rankine | Fahrenheit | Rankine | Fahrenheit | Rankine |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 55 | 515.0 | 69 | 529.0 | 83 | 543.0 | 97 | 557.0 |
| 56 | 516.0 | 70 | 530.0 | 84 | 544.0 | 98 | 558.0 |
| 57 | 517.0 | 71 | 531.0 | 85 | 545.0 | 99 | 559.0 |
| 58 | 518.0 | 72 | 532.0 | 86 | 546.0 | 100 | 560.0 |
| 59 | 519.0 | 73 | 533.0 | 87 | 547.0 | 200 | 660.0 |
| 60 | 520.0 | 74 | 534.0 | 88 | 548.0 | 300 | 760.0 |
| 61 | 521.0 | 75 | 535.0 | 89 | 549.0 | 400 | 860.0 |
| 62 | 522.0 | 76 | 536.0 | 90 | 550.0 | 500 | 960.0 |
| 63 | 523.0 | 77 | 537.0 | 91 | 551.0 | 600 | 1060 |
| 64 | 524.0 | 78 | 538.0 | 92 | 552.0 | 700 | 1160 |
| 65 | 525.0 | 79 | 539.0 | 93 | 553.0 | 800 | 1260 |
| 66 | 526.0 | 80 | 540.0 | 94 | 554.0 | 900 | 1360 |
| 67 | 527.0 | 81 | 541.0 | 95 | 555.0 |  |  |
| 68 | 528.0 | 82 | 542.0 | 96 | 556.0 |  |  |

Note: Rankine is a temperature scale named after Scottish engineer and physicist William Macquorn Rankine, and is based upon zero being Absolute Zero.
By permission: www.metric-conversions.org

### 2.0.27 Rankine to Fahrenheit Temperature Conversion

| Rankine | Fahrenheit | Rankine | Fahrenheit | Rankine | Fahrenheit | Rankine | Fahrenheit |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0 | -460.00 | 21 | -439.00 | 51 | -409.00 | 81 | -379.00 |
| 0.1 | -459.9 | 22 | -438.00 | 52 | -408.00 | 82 | -378.00 |
| 0.2 | -459.8 | 23 | -437.00 | 53 | -407.00 | 83 | -377.00 |
| 0.3 | -459.7 | 24 | -436.00 | 54 | -406.00 | 84 | -376.00 |
| 0.4 | -459.6 | 25 | -435.00 | 55 | -405.00 | 85 | -375.00 |
| 0.5 | -459.5 | 26 | -434.00 | 56 | -404.00 | 86 | -374.00 |
| 0.6 | -459.4 | 27 | -433.00 | 57 | -403.00 | 87 | -373.00 |
| 0.7 | -459.3 | 28 | -432.00 | 58 | -402.00 | 88 | -372.00 |
| 0.8 | -459.2 | 29 | -431.00 | 59 | -401.00 | 89 | -371.00 |
| 0.9 | -459.1 | 30 | -430.00 | 60 | -400.00 | 90 | -370.00 |
| 1 | -459.00 | 31 | -429.00 | 61 | -399.00 | 91 | -369.00 |
| 2 | -458.00 | 32 | -428.00 | 62 | -398.00 | 92 | -368.00 |
| 3 | -457.00 | 33 | -427.00 | 63 | -397.00 | 93 | -367.00 |
| 4 | -456.00 | 34 | -426.00 | 64 | -396.00 | 94 | -366.00 |
| 5 | -455.00 | 35 | -425.00 | 65 | -395.00 | 95 | -365.00 |
| 6 | -454.00 | 36 | -424.00 | 66 | -394.00 | 96 | -364.00 |
| 7 | -453.00 | 37 | -423.00 | 67 | -393.00 | 97 | -363.00 |
| 8 | -452.00 | 38 | -422.00 | 68 | -392.00 | 98 | -362.00 |
| 9 | -451.00 | 39 | -421.00 | 69 | -391.00 | 99 | -361.00 |
| 10 | -450.00 | 40 | -420.00 | 70 | -390.00 | 100 | -360.00 |
| 11 | -449.00 | 41 | -419.00 | 71 | -389.00 | 200 | -260.00 |
| 12 | -448.00 | 42 | -418.00 | 72 | -388.00 | 300 | -160.00 |
| 13 | -447.00 | 43 | -417.00 | 73 | -387.00 | 400 | -60.00 |
| 14 | -446.00 | 44 | -416.00 | 74 | -386.00 | 500 | 40.00 |
| 15 | -445.00 | 45 | -415.00 | 75 | -385.00 | 600 | 700 |
| 16 | -444.00 | 46 | -414.00 | 76 | -384.00 | 800 | 140.0 |
| 17 | -443.00 | 47 | -413.00 | 77 | -383.00 | -382.00 | 900 |
| 18 | -442.00 | 48 | -412.00 | 78 | -381.00 |  | 340.0 |
| 19 | -441.00 | 49 | -411.00 | 79 | -380.00 | 440.0 |  |
| 20 | -440.00 | 50 | -410.00 | 80 |  |  |  |

Note: Rankine is a temperature scale named after Scottish engineer and physicist William Macquorn Rankine, and is based upon zero being Absolute Zero.
By permission: www.metric-conversions.org

### 2.1.0 Converting Water from One Form to Another

This information is of general interest in converting water information from one form to another. This conversion information can be used in virtually all water measurements.

Acre-feet $\times 43560=$ cubic feet
Acre-feet $\times 1613.3=$ cubic yards
Acre Feet $\times 325851=$ gallons
Acre-feet/day $\times 0.5=$ acre-inches/hour
Acre-feet/day $\times 226.3=$ gallons $/$ minute
Acre-feet/day $\times 0.3259=$ million gallons/day
Cubic feet $\times 1728=$ cubic inches
Cubic feet $\times 0.03704=$ cubic yards
Cubic feet $\times 7.481=$ gallons
Cubic feet/second $\times 449=$ gallons $/$ minute
Cubic feet/second $\times 38.4=$ Colorado miners' inches
Cubic feet/second $\times 0.02832=$ cubic meters/second
Feet of water $\times .0295=$ atmospheres
Feet of water $\times 62.43=$ pounds/square foot
Feet of water $\times .4335=$ pounds/square inch
Gallons $\times .1337=$ cubic feet
Gallons $\times 3.785=$ liters
Gallons of water $\times 8.33=$ pounds of water
Liters $\times 61.02=$ cubic inches
Liters $\times .001=$ cubic meters
Liters $\times .001308=$ cubic yards
Liters $\times .2642=$ gallons
Courtesy of csgnetwork.com/waterconvinformation.html

### 2.1.1 U.S. and Metric Lumber Length Conversion Table

| Actual (ft) to | Metric $(\mathbf{m})$ | Metric $(\mathbf{m})$ to | Actual (ft) |
| :--- | :--- | :--- | :---: |
| 6 | 1.83 | 2 | 6.56 |
| 8 | 2.44 | 2.50 | 8.20 |
| 10 | 3.05 | 3 | 9.84 |
| 12 | 3.66 | 3.50 | 11.48 |
| 14 | 4.27 | 4 | 13.12 |
| 16 | 4.88 | 4.50 | 14.76 |
| 18 | 5.49 | 5 | 16.40 |
| 20 | 6.10 | 5.50 | 18.04 |
| 22 | 6.71 | 6 | 19.68 |
| 24 | 7.32 | 7.50 | 21.32 |
|  |  | 22.96 |  |
| Courtesy of csgnetwork.com/lmbrlengthcvttable.html |  |  |  |

### 2.2.0 Conversion Factors-Energy, Volume, Length, Weight, Liquid

Table of Conversion Factors

| Multiply | By | To Obtain |
| :---: | :---: | :---: |
| Amperes/sq.ft., | 0.108 | amperes/sq. dm. |
| Ampere hours, | 3600 | coulombs |
| Amperes/sq. dm., | 9.29 | amperes/sq. ft . |
| Angstrom units, | $1 \times 10^{-4}$ | microns |
| Centimeters | 0.394 | inches |
| Centimeters | 393.7 | mils |
| Centimeters | 0.0328 | feet |
| Cubic centimeters | $3.53 \times 10^{-5}$ | cubic feet |
| Cubic centimeters | 0.061 | cubic inches |
| Cubic centimeters | $2.64 \times 10^{-4}$ | gallons |
| Cubic centimeters | 0.0338 | ounces (fluid) |
| Cubic feet | 28317 | cubic centimeters |
| Cubic feet | 1728 | cubic inches |
| Cubic feet | 7.48 | gallons |
| Cubic feet of water $60^{\circ} \mathrm{F}$ | 62.37 | pounds |
| Cubic inches | 16.39 | cubic centimeters |
| Faradays | $9.65 \times 10^{-4}$ | coulombs |
| Faraday/second | 96500 | amperes |
| Feet | 30.48 | centimeters |
| Feet | 12 | inches |
| Feet | 0.3048 | meters |
| Gallons | 4 | quarts (liquid) |
| Gallons | 3785.4 | cubic centimeters |
| Gallons (U.S.) | 231 | cubic inches |
| Gallons (U.S.) | 3.785 | liters |
| Gallons (U.S.) | 128 | ounces (fluid) |
| Gallons (U.S.) | 8 | pints |
| Gallons (U.S.) | 8.34 | pounds (av.) of $\mathrm{H}_{2} \mathrm{O}$ at $62^{\circ} \mathrm{F}$. |
| Gallons (U.S.) | 1.2 | gallons (British) |
| Grains | 0.0648 | grams |
| Grains | 0.0023 | ounces (avoir.) |
| Grains | 0.0021 | ounces (troy) |
| Grains | 0.0417 | pennyweights (troy) |
| Grams | 15.43 | grains |
| Grams | 1000 | milligrams |
| Grams | 0.0353 | ounces (avoir.) |
| Grams | 0.0321 | ounces (troy) |
| Grams | 0.643 | pennyweights |
| Grams/liter | 0.122 | ounces/gallon (troy) |
| Grams/liter | 0.134 | ounces/gallon (avoir.) |
| Grams/liter | 1000 | parts per million |
| Grams/liter | 2.44 | pennyweights/gallon |
| Inches | 2.54 | centimeters |
| Inches | 1000 | mils |
| Kilograms | 1000 | grams |
| Kilograms | 2.205 | pounds (avoir.) |
| Kilograms | 2.679 | pounds (troy) |
| Liters | 1000 | milliliters |
| Liters | 0.264 | gallons |
| Meters | 100 | centimeters |


| Table of Conversion Factors—Cont'd |  |  |
| :--- | :--- | :--- |
| Multiply | By | To Obtain |
| Meters | 39.37 | inches |
| Microns | $3.9 \times 10^{-5}$ | inches |
| Milligrams | 0.001 | grams |
| Milliliters | 1.000027 | cubic centimeters |
| Mils | 0.001 | inches |
| Mils | 25.4 | microns |
| Ounces (avoir.) | 437.5 | grains |
| Ounces (avoir.) | 28.35 | grams |
| Ounces (avoir.) | 0.911 | ounces (troy) |
| Ounces (avoir.) | 18.23 | pennyweights |
| Ounces (avoir.) | 0.076 | pounds (troy) |
| Ounces/gallon (avoir.) | 7.5 | grams/liter |
| Ounces (troy) | 480 | grains |
| Ounces (troy) | 31.1 | grams |
| Ounces (troy) | 1.097 | ounces (avoir.) |
| Ounces (troy) | 20 | pennyweights |
| Ounces/gallon (troy) | 8.2 | grams/liter |
| Ounces (fluid) | 29.57 | cubic centimeters |
| Ounces/gallon (fluid) | 7.7 | cc/liter |
| Pennyweights | 24 | grains |
| Pennyweights | 1.56 | grams |
| Pennyweights/gallon | 0.41 | grams/liter |
| Pints | 16 | ounces (fluid) |
| Pounds (avoir.) | 453.6 | grams |
| Pounds (avoir.) | 16 | ounces (avoir.) |
| Pounds (avoir.) | 14.58 | ounces (troy) |
| Pounds (avoir.) | 1.215 | pounds (troy) |
| Pounds (troy) | 373.24 | grams |
| Pounds (troy) | 12 | ounces (troy) |
| Pounds (troy) | 0.823 | pounds (avoir.) |
| Quarts (liquid) | 946.4 | cubic centimeters |
| Quarts (liquid) | 2 | pints |
| Square feet | 929.23 | square centimeters |
| Square feet | 144 | square inches |
| Square inches | 6.45 | square centimeters |
| By |  |  |

By permission: Associated Rack Corp., Vero Beach, FL

### 2.3.0 Conversion of Liquids-Specific Gravity to Degrees Baume

Conversion Table - Specific Gravity, Degrees Baume, Pounds Per Cubic Foot
${ }^{\circ}$ Bé. $=145-\frac{145}{\text { sp.gr. }}\left(\right.$ theavier than $\left.\mathrm{H}_{2} \mathrm{O}\right) ;{ }^{\circ}$ Bé. $=\frac{140}{\text { sp.gr. }}-130\left(\right.$ lighter than $\left.\mathrm{H}_{2} \mathrm{O}\right)$

| Sp. gr. $60^{\circ} / 60^{\circ}$ | ${ }^{\circ}$ Bé | Lb. per gal. at $60^{\circ} \mathrm{F}$ wt. in air | Lb. per cu. ft. at $60^{\circ} \mathrm{F}$ wt. in air |
| :---: | :---: | :---: | :---: |
| 1.000 | 10.00 | 8.3283 | 62.300 |
| 1.005 | 0.72 | 8.3700 | 62.612 |
| 1.010 | 1.44 | 8.4117 | 62.924 |
| 1.015 | 2.14 | 8.4534 | 63.236 |


| Sp. gr. $60^{\circ} / 60^{\circ}$ | ${ }^{\circ}$ Bé | Lb. per gal. at $60^{\circ} \mathrm{F}$ wt. in air | Lb. per cu. ft. at $60^{\circ} \mathrm{F}$ wt. in air |
| :---: | :---: | :---: | :---: |
| 1.020 | 2.84 | 8.4950 | 63.547 |
| 1.025 | 3.54 | 8.5367 | 63.859 |
| 1.030 | 4.22 | 8.5784 | 64.171 |
| 1.035 | 4.90 | 8.6201 | 64.483 |
| 1.040 | 5.58 | 8.6618 | 64.795 |
| 1.045 | 6.24 | 8.7035 | 65.107 |
| 1.050 | 6.91 | 8.7452 | 65.419 |
| 1.055 | 7.56 | 8.7869 | 65.731 |
| 1.060 | 8.21 | 8.8286 | 66.042 |
| 1.065 | 8.85 | 8.8703 | 66.354 |
| 1.070 | 9.49 | 8.9120 | 66.666 |
| 1.075 | 10.12 | 8.9537 | 66.978 |
| 1.080 | 10.74 | 8.9954 | 67.290 |
| 1.085 | 11.36 | 9.0371 | 67.602 |
| 1.090 | 11.97 | 9.0787 | 67.914 |
| 1.095 | 12.58 | 9.1204 | 68.226 |
| 1.100 | 13.18 | 9.1621 | 68.537 |
| 1.105 | 13.78 | 9.2038 | 68.849 |
| 1.110 | 14.37 | 9.2455 | 69.161 |
| 1.115 | 14.96 | 9.2872 | 69.473 |
| 1.120 | 15.54 | 9.3289 | 60.785 |
| 1.125 | 16.11 | 9.3706 | 70.097 |
| 1.130 | 16.68 | 9.4123 | 70.409 |
| 1.135 | 17.25 | 9.4540 | 70.721 |
| 1.140 | 17.81 | 9.4957 | 71.032 |
| 1.145 | 18.36 | 9.5374 | 71.344 |
| 1.150 | 18.91 | 9.5790 | 71.656 |
| 1.155 | 19.46 | 9.6207 | 71.968 |
| 1.160 | 20.00 | 9.6624 | 72.280 |
| 1.165 | 20.54 | 9.7041 | 72.592 |
| 1.170 | 21.07 | 9.7458 | 72.904 |
| 1.175 | 21.6 | 9.7875 | 73.216 |
| 1.180 | 22.12 | 9.8292 | 73.528 |
| 1.185 | 22.64 | 9.8709 | 73.840 |
| 1.190 | 23.15 | 9.9126 | 74.151 |
| 1.195 | 23.66 | 9.9543 | 74.463 |
| 1.200 | 24.17 | 9.9960 | 74.775 |
| 1.205 | 24.67 | 10.0377 | 75.087 |
| 1.210 | 25.17 | 10.0793 | 75.399 |
| 1.215 | 25.66 | 10.1210 | 75.711 |
| 1.220 | 26.15 | 10.1627 | 76.022 |
| 1.225 | 26.63 | 10.2044 | 76.334 |
| 1.230 | 27.11 | 10.2461 | 76.646 |
| 1.235 | 27.59 | 10.2878 | 76.958 |
| 1.240 | 28.06 | 10.3295 | 77.270 |
| 1.245 | 28.53 | 10.3712 | 77.582 |
| 1.250 | 29.00 | 10.4129 | 77.894 |
| 1.255 | 29.46 | 10.4546 | 78.206 |
| 1.260 | 29.92 | 10.4963 | 78.518 |
| 1.265 | 30.38 | 10.5380 | 78.830 |
| 1.270 | 30.83 | 10.5797 | 79.141 |
| 1.275 | 31.27 | 10.6214 | 79.453 |
| 1.280 | 31.72 | 10.6630 | 79.765 |
| 1.285 | 32.16 | 10.7047 | 80.077 |


| Sp. gr. $60^{\circ} / 60^{\circ}$ | ${ }^{\circ}$ Bé | Lb. per gal. at $60^{\circ} \mathrm{F}$ wt. in air | Lb. per cu. ft. at $60^{\circ} \mathrm{F}$ wt. in air |
| :---: | :---: | :---: | :---: |
| 1.290 | 32.60 | 10.7464 | 80.389 |
| 1.295 | 33.03 | 10.7881 | 80.701 |
| 1.300 | 33.46 | 10.8298 | 81.013 |
| 1.305 | 33.89 | 10.8715 | 81.325 |
| 1.310 | 34.31 | 10.9132 | 81.636 |
| 1.315 | 34.73 | 10.9549 | 81.948 |
| 1.320 | 35.15 | 10.9966 | 82.260 |
| 1.325 | 35.57 | 11.0383 | 82.572 |
| 1.330 | 35.98 | 11.0800 | 82.884 |
| 1.335 | 36.39 | 11.1217 | 83.196 |
| 1.340 | 36.79 | 11.1634 | 83.508 |
| 1.345 | 37.19 | 11.2051 | 83.820 |
| 1.350 | 37.59 | 11.2467 | 84.131 |
| 1.355 | 37.99 | 11.2884 | 84.443 |
| 1.360 | 38.38 | 11.3301 | 84.755 |
| 1.365 | 38.77 | 11.3718 | 85.067 |
| 1.370 | 39.16 | 11.4135 | 85.379 |
| 1.375 | 39.55 | 11.4552 | 85.691 |
| 1.380 | 39.93 | 11.4969 | 86.003 |
| 1.385 | 40.31 | 11.5386 | 86.315 |
| 1.390 | 40.68 | 11.5803 | 86.626 |
| 1.395 | 41.06 | 11.6220 | 86.938 |
| 1.400 | 41.43 | 11.6637 | 87.250 |
| 1.405 | 41.80 | 11.7054 | 87.562 |
| 1.410 | 42.16 | 11.7471 | 87.874 |
| 1.415 | 42.53 | 11.7888 | 88.186 |
| 1.420 | 42.89 | 11.8304 | 88.498 |
| 1.425 | 43.25 | 11.8721 | 86.810 |
| 1.430 | 43.60 | 11.9138 | 89.121 |
| 1.435 | 43.95 | 11.9555 | 89.433 |
| 1.440 | 44.31 | 11.9972 | 89.745 |
| 1.445 | 44.65 | 12.0389 | 90.057 |
| 1.450 | 45.00 | 12.0806 | 90.369 |
| 1.455 | 45.34 | 12.1223 | 90.681 |
| 1.460 | 45.68 | 12.1640 | 90.993 |
| 1.465 | 46.02 | 12.2057 | 91.305 |
| 1.470 | 46.36 | 12.2473 | 91.616 |
| 1.475 | 46.69 | 12.2890 | 91.928 |
| 1.480 | 47.03 | 12.3307 | 92.240 |
| 1.485 | 47.36 | 12.3724 | 92.552 |
| 1.490 | 47.68 | 12.4141 | 92.864 |
| 1.495 | 48.01 | 12.4558 | 93.176 |
| 1.500 | 48.33 | 12.4975 | 93.488 |
| 1.505 | 48.65 | 12.5392 | 93.800 |
| 1.510 | 48.97 | 12.5809 | 94.112 |
| 1.515 | 49.29 | 12.6226 | 94.424 |
| 1.520 | 49.61 | 12.6643 | 94.735 |
| 1.525 | 49.92 | 12.7060 | 95.047 |
| 1.530 | 50.23 | 12.7477 | 95.359 |
| 1.535 | 50.54 | 12.7894 | 95.671 |
| 1.540 | 50.84 | 12.8310 | 95.983 |
| 1.545 | 51.15 | 12.8727 | 96.295 |
| 1.550 | 51.45 | 12.9144 | 96.606 |
| 1.555 | 51.75 | 12.9561 | 96.918 |
| 1.560 | 52.05 | 12.9978 | 97.230 |


| Sp. gr. $60^{\circ} / 60^{\circ}$ | ${ }^{\circ}$ Bé | Lb. per gal. at $60^{\circ} \mathrm{F}$ wt. in air | Lb. per cu. ft. at $60^{\circ} \mathrm{F}$ wt. in air |
| :---: | :---: | :---: | :---: |
| 1.565 | 52.35 | 13.0395 | 97.542 |
| 1.570 | 52.64 | 13.0812 | 97.854 |
| 1.575 | 52.94 | 13.1229 | 98.166 |
| 1.580 | 53.23 | 13.1646 | 98.478 |
| 1.585 | 53.52 | 13.2063 | 98.790 |
| 1.590 | 53.81 | 13.2480 | 99.102 |
| 1.595 | 54.09 | 13.2897 | 99.414 |
| 1.600 | 54.38 | 13.3313 | 99.725 |
| 1.605 | 54.66 | 13.3730 | 100.037 |
| 1.610 | 54.94 | 13.4147 | 100.349 |
| 1.615 | 55.22 | 13.4564 | 100.661 |
| 1.620 | 55.49 | 13.4981 | 100.973 |
| 1.625 | 55.77 | 13.5398 | 101.285 |
| 1.630 | 56.04 | 13.5815 | 101.597 |
| 1.635 | 56.32 | 13.6232 | 101.909 |
| 1.640 | 56.59 | 13.6649 | 102.220 |
| 1.645 | 56.85 | 13.7066 | 102.532 |
| 1.650 | 57.12 | 13.7483 | 102.844 |
| 1.655 | 57.39 | 13.7900 | 103.156 |
| 1.660 | 57.65 | 13.8317 | 103.468 |
| 1.665 | 57.91 | 13.8734 | 103.780 |
| 1.670 | 58.17 | 13.9150 | 104.092 |
| 1.675 | 58.43 | 13.9567 | 104.404 |
| 1.680 | 58.69 | 13.9984 | 104.715 |
| 1.685 | 58.95 | 14.0401 | 105.027 |
| 1.690 | 59.20 | 14.0818 | 105.339 |
| 1.695 | 59.45 | 14.1235 | 105.651 |
| 1.700 | 59.71 | 14.1652 | 105.963 |
| 1.705 | 59.96 | 14.2069 | 106.275 |
| 1.710 | 60.20 | 14.2486 | 106.587 |
| 1.715 | 60.45 | 14.2903 | 196.899 |
| 1.720 | 60.70 | 14.3320 | 107.210 |
| 1.725 | 60.94 | 14.3737 | 107.522 |
| 1.730 | 61.18 | 14.4153 | 107.834 |
| 1.735 | 61.34 | 14.4570 | 108.146 |
| 1.740 | 61.67 | 14.4987 | 108.458 |
| 1.745 | 61.91 | 14.5404 | 108.770 |
| 1.750 | 62.14 | 14.5821 | 109.082 |
| 1.755 | 62.38 | 14.6238 | 109.394 |
| 1.760 | 62.61 | 14.6655 | 109.705 |
| 1.765 | 62.85 | 14.7072 | 110.017 |
| 1.770 | 63.08 | 14.7489 | 110.329 |
| 1.775 | 63.31 | 14.7906 | 110.641 |
| 1.780 | 63.54 | 14.8323 | 110.953 |
| 1.785 | 63.77 | 14.8740 | 111.265 |
| 1.790 | 63.99 | 14.9157 | 111.577 |
| 1.795 | 64.22 | 14.9574 | 111.889 |
| 1.800 | 64.44 | 14.9990 | 112.200 |
| 1.805 | 64.67 | 15.0407 | 112.512 |
| 1.810 | 64.89 | 15.0824 | 112.824 |
| 1.815 | 65.11 | 15.1241 | 113.136 |
| 1.820 | 65.33 | 15.1658 | 113.448 |
| 1.825 | 65.55 | 15.2075 | 113.760 |
| 1.830 | 65.77 | 15.2492 | 114.072 |



### 2.4.0 Volume-to-Weight Conversion Table

The volume-to-weight conversion table presented on the following pages is a compilation of several sources. Materials converted from volume to weight include paper (high-grade and other), glass, plastic, metals, organics, and other materials (e.g., tires and oil).

It is important to note that although the weight (density) figures presented here are useful for determining rough estimates, they will not be as useful when precise measurements are required. Differences in the way a material is handled, processed, or in the amount of moisture present can make substantial differences in the amount a particular material weighs per specified volume. Because of these differences, it will be important to actually sort and weight materials in your program whenever precise measurements are needed (e.g., recycling contract agreements).

| Category | Material <br> (u/c = uncompacted/compacted \& baled) | Volume | Estimated Weight (in pounds) |
| :---: | :---: | :---: | :---: |
| High-Grade Paper | Computer Paper: |  |  |
|  | Uncompacted, stacked | $1 \mathrm{cu} . \mathrm{yd}$. | 655 |
|  | Compacted/baled | $1 \mathrm{cu} . \mathrm{yd}$. | 1,310 |
|  | 1 case | 2800 sheets | 42 |
|  | White Ledger: |  |  |
|  | (u)stacked/(c)stacked | $1 \mathrm{cu} . \mathrm{yd}$. | 375465/755-925 |
|  | (u)crumpled/(c)crumpled | $1 \mathrm{cu} . \mathrm{yd}$. | 11 0205/325 |
|  | Ream of 20\# bond; 8-1/2 $\times 11$ | 1 ream $=500$ sheets | 5 |
|  | Ream of 20\# bond; 8-1/2 $\times 14$ | 1 ream $=500$ sheets | 6.4 |
|  | White ledger pads | 1 case $=72$ pads | 38 |
|  | Tab Cards |  |  |
|  | Uncompacted | $1 \mathrm{cu} . \mathrm{yd}$. | 605 |
|  | Compacted/baled | $1 \mathrm{cu} . \mathrm{yd}$. | 1,215-1,350 |


| Category | Material <br> (u/c = uncompacted/compacted \& baled) | Volume | Estimated Weight (in pounds) |
| :---: | :---: | :---: | :---: |
| Other Paper | Cardboard (Corrugated): |  |  |
|  | Uncompacted | $1 \mathrm{cu} . \mathrm{yd}$. | 50-150 |
|  | Compacted | $1 \mathrm{cu} . \mathrm{yd}$. | 300-500 |
|  | Baled | $1 \mathrm{cu} . \mathrm{yd}$. | 7001,100 |
|  | Newspaper: |  |  |
|  | Uncompactad | $1 \mathrm{cu} . \mathrm{yd}$. | 360-505 |
|  | Compacted/baled | $1 \mathrm{cu} . \mathrm{yd}$. | 7,201,000 |
|  | 12 stack | - | 35 |
|  | Miscellaneous Paper: |  |  |
|  | Yellow legal pads | 1 case $=72$ pads | 38 |
|  | Colored message pads | 1 carton $=144$ pads | 22 |
|  | Self-carbon forms; 8-1/2 $\times 11$ | 1 ream $=500$ sheets | 50 |
|  | Mixed Ledger/Office Paper: |  |  |
|  | Flat (u/c) | $1 \mathrm{cu} . \mathrm{yd}$. | 380/755 |
|  | Crumpled (u/c) | $1 \mathrm{cu} . \mathrm{yd}$. | 110205/610 |
| Gass | Refillable Whole Bottles: |  |  |
|  | Refillable beer bottles | 1 case $=24$ bottles: | 14 |
|  | Refillable soft drink bottles | 1 case $=24$ bottles | 22 |
|  | 8 oz. glass container | 1 case $=24$ bottles | 12 |
|  | Bottles: |  |  |
|  | Whole | $1 \mathrm{cu} . \mathrm{yd}$ | 500-700 |
|  | Semi-crushed | $1 \mathrm{cu} . \mathrm{yd}$. | 1,0001,800 |
|  | Crushed (mechanically) | $1 \mathrm{cu} . \mathrm{yd}$. | 1,800-2,700 |
|  | Uncrushed to manually broken | 55 gallon drum | 300 |
| Plastic | PET (Soda Bottles): |  |  |
|  | Whole bottles, uncompacted | $1 \mathrm{cu} . \mathrm{yd}$. | 30-40 |
|  | Whole bottles, compacted | $1 \mathrm{cu} . \mathrm{yd}$. | 515 |
|  | Whole bottles, uncompacted | gaylord | 40-53 |
|  | Baled | $30^{\prime \prime} \times 62^{\prime \prime}$ | 500-550 |
|  | Granulated | gaylord | 700-750 |
|  | 8 bottles (2-liter size) |  | 1 |
|  | HDPE(Dairy): |  |  |
|  | Whole, uncompacted | $1 \mathrm{cu} . \mathrm{yd}$. | 24 |
|  | Whole, compacted | $1 \mathrm{cu} . \mathrm{yd}$. | 270 |
|  | Baled | $32^{\prime \prime} \times 60^{\prime \prime}$ | 400-500 |
|  | HDFE(Mixed): |  |  |
|  | Baled | $32^{\prime \prime} \times 60^{\prime \prime}$ | 900 |
|  | Granulated | semi-load | 42,000 |
|  | Odd Plastic: |  |  |
|  | Uncompacted | $1 \mathrm{cu} . \mathrm{yd}$. | 50 |
|  | Compacted/baled | 1 cu.yd. | 400-700 |
|  | Mixed PET and HDPE (Dairy): |  |  |
|  | Whole, uncompacted | $1 \mathrm{cu} . \mathrm{yd}$. | 32 |


| Category | Material <br> (u/c = uncompacted/compacted \& baled) | Volume | Estimated Weight (in pounds) |
| :---: | :---: | :---: | :---: |
| Metals | Aluminum (Cans): |  |  |
|  | Whole | 1 cu.yd. | 50-75 |
|  | Compacted (manually) | $1 \mathrm{cu} . \mathrm{yd}$. | 250-430 |
|  | Uncompacted | 1 full grocery bag | 1.5 |
|  |  | 1 case $=24$ cans | 0.9 |
|  | Ferrous (tin-coated steel cans): |  |  |
|  | Whole | $1 \mathrm{cu} . \mathrm{yd}$. | 150 |
|  | Flattened | $1 \mathrm{cu} . \mathrm{yd}$. | 850 |
|  | Whole | 1 case $=6$ cans | 22 |
| Organics | Yard trmming*: |  |  |
|  | Leaves (uncompacted) | $1 \mathrm{cu} . \mathrm{yd}$. | 200-250 |
|  | Leaves (compacted) | $1 \mathrm{cu} . \mathrm{yd}$. | 300-450 |
|  | Leaves, vacuumed | $1 \mathrm{cu} . \mathrm{yd}$. | 350 |
|  | Grass clippings (uncompacted) | $1 \mathrm{cu} . \mathrm{yd}$. | 350-450 |
|  | Grass clippings (compacted) | $1 \mathrm{cu} . \mathrm{yd}$. | 550-1,500 |
|  | Finished compost | $1 \mathrm{cu} . \mathrm{yd}$. | 600 |
|  | Scrap wood: |  |  |
|  | Pallets |  | 30-100 (40 avg.) |
|  | Wood chips | $1 \mathrm{cu} . \mathrm{yd}$. | 500 |
|  | Food Waste: |  |  |
|  | Solid/liquid fats | 55-gallon drum | 400-410 |
| Other Materials | Tires: |  |  |
|  | Car | 1 tire | 12-20 |
|  | Truck | 1 tire | 60-100 |
|  | Oil (Used Motor Oil) | 1 gallon | 7 |

*Density of yard trimmings is highly variable depending on moisture content.

### 2.5.0 Convert Old A.I.S.C. Structural Shapes to New Designations

## A.I.S.C. Hot-Rolled Structural Steel Shape Designations

| New Designation | Type of Shape | Old Designation |
| :---: | :---: | :---: |
| W $24 \times 76$ | W shape | 24 WF 76 |
| W $14 \times 26$ | W shape | 14 B 26 |
| S $24 \times 100$ | S shape | 241100 |
| M $8 \times 18.5$ | $M$ shape | 8 M 18.5 |
| M $10 \times 9$ | $M$ shape | 10 JR 9.0 |
| M $8 \times 34.3$ | $M$ shape | $8 \times 8 \mathrm{M} 34.3$ |
| C $12 \times 20.7$ | American Std. Channel | 12 C 20.7 |
| MC $12 \times 45$ | Miscellaneous Channel | $12 \times 4 \mathrm{C} 45.0$ |
| MC $12 \times 10.6$ | Miscellaneous Channel | 12 JR C 10.6 |
| HP $14 \times 73$ | HP shape | 14 BP 73 |
| L6 $\times 6 \times 3 / 4$ | Equal Leg Angle | L $6 \times 6 \times 3 / 4$ |

A.I.S.C. Hot-Rolled Structural Steel Shape Designations-Cont'd

| New Designation | Type of Shape | Old Designation |
| :---: | :---: | :---: |
| $\mathrm{L} 6 \times 4 \times 5 / 8$ | Unequal Leg Angle | L $6 \times 4 \times 5 / 8$ |
| WT $12 \times 38$ | Structural Tee | ST 12 WF 38 |
| WT $7 \times 13$ | Cut from W shape | ST 7 B 13 |
| ST $12 \times 50$ | Cut from $S$ shape | ST 12 / 50 |
| MT $4 \times 9.25$ | Cut from M shape | ST 4 M 9.25 |
| MT $5 \times 4.5$ | Cut from M shape | ST 5 JR 4.5 |
| MT $4 \times 17.15$ | Cut from M shape | ST 4 M 17.15 |
| PL $1 / 2 \times 18$ | Plate | PL $18 \times 1 / 2$ |
| Bar 1ם | Square Bar | Bar 1ם |
| Bar 1-1/4 0 | Round Bar | Bar 1-1/4 0 |
| Bar 2-1/2 $\times 1 / 2$ | Flat Bar | Bar 2-1/2 $\times 1 / 2$ |
| Pipe 4 Std. | Pipe | Pipe 4 Std. |
| Pipe $4 \times$. Strong | Pipe | Pipe $4 \times$-Strong |
| Pipe $4 \times \times$ Strong | Pipe | Pipe $4 \times \times$-Strong |
| TS $4 \times 4 \times .375$ | Structural Tubing: Square | Tube $4 \times 4 \times .375$ |
| TS $5 \times 3 \times .375$ | Rectangular | Tube $5 \times 3 \times .375$ |
| TS 3 OD $\times .250$ | Circular | Tube 3 OD $\times .250$ |

Source: Cardinal Metals, Irving, Texas

### 2.6.0 USA Mesh Size—Convert to International Particle Size (Microns)

International Sieve Chart / Micropowder Grit Chart

| $\begin{aligned} & \text { ASTM } \\ & \text { E-11 } \end{aligned}$ | ANSI B74.12-1992 | JIS | BSI | Particle <br> Diameter | AFNOR | DIN | Tyler ${ }^{\text {® }}$ | Angstrom <br> Units ( $\AA$ ) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| USA | USA | JPN | GBR | USA | FRA | DEU | USA | Global |
| USS <br> Mesh | d50 (microns) <br> Sedimentation tube method | Microns ( $\mu$ ) | Mesh | Microns ( $\mu$ ) | Microns ( $\mu$ ) | Microns ( $\mu$ ) | Mesh | Angstroms ( $\AA$ ) |
| 3 in . |  | 71 |  | 75 mm |  |  |  |  |
| 2 in . |  | 50 |  | 50 |  |  |  |  |
| 1.06 in. |  | 26.5 |  | 26.5 |  | 26.5 | 1.05 in. |  |
| 7/8 in. |  | 22.4 |  | 22.4 | 22.4 | 22.4 | 0.883 in. |  |
| 3/4 in. |  | 19 |  | 19.0 |  | 19 | 0.742 in . |  |
| 5/8 in. |  | 16 |  | 16.0 | 16 | 16 | 0.624 in. |  |
| 1/2 in. |  | 12.5 |  | 12.5 | 12.5 | 12.5 |  |  |
| 7/16 in. |  | 11.2 |  | 11.2 | 11.2 | 11.2 | 0.441 in. |  |
| 3/8 in. |  | 9.5 |  | 9.5 | 11.2 | 9.5 | 0.371 in. |  |
| 5/16 in. |  | 8 |  | 8.0 | 8 | 8 | 2.5 |  |
| 0.265 in . |  | 6.7 |  | 6.7 |  | 6.7 | 3 |  |
| 3.5 |  | 90 |  | 5.6 | 5.6 | 5.6 | 3.5 |  |
| 5 |  | 5 |  | 4.00 | 4 | 4 | 5 |  |
| 8 |  | 8 |  | 2.36 |  | 2.36 | 8 |  |
| 12 |  |  |  | 1.70 |  | 1.7 | 10 |  |
| 14 |  | 1.4 | 12 | 1.40 | 1.4 | 1.4 | 12 |  |
| 16 |  |  |  | 1.18 |  | 1.18 | 14 |  |
| 18 |  |  |  | 1.00 | 1,0 | 1.0 | 16 |  |
| 20 |  | 850 | 18 | 850 |  | 850 | 20 |  |

(Continued)

International Sieve Chart / Micropowder Grit Chart-Cont'd

| $\begin{aligned} & \text { ASTM } \\ & \text { E-11 } \end{aligned}$ | ANSI B74.12-1992 | JIS | BSI | Particle <br> Diameter | AFNOR | DIN | Tyler ${ }^{\text {® }}$ | Angstrom <br> Units ( A ) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| USA | USA | JPN | $G B R$ | USA | FRA | DEU | USA | Global |
| USS <br> Mesh | d50 (microns) <br> Sedimentation tube method | Microns ( $\boldsymbol{\mu}$ ) | Mesh | Microns ( $\mu$ ) | Microns ( $\boldsymbol{\mu}$ ) | Microns ( $\mu$ ) | Mesh | Angstroms ( $\AA$ ) |
| 25 |  | 710 | 22 | 710 | 710 | 710 | 24 |  |
| 30 |  | 600 | 25 | 600 |  | 600 | 28 |  |
| 35 |  | 500 | 30 | 500 | 500 | 500 | 32 |  |
| 40 |  | 425 | 36 | 425 |  | 425 | 35 |  |
| 45 |  | 355 | 44 | 355 | 355 | 355 | 42 |  |
| 50 |  | 300 | 52 | 300 |  | 300 | 48 |  |
| 60 |  | 250 | 60 | 250 | 250 | 250 | 60 |  |
| 70 |  | 212 | 72 | 212 |  | 212 | 65 |  |
| 80 |  | 180 | 85 | 180 | 180 | 180 | 80 |  |
| 100 |  | 150 | 100 | 150 |  | 150 | 100 |  |
| 120 |  | 125 | 120 | 125 | 125 | 125 | 115 |  |
| 140 |  | 106 |  | 106 |  | 106 | 150 |  |
| 170 |  | 90 | 170 | 90 | 90 | 90 | 170 |  |
| 200 |  | 75 | 200 | 75 |  | 75 | 200 |  |
| 230 | 240 grit $=53.5-50$ | 63 | 240 | 63 | 63 | 63 | 250 |  |
| 270 | 280 grit $=44-40.5$ | 53 | 300 | 53 |  | 53 | 270 |  |
| 325 | 320 grit $=36-32.5$ | 45 | 350 | 45 | 45 | 45 | 325 |  |
| 400 | 360 grit $=28.8-25.8$ | 38 | 400 | 38 |  | 38 | 400 |  |
| 450 | 400 grit $=23.6-20.6$ | 32 | 440 | 32 | 32 | 32 | 450 |  |
| 500 | 500 grit $=19.7-16.7$ |  |  | 25 | 25 | 25 | 500 |  |
| 635 | 600 grit $=16-13$ |  |  | 20 | 20 | 20 | 635 |  |
|  | 800 grit $=12.3-9.8$ |  |  | 16 |  | 16 |  |  |
|  | 1000 grit $=9.3-6.8$ |  |  | 10 |  | 10 | 1,250 |  |
|  | 1200 grit $=6.5-4.5$ |  |  | 5 |  | 5 | 5,000 |  |
| 1 |  |  |  |  |  | 1 | 10,000 |  |
|  |  |  |  | 0.1 |  |  |  | 1,000 |
|  |  |  |  | 0.01 |  |  |  | 100 |
|  |  |  |  | 0.001 |  |  |  | 10 |
|  |  |  |  | 0.0001 |  |  |  | 1 |
|  |  |  |  | 0 |  |  |  | 0 |

## Particle Size Conversion Chart

| 1 cm | 1 mm | 100 micrometers | 10 micrometers | 1 micrometer | 100 nanometers | 10 nanometers | 1 nanometer |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| 0.01 | 0.001 | 0.0001 meter | 0.00001 meter | 0.000001 <br> meter | 0.0000001 <br> meter | meter |  |

Source: READE.com

Applicable Standards Applicable standards are ISO 565 (1987), ISO 3310 (1999), ASTM E 11-70 (1995), DIN 4188 (1977), BS 410 (1986) and AFNOR NFX11-501 (1987).

### 2.7.0 Converting Wire Gauge from

1. Standard Wire Gauge (S.W.G.)
to
2. Wire Number
to
3. American Wire Gauge (A.W.G.)
to
4. A.W.G. and Brown \& Sharpe (B\&S)-Inches
to
5. A.W.G. to Metric

| S.W.G. <br> (Inches) | Wire Number <br> (Gauge) | A.W.G. or B\&S <br> (Inches) | A.W.G. Metric <br> (MM) |
| :--- | :--- | :--- | ---: |
| 0.500 | $0000000(7 / 0)$ | $\ldots \ldots \ldots \ldots \ldots \ldots \ldots$ | $\ldots \ldots \ldots \ldots \ldots$ |
| 0.464 | $000000(6 / 0)$ | 0.580000 | $\ldots \ldots \ldots \ldots$ |
| 0.432 | $00000(5 / 0)$ | 0.516500 | $\ldots \ldots \ldots \ldots$ |
| 0.400 | $0000(4 / 0)$ | 0.460000 | 11,684 |
| 0.372 | $000(3 / 0)$ | 0.409642 | 10,404 |
| 0.348 | $00(2 / 0)$ | 0.364796 | 9,266 |
| 0.324 | $0(1 / 0)$ | 0.324861 | 8,252 |
| 0.300 | 1 | 0.289297 | 7,348 |
| 0.276 | 2 | 0.257627 | 6,543 |
| 0.252 | 3 | 0.229423 | 5,827 |
| 0.232 | 4 | 0.2043 | 5,189 |
| 0.2120 | 5 | 0.1819 | 4,621 |
| 0.1920 | 6 | 0.1620 | 4,115 |
| 0.1760 | 7 | 0.1443 | 3,665 |
| 0.1600 | 8 | 0.1285 | 3,264 |
| 0.1440 | 9 | 0.1144 | 2,906 |
| 0.1280 | 10 | 0.1019 | 2,588 |
| 0.1160 | 11 | 0.0907 | 2,304 |
| 0.1040 | 12 | 0.0808 | 2,052 |
| 0.0920 | 13 | 0.0720 | 1,829 |
| 0.0800 | 14 | 0.0641 | 1,628 |
| 0.0720 | 15 | 0.0571 | 1,450 |
| 0.0640 | 16 | 0.0508 | 1,291 |
| 0.0560 | 17 | 0.0453 | 1,150 |
| 0.0480 | 18 | 0.0403 | 1,024 |
| 0.0400 | 19 | 0.0359 | 0,9119 |
| 0.0360 | 20 | 0.0320 | 0,8128 |
| 0.0320 | 21 | 0.0285 | 0,7239 |
| 0.0280 | 22 | 0.0253 | 0,6426 |
|  |  |  |  |


| S.W.G. <br> (Inches) | Wire Number <br> (Gauge) | A.W.G. or B\&S <br> (Inches) | A.W.G. Metric <br> (MM) |
| :--- | :--- | :--- | :--- |
| 0.0240 | 23 | 0.0226 | 0,5740 |
| 0.0220 | 24 | 0.0201 | 0,5106 |
| 0.0200 | 25 | 0.0179 | 0,4547 |
| 0.0180 | 26 | 0.0159 | 0,4038 |
| 0.0164 | 27 | 0.0142 | 0,3606 |
| 0.0148 | 28 | 0.0126 | 0,3200 |
| 0.0136 | 29 | 0.0113 | 0,2870 |
| 0.0124 | 30 | 0.0100 | 0,2540 |
| 0.0116 | 31 | 0.0089 | 0,2261 |
| 0.0108 | 32 | 0.0080 | 0,2032 |
| 0.0100 | 33 | 0.0071 | 0,1803 |
| 0.0092 | 34 | 0.0063 | 0,1601 |
| 0.0084 | 35 | 0.0056 | 0,1422 |
| 0.0076 | 36 | 0.0050 | 0,1270 |
| 0.0068 | 37 | 0.0045 | 0,1143 |
| 0.0060 | 38 | 0.0035 | 0,1016 |
| 0.0052 | 39 | 0.0031 | 0,0889 |
| 0.0048 | 40 | 0.0028 | 0,0787 |
| 0.0044 | 41 | 0.0025 | 0,0711 |
| 0.0040 | 42 | 0.0022 | 0,0635 |
| 0.0036 | 43 | 0.0020 | 0,0559 |
| 0.0032 | 44 | 0.0018 | 0,0508 |
| 0.0028 | 45 | 0.0016 | 0,0457 |
| 0.0024 | 46 | 0.0014 | 0,0406 |
| 0.0020 | 47 | 0.0012 | 0,0350 |
| 0.0016 | 48 | 0.0011 | 0,0305 |
| 0.0012 | 49 | 0.0010 | 0,0279 |
| 0.0010 | 50 | 0.00088 | 0,0254 |
|  | 51 | 0.00078 | 0,0224 |
|  | 52 | 0.00070 | 0,0198 |
|  | 53 | 0.00062 | 0,0178 |
| Source: READE.com | 0.00055 | 0,0158 |  |
|  | 54 | 0.00049 | 0,0124 |
|  |  |  |  |
|  |  |  |  |

### 2.8.0 Map of United States Showing Four Continental Times Zones


2.8.1 Convert Time Zones-UTC to Four Standard U.S. Time Zones

Conversions from UTC to some U.S. time zones

| UTC <br> (GMT) | Pacific <br> Standard | Mountain <br> Standard | Central <br> Standard | Eastern <br> Standard |
| :--- | :---: | :---: | :---: | :---: |
| 0 | $4 \mathrm{pm}^{*}$ | $5 \mathrm{pm}^{*}$ | $6 \mathrm{pm}^{*}$ | $7 \mathrm{pm}^{*}$ |
| 1 | $5 \mathrm{pm}^{*}$ | $6 \mathrm{pm}^{*}$ | $7 \mathrm{pm}^{*}$ | $8 \mathrm{pm}^{*}$ |
| 2 | $6 \mathrm{pm}^{*}$ | $7 \mathrm{pm}^{*}$ | $8 \mathrm{pm}^{*}$ | $9 \mathrm{pm}^{*}$ |
| 3 | $7 \mathrm{pm}^{*}$ | $8 \mathrm{pm}^{*}$ | $9 \mathrm{pm}^{*}$ | $10 \mathrm{pm}^{*}$ |
| 4 | $8 \mathrm{pm}^{*}$ | $9 \mathrm{pm}^{*}$ | $10 \mathrm{pm}^{*}$ | $11 \mathrm{pm}^{*}$ |
| 5 | $9 \mathrm{pm}^{*}$ | $10 \mathrm{pm}^{*}$ | $11 \mathrm{pm}^{*}$ | 12 mid |
| 6 | $10 \mathrm{pm}^{*}$ | $11_{\mathrm{pm}}{ }^{*}$ | $12 \mathrm{mid}^{12 \mathrm{mid}}$ | 1 am |
| 7 | $11 \mathrm{pm}^{*}$ | 12 mid | 1 am | 2 am |
| 8 | 12 mid | 1 am | 2 am | 3 am |
| 9 | 1 am | 2 am | 3 am | 4 am |
| 10 | 2 am | 3 am | 4 am | 5 am |

Conversions from UTC to some U.S. time zones-Cont'd

| UTC <br> (GMT) | Pacific <br> Standard | Mountain <br> Standard | Central <br> Standard | Eastern <br> Standard |
| :--- | :--- | :--- | :--- | :--- |
| 11 | 3 am | 4 am | 5 am | 6 am |
| 12 | 4 am | 5 am | 6 am | 7 am |
| 13 | 5 am | 6 am | 7 am | 8 am |
| 14 | 6 am | 7 am | 8 am | 9 am |
| 15 | 7 am | 8 am | 9 am | 10 am |
| 16 | 8 am | 9 am | 10 am | 11 am |
| 17 | 9 am | 10 am | 11 am | 12 noon |
| 18 | 10 am | 11 am | 12 noon | 1 pm |
| 19 | 11 am | 12 noon | 1 pm | 2 pm |
| 20 | 12 noon | 1 pm | 2 pm | 3 pm |
| 21 | 1 pm | 2 pm | 3 pm | 4 pm |
| 22 | 2 pm | 3 pm | 4 pm | 5 pm |
| 23 | 3 pm | 4 pm | 5 pm | 6 pm |

* = previous day


### 2.8.2 Convert Time Zones-UTC to Four Daylight U.S. Time Zones

Conversions from UTC to some U.S. time zones

| UTC <br> (GMT) | Pacific <br> Daylight | Mountain <br> Daylight | Central <br> Daylight | Eastern <br> Daylight |
| :--- | :---: | :---: | :---: | :---: |
| 0 | $5 \mathrm{pm}^{*}$ | $6 \mathrm{pm}^{*}$ | $7 \mathrm{pm}^{*}$ | $8 \mathrm{pm}^{*}$ |
| 1 | $6 \mathrm{pm}^{*}$ | $7 \mathrm{pm}^{*}$ | $8 \mathrm{pm}^{*}$ | $9 \mathrm{pm}^{*}$ |
| 2 | $7 \mathrm{pm}^{*}$ | $8 \mathrm{pm}^{*}$ | $9 \mathrm{pm}^{*}$ | $10 \mathrm{pm}^{*}$ |
| 3 | $8 \mathrm{pm}^{*}$ | $9 \mathrm{pm}^{*}$ | $10 \mathrm{pm}^{*}$ | $11 \mathrm{pm}{ }^{*}$ |
| 4 | $9 \mathrm{pm}^{*}$ | $10 \mathrm{pm}^{*}$ | $11 \mathrm{pm}^{*}$ | 12 mid |
| 5 | $10 \mathrm{pm}^{*}$ | $11 \mathrm{pm}^{*}$ | 12 mid | 1 am |
| 6 | $11 \mathrm{pm}^{*}$ | 12 mid | 1 am | 2 am |
| 7 | 12 mid | 1 am | 2 am | 3 am |
| 8 | 1 am | 2 am | 3 am | 4 am |
| 9 | 2 am | 3 am | 4 am | 5 am |
| 10 | 3 am | 4 am | 5 am | 6 am |
| 11 | 4 am | 5 am | 6 am | 7 am |
| 12 | 5 am | 6 am | 7 am | 8 am |
| 13 | 6 am | 7 am | 8 am | 9 am |
| 14 | 7 am | 8 am | 9 am | 10 am |
| 15 | 8 am | 9 am | 10 am | 11 am |
| 16 | 9 am | 10 am | 11 am | 12 noon |
| 17 | 10 am | 11 am | 12 noon | 1 pm |
| 18 | 11 am | 12 noon | 1 pm | 2 pm |
| 19 | 12 noon | 1 pm | 2 pm | 3 pm |
| 20 | 1 pm | 2 pm | 3 pm | 4 pm |
| 21 | 2 pm | 3 pm | 4 pm | 5 pm |
| 22 | 3 pm | 4 pm | 5 pm | 6 pm |
| 23 | 4 pm | 5 pm | 6 pm | 7 pm |

*= previous day

### 2.9.0 Convert Roman Numerals to Arabic Dates

Roman numerals from 1 to 1 million and their Arabic equivalent are listed below. Arabic to Roman dates for the 19th and 20th centuries and a portion of the 21st century are also included in this conversion exercise.

| Roman | Arabic |
| :--- | :--- |
| L | 1 |
| V | 5 |
| X | 10 |
| L | 50 |
| C | 100 |
| $D$ | 500 |
| M | 1,000 |
| $\mathbf{i}$ | 1,000 |
| v | 50,000 |
| x | 10,000 |
| l | 500,000 |
| c | $1,000,000$ |
| $d$ |  |
| $m$ |  |


| 19 th Century | $1851=$ MDCCCLI |
| :--- | :--- |
| $1801=$ MDCCCI | $1852=$ MDCCCLII |
| $1802=$ MDCCCII | $1853=$ MDCCCLIII |
| 1803 | $=$ MDCCCIIII |
| 1804 | $=$ MDCCCIV |
| 1805 | $=$ MDCCCV |
| 1806 | $=$ MDCCCVI |
| 1807 | $=$ MDCCCVII |
| 1808 | $=$ MDCCCVIIII |
| 1809 | $=$ MDCCCIX |

20th Century
1901 = MCMI
$1902=$ MCMII
1903 = MCMIII
$1904=$ MCMIV
$1905=$ MCMV
$1906=$ MCMVI
1907 = MCMVII
1908 = MCMVIII
$1909=$ MCMIX
$1910=$ MCMX
1911 = MCMXI
1912 = MCMXII
1913 = MCMXIII
$1914=$ MCMXIV
$1915=$ MCMXV
$1916=$ MCMXVI
1917 = MCMXVII
1918 = MCMXVIII
$1919=$ MCMXIX
$1920=$ MCMXX
1921 = MCMXXI
1922 = MCMXXII
1923 = MCMXXIII
$1924=$ MCMXXIV
$1925=$ MCMXXV
$1926=$ MCMXXVI
1927 = MCMXXVII
1928 = MCMXXVIII
$1929=$ MCMXXIX
$1930=$ MCMXXX
1931 = MCMXXXI
$1932=$ MCMXXXII
1933 = MCMXXXIII
1934 = MCMXXXIV
$1935=$ MCMXXXV
$1936=$ MCMXXXVI
1937 = MCMXXXVII
1938 = MCMXXXVIII
$1939=$ MCMXXXIX
$1940=$ MCMXL

|  | 1941 = MCMXLI |
| :---: | :---: |
|  | $1942=$ MCMXLII |
|  | 1943 = MCMXLIII |
|  | 1944 = MCMXLIV |
|  | 1945 = MCMXLV |
|  | $1946=$ MCMXLVI |
|  | 1947 = MCMXLVII |
|  | 1948 = MCMXLVIII |
|  | 1949 = MCMXLIX |
|  | 1950 = MCML |
|  | 1951 = MCMLI |
|  | 1952 = MCMLII |
|  | 1953 = MCMLIII |
|  | 1954 = MCMLIV |
|  | 1955 = MCMLV |
|  | $1956=$ MCMLVI |
|  | 1957 = MCMLVII |
|  | 1958 = MCMLVIII |
|  | 1959 = MCMLIX |
|  | 1960 = MCMLX |
|  | 1961 = MCMLXI |
|  | 1962 = MCMLXII |
|  | 1963 = MCMLXIII |
|  | 1964 = MCMLXIV |
|  | 1965 = MCMLXV |
|  | 1966 = MCMLXVI |
|  | 1967 = MCMLXVII |
|  | 1968 = MCMLXVIII |
|  | 1969 = MCMLXIX |
|  | 1970 = MCMLXX |
|  | 1971 = MCMLXXI |
|  | 1972 = MCMLXXII |
|  | 1973 = MCMLXXIII |
|  | 1974 = MCMLXXIV |
|  | 1975 = MCMLXXV |
|  | $1976=$ MCMLXXVI |
|  | 1977 = MCMLXXVII |
|  | 1978 = MCMLXXVIII |
|  | 1979 = MCMLXXIX |
|  | $1980=$ MCMLXXX |
|  | 1981 = MCMLXXXI |

$1982=$ MCMLXXXII
1983 = MCMLXXXIII
1984 = MCMLXXXIV
$1985=$ MCMLXXXV
$1986=$ MCMLXXXVI
1987 = MCMLXXXVII
$1988=$ MCMLXXXVIII
$1989=$ MCMLXXXIX
$1990=$ MCMXC
$1991=\mathrm{MCMXCI}$
$1992=$ MCMXCII
1993 = MCMXCIII
$1994=$ MCMXCIV
$1995=$ MCMXCV
$1996=$ MCMXCVI
1997 = MCMXCVII
$1998=$ MCMXCVIII
$1999=$ MCMXCIX
$2000=M M$
21st Century
2001 = MMI
2002 = MMII
$2003=$ MMIII
$2004=$ MMIV
$2005=$ MMV
$2006=$ MMVI
2007 = MMVII
2008 = MMVIII
$2009=$ MMIX
$2010=M M X$
2011 = MMXI
2012 = MMXII
2013 = MMXIII
2014 = MMXIV
2015 = MMXV
$2016=$ MMXVI
2017 = MMXVII
2018 = MMXVIII
2019 = MMXIX
$2020=$ MMXX

### 2.10.0 Converting Speed—Knots to MPH to Kilometers per Hour

| Speed Conversions - Knots, MPH, KPH |  |  |
| :--- | :---: | :---: |
| Knots | Miles per Hour | Kilometers per Hour |
| 1 | 1.152 | 1.85 |
| 2 | 2.303 | 3.70 |
| 3 | 3.445 | 5.55 |
| 4 | 4.606 | 7.41 |
| 5 | 5.758 | 9.26 |


| Speed Conversions - Knots, MPH, KPH-Cont'd |  |  |
| :--- | :---: | :--- |
| Knots | Miles per Hour | Kilometers per Hour |
| 6 | 6.909 | 11.13 |
| 7 | 8.061 | 12.98 |
| 8 | 9.212 | 14.83 |
| 9 | 10.364 | 16.68 |
| 10 | 11.515 | 18.55 |

Source: Glen-L Marine Designs-glen-I.com

### 2.11.0 Conversion Factors for Builders and Design Professionals Who Cook

## Common Kitchen Equivalents

| Standard | Equivalent | Liquid Equivalent |
| :--- | :--- | :--- |
| One pinch or dash | $1 / 16$ teaspoon |  |
| 1 teaspoon | 5 ml | $1 / 6$ ounce |
| 3 teaspoons | 1 tablespoon | $1 / 2$ ounce |
| 4 tablespoons | $1 / 4$ cup | 2 ounces |
| $1 / 3$ cup | 5 tablespoon +1 teaspoon | 3 ounces |
| $1 / 2$ cup | 8 tablespoons | 4 ounces |
| 1 gill | $1 / 2$ cup | 4 ounces |
| 1 cup | 16 tablespoons | 8 ounces |
| 2 cups |  | 1 pint $(16$ ounces $)$ |
| 4 cups |  | 1 quart $(32$ ounces $)$ |
| 2 pints |  | 1 quart $(32$ ounces $)$ |
| 4 quarts | 1 peck | 1 gallon $(128$ ounces) |
| 8 quarts | 1 bushel |  |
| 4 pecks | 1 pound dry measure |  |
| 16 ounces |  |  |

$$
\begin{aligned}
& \mathrm{tsp}=\text { teaspoon } \\
& \mathrm{t}=\text { tablespoon } \\
& \mathrm{oz}=\text { ounce } \\
& \mathrm{c}=\mathrm{cup} \\
& \mathrm{pt}=\text { pint } \\
& \mathrm{qt}=\text { quart } \\
& \mathrm{bu}=\text { bushel } \\
& \mathrm{lb}=\text { pound }
\end{aligned}
$$

Oven Temperatures

| $225^{\circ} \mathrm{F}$ | $110^{\circ} \mathrm{C}$ | Gas Mark 1/4 | Very slow / Very cool |
| :--- | :--- | :--- | :--- |
| 250 | 120 | Gas Mark 1/2 | Very slow / Very cool |
| 275 | 140 | Gas Mark 1 | Slow / Cool |
| 300 | 150 | Gas Mark 2 | Slow / Cool |
| 325 | 160 | Gas Mark 3 | Warm / Very Moderate |
| 350 | 180 | Gas Mark 4 | Moderate |
| 375 | 190 | Gas Mark 5 | Moderately hot / Fairly hot |
| 400 | 200 | Gas Mark 6 | Moderately hot / Fairly hot |
| 425 | 220 | Gas Mark 7 | Hot |
| 450 | 230 | Gas Mark 8 | Very hot |
| 475 | 240 | Gas Mark 9 | Very hot |

- Temperature equivalents are not exact; they are only a guide. Consult your oven's manual for its particular settings.
- Settings for convection ovens should be about $50^{\circ} \mathrm{F}\left(\mathrm{c} .25^{\circ} \mathrm{C}\right)$ less than the above.
- Remember to use an oven thermometer to check the actual temperature of your oven, as they do occasionally need to be recalibrated.
By permission: Fante's Kitchen Wares Shop, Philadelphia, PA-www.fantes.com


## Liquid (Fluid) Measure Equivalents

| 0.125 oz | 1 fl dram | 60 minims |  |  |  |
| :--- | :--- | :--- | :--- | :--- | :--- |
| 1 oz | 8 fl drams |  |  |  |  |
| 4 oz | 1 gill |  |  |  |  |
| 8 oz | 1 c |  |  |  |  |
| 16 oz | 2 c | 1 pt | qt |  |  |
| 32 oz | 4 c | 2 pt | 1.0567 qt | 0.264 gal | 1 l |
| 33.8 oz | 4.23 c | 2.1134 pt | 4 qt | 1 gal | 3.7853 l |
| 128 oz | 16 c | 8 pt | 7.875 qt | 31.5 gal |  |
| 4032 oz | 1 bbl | 3.94 pt | 63 gal |  |  |
| $\mathbf{8 0 6 4} \mathbf{~ o z}$ | 1 hhd | 7.875 pt | 15.75 qt | 6 |  |

$\mathrm{fl}=$ fluid
$\mathrm{oz}=$ ounce
$\mathrm{c}=$ cup
pt $=$ pint
$\mathrm{qt}=$ quart
gal $=$ gallon
bbl $=$ barrel
hhd $=$ hogshead
$1=$ liter

## Metric Equivalents, Liquid or Fluid Measure

|  | Or | Dry | Liquid |
| :--- | :--- | :--- | :--- |
| 1 centiliter |  | 0.6102 cubic inches | 0.338 ounces |
| 1 deciliter | 10 centiliters | 6.102 cubic inches | .0845 gill |
| 1 liter | 10 deciliters | 0.908 quart | 1.0567 quarts |
| 1 decaliter | 10 liters | 9.08 quarts | 2.64 gallons |

$\mathrm{cl}=$ centiliter
$\mathrm{dl}=$ deciliter
$1=$ liter
dal $=$ decaliter
cu in $=$ cubic inch
$\mathrm{oz}=$ ounce
$\mathrm{qt}=$ quart
gal = gallon
By permission: Fante's Kitchen Wares Shop, Philadelphia, PA-www.fantes.com

## Weight Equivalents

Avoirdupois

| 16 drams | 437.5 grains | 1 ounce | 28.35 grams |
| :--- | :--- | :--- | :--- |
| 16 ounces | 7000 grains | 1 pound | 453.59 grams |
| 1 pound | 0.45 kilograms |  |  |
| 1 kilogram | 2.2 pounds |  |  |
| 100 pounds | 1 central | 1 hundredweight |  |
| 2000 pounds | 1 short ton |  |  |
| 2204.6 pounds | 1 metric ton | 1000 kilograms |  |
| 2240 pounds | 1 long ton or gross ton |  |  |

## Also (in Great Britain)

| 14 pounds | 1 stone |  |
| :--- | :--- | :--- |
| 2 stones | 1 quarter |  |
| 4 quarters | 112 pounds | 1 hundredweight |
| 20 hundredweight | 1 long ton |  |

## Troy (Precious Metals)

| 24 grains | 1 pennyweight |  |
| :--- | :--- | :--- |
| 20 pennyweights | 480 grains <br> 12 ounces | 5760 grains |$\quad$| 1 ounce |
| :--- |
| 12 |

## Apothecaries' Weight

| 20 grains | 1 scruple |  |
| :--- | :--- | :--- |
| 3 scruples | 1 dram |  |
| 8 drams | 1 ounce |  |
| 12 ounces | 5760 grains | 1 pound |

## Paper

| 24 sheets | 1 quire |  |
| :--- | :--- | :--- |
| 20 quires | 1 short ream | 480 sheets |
| 500 sheets | 1 ream |  |
| 10 reams | 1 bale |  |

## Weight of Water

| 1 cubic inch | .0360 pound |
| :--- | :--- |
| 1 cubic foot | 62.3 pounds |
| 1 cubic foot | 7.48052 U.S. gallons |
| 1 Imperial gallon | 10.0 pounds |
| 1 U.S. gallon | 8.33 pounds |

By permission: Fante's Kitchen Wares Shop, Philadelphia, PA-www.fantes.com

## Calculations and Formulas-Geometry, Trigonometry, and Physics in Construction

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### 3.0.0 Useful Formulas-Water, Pressure, Heat, Cooling, Horsepower

Total Heat $(\mathrm{BTU} / \mathrm{hr})=4.5 \times \mathrm{cfm} \times \Delta \mathrm{h}(\mathrm{std}$ air $)$
Sensible Heat $(\mathrm{BTU} / \mathrm{hr})=1.1 \times \mathrm{cfm} \times \Delta \mathrm{t}(\mathrm{std}$ air $)$
Latent Heat $(\mathrm{BTU} / \mathrm{hr})=0.69 \times \mathrm{cfm} \times \Delta \mathrm{gr}(\mathrm{std}$ air $)$
NOTE: For conditions other than standard air please see this page.
Total Heat $(\mathrm{BTU} / \mathrm{hr})=500 \times \mathrm{gpm} \times \Delta \mathrm{t}($ water $)$
TONS $=24 \times \mathrm{gpm} \times \Delta \mathrm{t}$ (water)
GPM cooler $=(24 \times$ TONS $) / \Delta t($ water $)$
Fluid Mixture $\mathrm{T}_{\mathrm{m}}=\left(\mathrm{Xt}_{1}+\mathrm{Yt}_{2}\right) / \mathrm{X}+\mathrm{Y}$ (this works for air or water)
$\mathrm{BTU} / \mathrm{hr}=3.413 \times$ watts $=\mathrm{HP} \times 2546=\mathrm{Kg} \mathrm{Cal} \times 3.97$
$\mathrm{Lb} .=453.6$ grams $=7000$ grains
$\mathrm{psi}=\mathrm{ft}$ water $/ 2.31=\mathrm{in} . \mathrm{hg} / 2.03=\mathrm{in}$. water $/ 27.7=0.145 \times \mathrm{kPa}$
Ton $=12,000 \mathrm{BTU} / \mathrm{hr}=0.2843 \times \mathrm{KW}$
$\mathrm{HP}($ air $)=\mathrm{cfm} \times \Delta \mathrm{p}\left(\right.$ in. $\left.\mathrm{H}_{2} \mathrm{O}\right) / 6350 \times$ Eff
$\mathrm{HP}($ water $)=\mathrm{gpm} \times \Delta \mathrm{p}(\mathrm{ft}) / 3960 \times$ Eff
Gal. $=\mathrm{FT}^{3} / 7.48=3.785$ Liters $=8.33 \mathrm{lb}($ water $)=231 \mathrm{in} .{ }^{3}$
$\mathrm{gpm}=15.85 \times \mathrm{L} / \mathrm{S}$
$\mathrm{cfm}=2.119 \times \mathrm{L} / \mathrm{S}$
Liter $=3.785 \times \mathrm{gal}=0.946 \times$ quart $=28.32 \times \mathrm{ft}^{3}$
Therm $=100,000 \mathrm{BTU}=\mathrm{MJ} / 105.5$
Watt/sq ft $=0.0926 \times \mathrm{W} / \mathrm{M}^{2}$
$\mathrm{yd}=1.094 \times \mathrm{M}$
$\mathrm{ft}=3.281 \times \mathrm{M}$
$\mathrm{ft}^{2}=10.76 \times \mathrm{M}^{2}$
$\mathrm{ft}^{3}=35.31 \times \mathrm{M}^{3}$
$\mathrm{ft} / \mathrm{min}=196.9 \times \mathrm{M} / \mathrm{S}$
PPM (by mass) $=\mathrm{mg} / \mathrm{kg}$


NOTE: Liter $/ \mathrm{sec}$ is the proper SI term for liquid flow. $\mathrm{M}^{3} / \mathrm{sec}$ is the proper SI term for airflow. Due to the awkward nature of using $\mathrm{M}^{3} / \mathrm{S}$ at low air flow rates (lots of decimal points), $\mathrm{L} / \mathrm{S}$ is commonly used to express air flow for HVAC applications.

Source: www.gorhamschaffler.com/formulas.html

1. Water Measurement

1 cubic foot $=7.48$ gallons $=62.4$ pounds of water
1 acre-foot $=43,560$ cubic feet $=325,851$ gallons $=12$ acre-inches
1 acre-inch $=27,154$ gallons
1 acre-foot is the volume of water that would cover 1 acre of land 1 foot deep
1 acre-inch per hour $=450$ gallons per minute $(\mathrm{gpm})$
$=1$ cubic foot per second (cfs)
1 cubic meter $=1000$ liters $=264$ gallons
1 gallon $=128$ ounces $=3785$ mililiters
1 ounce $=29.56$ mililiters
1 liter $=1.06$ quarts
2. Pressure

1 pound per square inch $(\mathrm{psi})=2.31$ feet of water $=6.9 \mathrm{kpa}$ (kilopascal)

$$
\begin{aligned}
& =0.0703 \text { kilogram per square centimeter }\left(\mathrm{kg} / \mathrm{cm}^{2}\right) \\
& =0.704 \text { meter of water }
\end{aligned}
$$

A column of water 2.31 feet deep exerts a pressure of 1 psi at the bottom of the column.
Total dynamic head $(\mathrm{TDH})=$ pumping lift + elevation change + friction loss + irrigation system operating pressure
3. Area/Length/Weight/Yield

1 acre $=0.405$ hectare (ha) $=43,560$ feet $^{2}$
1 hectare $=2.47$ acres
1 mile $=5280$ feet $=1.61$ kilometers
1 foot $=0.305$ meter ( m )
1 meter $=3.28$ feet
1 inch $=2.54$ centimeters
1 pound $=454$ grams
1 kilogram per hectare $(\mathrm{kg} / \mathrm{ha})=1$ metric ton/ha $(\mathrm{MT} / \mathrm{ha})$
$=0.0149$ bushel ( 60 pounds) per acre
4. Temperature
${ }^{\circ} \mathrm{F}=1.8\left({ }^{\circ} \mathrm{C}\right)+32 \quad{ }^{\circ} \mathrm{C}=\left({ }^{\circ} \mathrm{F}-32\right) / 1.8$

5. Horsepower

1 horsepower $=0.746$ kilowatts $(\mathrm{kw})=33,000$ foot-pounds per minute
Water horsepower (WHP) is the power required to lift a given quantity of water against a given total dynamic head.

WHP $=(\mathrm{Q} \times \mathrm{H}) \div 3960$, where $\mathrm{Q}=$ flow rate in GPM and $\mathrm{H}=$ total dynamic head in feet
Brake horsepower (BHP) is the required power input to the pump.
$\mathrm{BHP}=\mathrm{WHP} / \mathrm{E}$, where $\mathrm{E}=$ pump efficiency

## Power unit horsepower

Electric power units: approximate name plate horsepower $=\mathrm{BHP} \div 0.9$

Internal combustion units:
Must derate $20 \%$ for continuous duty ( $=80 \%$ efficiency)
$5 \%$ for right-angle drive ( $=95 \%$ efficiency)
$3 \%$ for each 1000 feet above sea level ( $=91 \%$ for 3000 feet $)$
$1 \%$ for each $10^{\circ}$ above $60^{\circ} \mathrm{F}\left(=96 \%\right.$ for $100^{\circ} \mathrm{F}$ )
Approximate engine horsepower required $=\mathrm{BHP} \div$ deratings

$$
=\mathrm{BHP} \div(0.80 \times 0.95 \times 0.91 \times 0.96)
$$

Plastic Pipe Friction Loss (psi loss per 100 feet of pipe) for $C=150$

| Pipe size (inches) |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 10 | 25 | 50 | 75 | 100 | 150 |
|  |  |  | si loss p | ---- |  | ------ |
| $1 \frac{1}{2}$ | 0.26 | 1.40 | 5.50 | - | - | - |
| 2 | 0.09 | 0.52 | 1.90 | 4.10 | - |  |
| $2 \frac{1}{2}$ | 0.03 | 0.17 | 0.65 | 1.35 | 2.40 | 5.00 |
| 3 | - | 0.07 | 0.26 | 0.38 | 0.95 | 2.05 |
| 4 | - | 0.01 | 0.06 | 0.14 | 0.24 | 0.50 |
| Pipe size (inches) | Flow rate (gpm) - |  |  |  |  |  |
|  | 200 | 400 | 600 | 800 | 1000 | 1200 |
|  |  |  | - psi los | feet -- |  | ------ |
| 4 | 0.85 | 3.20 | - | - | - | - |
| 6 | 0.12 | 0.42 | 0.93 | 1.60 | 2.40 | 3.40 |
| 8 | 0.03 | 0.11 | 0.22 | 0.38 | 0.60 | 0.85 |
| 10 | - | 0.04 | 0.08 | 0.16 | 0.19 | 0.28 |
| 12 |  |  | 0.03 | 0.06 | 0.08 | 0.11 |

Economical Pipe Size Selection (flow in gpm)

| Size | Aluminum * | Plastic * |
| :--- | :---: | :---: |
| (inches) | -------------------- gpm ---------------------------------- |  |
| 4 | 200 | 620 |
| 6 | 450 | 1100 |
| 8 | 800 | 1720 |
| 10 | 1250 | 2480 |
| 12 | 1800 |  |

*Aluminum pipe velocity limited to $5 \mathrm{ft} / \mathrm{sec}$. Plastic pipe velocity limited to $7 \mathrm{ft} / \mathrm{sec}$.

## Maximum Economical Pipe-flow Capacities

A rule of thumb for coupled and gated pipe:


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Source: Kansas State University

### 3.1.0 Basic Mathematics—Algebra

Algebra is a type of mathematics that is used to determine unknown variables. This section is not designed to teach Algebra, but to help remind the reader what Algebra looks like (I know for me, the last time I looked at a variety of Algebra problems was well over 10 years ago!).

$$
x+y=z
$$

The above sample is an example of a simple algebra problem. To solve this equation, we must know at least two of the variables:

$$
\begin{aligned}
x & =2 \\
z & =4 \\
2+y & =4 \\
y & =2
\end{aligned}
$$

Proportions are just another version of ratios and are found in the following manner:

$$
\begin{aligned}
\frac{2}{4} & =\frac{x}{7} \\
4 * x & =2 * 7 \\
4 x & =14 \\
x & =3.5
\end{aligned}
$$

A monomial is an expression for a single number variable: $\mathrm{a}=5$ for example. Algebra is used to solve a combination of monomials such as binomials and polynomials. The method for solving a binomial or polynomial equation is called FOIL (First, Outer, Inner, Last).

A binomial:

$$
\begin{aligned}
& (3+7 x)(6+2 x) \\
& \text { F.O.I.L } \\
& 18+6 x+42 x+14 x^{2} \\
& 18+48 x+14 x^{2}
\end{aligned}
$$

A polynomial:

$$
\begin{aligned}
& x^{6}+x^{3} y^{4}+x^{3} y^{4}+y^{8} \\
& x^{6}+2 x^{3} y^{4}+y^{8}
\end{aligned}
$$

Other algebraic techniques include a distribution function. This is used to simplify two or more terms in order to make a problem easier to work:

$$
\begin{gathered}
4(2+3 x) \\
4 * 2+4 * 3 x \\
8+12 x
\end{gathered}
$$

Graphing is also a part of Algebra. Two famous types of equations that are used to plot a graph is the linear equation and the quadratic equation.

A linear equation is used to plot a single line based on a slope:
Following is a graph that plots the following linear equation:

$$
y=2 x+1
$$

Image Credit


The quadratic equation is used to plot curves and parabolas based on more than one variable:


Above is a graph of the following quadratic equation:

$$
y=2 x^{2}+2 x-2
$$

### 3.1.1 What Are Roots?

Roots are numbers that make an expression equal to zero. If it's an equation like $y=m x+b$ or $y=a x^{2}+b x+c$ or any other equation involving $x$ and $y$, the roots are those points where the graph crosses the $x$-axis.

Example 1: $y=2 x-3$. The root of this equation is 1.5 .


Example 2: $\mathrm{y}=2 \mathrm{x}^{2}-\mathrm{x}-15$. The roots of this equation are -2.5 and 3 .
Source: mathwizz.Com

### 3.1.2 Area and Circumference of a Circle by Archimedes

## Goals

1. Understand the relationship among $\Pi$, area, and circumference of a circle.
2. Understand how Archimedes used the unit circle (radius $=1$ ) and the concept of mathematical limits to show that $A=\Pi R^{2}$ and $C=2 \Pi R . b_{n} 2$.

## Basic Principles

1. The area of a polygon with n sides ( n -gon) inscribed in a unit circle approaches the special number pi as n increases.
2. The area of an inscribed polygon approaches the area of a circle as the number of sides on the polygon increases.
3. The perimeter of an inscribed polygon approaches the perimeter (circumference) of a circle as the number of sides on the polygon increases.

## Givens

- $\mathrm{n}=$ number of sides on the inscribed circle
- $1=$ radius of unit circle
- $h_{n}=$ height of an isosceles triangle inscribed in the inner circle
- $b_{n}=$ base of an isosceles triangle inscribed in the inner circle
- $\mathrm{R}=$ radius of the n -gon (Note that this radius is visualized in this applet as being greater than one, but R could be any value greater than zero.)
- $\mathrm{Rh}_{\mathrm{n}}=$ height of each inscribed isosceles triangle (based on $\mathrm{hn}=1$ )
- $\mathrm{Rb}_{\mathrm{n}}=$ base of each inscribed isosceles triangle (based on $\mathrm{hn}=1$ )
- $\mathrm{P}_{\mathrm{n}}=$ the perimeter of the unit circle
- $\mathrm{A}_{\mathrm{n}}=$ area of the unit circle
- $\mathrm{A}=$ the sum of the areas of the inscribed triangles, the limit of which is the area of the circle
- $\mathrm{C}=$ the sum of the "bases" of the inscribed isosceles triangles, the limit of which is the circumference of the circle.


## Knowns and Assumptions

- Area of a triangle $=\frac{(\text { base } * \text { height })}{2}$
- Other basic rules of algebra and geometry


## Method

With the unit circle (red) as the basis, Archimedes used the limiting process on the area and base of polygons ( n -gons) inscribed in circles (as n approaches infinity) to determine $\Pi$ and at the same time verify the formulas for the area and circumference of any circle (blue).

## What to Do

1. Notice how $R, h_{n}$, $b_{n}$, and the unit circle are used in the graphical Definition of Variables.
2. Observe how each of these variables is positioned in the equation in the center bottom of the screen.
3. Follow how the rules of algebra are used to rearrange the variables from the center equation outward to the left and outward to the right. Also, note the use of $\mathrm{P}_{\mathrm{n}}$ in the right-hand limit.

Source: math.psu.edu
4. Increase $n$. Note that as you increase $n$, the values of $n A_{n}$ on the left and the value of $P_{n} h_{n}$ on the right side simultaneously come closer and closer to the limiting value $\Pi$.
5. Notice: The limit at the left is A, the area of the circle. The limit at the right is $\frac{\mathrm{CR}}{2}$, the perimeter (or circumference) of the circle.

### 3.1.3 Explaining Exponential Functions-Concepts, Solutions

## Exponential Functions

Exponential functions are functions where $f(x)=a^{x}+B$ where $a$ is any real constant and $B$ is any expression. For example, $f(x)=\mathrm{e}^{-x}-1$ is an exponential function.

To graph exponential functions, remember that unless they are transformed, the graph will always pass through $(0,1)$ and will approach, but not touch or cross, the $x$-axis. Example:

1. Problem: Graph $f(x)=2^{x}$.

Solution: Plug in numbers for $x$ and find values for $y$, as we have done with the table below.

| $x$ | 0 | 1 | 2 | 3 |
| :---: | :---: | :---: | :---: | :---: |
| y | 1 | 2 | 4 | 8 |

Now plot the points and draw the graph (shown below).


Source: Thinkquest.org

### 3.2.0 General Geometric Formulas

## General Formulas

Here are some common simple geometric formulas useful in estimating sizes, quantities, and amounts:

Volume of a cube in cubic yards
cu yd's $=\left(\right.$ length $^{\prime} \times$ width $^{\prime} \times$ height' $\left.^{\prime}\right) / 27$
Why 27? Because $3^{\prime} \times 3^{\prime} \times 3^{\prime}=27 \mathrm{cu} \mathrm{ft}=1 \mathrm{cu} y \mathrm{~d}$

## Area of a parallelogram


area $=\mathrm{a} \times \mathrm{b}$

## Area of a trapezoid



$$
\text { area }=((\mathrm{a}+\mathrm{b}) / 2) \times \mathrm{c}
$$

## Hypotenuse of a right triangle


hypotenuse $(c)=\sqrt{\mathrm{a}^{2}+\mathrm{b}^{2}}$

## Area of any triangle


area $=(\mathrm{a} \times \mathrm{b}) / 2$

## pi

What exactly is pi?
Why is it 3.1416 ?
If you take any circle, measure its circumference and its diameter, and divide the circle's circumference by its diameter, the answer is always 3.1416 . This ratio of the circumference to the diameter is always the same, regardless of the size of the circle or its units of measure.

## Volume of a cylinder


volume $=(p i / 4) \times \mathrm{a}^{2} \times \mathrm{b}$ and volume $=\mathrm{pi} \times \mathrm{a} / 2 \times \mathrm{a} / 2 \times \mathrm{b}$

## Volume of a frustum of a pyramid


volume $=(e / 3) \times((a \times b)+(c \times d)+\sqrt{(a \times b) \times(c \times d)})$

## Volume of a cone


volume $=(\mathrm{pi} / 3) \times \mathrm{b}^{2} \times \mathrm{c}$

Volume of a frustum of a cone

volume $=(p i / 12) \times c \times\left(\mathrm{a}^{2}+(\mathrm{a} \times \mathrm{b})+\mathrm{b}^{2}\right)$

## Frustum's surface area



Angle of the cone
Frustum's altitude
Triangle's base
Length of Frustum's side
Frustum's surface area
$F^{\circ}$
$\mathrm{e}=$ tangent $\mathrm{F}^{\circ} \times \mathrm{c}$
$c=(b-a) / 2$
$d=\sqrt{c^{2}+e^{2}}$
area $=(\pi / 2) \times d \times(a+b)$

Tangent Examples:
tangent $30^{\circ}=0.57735$
tangent $35^{\circ}=0.70021$

```
tangent \(40^{\circ}=0.83910\)
tangent \(45^{\circ}=1.00000\)
tangent \(50^{\circ}=1.19175\)
tangent \(55^{\circ}=1.42815\)
tangent \(60^{\circ}=1.73205\)
tangent \(65^{\circ}=2.14451\)
tangent \(70^{\circ}=2.74748\)
```


## Volume of a sphere


volume $=(\mathrm{pi} / 6) \times \mathrm{a}^{3}$

## Area of a circle



A circle's area can be determined by using either the radius or the diameter

```
area = pi a 
area = 0.785 b
```


## Circumference of a circle


circumference $=$ pi $b$

Volume of a pyramid

volume $=(\mathrm{a} \times \mathrm{b} \times \mathrm{c}) / 3$

### 3.2.1 Volume of Prisms

## Right Prism Volume Postulate

The volume $V$ of any right prism is the product of $B$, the area of the base, and the height $h$ of the prism.
Formula: $V=B h$
$B=1 w$

(The base's formula could change depending on the base's shape.)

### 3.2.2 Volume of Pyramids

## Pyramids

A pyramid is a polyhedron with a single base and lateral faces that are all triangular. All lateral edges of a pyramid meet at a single point, or vertex.

## Pyramid Volume Theorem

The volume $V$ of any pyramid with height $h$ and a base with area $B$ is equal to one-third the product of the height and the area of the base.

Formula: $\mathrm{V}=(1 / 3) \mathrm{Bh}$

### 3.2.3 Area of Prisms and Right Area Prisms

There are special formulas that deal with prisms, but they only deal with right prisms. Right prisms are prisms that have two special characteristics-all lateral edges are perpendicular to the bases, and lateral faces are rectangular. The figure below depicts a right prism.


## Right Prism Area

The lateral area $L$ (area of the vertical sides only) of any right prism is equal to the perimeter of the base times the height of the prism $(L=P h)$.

The total area $T$ of any right prism is equal to two times the area of the base plus the lateral area.

Formula: $\mathrm{T}=2 \mathrm{~B}+\mathrm{Ph}$
$B=1 w$
$P=2 l+2 w$

(The base's formula could change depending on the base's shape.)
(The perimeter's formula could change depending on the base's shape.)

### 3.2.4 Cylinder Volume Theorem

## Cylinders

## Cylinder Volume Theorem

The volume $V$ of any cylinder with radius $r$ and height $h$ is equal to the product of the area of a base and the height.

Formula: $\mathrm{V}=(\mathrm{PI}) \mathrm{r}^{2} \mathrm{~h}$


## Cylinder Area Theorem

For any right circular cylinder with radius $r$ and height $h$, the total area $T$ is two times the area of the base plus the lateral area (2(PI)rh).

Formula: $\mathrm{T}=2(\mathrm{PI}) \mathrm{rh}+2(\mathrm{PI}) \mathrm{r}^{2}$

### 3.2.5 Cone Volume Theorem

## Cones

## Cone Volume Theorem

The volume $V$ of any cone with radius $r$ and height $h$ is equal to one-third the product of the height and the area of the base.

Formula: $\left.\mathrm{V}=(1 / 3)(\mathrm{PI}) \mathrm{r}^{2} \mathrm{~h}\right]$


## Cone Area Theorem

The total area $T$ of a cone with radius $r$ and slant height $l$ is equal to the area of the base plus PI times the product of the radius and the slant height.

Formula: $\mathrm{T}=(\mathrm{PI}) \mathrm{rl}+(\mathrm{PI}) \mathrm{r}^{2}$

### 3.2.6 Sphere Volume and Area Theorem

## Spheres

## Sphere Volume and Area Theorem

The volume $V$ for any sphere with radius $r$ is equal to four-thirds times the product of PI and the cube of the radius. The area $A$ of any sphere with radius $r$ is equal to $4(\mathrm{PI})$ times the square of the radius.

Volume Formula: $V=(4 / 3)(P I) r^{3}$
Area Formula: $A=4(P I) r^{2}$


### 3.2.7 The Isosceles and Equilateral Triangles

An isosceles triangle has two congruent sides called legs and a third side called the base. The vertex angle is the angle included by the legs. The other two angles are called base angles. The base angles are congruent. The figure below depicts an isosceles triangle with all the parts labeled.


Isosceles Triangle
An equilateral triangle is a special isosceles triangle in which all three sides are congruent. Equilateral triangles are also equiangular, which means all three angles are congruent. The measure of each angle is 60 degrees. The figure below depicts an equilateral triangle with all the parts labeled.


Equilateral Triangle

### 3.2.8 Theorems That Apply to Parallelograms

A parallelogram is so named because it has two pairs of opposite sides that are parallel.
Four theorems apply to parallelograms only. They are as follows.

1. A diagonal of any parallelogram forms two congruent triangles. Example:

Problem: Prove triangle $A B C$ is congruent to triangle $C D A$.


Solution: Since the figure is a parallelogram, segment $A B$ is parallel to segment $D C$ and the two segments are also congruent.
Angle 2 is congruent to angle 4 and angle 1 is congruent to angle 3 . This is true because alternate interior angles are congruent when parallel lines are cut by a transversal.
Segment $A C$ is congruent to segment $C A$ by the Reflexive Property of Congruence, which says any figure is congruent to itself.
Triangle $A B C$ is congruent triangle CDA by Angle-Side-Angle.
2. Both pairs of opposite sides of a parallelogram are congruent.
3. Both pairs of opposite angles of a parallelogram are congruent.
4. The diagonals of any parallelogram bisect each other. Example:

Problem: Prove segment $A E$ is congruent to segment $C E$ and segment $D E$ is congruent to segment $B E$.


Solution: By the definition of a parallelogram, segment $A D$ and segment $B C$ are parallel and congruent. Angle 1 is congruent to angle 3, and angle 2 is congruent to angle 4. This is true because alternate interior angles are congruent when parallel lines are cut by a transversal.
Triangle $A E D$ and triangle $C E B$ are congruent by Angle-Side-Angle.
The segments we were asked to prove as congruent are congruent by CPCTC.
The three theorems that tell us how to find a parallelogram are as follows.

1. If both pairs of opposite sides of a quadrilateral are congruent, the quadrilateral is a parallelogram.
2. If the diagonals of a quadrilateral bisect each other, then the quadrilateral is a parallelogram.
3. If one pair of opposite sides of a quadrilateral is both parallel and congruent, then the quadrilateral is a parallelogram.

### 3.2.9 The Small Angle Formula

There is a very powerful formula relating the size of an object to its distance and its angular size. This formula, the small angle formula, comes from considering a circle of radius $\mathbf{r}$. Remember that the circumference $\mathbf{c}$ is the distance all the way around the circle and $\mathrm{c}=2 \pi \mathbf{r}$. What if we are not interested in the distance all the way around the circle, but instead want to know the distance around part of the circle, say the length of the arc marked s?


For this we can set up a ratio:

$$
\frac{\text { length of } \mathrm{s}}{\text { distance around whole circle }}=\frac{\text { angle subtended by s }}{\text { angle around whole circle }}
$$

so that

$$
\frac{\mathrm{s}}{2 \pi \mathrm{r}}=\frac{\theta}{360^{\circ}}
$$

This is the small angle formula.
Why is this formula so great? Because it can even be used for things that are not part of a circle, as long as the angle is small! For example, when the angle is small (say less than 25), the triangle below looks an awful lot like the wedge from the circle above.


Now we have a very powerful tool indeed because we can turn a lot of astronomy problems into pictures involving skinny triangles-as you will see as you read on!

## Angular Sizes

In astronomy, we study the universe while sitting comfortably here on good ol' Terra Firma. This means that we cannot generally measure the sizes of objects using rulers. Let's face it, even if we were to visit Jupiter, it would be awfully hard to find a ruler big enough to measure it. . .

So from our earthly vantage point, we often describe the size of an object using an angular measure rather than a linear (ruler-like) one. If we are lucky enough to know something about an object's distance, then we can relate its angular size to its linear size using the small angle formula. This method is very frequently used to measure things in astronomy.

As an example, imagine that you are looking at the Green Hall Tower from a distance of 200 meters. You estimate from your point of view that the Tower covers an angle of $10^{\circ}$. We can draw the following picture:


Note that this looks very similar to the skinny triangle picture above. In fact, we can apply the small angle formula to the triangle originating at your eyeball to get the height of the Tower:

$$
\begin{aligned}
\text { size } & =\frac{\theta}{360^{\circ}} \times 2 \pi \times \text { distance } \\
& =\frac{\theta}{57.3^{\circ}} \times \text { distance } \\
& =\frac{10^{\circ}}{57.3^{\circ}} \times 200 \mathrm{~m}=35 \mathrm{~m}
\end{aligned}
$$

So we were able to measure the height of the Tower without actually going there!

The angle that an object covers when we trace it back to your eyeball is called its angular size. Consider the following pictures.


The top drawing demonstrates that two objects having different linear sizes can have the same angular size () if they are viewed from different distances. The object's angular size is determined by the ratio size/distance. The quarter's linear size $(2.5 \mathrm{~cm})$ is 1.4 times as big as the dime's $(1.8 \mathrm{~cm})$ and so must be placed 1.4 times farther away to subtend the same angle. Now move the quarter two times closer as in the lower drawing, and its angular size is twice as big ( 20 degrees instead of 10 degrees).

You can practice measuring the angular sizes of things (trees, constellations, friends) using various body parts! As shown in the picture below, the angular size of your fist when you put your arm straight out in front of you is approximately 10 degrees. Also with a straight arm, your pinky fingernail subtends about 1 degree.


## Summary



$$
\text { size }=\frac{\theta}{57.3^{\circ}} \times \text { distance }
$$

Note that you have to express the angle in degrees in order to get the units to work out properly. In astronomy we are often working with very small angles, measured in arcseconds. We can change the degrees in this equation to arcseconds using our units conversion methods:

$$
\text { size }=\frac{\theta}{57.3^{\circ}} \times \text { distance } \times \frac{1^{\circ}}{60^{\prime}} \times \frac{1^{\circ}}{60^{\prime \prime}}=\frac{\theta}{206265^{\prime \prime}} \times \text { distance }
$$

You will often see the small angle formula written in the textbooks in this form.

### 3.2.10 Geometric Surface Area Formulas for Cubes, Spheres, Cones

## Geometric Surface Area Formulas-Cube, Sphere, Cone

## Surface Area of a Cube

$\mathrm{SA}=6 \times^{\wedge} 2$
where: SA is the surface area and $\times$ is the side of the cube

## Surface Area of a Cuboid

$$
\mathrm{SA}=2(\mathrm{xy}+\mathrm{yz}+\mathrm{zx})
$$

where: $\mathrm{x}, \mathrm{y}$, and z are the adjacent three sides of the cuboid

## Surface Area of a Sphere

$\mathrm{SA}=4 \mathrm{pi} \mathrm{r}^{2}$
where: r is the radius of the sphere and $\mathrm{pi}=3.14$

## Surface Area of a Cone

SA $=\mathrm{pi} \times \mathrm{r} \times \mathrm{L}$
where: $r$ is the radius of the cone and $L$ is the slant height of the cone and $\mathrm{pi}=3.14$

## Total Surface Area of a Cone (including the area of the base)

$$
\mathrm{SA}=(\mathrm{pi} \times \mathrm{r} \times \mathrm{L})+\mathrm{pi} \times \mathrm{r}^{2}
$$

where: $r$ is the radius of the cone and L is the slant height of the cone and $\mathrm{pi}=3.14$

### 3.2.11 Angles of an $\mathbf{N}$-gon

Although you won't encounter many odd shapes, such as shapes with $t 12$ sides, it can happen. On most instances of this, you will need to find the sum of the measures of the angles. There is a special theorem that says, if $n$ is the number of sides of any polygon, the sum $(S)$ of the measure of its angles is given by the formula

$$
S=(n-2) 180^{\circ}
$$

The figure and table below will help this theorem make more sense.

## Hexagon


(3 diagonals from one vertex.)
(4 triangles formed.)

| Polygon | No. Sides | Total No. of <br> Diagonals fr. 1 vertex | No. Triangles <br> Formed | Sum of Angle <br> Measures |
| :--- | :--- | :--- | :--- | :--- |
| Triangle | 3 | 0 | 1 | $180^{\circ}$ |
| Quad. | 4 | 1 | 2 | $360^{\circ}$ |
| Pentagon | 5 | 2 | 3 | $540^{\circ}$ |
| Hexagon | 6 | 3 | 4 | $720^{\circ}$ |
| $\cdot$ | $\cdot$ | $\cdot$ | $\cdot$ | $\cdot$ |
| $\cdot$ | $\cdot$ | $n-3$ | $\cdot$ | $\cdot$ |
| $\cdot$ | $n$ |  | $n-2$ | $(\mathrm{n}-2)\left(180^{\circ}\right)$ |

### 3.3.0 Basic Trigonometric Functions

## Basic Trigonometric Functions:

Trigonometry is a specialty of Geometry that focuses mostly on angles. If we want the sum of all internal angles or if we want a ratio of angles for example, we will turn to Trigonometry.

Two recurrent terms to know are the Sine and Cosine

- Sine $=$ ratio of the height to the hypotenuse (see image below)
- Cosine = ratio of the base of the hypotenuse (see image below)

A simple example: A straight line has an angle-its 0 degrees.
Sine $=0^{\circ}$
Cosine $=1^{\circ}$
To introduce the working formulas for sine and cosine, we will use a right triangle.


Pythagorean Theorem:

$$
A^{2}+B^{2}=C^{2}
$$

Source: astronomyonline.org

### 3.3.1 Trigonometry's Sine and Cosine

A right-angled triangle has a hypotenuse, base, and perpendicular side. Let angle A be the angle between the base and the hypotenuse.

Then
Sin A = perpendicular / hypotenuse
$\operatorname{Cos} \mathrm{A}=$ base $/$ hypotenuse
$\operatorname{Tan} \mathrm{A}=(\operatorname{Sin} \mathrm{A}) /(\operatorname{Cos} \mathrm{A})=$ perpendicular $/$ base
$\operatorname{Cot} \mathrm{A}=1 / \operatorname{Tan} \mathrm{A}=(\operatorname{Cos} \mathrm{A}) /(\operatorname{Sin} \mathrm{A})$
$\operatorname{Sec} \mathrm{A}=1 / \operatorname{Cos} \mathrm{A}$
$\operatorname{Cosec} \mathrm{A}=1 / \operatorname{Sin} \mathrm{A}$
Certain basic trigonometric identities applicable to any angle are:
$\operatorname{Sin}(-\mathrm{A})=-\operatorname{Sin} \mathrm{A}$
$\operatorname{Cos}(-\mathrm{A})=\operatorname{Cos} \mathrm{A}$
$\operatorname{Tan}(-\mathrm{A})=-\operatorname{Tan} \mathrm{A}$
$\operatorname{Sin}^{2} \mathrm{~A}+\operatorname{Cos}^{2} \mathrm{~A}=1$
$1+\operatorname{Tan}^{2} \mathrm{~A}=\operatorname{Sec}^{2} \mathrm{~A}$
$1+\operatorname{Cot}^{2} \mathrm{~A}=\operatorname{Cosec}^{2} \mathrm{~A}$
Source:Tutor4Physics.com

### 3.3.2 Trigonometric Ratios

## Ratios

The trigonometric ratios, sine, cosine, and tangent are based on properties of right triangles. The function values depend on the measure of the angle. The functions are outlined below.
sine $x=($ side opposite $x) /$ hypotenuse
cosine $x=($ side adjacent $x$ )/hypotenuse
tangent $x=($ side opposite $x) /($ side adjacent $x)$
In the figure below, $\sin A=\mathrm{a} / \mathrm{c}, \operatorname{cosine} A=\mathrm{b} / \mathrm{c}$, and tangent $A=\mathrm{a} / \mathrm{b}$.


There are two special triangles you need to know, 45-45-90 and 30-60-90 triangles. They are depicted in the figures below.


### 3.3.3 Basic Trigonometry Formulas



Law of Sines

$$
\frac{\sin \alpha}{a}=\frac{\sin \beta}{b}=\frac{\sin \gamma}{c}
$$

Law of Cosines

$$
\begin{aligned}
& a^{2}=b^{2}+c^{2}-2 b c \cos \alpha \\
& b^{2}=a^{2}+c^{2}-2 a c \cos \beta \\
& c^{2}=a^{2}+b^{2}-2 a b \cos \gamma
\end{aligned}
$$

## Law of Tangents

$$
\begin{aligned}
& \frac{a+b}{a-b}=\frac{\tan \frac{1}{2}(\alpha+\beta)}{\tan \frac{1}{2}(\alpha-\beta)} \\
& \frac{a+c}{a-c}=\frac{\tan \frac{1}{2}(\alpha+\gamma)}{\tan \frac{1}{2}(\alpha-\gamma)} \\
& \frac{b+c}{b-c}=\frac{\tan \frac{1}{2}(\beta+\gamma)}{\tan \frac{1}{2}(\beta-\gamma)}
\end{aligned}
$$

## Mollweide's Formulas

$$
\begin{aligned}
& \frac{b-c}{a}=\frac{\sin \frac{1}{2}(\beta-\gamma)}{\cos \frac{1}{2} \alpha} \\
& \frac{c-a}{b}=\frac{\sin \frac{1}{2}(\gamma-\alpha)}{\cos \frac{1}{2} \beta} \\
& \frac{a-b}{c}=\frac{\sin \frac{1}{2}(\alpha-\beta)}{\cos \frac{1}{2} \gamma}
\end{aligned}
$$

## Newton's Formulas

$$
\begin{aligned}
& \frac{b+c}{a}=\frac{\cos \frac{1}{2}(\beta-\gamma)}{\sin \frac{1}{2} \alpha} \\
& \frac{c+a}{b}=\frac{\cos \frac{1}{2}(\gamma-\alpha)}{\sin \frac{1}{2} \beta} \\
& \frac{a+b}{c}=\frac{\cos \frac{1}{2}(\alpha-\beta)}{\sin \frac{1}{2} \gamma}
\end{aligned}
$$



$$
\text { Semiperimeter }=s=\frac{1}{2}(a+b+c)
$$

## Heron's Formula

$$
\text { Area }=\sqrt{s(s-a)(s-b)(s-c)}
$$

## Other Area Formulas

$$
\begin{aligned}
\text { Area } & =\frac{b c \sin \alpha}{2}=\frac{a c \sin \beta}{2}=\frac{a b \sin \gamma}{2} \\
\text { Area } & =\frac{c^{2} \sin \alpha \sin \beta}{2 \sin \gamma} \\
& =\frac{b^{2} \sin \alpha \sin \gamma}{2 \sin \beta} \\
& =\frac{a^{2} \sin \beta \sin \gamma}{2 \sin \alpha}
\end{aligned}
$$

## Triangle Sides

$$
\begin{aligned}
& a=b \cos \gamma+\mathrm{c} \cos \beta \\
& b=c \cos \beta+\mathrm{a} \cos \alpha \\
& c=a \cos \alpha+\mathrm{b} \cos \gamma
\end{aligned}
$$

## Radius of Inscribed Circle

$$
r=\sqrt{\frac{(s-a)(s-b)(s-c)}{s}}=\frac{\text { Area }}{s}
$$

## Radius of Circumscribed Circle

$$
R=\frac{a}{2 \sin \alpha}=\frac{b}{2 \sin \beta}=\frac{c}{2 \sin \gamma}=\frac{a b c}{4(\text { Area })}
$$

## Angles

$$
\begin{aligned}
& \sin \alpha=\frac{2}{b c}=(\text { Area }) \\
& \cos \alpha=\frac{c^{2}+b^{2}-a^{2}}{2 b c} \\
& \sin \frac{\alpha}{2}=\sqrt{\frac{(s-b)(s-c)}{b c}} \\
& \cos \frac{\alpha}{2}=\sqrt{\frac{s(s-a)}{b c}}
\end{aligned}
$$

Analogous formulas hold for other angles.

## Right Triangle Definitions



$$
\begin{aligned}
& \sin \theta=\frac{\text { opp }}{\text { hyp }} \quad \cos \theta=\frac{\text { adj }}{\text { hyp }} \quad \tan \theta=\frac{\text { opp }}{\text { adj }} \\
& \csc \theta=\frac{\text { hyp }}{\text { opp }} \quad \sec \theta=\frac{\text { hyp }}{\text { adj }} \quad \cot \theta=\frac{\text { adj }}{\text { opp }}
\end{aligned}
$$

## Basic Identities

$$
\begin{array}{lll}
\sin x=\frac{1}{\csc x} & \cos x=\frac{1}{\sec x} & \tan x=\frac{1}{\cot x} \\
\csc x=\frac{1}{\sin x} & \sec x=\frac{1}{\cos x} & \cot x=\frac{1}{\tan x} \\
\tan x=\frac{\sin x}{\cos x} & \cot x=\frac{\cos x}{\sin x} &
\end{array}
$$

## Pythagorean Identities

$$
\begin{aligned}
& \sin ^{2} x+\cos ^{2} x=1 \\
& \tan ^{2} x+1=\sec ^{2} x \\
& 1+\cot ^{2} x=\csc ^{2} x
\end{aligned}
$$

## Symmetry Properties

$$
\left.\begin{array}{ll}
\cos (-x) & =\cos x
\end{array} \quad \sec (-x)=\sec x\right] \text { (-x)}=-\sin x \quad \csc (-x)=-\csc x .
$$

## Sum and Difference Formulas

$$
\begin{aligned}
& \sin (x \pm y)=\sin x \cos y \pm \cos x \sin y \\
& \cos (x \pm y)=\cos x \cos y \mp \sin x \sin y \\
& \tan (x \pm y)=\frac{\tan x \pm \tan y}{1 \mp \tan x \tan y}
\end{aligned}
$$



## Double Angle Formulas

$$
\begin{aligned}
\sin 2 x & =2 \sin x \cos x \\
\cos 2 x & =\cos ^{2} x-\sin ^{2} x=2 \cos ^{2} x-1=1-2 \sin ^{2} x \\
\tan 2 x & =\frac{2 \tan x}{1-\tan ^{2} x}
\end{aligned}
$$

Power-Reducing Formulas

$$
\begin{aligned}
& \sin ^{2} x=\frac{1-\cos 2 x}{2} \\
& \cos ^{2} x=\frac{1+\cos 2 x}{2} \\
& \tan ^{2} x=\frac{1-\cos 2 x}{1+\cos 2 x}
\end{aligned}
$$

## Product-to-Sum Formulas

$$
\begin{aligned}
& \sin x \sin y=\frac{1}{2}[\cos (x-y)-\cos (x+y)] \\
& \sin x \cos y=\frac{1}{2}[\sin (x+y)+\sin (x-y)] \\
& \cos x \cos y=\frac{1}{2}[\cos (x-y)+\cos (x+y)]
\end{aligned}
$$

## Sum-to-Product Formulas

$$
\begin{aligned}
& \sin x+\sin y=2 \sin \left(\frac{\mathrm{x}+\mathrm{y}}{2}\right) \cos \left(\frac{\mathrm{x}-\mathrm{y}}{2}\right) \\
& \sin x-\sin y=2 \cos \left(\frac{\mathrm{x}+\mathrm{y}}{2}\right) \sin \left(\frac{\mathrm{x}-\mathrm{y}}{2}\right) \\
& \cos x+\cos y=2 \cos \left(\frac{\mathrm{x}+\mathrm{y}}{2}\right) \cos \left(\frac{\mathrm{x}-\mathrm{y}}{2}\right) \\
& \cos x-\cos y=-2 \sin \left(\frac{\mathrm{x}+\mathrm{y}}{2}\right) \sin \left(\frac{\mathrm{x}-\mathrm{y}}{2}\right)
\end{aligned}
$$

## Pascal's Triangle

|  | 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  | 1 |  | 2 | 2 |  | 1 |  |  |  |  |  |
|  |  |  | 1 |  | 3 |  |  | 3 |  | 1 |  |  |  |  |
|  |  | 1 |  | 4 |  | 6 | 6 |  | 4 |  | 1 |  |  |  |
|  |  | 1 | 5 |  | 10 |  |  | 10 |  | 5 |  | 1 |  |  |
|  | 1 | 6 |  | 15 |  | 20 | 0 |  | 15 |  | 6 |  | 1 |  |
|  |  | 7 | 21 |  | 35 |  |  | 35 |  | 21 |  | 7 |  |  |
| 1 | 8 | 28 |  | 56 |  | 70 | 0 |  | 56 |  | 28 |  | 8 |  |

The coefficients of $(x+y)^{n}$ can be obtained from the $(n+1)^{\text {st }}$ row of Pascal's Triangle.
For example:

$$
\begin{aligned}
(x+y)^{4} & =x^{4}+4 x^{3} y+6 x^{2} y^{2}+4 x y^{3}+y^{4} \\
(x-y)^{3} & =x^{3}-3 x^{2} y+3 x y^{2}-y^{3}
\end{aligned}
$$

## Quadratic Formula

$$
a x^{2}+b x+c=0 \Longleftrightarrow x=\frac{-b \pm \sqrt{b^{2}-4 a c}}{2 a}
$$

## Completing the Square

$$
a x^{2}+b x+c=a\left(x+\frac{b}{2 a}\right)^{2}+\left(c-\frac{b^{2}}{4 a}\right)
$$

## Special Factors

$$
\begin{aligned}
& A^{2}-B^{2}=(A-B)(A+B) \\
& A^{3}-B^{3}=(A-B)\left(A^{2}+A B+B^{2}\right) \\
& A^{3}+B^{3}=(A+B)\left(A^{2}-A B+B^{2}\right) \\
& A^{4}-B^{4}=(A+B)(A-B)\left(A^{2}+B^{2}\right)
\end{aligned}
$$

## Logarithms

$$
\begin{aligned}
\log _{a} 1 & =0 \\
\log _{a} a^{x} & =x \\
\log _{a} x y & =\log _{a} x+\log _{a} y \\
\log _{a} \frac{x}{y} & =\log _{a} x-\log _{a} y \\
\log _{a} x^{y} & =y \log _{a} x \\
\log _{a} x & =\frac{\log _{b} x}{\log _{b} a}
\end{aligned}
$$

## Absolute Value

$$
\begin{aligned}
& |x|= \begin{cases}x & \text { if } x \geq 0 \\
-x & \text { if } x<0\end{cases} \\
& |-x|=|x| \\
& |x y|=|x| \cdot|y| \\
& \left|\frac{x}{y}\right|=\left|\frac{x}{y}\right| \\
& |x|^{2}=x^{2} \\
& \sqrt{x^{2}}=|x| \\
& |x|=p \quad \Longleftrightarrow \quad x=-p \text { or } x=p \\
& |x|<p \quad \Longleftrightarrow \quad-p<x<p \\
& |x|>p \quad \Longleftrightarrow \quad x<-p \text { or } x>p
\end{aligned}
$$

## Triangle Inequality

$$
|x+y| \leq|x|+|y|
$$

## Lines

Slope: $m=\frac{y_{2}-y_{1}}{x_{2}-x_{1}}$
Point-Slope Form: $y-y_{1}=m\left(x-x_{1}\right)$
Slope-Intercept Form: $y=m x+b$
Standard Form: $A x+B y=C$
Vertical Lines: $x=a$
Horizontal Lines: $y=b$

## Distance Formula

$$
D=\sqrt{\left(x_{2}-x_{1}\right)^{2}+\left(y_{2}-y_{1}\right)^{2}}
$$

## Midpoint Formula

$$
\left(\frac{x_{1}+x_{2}}{2}, \frac{y_{1}+y_{2}}{2}\right)
$$

## Inverse Functions

$$
y=f(x) \Longleftrightarrow x=f^{-1}(y)
$$

## Compound Interest

$n$ times per year: $A=P\left(1+\frac{i}{n}\right)^{n t}$
Continuously: $A=P e^{i t}$

### 3.3.4 What Is the Pythagorean Theorem?

## What Is the Pythagorean Theorem?

The Pythagorean theorem states that if you have a right-angle triangle, the square of the hypotenuse (that's the side opposite the right angle) equals the sum of the squares of the other two sides.


Using this triangle as an example, the hypotenuse is side c . We have $\mathrm{a}=3$, and $\mathrm{b}=4$. So:

$$
\begin{aligned}
& \mathrm{c}^{2}=3^{2}+4^{2} \\
& \mathrm{c}^{2}=9+16 \\
& \mathrm{c}^{2}=25
\end{aligned}
$$

To find the length of c , all we do is take the square root of both sides. In this case, $\mathrm{c}=5$.
Source: mathwizz.com

### 3.3.5 Pythagorean and Quotient Identities

There are two quotient identities. They tell us that the tangent and cotangent functions can be expressed in terms of the sine and cosine functions. They are as follows.

$$
\begin{aligned}
& \tan x=\frac{\sin x}{\cos x}, \cos x<>0 \\
& \cot x=\frac{\cos x}{\sin x}, \sin x<>0
\end{aligned}
$$

Three other identities are very important. They are called the Pythagorean Identities. They will come in handy later on when you need to prove more complicated trigonometric identities equal. The Pythagorean Identities are:

$$
\begin{aligned}
& \sin ^{2} x+\cos ^{2} x=1 \\
& 1+\cot ^{2} x=\csc ^{2} x \\
& 1+\tan ^{2} x=\sec ^{2} x
\end{aligned}
$$

Remember that $\sin ^{2} x=(\sin x)^{2}$.

## Special Rules

There are a few special rules you ought to remember when dealing with isosceles and/or equilateral triangles, notably:

1. If a triangle is equilateral, it is equiangular.
2. If two angles of a triangle are congruent, they are the base angles of an isosceles triangle.
3. If a triangle is equiangular, it is equilateral.

### 3.3.6 Properties of Triangles

## Pythagorus's Theorem

$$
\mathrm{a}^{2}+\mathrm{b}^{2}=\mathrm{c}^{2}
$$

where:
c is the hypotenuse of a right angle triangle, and a and b are two sides containing the right angle.

## Cosine Law

$$
c^{2}=a^{2}+b^{2}-2 a b(\operatorname{Cos} C)
$$

This formula is applicable to any triangle. Here $\mathrm{a}, \mathrm{b}$, and c are the three sides of the triangle, and $\mathrm{A}, \mathrm{B}$, and C are the angle opposite to these sides, respectively.

Similarly

$$
\mathrm{a}^{2}=\mathrm{b}^{2}+\mathrm{c}^{2}-2 \mathrm{bc}(\operatorname{Cos} \mathrm{~A})
$$

and

$$
\mathrm{b}^{2}=\mathrm{c}^{2}+\mathrm{a}^{2}-2 \mathrm{ca}(\operatorname{Cos} \mathrm{~B})
$$

Note: This is a general law, and if you make any of the angles equal to 90 degrees it gives the Pythagorean theorem as Cos of 90 degrees $=0$.

## Sine Law

$$
(\operatorname{Sin} \mathrm{A}) / \mathrm{a}=(\operatorname{Sin} \mathrm{B}) / \mathrm{b}=(\operatorname{Sin} \mathrm{C}) / \mathrm{c}
$$

This formula is applicable to any triangle. Here a, b and c are the three sides of the triangle, and A, B and C are the angle opposite to these sides, respectively.

### 3.3.7 Law of Sines



Given a triangle with sides A, B, and C and opposite angles a, b, and c, the Law of Sines states

$$
\frac{\sin a}{A}=\frac{\sin b}{B}=\frac{\sin \mathrm{c}}{C}
$$

Of course, since we can cross-multiply, we can flip the fractions to get

$$
\frac{A}{\sin a}=\frac{B}{\sin b}=\frac{C}{\sin c}
$$

Source: mathwizz.com

### 3.3.8 Law of Cosines



Given a triangle with sides $\mathrm{a}, \mathrm{b}$, and c and angle X , the Law of Cosines states

$$
\mathrm{c}^{2}=\mathrm{a}^{2}+\mathrm{b}^{2}-(2)(\mathrm{a})(\mathrm{b})(\cos \mathrm{X})
$$

Of course, the Pythagorean theorem is a special case of the Law of Cosines because we have a right angle triangle and in this case angle $\mathrm{X}=90^{\circ}$, and we know that $\cos 90^{\circ}=0$ and so we are left with $\mathrm{c}^{2}=\mathrm{a}^{2}+\mathrm{b}^{2}$.

Source: mathwizz.com

### 3.3.9 Reciprocal Ratios

The reciprocal ratios are trigonometric ratios, too. They are as follows:
cotangent $x=1 / \tan x=($ adjacent side $) /($ opposite side $)$
secant $x=1 / \cos x=$ (hypotenuse)/(adjacent side)
cosecant $x=1 / \sin x=$ (hypotenuse)/(opposite side)
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### 3.3.10 Cofunctions

In a right triangle, the two acute angles are complementary. Thus, if one acute angle of a right triangle is $x$, the other is $90^{\circ}-x$. Therefore, if $\sin x=(a / c)$ then $\cos \left(90^{\circ}-x\right)=(a / c)$. A table of all the cofunctions is displayed below.

$$
\begin{aligned}
& \sin x=\cos \left(90^{\circ}-x\right) \\
& \tan x=\cot \left(90^{\circ}-x\right) \\
& \sec x=\csc \left(90^{\circ}-x\right) \\
& \cos x=\sin \left(90^{\circ}-x\right) \\
& \cot x=\tan \left(90^{\circ}-x\right)
\end{aligned}
$$

## A Table of the Common Logarithms

$\begin{array}{lllllllllllllllllllllllllll}1.026 & 0.01114736 & 1.26 & 0.1003705 & 2.26 & 0.3541084 & 3.26 & 0.5132176 & 4.26 & 0.6294096 & 5.26 & 0.7209857 & 6.26 & 0.7965743 & 7.26 & 0.8609366 & 8.26 & 0.9169800 & 9.26 & 0.9666110\end{array}$

 $\begin{array}{llllllllllllllllllllllllllllllllllll}1.029 & 0.01241537 & 1.29 & 0.1105897 & 2.29 & 0.3598355 & 3.29 & 0.5171959 & 4.29 & 0.6324573 & 5.29 & 0.7234557 & 6.29 & 0.7986506 & 7.29 & 0.8627275 & 8.29 & 0.9185545 & 9.29 & 0.9680157\end{array}$ $\begin{array}{llllllllllllllllllllllllllllll}1.030 & 0.01283722 & 1.30 & 0.1139434 & 2.30 & 0.3617278 & 3.30 & 0.5185139 & 4.30 & 0.6334685 & 5.30 & 0.7242759 & 6.30 & 0.7993405 & 7.30 & 0.8633229 & 8.30 & 0.9190781 & 9.30 & 0.9684829\end{array}$

 $\begin{array}{lllllllllllllllllllllllllllllllllllll}1.033 & 0.01410032 & 1.33 & 0.1238516 & 2.33 & 0.3673559 & 3.33 & 0.5224442 & 4.33 & 0.6364879 & 5.33 & 0.7267272 & 6.33 & 0.8014037 & 7.33 & 0.8651040 & 8.33 & 0.9206450 & 9.33 & 0.9698816\end{array}$ $\begin{array}{lllllllllllllllllllllllllllllllllll}1.034 & 0.01452054 & 1.34 & 0.1271048 & 2.34 & 0.3692159 & 3.34 & 0.5237465 & 4.34 & 0.6374897 & 5.34 & 0.7275413 & 6.34 & 0.8020893 & 7.34 & 0.8656961 & 8.34 & 0.9211661 & 9.34 & 0.9703469\end{array}$


 $\begin{array}{lllllllllllllllllllllllllllllll}1.038 & 0.01619735 & 1.38 & 0.1398791 & 2.38 & 0.3765770 & 3.38 & 0.5289167 & 4.38 & 0.6414741 & 5.38 & 0.7307823 & 6.38 & 0.8048207 & 7.38 & 0.8680564 & 8.38 & 0.9232440 & 9.38 & 0.9722028\end{array}$


 $\begin{array}{lllllllllllllllllllllllllll}1.042 & 0.01786772 & 1.42 & 0.1522883 & 2.42 & 0.3838154 & 3.42 & 0.5340261 & 4.42 & 0.6454223 & 5.42 & 0.7339993 & 6.42 & 0.8075350 & 7.42 & 0.8704039 & 8.42 & 0.9253121 & 9.42 & 0.9740509\end{array}$ $\begin{array}{lllllllllllllllllllllllllllllllllll}1.043 & 0.01828431 & 1.43 & 0.1553360 & 2.43 & 0.3856063 & 3.43 & 0.5352941 & 4.43 & 0.6464037 & 5.43 & 0.7347998 & 6.43 & 0.8082110 & 7.43 & 0.8709888 & 8.43 & 0.9258276 & 9.43 & 0.9745117\end{array}$

 $\begin{array}{llllllllllllllllllllllll}1.046 & 0.01953168 & 1.46 & 0.1643529 & 2.46 & 0.3909351 & 3.46 & 0.5390761 & 4.46 & 0.6493349 & 5.46 & 0.7371926 & 6.46 & 0.8102325 & 7.46 & 0.8727388 & 8.46 & 0.9273704 & 9.46 & 0.9758911\end{array}$






 $\begin{array}{lllllllllllllllllllllllllllllllllll}1.054 & 0.02284061 & 1.54 & 0.1875207 & 2.54 & 0.4048337 & 3.54 & 0.5490033 & 4.54 & 0.6570559 & 5.54 & 0.7435098 & 6.54 & 0.8155777 & 7.54 & 0.8773713 & 8.54 & 0.9314579 & 9.54 & 0.9795484\end{array}$
 $\begin{array}{lllllllllllllllllllllllllllllllllllll}1.056 & 0.02366392 & 1.56 & 0.1931246 & 2.56 & 0.4082400 & 3.56 & 0.5514500 & 4.56 & 0.6589648 & 5.56 & 0.7450748 & 6.56 & 0.8169038 & 7.56 & 0.8785218 & 8.56 & 0.9324738 & 9.56 & 0.9804579\end{array}$
 $\begin{array}{lllllllllllllllllllllllllllllll}1.058 & 0.02448567 & 1.58 & 0.1986571 & 2.58 & 0.4116197 & 3.58 & 0.5538830 & 4.58 & 0.6608655 & 5.58 & 0.7466342 & 6.58 & 0.8182259 & 7.58 & 0.8796692 & 8.58 & 0.9334873 & 9.58 & 0.9813655\end{array}$ $\begin{array}{lllllllllllllllllllllllllllllllll}1.059 & 0.02489596 & 1.59 & 0.2013971 & 2.59 & 0.4132998 & 3.59 & 0.5550944 & 4.59 & 0.6618127 & 5.59 & 0.7474118 & 6.59 & 0.8188854 & 7.59 & 0.8802418 & 8.59 & 0.9339932 & 9.59 & 0.9818186\end{array}$ 1.0600 .025305871 .600 .20412002 .600 .41497333 .600 .55630254 .600 .6627578 5.60 0.74818806 .600 .81954397 .600 .88081368 .600 .93449859 .600 .9822712 1.0610 .025715381 .610 .20682592 .610 .41664053 .610 .55750724 .610 .66370095 .610 .74896296 .610 .82020157 .610 .88138478 .610 .93500329 .610 .9827234 $\begin{array}{lllllllllllllllllllllllllll}1.062 & 0.02612452 & 1.62 & 0.2095150 & 2.62 & 0.4183013 & 3.62 & 0.5587086 & 4.62 & 0.6646420 & 5.62 & 0.7497363 & 6.62 & 0.8208580 & 7.62 & 0.8819550 & 8.62 & 0.9355073 & 9.62 & 0.9831751\end{array}$ $\begin{array}{llllllllllllllllllllllllllllllllllll}1.063 & 0.02653326 & 1.63 & 0.2121876 & 2.63 & 0.4199557 & 3.63 & 0.5599066 & 4.63 & 0.6655810 & 5.63 & 0.7505084 & 6.63 & 0.8215135 & 7.63 & 0.8825245 & 8.63 & 0.9360108 & 9.63 & 0.9836263\end{array}$ $\begin{array}{lllllllllllllllllllllllllllllllllllll}1.064 & 0.02694163 & 1.64 & 0.2148438 & 2.64 & 0.4216039 & 3.64 & 0.5611014 & 4.64 & 0.6665180 & 5.64 & 0.7512791 & 6.64 & 0.8221681 & 7.64 & 0.8830934 & 8.64 & 0.9365137 & 9.64 & 0.9840770\end{array}$ 1.0650 .027349611 .650 .21748392 .650 .42324593 .650 .56229294 .650 .66745305 .650 .75204846 .650 .82282167 .650 .88366148 .650 .93701610 .650 .9845273 $\begin{array}{lllllllllllllllllllllllllllllllll}1.066 & 0.02775720 & 1.66 & 0.2201081 & 2.66 & 0.4248816 & 3.66 & 0.5634811 & 4.66 & 0.6683859 & 5.66 & 0.7528164 & 6.66 & 0.8234742 & 7.66 & 0.8842288 & 8.66 & 0.9375179 & 9.66 & 0.9849771\end{array}$ $\begin{array}{llllllllllllllllllllllllllllllllllll}1.067 & 0.02816442 & 1.67 & 0.2227165 & 2.67 & 0.4265113 & 3.67 & 0.5646661 & 4.67 & 0.6693169 & 5.67 & 0.7535831 & 6.67 & 0.8241258 & 7.67 & 0.8847954 & 8.67 & 0.9380191 & 9.67 & 0.9854265\end{array}$


1.0700 .029383781 .700 .23044892 .700 .43136383 .700 .56820174 .700 .67209795 .700 .75587496 .700 .82607487 .700 .88649078 .700 .93951939 .700 .9867717
 $\begin{array}{llllllllllllllllllllllll}1.072 & 0.03019479 & 1.72 & 0.2355284 & 2.72 & 0.4345689 & 3.72 & 0.5705429 & 4.72 & 0.6739420 & 5.72 & 0.7573960 & 6.72 & 0.8273693 & 7.72 & 0.8876173 & 8.72 & 0.9405165 & 9.72 & 0.9876663\end{array}$ $\begin{array}{lllllllllllllllllllllllllll}1.073 & 0.03059972 & 1.73 & 0.2380461 & 2.73 & 0.4361626 & 3.73 & 0.5717088 & 4.73 & 0.6748611 & 5.73 & 0.7581546 & 6.73 & 0.8280151 & 7.73 & 0.8881795 & 8.73 & 0.9410142 & 9.73 & 0.9881128\end{array}$

 $\begin{array}{llllllllllllllllllllllllll}1.076 & 0.03181227 & 1.76 & 0.2455127 & 2.76 & 0.4409091 & 3.76 & 0.5751878 & 4.76 & 0.6776070 & 5.76 & 0.7604225 & 6.76 & 0.8299467 & 7.76 & 0.8898617 & 8.76 & 0.9425041 & 9.76 & 0.9894498\end{array}$



 1.0810 .033825691 .810 .25767862 .810 .44870633 .810 .58092504 .810 .68214515 .810 .76417616 .810 .83314717 .810 .89265108 .810 .94497599 .810 .9916690


 $\begin{array}{llllllllllllllllllllllllllll}1.085 & 0.03542974 & 1.85 & 0.2671717 & 2.85 & 0.4548449 & 3.85 & 0.5854607 & 4.85 & 0.6857417 & 5.85 & 0.7671559 & 6.85 & 0.8356906 & 7.85 & 0.8948697 & 8.85 & 0.9469433 & 9.85 & 0.9934362\end{array}$ 1.0860 .035829831 .860 .26951292 .860 .45636603 .860 .58658734 .860 .68663635 .860 .76789766 .860 .83632417 .860 .89542258 .860 .94743379 .860 .9938769
 $\begin{array}{lllllllllllllllllllllllllllll}1.088 & 0.03662890 & 1.88 & 0.2741578 & 2.88 & 0.4593925 & 3.88 & 0.5888317 & 4.88 & 0.6884198 & 5.88 & 0.7693773 & 6.88 & 0.8375884 & 7.88 & 0.8965262 & 8.88 & 0.9484130 & 9.88 & 0.9947569\end{array}$
 1.0900 .037426501 .900 .27875362 .900 .46239803 .900 .59106464 .900 .69019615 .900 .77085206 .900 .83884917 .900 .89762718 .900 .94939009 .900 .9956352
 $\begin{array}{llllllllllllllllllllllll}1.092 & 0.03822264 & 1.92 & 0.2833012 & 2.92 & 0.4653829 & 3.92 & 0.5932861 & 4.92 & 0.6919651 & 5.92 & 0.7723217 & 6.92 & 0.8401061 & 7.92 & 0.8987252 & 8.92 & 0.9503649 & 9.92 & 0.9965117\end{array}$ $\begin{array}{llllllllllllllllllllllllllll}1.093 & 0.03862016 & 1.93 & 0.2855573 & 2.93 & 0.4668676 & 3.93 & 0.5943926 & 4.93 & 0.6928469 & 5.93 & 0.7730547 & 6.93 & 0.8407332 & 7.93 & 0.8992732 & 8.93 & 0.9508515 & 9.93 & 0.9969492\end{array}$
 1.0950 .039414121 .950 .2900346 2.95 0.46982203 .950 .59659714 .950 .69460525 .950 .77451706 .950 .84198487 .950 .90036718 .950 .95182309 .950 .9978231 $\begin{array}{llllllllllllllllllllllll}1.096 & 0.03981055 & 1.96 & 0.2922561 & 2.96 & 0.4712917 & 3.96 & 0.5976952 & 4.96 & 0.6954817 & 5.96 & 0.7752463 & 6.96 & 0.8426092 & 7.96 & 0.9009131 & 8.96 & 0.9523080 & 9.96 & 0.9982593\end{array}$ $\begin{array}{lllllllllllllllllllllllll}1.097 & 0.04020663 & 1.97 & 0.2944662 & 2.97 & 0.4727564 & 3.97 & 0.5987905 & 4.97 & 0.6963564 & 5.97 & 0.7759743 & 6.97 & 0.8432328 & 7.97 & 0.9014583 & 8.97 & 0.9527924 & 9.97 & 0.9986952\end{array}$ $\begin{array}{llllllllllllllllllllllllllllllllllll}1.098 & 0.04060234 & 1.98 & 0.2966652 & 2.98 & 0.4742163 & 3.98 & 0.5998831 & 4.98 & 0.6972293 & 5.98 & 0.7767012 & 6.98 & 0.8438554 & 7.98 & 0.9020029 & 8.98 & 0.9532763 & 9.98 & 0.9991305\end{array}$


Table of Common Logs, p. 3
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### 3.3.11 A Table of Common Logarithms

The table below lists the common logarithms (with base 10) for numbers between 1 and 10 .
The logarithm is denoted in bold face. For instance, the first entry in the third column means that the common log of 2.00 is 0.3010300 .
1.0000 .00000000
1.0010 .00043408 1.0020 .00086772 1.0030 .00130093 1.0040 .00173371 1.0050 .00216606 1.0060 .00259798 1.0070 .00302947 1.0080 .00346053 1.0090 .00389117
2.000 .30103003 .000 .47712134 .000 .60206005 .000 .69897006 .000 .77815137 .000 .84509808 .000 .90309009 .000 .9542425 $\begin{array}{llllllllllllllllllllll}2.01 & 0.3031961 & 3.01 & 0.4785665 & 4.01 & 0.6031444 & 5.01 & 0.6998377 & 6.01 & 0.7788745 & 7.01 & 0.8457180 & 8.01 & 0.9036325 & 9.01 & 0.9547248\end{array}$ 2.020 .30535143 .020 .48000694 .020 .60422615 .020 .7007037 6.02 0.77959657 .020 .84633718 .020 .90417449 .020 .9552065 $\begin{array}{lllllllllllllllllllll}2.03 & 0.3074960 & 3.03 & 0.4814426 & 4.03 & 0.6053050 & 5.03 & 0.7015680 & 6.03 & 0.7803173 & 7.03 & 0.8469553 & 8.03 & 0.9047155 & 9.03 & 0.9556878\end{array}$ $\begin{array}{llllllllllllllllllllllllll}2.04 & 0.3096302 & 3.04 & 0.4828736 & 4.04 & 0.6063814 & 5.04 & 0.7024305 & 6.04 & 0.7810369 & 7.04 & 0.8475727 & 8.04 & 0.9052560 & 9.04 & 0.9561684\end{array}$ 2.050 .31175393 .050 .48429984 .050 .60745505 .050 .70329146 .050 .78175547 .050 .84818918 .050 .90579599 .050 .9566486 $\begin{array}{llllllllllllllllllllll}2.06 & 0.3138672 & 3.06 & 0.4857214 & 4.06 & 0.6085260 & 5.06 & 0.7041505 & 6.06 & 0.7824726 & 7.06 & 0.8488047 & 8.06 & 0.9063350 & 9.06 & 0.9571282\end{array}$ $\begin{array}{lllllllllllllllllllllllllll}2.07 & 0.3159703 & 3.07 & 0.4871384 & 4.07 & 0.6095944 & 5.07 & 0.7050080 & 6.07 & 0.7831887 & 7.07 & 0.8494194 & 8.07 & 0.9068735 & 9.07 & 0.9576073\end{array}$ $\begin{array}{lllllllllllllllllllllllll}2.08 & 0.3180633 & 3.08 & 0.4885507 & 4.08 & 0.6106602 & 5.08 & 0.7058637 & 6.08 & 0.7839036 & 7.08 & 0.8500333 & 8.08 & 0.9074114 & 9.08 & 0.9580858\end{array}$ 2.090 .32014633 .090 .48995854 .090 .61172335 .090 .7067178 6.09 0.78461737 .090 .85064628 .090 .90794859 .090 .9585639

 $\begin{array}{lllllllllllllllllllllllllllllllllll}1.012 & 0.00518051 & 1.12 & 0.0492180 & 2.12 & 0.3263359 & 3.12 & 0.4941546 & 4.12 & 0.6148972 & 5.12 & 0.7092700 & 6.12 & 0.7867514 & 7.12 & 0.8524800 & 8.12 & 0.9095560 & 9.12 & 0.9599948\end{array}$ $\begin{array}{llllllllllllllllllllllllllll}1.013 & 0.00560945 & 1.13 & 0.0530784 & 2.13 & 0.3283796 & 3.13 & 0.4955443 & 4.13 & 0.6159501 & 5.13 & 0.7101174 & 6.13 & 0.7874605 & 7.13 & 0.8530895 & 8.13 & 0.9100905 & 9.13 & 0.9604708\end{array}$ 1.0140 .006037951 .140 .05690492 .140 .33041383 .140 .49692964 .140 .61700035 .140 .71096316 .140 .78816847 .140 .85369828 .140 .91062449 .140 .9609462
 $\begin{array}{lllllllllllllllllllllllllll}1.016 & 0.00689371 & 1.16 & 0.0644580 & 2.16 & 0.3344538 & 3.16 & 0.4996871 & 4.16 & 0.6190933 & 5.16 & 0.7126497 & 6.16 & 0.7895807 & 7.16 & 0.8549130 & 8.16 & 0.9116902 & 9.16 & 0.9618955\end{array}$ 1.0170 .007320951 .170 .06818592 .170 .33645973 .170 .50105934 .170 .62013615 .170 .71349056 .170 .79028527 .170 .85551928 .170 .91222219 .170 .9623693
 $\begin{array}{llllllllllllllllllllllllll}1.019 & 0.00817418 & 1.19 & 0.0755470 & 2.19 & 0.3404441 & 3.19 & 0.5037907 & 4.19 & 0.6222140 & 5.19 & 0.7151674 & 6.19 & 0.7916906 & 7.19 & 0.8567289 & 8.19 & 0.9132839 & 9.19 & 0.9633155\end{array}$

 $\begin{array}{lllllllllllllllllllllllllllll}1.022 & 0.00945090 & 1.22 & 0.0863598 & 2.22 & 0.3463530 & 3.22 & 0.5078559 & 4.22 & 0.6253125 & 5.22 & 0.7176705 & 6.22 & 0.7937904 & 7.22 & 0.8585372 & 8.22 & 0.9148718 & 9.22 & 0.9647309\end{array}$ $\begin{array}{lllllllllllllllllllllllllllllllllll}1.023 & 0.00987563 & 1.23 & 0.0899051 & 2.23 & 0.3483049 & 3.23 & 0.5092025 & 4.23 & 0.6263404 & 5.23 & 0.7185017 & 6.23 & 0.7944880 & 7.23 & 0.8591383 & 8.23 & 0.9153998 & 9.23 & 0.9652017\end{array}$
 $\begin{array}{llllllllllllllllllllllllllllllllllll}1.025 & 0.01072387 & 1.25 & 0.0969100 & 2.25 & 0.3521825 & 3.25 & 0.5118834 & 4.25 & 0.6283889 & 5.25 & 0.7201593 & 6.25 & 0.7958800 & 7.25 & 0.8603380 & 8.25 & 0.9164539 & 9.25 & 0.9661417\end{array}$

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### 3.4.0 Physics—Basic Formulas

## Reference Guide \& Formula Sheet for Physics

Dr. Hoselton \& Mr. Price
\#3 Components of a Vector
if $V=\mathbf{3 4} \mathrm{m} / \mathrm{sec} \angle \mathbf{4 8}{ }^{\circ}$
then
$\mathbf{V}_{\mathbf{i}}=\mathbf{3 4} \mathrm{m} / \mathrm{sec} \cdot\left(\boldsymbol{\operatorname { c o s }} \mathbf{4 8} \mathbf{8}^{\circ}\right)$, and $\mathbf{V}_{\mathbf{J}}=\mathbf{3 4} \mathrm{m} / \mathrm{sec} \cdot\left(\boldsymbol{\operatorname { s i n }} 48^{\circ}\right)$
\#4 $\quad$ Weight $=\mathbf{m} \cdot \mathbf{g}$
$\mathrm{g}=9.81 \mathrm{~m} / \mathrm{sec}^{2}$ near the surface of the Earth
$=9.795 \mathrm{~m} / \mathrm{sec}^{2}$ in Fort Worth, TX
Density $=$ mass $/$ volume
$\rho=\frac{m}{V}$ (unit : kg/m ${ }^{3}$ )
\#7 $\quad$ Ave speed $=$ distance $/$ time $=v=d / t$
Ave velocity $=$ displacement $/$ time $=v=d / t$
Ave acceleration $=$ change in velocity $/$ time

## Friction Force

$\mathrm{F}_{\mathrm{F}}=\mu \cdot \mathrm{F}_{\mathrm{N}}$
If the object is not moving, you are dealing with static friction and it can have any value from zero up to $\mu \mathrm{F} \mathrm{F}_{\mathrm{N}}$.
If the object is sliding, then you are dealing with kinetic friction and it will be constant and equal to $\mu_{\mathrm{k}} \mathrm{F}_{\mathrm{N}}$.

## \#9 Torque

$\tau=\mathrm{F} \cdot \mathrm{L} \cdot \sin \theta$
where $\theta$ is the angle between F and L ; unit: Nm
\#11 Newton's Second Law
$\mathrm{F}_{\text {net }}=\sum \mathrm{F}_{\mathrm{Ext}}=\mathrm{m} \cdot \mathrm{a}$
\#12 Work $=\mathbf{F} \cdot \mathrm{D} \cdot \cos \boldsymbol{\theta}$
where D is the distance moved and $\theta$ is the angle between $\mathbf{F}$ and the direction of motion, unit: J
\#16 $\quad$ Power $=$ rate of work done
Power $=\frac{\text { Work }}{\text { time }} \quad$ unit:watt
Efficiency $=$ Work $_{\text {out }} /$ Energy $_{\text {in }}$
Mechanical Advantage $=$ force out $/$ force in
M.A. $=\mathrm{F}_{\text {out }} / \mathrm{F}_{\text {in }}$
\#19 Constant-Acceleration Linear Motion

$$
\begin{array}{rlrl}
\mathrm{v} & =\mathrm{v}_{\mathrm{o}}+\mathrm{a} \cdot \mathrm{t} & \mathrm{x} \\
\left(\mathrm{x}-\mathrm{x}_{\mathrm{o}}\right) & =\mathrm{v}_{\mathrm{o}} \cdot \mathrm{t}+1 / 2 \cdot \mathrm{a} \cdot \mathrm{t}^{2} & \mathrm{v} \\
\mathrm{v}^{2} & =\mathrm{v}_{\mathrm{o}}^{2}+2 \cdot a \cdot\left(\mathrm{x}-\mathrm{x}_{\mathrm{o}}\right) & \mathrm{t} \\
\left(\mathrm{x}-\mathrm{x}_{\mathrm{o}}\right) & =1 / 2 \cdot\left(\mathrm{v}_{\mathrm{o}}+\mathrm{v}\right) \cdot \mathrm{t} & \mathrm{a} \\
\left(\mathrm{x}-\mathrm{x}_{\mathrm{o}}\right) & =\mathrm{v} \cdot \mathrm{t}-1 / 2 \cdot \mathrm{a} \cdot \mathrm{t}^{2} & v_{\mathrm{o}}
\end{array}
$$

\#20 Heating a Solid, Liquid, or Gas
$\mathrm{Q}=\mathrm{m} \cdot \mathrm{c} \cdot \Delta \mathrm{T} \quad$ (no phase changes!)
$\mathrm{Q}=$ the heat added
$\mathrm{c}=$ specific heat
$\Delta \mathrm{T}=$ temperature change, K
\#21 Linear Momentum
momentum $=\mathbf{p}=\mathrm{m} \cdot \mathrm{v}=$ mass $\cdot$ velocity Momentum is conserved in collisions.
\#23 Center of Mass - point masses on a line
$\mathrm{x}_{\mathrm{cm}}=\sum(\mathrm{mx}) / \mathrm{M}_{\mathrm{total}}$
\#25 Angular Speed vs. Linear Speed
Linear speed $=\mathrm{v}=\mathrm{r} \cdot \omega=\mathrm{r} \cdot$ angular speed
\#26 Pressure under Water
$\mathrm{P}=\rho \cdot \mathrm{g} \cdot \mathrm{h}$
$\mathrm{h}=$ depth of water
$\rho=$ density of water
\#28 Universal Gravitation
$F=G \frac{m_{1} m_{2}}{r^{2}}$
$\mathrm{G}=6.67 \mathrm{E}-11 \mathrm{~N} \mathrm{~m}^{2} / \mathrm{kg}^{2}$
\#29 Mechanical Energy
$\mathrm{PE}_{\text {Grav }}=\mathrm{P}=\mathrm{m} \cdot \mathrm{g} \cdot \mathrm{h}$
$\mathrm{KE}_{\text {Linear }}=\mathrm{K}=\frac{1}{2} \cdot \mathrm{~m} \cdot \mathrm{v}^{2}$
\#30 Impulse $=$ Change in Momentum
$\mathrm{F} \cdot \Delta \mathrm{t}=\Delta(\mathrm{m} \cdot \mathrm{v})$
\#31 Snell's Law
$\mathrm{n}_{1} \cdot \sin \theta_{1}=\mathrm{n}_{2} \cdot \sin \theta_{2}$
Index of Refraction
$\mathrm{n}=\mathrm{c} / \mathrm{v}$
$c=$ speed of light $=3 \mathrm{E}+8 \mathrm{~m} / \mathrm{s}$
\#32 Ideal Gas Law
$\mathrm{P} \cdot \mathrm{V}=\mathrm{n} \cdot \mathrm{R} \cdot \mathrm{T}$
$\mathrm{n}=\#$ of moles of gas
$\mathrm{R}=$ gas law constant
$=8.31 \mathrm{~J} / \mathrm{K}$ mole.
\#34 Periodic Waves
$\mathrm{v}=f \cdot \lambda$
$f=1 / \mathrm{T} \quad \mathrm{T}=$ period of wave
\#35 Constant-Acceleration Circular Motion

$$
\begin{array}{rlrl}
\omega & =\omega_{\mathrm{o}}+\alpha \cdot \mathrm{t} & & \theta \\
\theta-\theta_{\mathrm{o}} & =\omega_{\mathrm{o}} \cdot \mathrm{t}+1 / 2 \cdot \alpha \cdot \alpha \cdot \mathrm{t}^{2} & & \omega \\
\omega^{2} & =\omega_{\mathrm{o}}+2 \cdot \alpha \cdot\left(\theta-\theta_{\mathrm{o}}\right) & \mathrm{t} \\
\theta-\theta_{\mathrm{o}} & =1 / 2 \cdot\left(\omega_{\mathrm{o}}+\omega\right) \cdot \mathrm{t} & & \alpha \\
\theta-\theta_{\mathrm{o}} & =\omega \cdot \mathrm{t}-1 / 2 \cdot \alpha \cdot \mathrm{t}^{2} & & \omega_{\mathrm{o}}
\end{array}
$$

\#36 Buoyant Force - Buoyancy
$\mathrm{F}_{\mathrm{B}}=\rho \cdot \mathrm{V} \cdot \mathrm{g}=\mathrm{m}_{\text {Displaced fluid }} \cdot \mathrm{g}=$ weight $_{\text {Displaced fluid }}$
$\rho=$ density of the fluid
$\mathrm{V}=$ volume of fluid displaced

Hooke's Law
F $=\mathrm{k} \cdot \mathrm{x}$
Potential Energy of a spring $\mathrm{W}=\frac{1}{2} \cdot \mathrm{k} \cdot \mathrm{x}^{2}=$ work done on spring

## Electric Power

$\mathrm{p}=\mathrm{I}^{2} \cdot \mathrm{R}=\mathrm{V}^{2} / \mathrm{R}=\mathrm{I} \cdot \mathrm{V}$
\#44 Speed of a Wave on a String
$T=\frac{m v^{2}}{L}$
$\mathrm{T}=$ tension in string
$\mathrm{m}=$ mass of string
$\mathrm{L}=$ length of string
\#45 Projectile Motion
Horizontal: $\mathrm{x}-\mathrm{x}_{\mathrm{o}}=\mathrm{v}_{\mathrm{o}} \cdot \mathrm{t}+0$
Vertical: $y-y_{o}=v_{o} \cdot t+\frac{1}{2} \cdot a \cdot t^{2}$

## \#46 Centripetal Force

$F=\frac{m v^{2}}{r}=m \omega^{2} r$
\#47 Kirchhoff's Laws
Loop Rule: $\sum_{\text {Around any loop }} \Delta \mathrm{V}_{\mathrm{i}}=0$
Node Rule: $\sum_{\text {at any node }} I_{i}=0$
\#51 Minimum Speed at the top of a Vertical Circular Loop
$v=\sqrt{r g}$
SERIES
$\mathrm{R}_{e q}=\mathrm{R}_{1}+\mathrm{R}_{2}+\mathrm{R}_{3}+\ldots$
PARALLEL
$\frac{1}{R_{e q}}=\frac{1}{R_{1}}+\frac{1}{R_{2}}+\ldots+\frac{1}{R_{n}}=\sum_{i=1}^{n} \frac{1}{R_{i}}$
\#54 Newton's Second Law and Rotational Inertia
$\tau=$ torque $=\mathrm{I} \cdot \alpha$
$\mathrm{I}=$ moment of inertia $=\mathrm{m} \cdot \mathrm{r}^{2}$ (for a point mass)
(See table in Lesson 58 for I of 3D shapes.)
$\mathrm{Q}=\mathrm{C} \cdot \mathrm{V}$
$\mathrm{Q}=$ charge on the capacitor
$\mathrm{C}=$ capacitance of the capacitor
$\mathrm{V}=$ voltage applied to the capacitor
RC Circuits (Discharging)
$-\mathrm{t} / \mathrm{RC}$
$\mathrm{V}_{\mathrm{c}}=\mathrm{V}_{\mathrm{o}} \cdot \mathrm{e}$
$\mathrm{V}_{\mathrm{c}}-\mathrm{I} \cdot \mathrm{R}=0$
\#60 Thermal Expansion
Linear: $\Delta \mathrm{L}=\mathrm{L}_{\mathrm{o}} \cdot \alpha \cdot \Delta \mathrm{T}$
Volume: $\Delta \mathrm{V}=\mathrm{V}_{\mathrm{o}} \cdot \beta \cdot \Delta \mathrm{T}$
Bernoulli's Equation
$\mathrm{P}+\rho \cdot \mathrm{g} \cdot \mathrm{h}+\frac{1}{2} \cdot \rho \cdot \mathrm{v}^{2}=$ constant
$\mathrm{Q}_{\text {volume Flow Rate }}=\mathrm{A}_{1} \cdot \mathrm{v}_{1}=\mathrm{A}_{2} \cdot \mathrm{v}_{2}=$ constant
Circular Unbanked Tracks $\frac{m v^{2}}{r}=\mu m g$

Continuity of Fluid Flow
$\mathrm{A}_{\text {in }} \cdot \mathrm{v}_{\text {in }}=\mathrm{A}_{\text {out }} \cdot \mathrm{v}_{\text {out }}$
$\mathrm{A}=$ Area
$\mathrm{v}=$ velocity
Moment of Inertia - I
cylindrical hoop $\quad \mathrm{m} \cdot \mathrm{r}^{2}$
solid cylinder or disk $\quad 1 / 2 \mathrm{~m} \cdot \mathrm{r}^{2}$
solid sphere $\quad 1 / 5 \mathrm{~m} \cdot \mathrm{r}^{2}$
hollow sphere $\quad 2 / 3 \mathrm{~m} \cdot \mathrm{r}^{2}$
thin rod (center) $\quad \frac{1}{12} \mathrm{~m} \cdot \mathrm{~L}^{2}$
thin $\operatorname{rod}(e n d) \quad 1 / 3 \mathrm{~m} \cdot \mathrm{~L}^{2}$
Capacitors

Rotational Kinetic Energy (See LEM, pg 8)
$\mathrm{KE}_{\text {rotational }}=\frac{1}{2} \cdot \mathrm{I} \cdot \omega^{2}=\frac{1}{2} \cdot \mathrm{I} \cdot(\mathrm{v} / \mathrm{r})^{2}$
$\mathrm{KE}_{\text {rolling }} \mathrm{w} / \mathrm{o}$ slipping $=\frac{1}{2} \cdot \mathrm{~m} \cdot \mathrm{v}^{2}+\frac{1}{2} \cdot \mathrm{I} \cdot \omega^{2}$
Angular Momentum $=\mathrm{L}=\mathrm{I} \cdot \omega=\mathrm{m} \cdot \mathrm{v} \cdot \mathrm{r} \cdot \sin \theta$ Angular Impulse equals CHANGE IN Angular Momentum
$\Delta \mathrm{L}=\tau_{\text {orque }} \cdot \Delta \mathrm{t}=\Delta(\mathrm{I} \cdot \omega)$

## \#63 Period of Simple Harmonic Motion

$T=2 \pi \sqrt{\frac{m}{k}}$
where $\mathrm{k}=$ spring constant
$f=1 / \mathrm{T}=1 /$ period

Banked Circular Tracks

$$
\mathrm{v}^{2}=\mathrm{r} \cdot \mathrm{~g} \cdot \tan \theta
$$

First Law of Thermodynamics
$\Delta \mathrm{U}=\mathrm{Q}_{\mathrm{Net}}+\mathrm{W}_{\mathrm{Net}}$
Change in Internal Energy of a system $=$

+ Net Heat added to the system
+ Net Work done on the system
Flow of Heat through a Solid
$\Delta \mathrm{Q} / \Delta \mathrm{t}=\mathrm{k} \cdot \mathrm{A} \cdot \Delta \mathrm{T} / \mathrm{L}$
$\mathrm{k}=$ thermal conductivity
A = area of solid
$\mathrm{L}=$ thickness of solid

Simple Pendulum
$T=2 \pi \sqrt{\frac{L}{g}}$ and $f=1 / T$
Sinusoidal motion
$\mathrm{x}=\mathrm{A} \cdot \cos (\omega \cdot \mathrm{t})=\mathrm{A} \cdot \cos (2 \cdot \pi \cdot \mathrm{f} \cdot \mathrm{t})$
$\omega=$ angular frequency
$f=$ frequency
Doppler Effect
$f^{\prime}=f \frac{343 \pm_{\text {Away }}^{\text {Toward }} v_{o}}{343 \mp_{\text {Away }}^{\text {Toward }} v_{s}}$
$v_{\mathrm{o}}=$ velocity of observer: $v_{\mathrm{s}}=$ velocity of source
$2^{\text {nd }}$ Law of Thermodynamics
The change in internal energy of a system is
$\Delta \mathrm{U}=\mathrm{Q}_{\text {Added }}+\mathrm{W}_{\text {Done On }}-\mathrm{Q}_{\text {lost }}-\mathrm{W}_{\text {Done By }}$
Maximum Efficiency of a Heat Engine (Carnot Cycle) (Temperatures in Kelvin)
$\% E f f=\left(1-\frac{T_{c}}{T_{h}}\right) \cdot 100 \%$
$\frac{1}{f}=\frac{1}{D_{o}}+\frac{1}{D_{i}}=\frac{1}{o}+\frac{1}{i}$
$f=$ focal length
$\mathrm{i}=$ image distance
$\mathrm{O}=$ object distance

## Magnification

$$
M=-D_{i} / D_{o}=-i / o=H_{i} / H_{o}
$$

Helpful reminders for mirrors and lenses

Focal Length of:
Mirror
Lens
Object distance $=0$
Object height $=\mathrm{H}_{\mathrm{o}}$
Image distance $=\mathrm{i}$
Image height $=\mathrm{H}_{\mathrm{i}}$ Magnification

Positive concave converging all objects all objects real virtual, upright virtual, upright

Negative convex diverging
virtual real, inverted real, inverted
\#76 Coulomb's Law

$$
\begin{gathered}
F=k \frac{q_{1} q_{2}}{r^{2}} \\
k=\frac{1}{4 \pi \epsilon_{o}}=9 E 9 \frac{N \cdot m^{2}}{C^{2}}
\end{gathered}
$$

\#77 Capacitor Combinations PARALLEL
$\mathrm{C}_{\mathrm{eq}}=\mathrm{C}_{1}+\mathrm{C}_{2}+\mathrm{C}_{3}+\ldots$
SERIES
$\frac{1}{C_{e q}}=\frac{1}{C_{1}}+\frac{1}{C_{2}}+\ldots+\frac{1}{C_{n}}=\sum_{i=1}^{n} \frac{1}{C_{i}}$
\#78 Work Done on a Gas or by a Gas
$\mathrm{W}=\mathrm{P} \cdot \Delta \mathrm{V}$
\#80 Electric Field around a Point Charge

$$
E=k \frac{q}{r^{2}}
$$

$k=\frac{1}{4 \pi \varepsilon_{o}}=9 E 9 \frac{\mathrm{~N} \cdot \mathrm{~m}^{2}}{\mathrm{C}^{2}}$
\#82 Magnetic Field around a Wire
$B=\frac{\mu_{o} I}{2 \pi r}$
Magnetic Flux
$\Phi=\mathrm{B} \cdot \mathrm{A} \cdot \cos \theta$
Force caused by a magnetic field on a moving charge
$\mathrm{F}=\mathrm{q} \cdot \mathrm{v} \cdot \mathrm{B} \cdot \sin \theta$
\#83 Entropy Change at Constant T
$\Delta \mathrm{S}=\mathrm{Q} / \mathrm{T}$
(Phase changes only: melting, boiling, freezing, etc)
\#84 Capacitance of a Capacitor
$\mathrm{C}=\kappa \cdot \epsilon_{\mathrm{o}} \cdot \mathrm{A} / \mathrm{d}$
$\kappa=$ dielectric constant
$\mathrm{A}=$ area of plates
$\mathrm{d}=$ distance between plates
$\varepsilon_{0}=8.85 \mathrm{E}(-12) \mathrm{F} / \mathrm{m}$

Lenz's Law-induced current flows to create a B-field opposing the change in magnetic flux.
\#86 Inductors during an Increase in Current
$\mathrm{V}_{\mathrm{L}}=\mathrm{V}_{\text {cell }} \cdot \mathrm{e}^{-\mathrm{t} /(\mathrm{L} / \mathrm{R})}$
$\mathrm{I}=\left(\mathrm{V}_{\text {cell }} / \mathrm{R}\right) \cdot\left[1-\mathrm{e}^{-\mathrm{t} /(\mathrm{L} / \mathrm{R})}\right]$
$\mathrm{L} / \mathrm{R}=\tau=$ time constant

Transformers
$\mathrm{N}_{1} / \mathrm{N}_{2}=\mathrm{V}_{1} / \mathrm{V}_{2}$
$\mathrm{I}_{1} \cdot \mathrm{~V}_{1}=\mathrm{I}_{2} \cdot \mathrm{~V}_{2}$
Decibel Scale
B $($ Decibel level of sound $)=10 \log \left(I / I_{o}\right)$
I = intensity of sound
$\mathrm{I}_{\mathrm{o}}=$ intensity of softest audible sound
Poiseuille's Law
$\Delta \mathrm{P}=8 \cdot \eta \cdot \mathrm{~L} \cdot \mathrm{Q} /\left(\pi \cdot \mathrm{r}^{4}\right)$
$\eta=$ coefficient of viscosity
$\mathrm{L}=$ length of pipe
$\mathrm{r}=$ radius of pipe
$\mathrm{Q}=$ flow rate of fluid
Stress and Strain
$\mathbf{Y}$ or $\mathbf{S}$ or $\mathbf{B}=$ stress / strain
stress $=\mathbf{F} / \mathbf{A}$
Three kinds of strain: unit-less ratios
I. Linear: strain $=\Delta \mathrm{L} / \mathrm{L}$
II. Shear: strain $=\Delta x / L$
III. Volume: strain $=\Delta \mathrm{V} / \mathrm{V}$

Postulates of Special Relativity

1. Absolute, uniform motion cannot be detected.
2. No energy or mass transfer can occur at speeds faster than the speed of light.

Lorentz Transformation Factor
$\beta=\sqrt{1-\frac{v^{2}}{c^{2}}}$
$\Delta \mathrm{t}=\Delta \mathrm{t}_{\mathrm{o}} / \beta$
Relativistic Length Contraction
$\Delta \mathrm{x}=\beta \cdot \Delta \mathrm{x}_{\mathrm{o}}$
Relativistic Mass Increase
$\mathrm{m}=\mathrm{m}_{\mathrm{o}} / \beta$
Energy of a Photon or a Particle
$\mathrm{E}=\mathrm{h} \cdot f=\mathrm{m} \cdot \mathrm{c}^{2}$
$\mathrm{h}=$ Planck's constant $=6.63 \mathrm{E}(-34) \mathrm{J}$ sec
$f=$ frequency of the photon
Radioactive Decay Rate Law
$\mathrm{A}=\mathrm{A}_{\mathrm{o}} \cdot \mathrm{e}^{-\mathrm{kt}}=\left(1 / 2^{\mathrm{n}}\right) \cdot \mathrm{A}_{0}$ (after n half-lives)
Where $\mathrm{k}=(\ln 2) /$ half-life
\#99 Blackbody Radiation and the Photoelectric Effect
$\mathrm{E}=\mathrm{n} \cdot \mathrm{h} \cdot f$ where $\mathrm{h}=$ Planck's constant
\#100
Early Quantum Physics Rutherford-Bohr Hydrogen-like Atoms
$\frac{1}{\lambda}=R \cdot\left(\frac{1}{n_{s}^{2}}-\frac{1}{n^{2}}\right)$ meters $^{-1}$
or
$f=\frac{c}{\lambda}=c R\left(\frac{1}{n_{s}^{2}}-\frac{1}{n^{2}}\right) H z$
$\mathrm{R}=$ Rydberg's Constant
$=1.097373143 \mathrm{E} 7 \mathrm{~m}^{-1}$
$\mathrm{n}_{\mathrm{s}}=$ series integer $(2=$ Balmer $)$
$\mathrm{n}=$ an integer $>\mathrm{n}_{\mathrm{s}}$
Mass-Energy Equivalence
$\mathrm{m}_{\mathrm{v}}=\mathrm{m}_{\mathrm{o}} / \beta$
Total Energy $=K E+m_{0} c^{2}=m_{0} c^{2} / \beta$
Usually written simply as $\quad \mathrm{E}=\mathrm{m} \mathrm{c}^{2}$
de Broglie Matter Waves
For light: $\quad \mathrm{E}_{\mathrm{p}}=\mathrm{h} \cdot f=\mathrm{h} \cdot \mathrm{c} / \lambda=\mathrm{p} \cdot \mathrm{c}$
Therefore, momentum: $\mathrm{p}=\mathrm{h} / \lambda$
Similarly for particles, $\mathrm{p}=\mathrm{m} \cdot \mathrm{v}=\mathrm{h} / \lambda$,
so the matter wave's wavelength must be
$\lambda=\mathrm{h} / \mathrm{mv}$
Energy Released by Nuclear Fission or Fusion Reaction
$\mathrm{E}=\Delta \mathrm{m}_{\mathrm{o}} \cdot \mathrm{c}^{2}$

## Miscellaneous Formulas

## Quadratic Formula

if $a x^{2}+b x+c=0$
then

Trigonometric Definitions

$$
x=\frac{-b \pm \sqrt{b^{2}-4 a c}}{2 a}
$$

$$
\begin{aligned}
\sin \theta & =\text { opposite } / \text { hypotenuse } \\
\cos \theta & =\text { adjacent } / \text { hypotenuse } \\
\tan \theta & =\text { opposite } / \text { adjacent } \\
\sec \theta & =1 / \cos \theta=\text { hyp } / \text { adj } \\
\csc \theta & =1 / \sin \theta=\text { hyp } / \text { opp } \\
\cot \theta & =1 / \tan \theta=\text { adj } / \text { opp }
\end{aligned}
$$

## Inverse Trigonometric Definitions

$$
\begin{aligned}
& \theta=\sin ^{-1}(\text { opp } / \mathrm{hyp}) \\
& \theta=\cos ^{-1}(\mathrm{adj} / \mathrm{hyp}) \\
& \theta=\tan ^{-1}(\mathrm{opp} / \mathrm{hyp})
\end{aligned}
$$

## Law of Sines

$$
\mathrm{a} / \sin \mathrm{A}=\mathrm{b} / \sin \mathrm{B}=\mathrm{c} / \sin \mathrm{C}
$$

or

$$
\sin A / a=\sin B / b=\sin C / c
$$

## Law of Cosines

$$
\begin{aligned}
& \mathrm{a}^{2}=\mathrm{b}^{2}+\mathrm{c}^{2}-2 \mathrm{bc} \mathrm{c} \cos \mathrm{~A} \\
& \mathrm{~b}^{2}=\mathrm{c}^{2}+\mathrm{a}^{2}-2 \mathrm{ca} \cos \mathrm{~B} \\
& \mathrm{c}^{2}=\mathrm{a}^{2}+\mathrm{b}^{2}-2 \mathrm{ab} \cos \mathrm{C}
\end{aligned}
$$

## T-Pots

For the functional form

$$
\frac{1}{A}=\frac{1}{B}+\frac{1}{C}
$$

You may use "The Product over the Sum" rule.

$$
A=\frac{B \cdot C}{B+C}
$$

For the Alternate Functional form

$$
\frac{1}{A}=\frac{1}{B}-\frac{1}{C}
$$

You may substitute T-Pot-d

$$
A=\frac{B \cdot C}{C-B}=-\frac{B \cdot C}{B-C}
$$

## Fundamental SI Units

| Unit | Base Unit | Symbol |
| :--- | :--- | :--- |
|  | $\ldots \ldots \ldots \ldots \ldots \ldots \ldots \ldots$ |  |
| Length | meter | m |
| Mass | kilogram | kg |
| Time | second | s |
| Electric Current | ampere | A |
| Thermodynamic Temperature | kelvin | K |
| Luminous Intensity | candela | cd |
| Quantity of Substance | moles | mol |
| Plane Angle | radian | rad |
| Solid Angle | steradian | sr or str |

## Some Derived SI Units

Symbol/Unit
Quantity
Electric Charge
C—coulomb
Capacitance
H-henry
Inductance

Base Units

$$
\begin{aligned}
& \mathrm{A} \cdot \mathrm{~s} \\
& \mathrm{~A}^{2} \cdot \mathrm{~s} 4 /\left(\mathrm{kg} \cdot \mathrm{~m}^{2}\right) \\
& \mathrm{kg} \cdot \mathrm{~m}^{2} /\left(\mathrm{A}^{2} \cdot \mathrm{~s}^{2}\right)
\end{aligned}
$$

| Hz-hertz | Frequency | $\mathrm{s}^{-1}$ |
| :---: | :---: | :---: |
| J-joule | Energy \& Work | $\mathrm{kg} \cdot \mathrm{m}^{2} / \mathrm{s}^{2}=\mathrm{N} \cdot \mathrm{m}$ |
| N -newton | Force | $\mathrm{kg} \cdot \mathrm{m} / \mathrm{s}^{2}$ |
| $\Omega-\mathrm{ohm}$ | Elec Resistance | $\mathrm{kg} \cdot \mathrm{m}^{2} /\left(\mathrm{A}^{2} \cdot \mathrm{~s}^{2}\right)$ |
| Pa -pascal | Pressure | $\mathrm{kg} /\left(\mathrm{m} \cdot \mathrm{s}^{2}\right)$ |
| T-tesla | Magnetic Field | $\mathrm{kg} /\left(\mathrm{A} \cdot \mathrm{s}^{2}\right)$ |
| V—volt | Elec Potential | $\mathrm{kg} \cdot \mathrm{m}^{2} /\left(\mathrm{A} \cdot \mathrm{s}^{3}\right)$ |
| W-watt | Power | $\mathrm{kg} \cdot \mathrm{m}^{2} / \mathrm{s}^{3}$ |

## Non-SI Units

| ${ }^{\circ} \mathrm{C}$ | degrees Celsius | Temperature |
| :--- | :--- | :--- |
| eV | electron-volt | Energy, Work |

Aa Acceleration, Area, $\mathrm{A}_{\mathrm{x}}=$ Cross-sectional Area, Amperes, Amplitude of a Wave, Angle
Bb Magnetic Field, Decibel Level of Sound, Angle
Cc Specific Heat, Speed of Light, Capacitance, Angle, Coulombs, ${ }^{\circ}$ Celsius, Celsius Degrees, Candela
Dd Displacement, Differential Change in a Variable, Distance, Distance Moved, Distance
Ee Base of the Natural Logarithms, Charge on the Electron, Energy
Ff Force, Frequency of a Wave or Periodic Motion, Farads
Gg Universal Gravitational Constant, Acceleration due to Gravity, Gauss, Grams, Giga-
Hh Depth of a Fluid, Height, Vertical Distance, Henrys, Hz = Hertz
Ii Current, Moment of Inertia, Image Distance, Intensity of Sound
Jj Joules
Kk K or KE = Kinetic Energy, Force Constant of a Spring, Thermal Conductivity, Coulomb's Law Constant, $\mathrm{kg}=$ Kilograms, Kelvins, Kilo-, Rate Constant for Radioactive Decay $=1 / \tau=\ln 2 /$ half-life
Ll Length, Length of a Wire, Latent Heat of Fusion or Vaporization, Angular Momentum, Thickness, Inductance
Mm Mass, Total Mass, Meters, Milli-, Mega-, $\mathrm{m}_{\mathrm{o}}=$ Rest Mass, Mol $=$ Moles
Nn index of refraction, Moles of a Gas, Newtons, Number of Loops, Nano-
Pp Power, Pressure of a Gas or Fluid, Potential Energy, Momentum, Power, Pa = Pascal
Qq Heat Gained or Lost, Maximum Charge on a Capacitor, Object Distance, Flow Rate
Rr Radius, Ideal Gas Law Constant, Resistance, Magnitude or Length of a Vector, Rad = Radians
Ss Speed, Seconds, Entropy, Length along an Arc
Tt Time, Temperature, Period of a Wave, Tension, Teslas, $\mathrm{t}_{1 / 2}=$ Half-life
Uu Potential Energy, Internal Energy
Vv velocity, Velocity, Volume of a Gas, Velocity of Wave, Volume of Fluid Displaced, Voltage, Volts
Ww Weight, Work, Watts, Wb = Weber
Xx Distance, Horizontal Distance, x-Coordinate East-and-West Coordinate
Yy Vertical distance, y-Coordinate, North-and-South Coordinate
Zz Z-coordinate, Up-and-Down Coordinate, Linear Expansion
B $\boldsymbol{\beta}$ Beta Coefficient of Volume Expansion, Lorentz Transformation Factor
X $\chi$ Chi
$\Delta \boldsymbol{\delta}$ Delta $\Delta=$ Change in a Variable
Eє Epsilon $\epsilon_{\mathrm{o}}=$ Permittivity of Free Space
$\Phi \phi$ Phi Magnetic Flux, Angle
$\Gamma \gamma$ Gamma Surface Tension $=\mathrm{F} / \mathrm{L}, 1 / \gamma=$ Lorentz Transformation Factor

## H $\eta$ Eta

It Iota
$\boldsymbol{\vartheta} \boldsymbol{\phi}$ Theta and Phi Lower-case Alternates
Kк Kappa Dielectric Constant
$\boldsymbol{\Lambda} \boldsymbol{\lambda}$ Lambda Wavelength of a Wave, Rate Constant for Radioactive Decay $=1 / \tau=\ln 2 /$ half-life
$\mathbf{M} \boldsymbol{\mu}$ Mu Friction, $\mu_{0}=$ Permeability of Free Space, Micro-
$\mathrm{N} v \mathbf{N u}$ Alternate Symbol for Frequency

## Oo Omicron

Пл Pi 3.1425926536...
© $\theta$ Theta Angle between Two Vectors
P $\rho$ Rho Density of a Solid or Liquid, Resistivity
$\Sigma \sigma$ Sigma Summation, Standard Deviation
$\mathrm{T} \tau$ Tau Torque, Time Constant for a Exponential Processes; eg $\tau=\mathrm{RC}$ or $\tau=\mathrm{L} / \mathrm{R}$ or $\tau=1 / \mathrm{k}=1 / \lambda$
Yo Upsilon
$\varsigma \omega$ Zeta and Omega Lower-case Alternates
$\Omega \omega$ Omega Angular Speed or Angular Velocity, Ohms
$\Xi \xi \mathrm{Xi}$
$\Psi \psi$ Psi
Zち Zeta

Values of Trigonometric Functions for First Quadrant Angles(simple mostly rational approximations)

| $\theta$ | $\boldsymbol{\operatorname { s i n }} \boldsymbol{\theta}$ | $\cos \theta$ | $\boldsymbol{t a n} \theta$ |
| :---: | :---: | :---: | :---: |
| $0^{\circ}$ | 0 | 1 | 0 |
| $10^{\circ}$ | 1/6 | 65/66 | 11/65 |
| $15^{\circ}$ | 1/4 | 28/29 | 29/108 |
| $20^{\circ}$ | 1/3 | 16/17 | 17/47 |
| $29^{\circ}$ | $15^{1 / 2} / 8$ | 7/8 | $15^{1 / 2 / 7}$ |
| $30^{\circ}$ | 1/2 | $3^{1 / 2 / 2}$ | $1 / 3^{1 / 2}$ |
| $37^{\circ}$ | 3/5 | 4/5 | 3/4 |
| $42^{\circ}$ | 2/3 | 3/4 | 8/9 |
| $45^{\circ}$ | $2^{1 / 2} / 2$ | $2^{1 / 2} / 2$ | 1 |
| $49^{\circ}$ | 3/4 | 2/3 | 9/8 |
| $53^{\circ}$ | 4/5 | 3/5 | 4/3 |
| 60 | $3^{1 / 2} / 2$ | 1/2 | $3^{1 / 2}$ |
| $61^{\circ}$ | 7/8 | $15^{1 / 2} / 8$ | 7/15 ${ }^{1 / 2}$ |
| $70^{\circ}$ | 16/17 | 1/3 | 47/17 |
| $75^{\circ}$ | 28/29 | 1/4 | 108/29 |
| $80^{\circ}$ | 65/66 | 1/6 | 65/11 |
| $90^{\circ}$ | 1 | 0 | $\infty$ |

(Memorize the Bold rows for future reference.)

## Derivatives of Polynomials

For polynomials, with individual terms of the form $\mathrm{Ax}^{\mathrm{n}}$, we define the derivative of each term as

$$
\frac{d}{d x}\left(A x^{n}\right)=n A x^{n-1}
$$

To find the derivative of the polynomial, simply add the derivatives for the individual terms:

$$
\frac{d}{d x}\left(3 x^{2}+6 x-3\right)=6 x+6
$$

## Integrals of Polynomials

For polynomials, with individual terms of the form $\mathrm{Ax}^{\mathrm{n}}$, we define the indefinite integral of each term as

$$
\int\left(A x^{n}\right) d x=\frac{1}{n+1} A x^{n+1}
$$

To find the indefinite integral of the polynomial, simply add the integrals for the individual terms and the constant of integration, C.

$$
\int(6 x+6) d x=\left[3 x^{2}+6 x+C\right]
$$

## Prefixes

| Factor | Prefix | Symbol | Example |
| :---: | :---: | :---: | :---: |
| $10^{18}$ | exa- | E | 38 Es (Age of the Universe in Seconds) |
| $10^{15}$ | peta- | P |  |
| $10^{12}$ | tera- | T | 0.3 TW (Peak power of a 1 ps pulse from a typical Nd-glass laser) |
| $10^{9}$ | giga- | G | 22 G\$ (Size of Bill \& Melissa Gates' Trust) |
| $10^{6}$ | mega- | M | 6.37 Mm (The radius of the Earth) |
| $10^{3}$ | kilo- | k | 1 kg (SI unit of mass) |
| $10^{-1}$ | deci- | d | 10 cm |
| $10^{-2}$ | centi- | c | 2.54 cm ( $=1 \mathrm{in}$ ) |
| $10^{-3}$ | milli- | m | $\mathbf{1 ~ m m}$ (The smallest division on a meter stick) |
| $10^{-6}$ | micro- | $\mu$ |  |
| $10^{-9}$ | nano- | n | $510 \mathbf{n m}$ (Wavelength of green light) |
| $10^{-12}$ | pico- | p | $\mathbf{1} \mathbf{~ p g}$ (Typical mass of a DNA sample used in genome studies) |
| $10^{-15}$ | femto- | f |  |
| $10^{-18}$ | atto- | a | 600 as (Time duration of the shortest laser pulses) |

## Linear Equivalent Mass

Rotating systems can be handled using the linear forms of the equations of motion. To do so, however, you must use a mass equivalent to the mass of a nonrotating object. We call this the Linear Equivalent Mass (LEM). (See Example I.)

For objects that are both rotating and moving linearly, you must include them twice-once as a linearly moving object (using m) and once more as a rotating object (using LEM). (See Example II.)

The LEM of a rotating mass is easily defined in terms of its moment of inertia, I.

$$
\mathrm{LEM}=\mathrm{I} / \mathrm{r}^{2}
$$

For example, using a standard table of Moments of Inertia, we can calculate the LEM of simple objects rotating on axes through their centers of mass:

|  | I | LEM |
| :--- | :--- | :--- |
| Cylindrical hoop | $\mathrm{mr}^{2}$ | m |
| Solid disk | $1 / 2 \mathrm{mr}^{2}$ | $1 / 2 \mathrm{~m}$ |
| Hollow sphere | $2 / \mathrm{sr}^{2}$ | $2 / \mathrm{sm}$ |
| Solid sphere | $2 / 3 \mathrm{mr}^{2}$ | $2 / 3 \mathrm{~m}$ |

## Example I

A flywheel, $\mathrm{M}=4.80 \mathrm{~kg}$ and $\mathrm{r}=0.44 \mathrm{~m}$, is wrapped with a string. A hanging mass, m , is attached to the end of the string.

When the hanging mass is released, it accelerates downward at $1.00 \mathrm{~m} / \mathrm{s}^{2}$. Find the hanging mass.


To handle this problem using the linear form of Newton's Second Law of Motion, all we have to do is use the LEM of the flywheel. We will assume, here, that it can be treated as a uniform solid disk.

The only external force on this system is the weight of the hanging mass. The mass of the system consists of the hanging mass plus the linear equivalent mass of the flywheel. From Newton's Second Law we have
$\mathrm{F}=\mathrm{ma}$, therefore, $\quad \mathrm{mg}=[\mathrm{m}+(\mathrm{LEM}=1 / 2 \mathrm{M})] \mathrm{a}$

$$
\mathrm{mg}=[\mathrm{m}+1 / 2 \mathrm{M}] \mathrm{a}
$$

$$
(\mathrm{mg}-\mathrm{ma})=1 / 2 \mathrm{Ma}
$$

$$
\mathrm{m}(\mathrm{~g}-\mathrm{a})=1 / 2 \mathrm{Ma}
$$

$$
\mathrm{m}=1 / 2 \cdot \mathrm{M} \cdot \mathrm{a} /(\mathrm{g}-\mathrm{a})
$$

$$
\mathrm{m}=1 / 2 \cdot 4.8 \cdot 1.00 /(9.81-1)
$$

$$
\mathrm{M}=0.27 \mathrm{~kg}
$$

If $\mathrm{a}=\mathrm{g} / 2=4.905 \mathrm{~m} / \mathrm{s}^{2}, \quad \mathrm{~m}=2.4 \mathrm{~kg}$
If $\mathrm{a}=3 / 4 \mathrm{~g}=7.3575 \mathrm{~m} / \mathrm{s}^{2}, \quad \mathrm{~m}=7.2 \mathrm{~kg}$
Note, too, that we do not need to know the radius unless the angular acceleration of the flywheel is requested. If you need $\alpha$, and you have r , then $\alpha=\mathrm{a} / \mathrm{r}$.

## Example II

Find the kinetic energy of a disk, $\mathrm{m}=6.7 \mathrm{~kg}$, that is moving at $3.2 \mathrm{~m} / \mathrm{s}$ while rolling without slipping along a flat, horizontal surface. ( $\mathrm{I}_{\text {DISK }}=\frac{1}{2} \mathrm{mr}^{2} ;$ LEM $=\frac{1}{2} \mathrm{~m}$ )

The total kinetic energy consists of the linear kinetic energy, $\mathrm{K}_{\mathrm{L}}=\frac{1}{2} \mathrm{mv}^{2}$, plus the rotational kinetic energy,

$$
\begin{aligned}
& \mathrm{K}_{\mathrm{R}}=1 / 2(\mathrm{I})(\omega)^{2}=1 / 2(\mathrm{I})(\mathrm{v} / \mathrm{r})^{2}=1 / 2\left(\mathrm{I} / \mathrm{r}^{2}\right) \mathrm{v}^{2}=1 / 2(\mathrm{LEM}) \mathrm{v}^{2} . \\
& \mathrm{KE}=1 / 2 \mathrm{mv}^{2}+1 / 2 \cdot(\mathrm{LEM}=1 / 2 \mathrm{~m}) \cdot \mathrm{v}^{2} \\
& \mathrm{KE}=1 / 2 \cdot 6.7 \cdot 3.2^{2}+1 / 2 \cdot(1 / 2 \cdot 6.7) \cdot 3.2^{2} \\
& \mathrm{KE}=34.304+17.152=51 \mathrm{~J}
\end{aligned}
$$

### 3.4.1 Physics Concepts-Force, Pressure, and Energy

## Physics Concepts-Force, Pressure, and Energy

Force, pressure, and energy are some basic tenets of Physics. The following terms will hopefully serve as a reminder of Physics 101.

Force
Pressure
Energy

## Force

Force is described as what is required to change the velocity or acceleration of an object. Recall that acceleration is any change in vector. The formula for force is rather simple:

$$
\begin{aligned}
& F=m * a \\
& F=\text { Force } \\
& m \text { massofobject } \\
& a=\text { accleration }
\end{aligned}
$$

Source: astronomyonline.org
Here is Newton's famous Force equation:
$\mathrm{F}=$ force required, given in Newton's or Dyne's
$\mathrm{m}=$ the mass of the object, and
$\mathrm{a}=$ acceleration
The Newton (named after Sir Isaac Newton) is the amount required to move 1 kilogram at a distance of 1 meter in 1 second. It is written as:

$$
\begin{aligned}
& \frac{k g(m)}{s^{2}} \\
& o r: \\
& (\mathrm{kg}) \mathrm{m} / \mathrm{s}^{2} \\
& \mathrm{or}: \\
& \mathrm{kg}(\mathrm{~m}) \mathrm{s}^{-2}
\end{aligned}
$$

The equation of force is Newton's second Law of Physics. The three laws are:

- A body at rest must remain at rest. and a body in motion remains in motion unless acted upon by an external force.
- A force ( F ) on a body (m) gives it an acceleration (a) in the direction on the force and is inversely proportional to the mass.
- Whenever a body exerts a force from another body, that body exerts a force equal in magnitude and in the opposite direction of the initial mass.

$$
F=G\left(\frac{m_{1} m_{2}}{r^{2}}\right)
$$

$F=$ gravitational force between two objects
$m_{1}=$ mass of first object in kilograms
$m_{2}=$ mass of second object in kilograms
$r=$ distance between objects
$G=$ gravitational constant
Examples of force: Gravity and Friction

## Pressure

Pressure is very similar to Force, but applies force over a particular area.

$$
\text { Pressure }=\frac{\text { Force }}{\text { Area }}
$$

The units of this equation will be:

$$
\mathrm{kg} / \mathrm{ms}^{2}=1 \text { Pascal }=\mathrm{N} / \mathrm{m}^{2}
$$

### 3.4.2 Physics—Circular Motion

## Circular Motion



In the diagram $\mathbf{v}$ is the tangential velocity of the object, $\mathbf{a}$ is the centripetal (acting towards the center of the circle) acceleration, $\mathbf{F}$ is the centripetal force, $\mathbf{r}$ is the radius of the circle, and $m$ is the mass of the object.

$$
\begin{gathered}
\mathrm{a}=\mathrm{v}^{2} / \mathrm{r} \\
\mathrm{~F}=\mathrm{ma}=\mathrm{mv}^{2} / \mathrm{r}
\end{gathered}
$$

Source: Tutor4Physics.com

### 3.4.3 Physics—Gravitation

## Physics—Gravitation

## Kepler's Laws

Toward the end of the sixteenth century, Tycho Brahe collected a huge amount of data giving precise measurements of the position of planets. Johannes Kepler, after a detailed analysis of the measurements, announced three laws in 1619.

1. The orbit of each planet is an ellipse that has the Sun at one of its foci.
2. Each planet moves in such a way that the (imaginary) line joining it to the Sun sweeps out equal areas in equal times.
3. The squares of the periods of revolution of the planets about the Sun are proportional to the cubes of their mean distances from it.

## Newton's Law of Universal Gravitation

About 50 years after Kepler announced the laws now named after him, Isaac Newton showed that every particle in the universe attracts every other with a force that is proportional to the products of their masses and inversely proportional to the square of their separation.

Hence:
If $\mathbf{F}$ is the force due to gravity, $\mathbf{g}$ the acceleration due to gravity, $\mathbf{G}$ the Universal Gravitational Constant $\left(6.67 \times 10^{-11} \mathrm{~N} \mathrm{~m}^{2} / \mathrm{kg}^{2}\right), \mathbf{m}$ the mass, and $\mathbf{r}$ the distance between two objects, then

$$
\mathrm{F}=\mathrm{Gm}_{1} \mathrm{~m}_{2} / \mathrm{r}^{2}
$$

## Acceleration Due to Gravity Outside the Earth

It can be shown that the acceleration due to gravity outside of a spherical shell of uniform density is the same as it would be if the entire mass of the shell were to be concentrated at its center.

Using this principle, we can express the acceleration due to gravity ( $\mathbf{g}^{\prime}$ ) at a radius ( $\mathbf{r}$ ) outside the Earth in terms of the Earth's radius $\left(\mathbf{r}_{\mathbf{e}}\right)$ and the acceleration due to gravity at the Earth's surface ( $\mathbf{g}$ ).

$$
\mathrm{g}^{\prime}=\left(\mathrm{r}_{\mathrm{e}}^{2} / \mathrm{r}^{2}\right) \mathrm{g}
$$

## Acceleration Due to Gravity Inside the Earth

Here let $\mathbf{r}$ represent the radius of the point inside the Earth. The formula for determining the acceleration due to gravity at this point becomes:

$$
\mathrm{g}^{\prime}=\left(\mathrm{r} / \mathrm{r}_{\mathrm{e}}\right) \mathrm{g}
$$

In both of the above formulas, as expected, $g^{\prime}$ becomes equal to $g$ when $r=r_{e}$.

### 3.4.4 Physics—Work Energy Power

## Work and Energy

As we know from the law of conservation of energy, energy is always conserved.
Work is the product of force and the distance over which it moves. Imagine you are pushing a heavy box across the room. The further you move, the more work you do! If W is work, F the force, and x the distance, then.

$$
\mathrm{W}=\mathrm{Fx}
$$

Energy comes in many shapes. The energies we see over here are kinetic energy (KE) and potential energy (PE).

$$
\begin{aligned}
\text { Transitional KE } & =\frac{1}{2} \mathrm{mv}^{2} \\
\text { Rotational KE } & =\frac{1}{2} \mathrm{lw}^{2}
\end{aligned}
$$

Here $\mathbf{I}$ is the moment of inertia of the object (a simple manner in which one can understand moment of inertia is to consider it to be similar to mass in transitional KE ) and w is angular velocity

$$
\text { Gravitational } \mathrm{PE}=\mathrm{mgh}
$$

where $\mathbf{h}$ is the height of the object

$$
\text { Elastic PE }=\frac{1}{2} \mathrm{k}^{2}
$$

where $\mathbf{k}$ is the spring constant (it gives how much a spring will stretch for a unit force) and L is the length of the spring. Simple isn't it!!

## Power

Power $(\mathbf{P})$ is work $(\mathbf{W})$ done in unit time $(\mathrm{t})$.

$$
\mathrm{P}=\mathrm{W} / \mathrm{t}
$$

As work and energy $(\mathbf{E})$ are the same, it follows that power is also energy consumed or generated per unit time.

$$
\mathrm{P}=\mathrm{E} / \mathrm{t}
$$

In measuring power, horsepower is a unit that is in common use. However, in physics we use Watt. So the first thing to do in solving any problem related to power is to convert horsepower to Watts. 1 horsepower $(\mathrm{hp})=746$ Watts

### 3.4.5 Physics—Laws of Motion

## Newton's Laws of Motion

Through Newton's second law-The acceleration of a body is directly proportional to the net unbalanced force and inversely proportional to the body's mass-a relationship is established between Force (F), Mass (m), and Acceleration (a). This is of course a wonderful relation and of immense usefulness.

$$
\mathrm{F}=\mathrm{m} \times \mathrm{a}
$$

Knowing any two of the quantities automatically gives you the third !!

## Momentum

Momentum ( $\mathbf{p}$ ) is the quantity of motion in a body. A heavy body moving at a fast velocity is difficult to stop. A light body at a slow speed, on the other hand, can be stopped easily. So momentum has to do with both mass and velocity.

$$
\mathbf{p}=\mathbf{m v}
$$

Often physics problems deal with momentum before and after a collision. In such cases the total momentum of the bodies before collision is taken as equal to the total momentum of the bodies after collision. That is to say: momentum is conserved.

## Impulse

This is the change in the momentum of a body caused over a very short time. Let $\mathbf{m}$ be the mass and $\mathbf{v}$ and $\mathbf{u}$ be the final and initial velocities of a body.

$$
\text { Impulse }=\mathrm{Ft}=\mathrm{mv}-\mathrm{mu}
$$

Source: Tutor4Physics.com

### 3.4.6 Physics-One-, Two-, and Three-Dimensional Motion

## One-Dimensional Motion

By one dimension we mean that the body is moving only in one plane and in a straight line. We would be undergoing one-dimensional motion, for example, if we were to roll a marble on a flat table, and if we rolled it in a straight line (not easy!).

Four variables put together in an equation can describe this motion. These are Initial Velocity (u); Final Velocity (v); Acceleration (a); Distance Traveled (s) and Time Elapsed (t). The equations that tell us the relationship between these variables are as follows:

$$
\begin{gathered}
\mathrm{v}=\mathrm{u}+\mathrm{at} \\
\mathrm{v}^{2}=\mathrm{u}^{2}+2 \mathrm{as} \\
\mathrm{~s}=\mathrm{ut}+1 / 2 \mathrm{at}^{2} \\
\text { average velocity }=(\mathrm{v}+\mathrm{u}) / 2
\end{gathered}
$$

Armed with these equations you can do wonderful things such as calculating a car's acceleration from zero to whatever in 60 seconds !!

Source: Tutor4physics

## Two- and Three-Dimensional Motion

## Scalar or Vector?

To explain the difference between scalar and vector, we use two words: magnitude and direction. By magnitude we mean how much of the quantity is there. By direction we mean the quantity having a direction that defines it. Physical quantities that are completely specified by just giving out their magnitude are known as scalars. Examples of scalar quantities are distance, mass, speed, volume, density, and temperature. Other physical quantities cannot be defined by just their magnitude. To define them completely, we must also specify their direction. Examples of these quantities are velocity, displacement, acceleration, force, torque, and momentum.

## Vector Addition

## Parallelogram Law of Vector Addition

Let us assume we were to represent two vectors' magnitude and direction by two adjacent sides of a parallelogram. The resultant could then be represented in magnitude and direction by the diagonal. This diagonal is the one that passes through the point of intersection of these two sides.

## Resolution of a Vector

It is often necessary to split a vector into its components. Splitting a vector into its component parts is called resolution of the vector. The original vector is the resultant of these components. When the components of a vector are at right angles to each other, they are called the rectangular components of a vector.

## Rectangular Components of a Vector

As the rectangular components of a vector are perpendicular to each other, we can do mathematics on them. This allows us to solve many real-life problems. After all, the best thing about physics is that it can be used to solve real-world problems.

Note: As it is difficult to use vector notations on computer word processors, we will coin our own notation. We will show all vector quantities in bold. For example, A will be scalar quantity and $\mathbf{A}$ will be a vector quantity.

Let $\mathbf{A}_{\mathbf{x}}$ and $\mathbf{A}_{\mathbf{y}}$ be the rectangular components of a vector $\mathbf{A}$
then

$$
\mathbf{A}=\mathbf{A}_{\mathbf{x}}+\mathbf{A}_{\mathbf{y}}
$$

This means that vector $\mathbf{A}$ is the resultant of vectors $\mathbf{A}_{\mathbf{x}}$ and $\mathbf{A}_{\mathbf{y}}$.
A is the magnitude of vector $\mathbf{A}$, and similarly $\mathrm{A}_{\mathrm{x}}$ and $\mathrm{A}_{\mathrm{y}}$ are the magnitudes of vectors $\mathbf{A}_{\mathbf{x}}$ and $\mathbf{A}_{\mathbf{y}}$.
As we are dealing with rectangular components that are at right angles to each other, we can say that:

$$
\mathrm{A}=\left(\mathrm{A}_{\mathrm{x}}+\mathrm{A}_{\mathrm{y}}\right)^{1 / 2}
$$

Similarly, the angle Q that the vector $\mathbf{A}$ makes with the horizontal direction will be

$$
\mathrm{Q}=\tan ^{-1}\left(\mathrm{~A}_{\mathrm{x}} / \mathrm{A}_{\mathrm{y}}\right)
$$

Physics-One-,Two-,Three-Dimension Motion, p. 3
Source:Tutor4Physics.com

### 3.4.7 Physics—Electricity

## Electricity

According to Ohm's Law, electric potential difference $(\mathbf{V})$ is directly proportional to the product of the current( $\mathbf{I}$ ) times the resistance $(\mathbf{R})$.

$$
\mathrm{V}=\mathrm{I} \mathrm{R}
$$

The relationship between power $(\mathbf{P})$ and current and voltage is

$$
\mathrm{P}=\mathrm{IV}
$$

Using the equations above, we can also write

$$
\mathrm{P}=\mathrm{V}^{2} / \mathrm{R}
$$

and

$$
\mathrm{P}=\mathrm{I}^{2} \mathrm{R}
$$

## Resistance of Resistors in Series

The equivalent resistance ( $\mathrm{R}_{\text {eq }}$ ) of a set of resistors connected in series is

$$
\mathrm{R}_{\mathrm{eq}}=\mathrm{R}_{1}+\mathrm{R}_{2}+\mathrm{R}_{3}+---
$$

## Resistance of Resistors in Parallel

The equivalent resistance ( $\mathrm{R}_{\mathrm{eq}}$ ) of a set of resistors connected in parallel is

$$
1 / \mathrm{R}_{\mathrm{eq}}=1 / \mathrm{R}_{1}+1 / \mathrm{R}_{2}+1 / \mathrm{R}_{3}
$$

### 3.5.0 Financial Formula Calculations-Net Present Value and Compounding

 Present Value FormulasCalculate the Present Value of a Single Sum

$$
\mathrm{PV}=\frac{\mathrm{FV}}{(1+r)^{n}}
$$

Calculate the Present Value with Compounding

$$
\mathrm{PV}=\frac{\mathrm{FV}}{\left(1+\frac{r}{m}\right)^{(n-m)}}
$$

Calculate the Present Value of a Cash-Flow Series

$$
\mathrm{PV}=\sum_{j=1}^{n}\left[\frac{\mathrm{FV}_{j}}{\left(1+\frac{r}{m}\right)^{j}}\right]
$$

Calculate the Present Value of an Annuity with Continuous Compounding

$$
\mathrm{PV}_{a c p}=\frac{1-e^{(-r t)}}{r}
$$

Calculate the Present Value of a Growing Annuity with Continuous Compounding

$$
P V_{g a}=\frac{\operatorname{PMT}\left(1-e^{-(r-g) t}\right)}{e^{(r-g)}-1}
$$

Calculate the Net Present Value of a Cash-Flow Series

$$
\mathrm{NPV}=\sum_{j-1}^{n} \frac{\mathrm{CF}_{j}}{(1+r)^{j}}=0
$$

Calculate the Future Value with Compounding

$$
\mathrm{FV}=\mathrm{PV}\left(1+\frac{r}{m}\right)^{(n-m)}
$$

Calculate the Future Value of a Cash-Flow Series

$$
\mathrm{FV}=\sum_{j-1}^{n} \mathrm{CF}_{j}(1+r)^{j}
$$

Calculate the Future Value of an Annuity

$$
\mathrm{FV}_{a}=\mathrm{PMT}\left[\frac{(1+r)^{n}-1}{r}\right]
$$

Calculate the Future Value of an Annuity Due

$$
\mathrm{FV}_{\mathrm{ad}}=\mathrm{PMT}\left[\frac{(1+r)^{n}-1}{r}\right](1+r)
$$

Calculate the Future Value of an Annuity with Compounding

$$
\mathrm{FV}_{a}=\mathrm{PMT} \cdot \frac{\left(1+\frac{r}{m}\right)^{(m-n)}-1}{r / m}
$$

## Payment Calculations

Calculate Monthly Payment

$$
\mathrm{PMT}=P \cdot \frac{r(1+r)^{n}}{(1+r)^{n}-1}
$$

## Symbols and Variables in Financial Formulas

One important key to understanding formulas for financial math is knowledge of what financial symbols and variables represent.

Where $N=$ Number of Periods $g=$ Rate of Growth $m=$ Compounding Frequency $r=$ Interest Rate PMT $=$ Periodic Payment

The chart of symbols (located at the right) offers an explanation of the financial variables used in the formulas provided below.
$F V=$ Future Value
$\mathrm{PV}=$ Present Value
$\mathrm{CF}=$ Cash Flow
$j=$ the jth Period

Calculate the Number of Payments

$$
\mathrm{N}=\frac{-\log (1-\mathrm{rFV} / \mathrm{PMT})}{\log (1+\mathrm{r})}
$$

## Convert Interest Rate Compounding Bases

Where $r_{1}=$ Original Interest Rate with Compounding Frequency $n_{1}$ and $r_{2}$ is the stated interest rate with compounding frequency $n_{2}$

$$
r_{2}=\left[\left(1+\frac{r_{1}}{n_{2}}\right)^{\frac{n_{1}}{n_{2}}}-1\right] \cdot n_{2}
$$

Calculate Sinking Fund

$$
\text { PMT }=\text { Goal } \cdot \frac{r / m}{\left(1+\frac{r}{m}\right)^{(n-m)}-1}
$$

## Future Value Formulas

Calculate the Future Value of a Single Sum

$$
\mathrm{FV}=\mathrm{PV}(1+r)^{n}
$$

## Cash-Flow Series Calculations

Calculate the Present Worth Cost of a Cash-Flow Series

$$
\mathrm{PWC}=\sum_{j-1}^{n} \frac{\mathrm{CF}_{j}}{(1+r)^{j}} \text { where } \mathrm{CF}_{j}<0
$$

Calculate the Present Worth Revenue of a Cash-Flow Series

$$
\mathrm{PWR}=\sum_{j-1}^{n} \frac{\mathrm{CF}_{j}}{(1+r)^{j}} \text { where } \mathrm{CF}_{j}>0
$$

### 3.6.0 Formulas for Calculating the Volume of Cylindrical Tanks

## Cylindrical Tank Capacity

We establish capacity in gallons of water in a cylindrical tank by using formulas. The plumbing trade rarely exposes a need for such calculations on a job site. Most state plumbing exams require an individual to be knowledgeable pertaining to a tank's capacity in gallons of water.

Several steps are required, and you must know how many cubic inches are in 1 gallon of water and how many gallons are in a cubic foot. Dimensions of a tank are in feet and inches or may only be in feet form. You must use the relevant formula for each dimensional situation. When a dimension is in feet and inches, calculations must be used in inch form. Conversion from fraction of an inch to decimal of an inch is also required.

Example: $10^{\prime}-2-1 / 2^{\prime \prime}=122-1 / 2^{\prime \prime}$, which converts to $122.50^{\prime \prime}$
The first approach is to find an area of the base of a tank. This is achieved by knowing the diameter of a tank, then multiplying by the diameter by the diameter ( $\mathrm{D} \times \mathrm{D}$ ), and next by multiplying by 0.7854 . At this point in your calculations, you have achieved the square area of a circle.

Area of a cylinder base $=$ Diameter $\times$ Diameter $\times 0.7854(\mathrm{D} \times \mathrm{D} \times 0.7854)$
The next step is to know a length or height of a tank. You multiply the square area of a tank's base by the length or height dimension. This gives you the volume of a tank or pipe.

Volume of a cylinder $=$ Height $\times$ Square area of cylindrical base (height also $=$ length or depth)
The next step is to calculate the volume when dimensions are in inch form in order to determine the capacity in gallons of a tank. There are 231 cubic inches in 1 gallon of water, so you would divide the volume of a tank by $\underline{231}$ to achieve the gallon capacity.

Tank capacity in gallons when measurements are in inch form $=$ Volume $\div 231$
Diameter $\times$ Diameter $\times 0.7854 \times$ Length $\div 231(\mathrm{D} \times \mathrm{D} \times 0.7854 \times \mathrm{L} \div 231)$
The next step is to calculate the volume when dimensions are in feet form in order to determine the capacity in gallons of a tank. There are 7.48 gallons in 1 cubic foot, so you would multiply the volume of a tank by 7.48.

Tank capacity in gallons when measurements are in feet form $=$ Volume $\times 7.48$ Diameter $\times$ Diameter $\times$ $0.7854 \times$ Length $\times 7.48(\mathrm{D} \times \mathrm{D} \times 0.7854 \times \mathrm{L} \times 7.48)$

A problem you may have is remembering what formula to use correctly. Because you are rarely faced with a need to make such calculations on a job site, you must understand what the formulas represent. The formula 0.7854 is derived from the circular area of a square or $78-1 / 2 \%$ of a square. The remaining $21-1 / 2 \%$ is not relevant because we are calculating a circular area. Imagine placing a circle of equal diameter into a square of equal length (an 8 -foot diameter into an 8 -foot square).


When a circle's diameter is equal size of a square, the area of a circle consumes 0.7854 or $78-1 / 2 \%$ of a square. The remaining $21-1 / 2 \%$ is not used in cylindrical calculations.

By permission: Joyce Company, Inc., Cary, North Carolina

## Cylindrical Tank WorkSheet

## Work Sheet

Below is an illustration of a tank that can be viewed as horizontal or vertical. Its dimensions are in feet and inch form. This requires you to calculate all dimensions in inch form and convert to decimal form.


Formulas must relate to cubic inches because the physical dimensions of the tank are in feet and inch form.

- Tank dimensions are: Length $=12^{\prime}-8-5 / 8^{\prime \prime} \times$ Diameter $=4^{\prime}-2-1 / 8^{\prime \prime}$
- Converts to 152.625 inches $\times 50.125^{\prime \prime}$
- Formula is $\mathrm{D} \times \mathrm{D} \times 7854 \times \mathrm{L} \div 231=$ Gallon Capar

| Diameter | $\times$ | Diameter | $\times$ | Percent | $\times$ | Length | $=$ | Cub | $\div$ | Cu. In./1 Gal. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |$=$ Gallon Capacity

An exam answer in a multiple-choice format may have possible selections that indicate an exact answer remaining in its decimal form. If gallons were rounded to a nearest whole number, the answer would be 1304 gallons.

Use the following dimensions to calculate the gallon capacity of tanks listed below.

| Diameter | Length or Height | Gallon Capacity |
| :--- | :--- | :--- |
| $4^{\prime}-0^{\prime \prime}$ | $16^{\prime}-0^{\prime \prime}$ |  |
| $6^{\prime}-3^{\prime \prime}$ | $12^{\prime}-8^{\prime \prime}$ |  |
| $8^{\prime}-1-5 / 8^{\prime \prime}$ | $11^{\prime} 7-3 / 8^{\prime \prime}$ |  |
| $7^{\prime}-0^{\prime \prime}$ | $9^{\prime}-2^{\prime \prime}$ |  |
| $5^{\prime}-6-7 / 8^{\prime \prime}$ | $5^{\prime}-6-7 / 8^{\prime \prime}$ |  |

The next illustration can help you remember that there are 231 cubic inches in a gallon of water. Imagine a narrow structure that is $19-1 / 4^{\prime \prime}\left(19.25^{\prime \prime}\right)$ long, $1^{\prime \prime}$ wide, and $12^{\prime \prime}$ high. It would take 1 gallon of water to fill such a structure. Now picture freezing 231 ice cubes that are $1^{\prime \prime}$ square and $1^{\prime \prime}$ high: this is how many $1^{\prime \prime}$ cubed-shaped ice cubes you can freeze from a gallon of water. You would have 12 rows high of $1^{\prime \prime}$ square and $1^{\prime \prime}$ high ice cubes with 19-1/4 cubes per row.


To arrive at the dimensions of this structure, divide 231 cubic inches by the structure height (12). This structure changes dimensionally if you divide by any height. Imagine a structure that is only $1^{\prime \prime}$ high. You divide 231 by 1 and have a structure that is dimensioned to be $1^{\prime \prime}$ high by $1^{\prime \prime}$ wide and 231 inches long. So regardless of the size of a tank, pipe, or structure, there are 231 cubic inches in a gallon of water.

Another portion of a calculating formula that you have to know concerns when measurements are given in feet form. Remember 1 cubic foot holds 7.48 (7-1/2) gallons of water. This next illustration will help you remember how many gallons are in a cubic foot.


When using this formula for exam purposes and multiple-choice selections are present; you may calculate all gallons in the form of 7.5. If you are faced with answers that are close in numerical sequence or if exact answers are required, use the 7.48 formula, not 7.5 .

### 3.7.0 Round Tank Volume Tables for One Foot of Depth-1 to 32 Feet in Diameter

## Calculating the Volume of a Round Tank 1 Foot to 32 Feet in Diameter

Tank Volume
U.S. Gallons in Round Tanks for One Foot in Depth

| Diameter of Tank |  | No. U.S.Gals. | Cu Ft and Area in Sq Ft |
| :---: | :---: | :---: | :---: |
| Ft | In. |  |  |
| 1 | 0 | 5.87 | . 785 |
| 1 | 1 | 6.89 | . 922 |
| 1 | 2 | 8.00 | 1.069 |
| 1 | 3 | 9.18 | 1.227 |
| 1 | 4 | 10.44 | 1.396 |
| 1 | 5 | 11.79 | 1.576 |
| 1 | 6 | 13.22 | 1.767 |
| 1 | 7 | 14.73 | 1.969 |
| 1 | 8 | 16.32 | 2.182 |
| 1 | 9 | 17.99 | 2.405 |
| 1 | 10 | 19.75 | 2.640 |
| 1 | 11 | 21.58 | 2.885 |
| 2 | 0 | 23.50 | 3.142 |
| 2 | 1 | 25.50 | 3.409 |
| 2 | 2 | 27.58 | 3.687 |
| 2 | 3 | 29.74 | 3.976 |
| 2 | 4 | 31.99 | 4.276 |
| 2 | 5 | 34.31 | 4.587 |
| 2 | 6 | 36.72 | 4.909 |
| 2 | 7 | 39.21 | 5.241 |
| 2 | 8 | 41.78 | 5.585 |
| 2 | 9 | 44.43 | 5.940 |
| 2 | 10 | 47.16 | 6.305 |
| 2 | 11 | 49.98 | 6.681 |
| 3 | 0 | 52.88 | 7.069 |
| 3 | 1 | 55.86 | 7.467 |
| 3 | 2 | 58.92 | 7.876 |
| 3 | 3 | 62.06 | 8.296 |
| 3 | 4 | 65.28 | 8.727 |
| 3 | 5 | 68.58 | 9.168 |
| 3 | 6 | 71.97 | 9.621 |
| 3 | 7 | 75.44 | 10.085 |
| 3 | 8 | 78.99 | 10.559 |
| 3 | 9 | 82.62 | 11.045 |
| 3 | 10 | 86.33 | 11.541 |
| 3 | 11 | 90.13 | 12.048 |
| 4 | 0 | 94.00 | 12.566 |
| 4 | 1 | 97.96 | 13.095 |
| 4 | 2 | 102.00 | 13.635 |
| 4 | 3 | 106.12 | 14.186 |
| 4 | 4 | 110.32 | 14.748 |
| 4 | 5 | 114.61 | 15.321 |


| Tank Volume-Cont'd U.S. Gallons in Round Tanks for One Foot in Depth |  |  |  |
| :---: | :---: | :---: | :---: |
| Diameter of Tank |  |  |  |
| Ft | In. | No. U.S.Gals. | Cu Ft and Area in Sq Ft |
| 4 | 6 | 118.97 | 15.90 |
| 4 | 7 | 123.42 | 16.50 |
| 4 | 8 | 127.95 | 17.10 |
| 4 | 9 | 132.56 | 17.72 |
| 4 | 10 | 137.25 | 18.35 |
| 4 | 11 | 142.02 | 18.99 |
| 5 | 0 | 146.88 | 19.63 |
| 5 | 1 | 151.82 | 20.29 |
| 5 | 2 | 156.83 | 20.97 |
| 5 | 3 | 161.93 | 21.65 |
| 5 | 4 | 167.12 | 22.34 |
| 5 | 5 | 177.38 | 23.04 |
| 5 | 6 | 177.72 | 23.76 |
| 5 | 7 | 183.15 | 24.48 |
| 5 | 8 | 188.66 | 25.22 |
| 5 | 9 | 194.25 | 25.97 |
| 5 | 10 | 199.92 | 26.73 |
| 5 | 11 | 205.67 | 27.49 |
| 6 | 0 | 211.51 | 28.27 |
| 6 | 3 | 229.50 | 30.68 |
| 6 | 6 | 248.23 | 33.18 |
| 6 | 9 | 267.69 | 35.78 |
| 7 | 0 | 287.88 | 38.48 |
| 7 | 3 | 308.81 | 41.28 |
| 7 | 6 | 330.48 | 44.18 |
| 7 | 9 | 352.88 | 47.17 |
| 8 | 0 | 376.01 | 50.27 |
| 8 | 3 | 399.88 | 53.46 |
| 8 | 6 | 424.48 | 56.75 |
| 8 | 9 | 449.82 | 60.13 |
| 9 | 0 |  | 63.62 |
| 9 | 3 | 502.70 | 67.20 |
| 9 | 6 | 530.24 | 70.88 |
| 9 | 9 | 558.51 | 74.66 |
| 10 | 0 | 587.52 | 78.54 |
| 10 | 3 | 617.26 | 82.52 |
| 10 | 6 | 647.74 | 86.59 |
| 10 | 9 | 678.95 | 90.76 |
| 11 | 0 | 710.90 | 95.03 |
| 11 | 3 | 743.58 | 99.40 |
| 11 | 6 | 776.99 | 103.87 |
| 11 | 9 | 811.14 | 108.43 |

## Tank Volume-Cont'd

U.S. Gallons in Round Tanks for One Foot in Depth

| Diameter of Tank |  | No. U.S.Gals. | Cu Ft and Area in Sq Ft |
| :---: | :---: | :---: | :---: |
| Ft | In. |  |  |
| 12 | 0 | 846.03 | 113.10 |
| 12 | 3 | 881.65 | 117.86 |
| 12 | 6 | 918.00 | 122.72 |
| 12 | 9 | 955.09 | 127.68 |
| 13 | 0 | 201.06 | 132.73 |
| 13 | 3 | 1031.50 | 137.89 |
| 13 | 6 | 1070.80 | 143.14 |
| 13 | 9 | 1110.80 | 148.49 |
| 14 | 0 | 1151.50 | 153.94 |
| 14 | 3 | 1193.00 | 159.48 |
| 14 | 6 | 1235.30 | 165.13 |
| 14 | 9 | 1278.20 | 170.87 |
| 15 | 0 | 1321.90 | 176.71 |
| 15 | 3 | 1366.40 | 182.65 |
| 15 | 6 | 1411.50 | 188.69 |
| 15 | 9 | 1457.40 | 194.83 |
| 16 | 0 | 1504.10 | 201.06 |
| 16 | 3 | 1551.40 | 207.39 |
| 16 | 6 | 1599.50 | 213.82 |
| 16 | 9 | 1648.40 | 220.35 |
| 17 | 0 | 1697.90 | 226.98 |
| 17 | 3 | 1748.20 | 233.71 |
| 17 | 6 | 1799.30 | 240.53 |
| 17 | 9 | 1851.10 | 247.45 |
| 18 | 0 | 1903.60 | 254.47 |
| 18 | 3 | 1956.80 | 261.59 |
| 18 | 6 | 2010.80 | 268.80 |
| 18 | 9 | 2065.50 | 276.12 |
| 19 | 0 | 2120.90 | 283.53 |
| 19 | 3 | 2177.10 | 291.04 |
| 19 | 6 | 2234.00 | 298.65 |
| 19 | 9 | 2291.70 | 306.35 |
| 20 | 0 | 2350.10 | 314.16 |
| 20 | 3 | 2409.20 | 322.06 |
| 20 | 6 | 2469.10 | 330.06 |
| 20 | 9 | 2529.60 | 338.16 |
| 21 | 0 | 2591.00 | 346.36 |
| 21 | 3 | 2653.00 | 354.66 |
| 21 | 6 | 2715.80 | 363.05 |
| 21 | 9 | 2779.30 | 371.54 |
| 22 | 0 | 2843.60 | 380.13 |
| 22 | 3 | 2908.60 | 388.82 |
| 22 | 6 | 2974.30 | 397.61 |
| 22 | 9 | 3040.80 | 406.49 |

## Tank Volume-Cont'd <br> U.S. Gallons in Round Tanks for One Foot in Depth

| Diameter of Tank |  | No. U.S.Gals. | Cu Ft and Area in Sq Ft |
| :---: | :---: | :---: | :---: |
| Ft | In. |  |  |
| 23 | 0 | 3108.00 | 415.48 |
| 23 | 3 | 3175.90 | 424.56 |
| 23 | 6 | 3244.60 | 433.74 |
| 23 | 9 | 3314.00 | 443.01 |
| 24 | 0 | 3384.10 | 452.39 |
| 24 | 3 | 3455.00 | 461.86 |
| 24 | 6 | 3526.60 | 471.44 |
| 24 | 9 | 3598.90 | 481.11 |
| 25 | 0 | 3672.00 | 490.87 |
| 25 | 3 | 3745.80 | 500.74 |
| 25 | 6 | 3820.30 | 510.71 |
| 25 | 9 | 3895.60 | 520.77 |
| 26 | 0 | 3971.60 | 530.93 |
| 26 | 3 | 4048.40 | 541.19 |
| 26 | 6 | 4125.90 | 551.55 |
| 26 | 9 | 4204.10 | 562.00 |
| 27 | 0 | 4283.00 | 572.56 |
| 27 | 3 | 4362.70 | 583.21 |
| 27 | 6 | 4443.10 | 593.96 |
| 27 | 9 | 4524.30 | 604.81 |
| 28 | 0 | 4606.20 | 615.75 |
| 28 | 3 | 4688.80 | 626.80 |
| 28 | 6 | 4772.10 | 637.94 |
| 28 | 9 | 4856.20 | 649.18 |
| 29 | 0 | 4941.00 | 660.52 |
| 29 | 3 | 5026.60 | 671.96 |
| 29 | 6 | 5112.90 | 683.49 |
| 29 | 9 | 5199.90 | 695.13 |
| 30 | 0 | 5287.70 | 706.86 |
| 30 | 3 | 5376.20 | 718.69 |
| 30 | 6 | 5465.40 | 730.62 |
| 30 | 9 | 5555.40 | 742.64 |
| 31 | 0 | 5646.10 | 754.77 |
| 31 | 3 | 5737.50 | 766.99 |
| 31 | 6 | 5829.70 | 779.31 |
| 31 | 9 | 5922.60 | 791.73 |
| 32 | 0 | 6016.20 | 804.25 |
| 32 | 3 | 6110.60 | 816.86 |
| 32 | 6 | 6205.70 | 829.58 |
| 32 | 9 | 6301.50 | 842.39 |

31-1/2 Gallons equal 1 Barrel
To find the capacity of tanks greater than the largest given in the table, look in the table for a tank of one-half of the given size and multiply its capacity by 4 , or one of one-third its size and multiply its capacity by 9 , etc. Source: Cardinal Metal, Inc., Irving, TX

### 3.8.0 Capacity of Round Tanks-1 to $\mathbf{3 0}$ Feet in Diameter

| Diameter | gal/ft* |
| :--- | ---: |
| $1^{\prime}-0^{\prime \prime}$ | 5.88 |
| $1^{\prime}-3^{\prime \prime}$ | 9.18 |
| $1^{\prime}-6^{\prime \prime}$ | 13.22 |
| $1^{\prime}-9^{\prime \prime}$ | 17.99 |
| $2^{\prime}-0^{\prime \prime}$ | 23.50 |
| $2^{\prime}-3^{\prime \prime}$ | 29.75 |
| $2^{\prime}-6^{\prime \prime}$ | 36.72 |
| $2^{\prime}-9^{\prime \prime}$ | 44.43 |
| $3^{\prime}-0^{\prime \prime}$ | 52.88 |
| $3^{\prime}-6^{\prime \prime}$ | 71.98 |
| $4^{\prime}-0^{\prime \prime}$ | 94.01 |
| $4^{\prime}-6^{\prime \prime}$ | 118.98 |
| $5^{\prime}$ | 146.89 |
| $6^{\prime}$ | 211.52 |
| $7^{\prime}$ | 287.90 |
| $8^{\prime}$ | 376.04 |
| $9^{\prime}$ | 475.92 |
| $10^{\prime}$ | 587.56 |
| $11^{\prime}$ | 710.94 |
| $12^{\prime}$ | 846.08 |
| $15^{\prime}$ | 1322.00 |
| $20^{\prime}$ | 2350.23 |
| $25^{\prime}$ | 3672.23 |
| $30^{\prime}$ | 5288.01 |
| Capacity of Round Tanks by Diameter \& Gallons per Foot |  |
| Basis: Inside Measure, 1 cubic foot $=7.481$ gallons $\bullet$ Table Gives Gallons Per Foot of |  |
| Height |  |

### 3.9.0 Capacity of Rectangular Tanks-1 to 6 Feet in Depth, 1 to 10 Feet in Length

Capacity of Rectangular Tanks

| Length | $1^{\prime}-0^{\prime \prime}$ | $1^{\prime}-6^{\prime \prime}$ | $2^{\prime}-0^{\prime \prime}$ | $2^{\prime}-6^{\prime \prime}$ | $3^{\prime}-0^{\prime \prime}$ | $4^{\prime}-0^{\prime \prime}$ | $5^{\prime}-0^{\prime \prime}$ | $6^{\prime}-0^{\prime \prime}$ | $7^{\prime}-0^{\prime \prime}$ | $8^{\prime}-0^{\prime \prime}$ | $9^{\prime}-0^{\prime \prime}$ | $10^{\prime}-0^{\prime \prime}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $1^{\prime}-0^{\prime \prime}$ | 7.48 | 11.22 | 14.96 | 18.70 | 22.44 | 29.92 | 37.41 | 44.89 | 52.37 | 59.85 | 67.33 | 74.81 |
| $1^{\prime}-3^{\prime \prime}$ | 9.35 | 14.03 | 18.70 | 23.38 | 28.05 | 37.41 | 46.76 | 56.11 | 65.46 | 74.81 | 84.16 | 93.51 |
| $1^{\prime}-6{ }^{\prime \prime}$ | 11.22 | 16.83 | 22.44 | 28.05 | 33.66 | 44.89 | 56.11 | 67.33 | 78.55 | 89.77 | 100.99 | 112.22 |
| $1^{\prime}-9^{\prime \prime}$ | 13.09 | 19.64 | 26.18 | 32.73 | 39.28 | 52.37 | 65.46 | 78.55 | 91.64 | 104.73 | 117.83 | 130.92 |
| $2^{\prime}-0^{\prime \prime}$ | 14.96 | 22.44 | 29.92 | 37.41 | 44.89 | 59.85 | 74.81 | 89.77 | 104.73 | 119.70 | 134.66 | 149.62 |
| $2^{\prime}-3^{\prime \prime}$ | 16.83 | 25.25 | 33.66 | 42.08 | 50.50 | 67.33 | 84.16 | 100.99 | 117.83 | 134.66 | 151.49 | 168.32 |
| $2^{\prime}-6^{\prime \prime}$ | 18.70 | 28.05 | 37.41 | 46.76 | 56.11 | 74.81 | 93.51 | 112.22 | 130.92 | 149.62 | 168.32 | 187.03 |
| $2^{\prime}-9^{\prime \prime}$ | 20.57 | 30.86 | 41.15 | 51.43 | 61.72 | 82.29 | 102.86 | 123.44 | 144.01 | 164.58 | 185.15 | 205.73 |
| $3^{\prime}-0^{\prime \prime}$ | 22.44 | 33.66 | 44.89 | 56.11 | 67.33 | 89.77 | 112.22 | 134.66 | 157.10 | 179.54 | 201.99 | 224.43 |
| $3^{\prime}-6^{\prime \prime}$ | 26.18 | 39.28 | 52.37 | 65.46 | 78.55 | 104.73 | 130.92 | 157.10 | 183.28 | 209.47 | 235.65 | 261.84 |

Capacity of Rectangular Tanks-Cont'd

| Length | $1^{\prime}-0^{\prime \prime}$ | $1^{\prime}-6^{\prime \prime}$ | $2^{\prime}-0^{\prime \prime}$ | $2^{\prime}-6^{\prime \prime}$ | $3^{\prime}-0^{\prime \prime}$ | $4^{\prime}-0^{\prime \prime}$ | $5^{\prime}-0^{\prime \prime}$ | $6^{\prime}-0^{\prime \prime}$ | $7^{\prime}-0^{\prime \prime}$ | $8^{\prime}-0^{\prime \prime}$ | $9^{\prime}-0^{\prime \prime}$ | $10^{\prime}-0^{\prime \prime}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $4^{\prime}-0^{\prime \prime}$ | 29.92 | 44.89 | 59.85 | 74.81 | 89.77 | 119.70 | 149.62 | 179.54 | 209.47 | 239.39 | 269.32 | 299.24 |
| $4^{\prime}-6{ }^{\prime \prime}$ | 33.66 | 50.50 | 67.33 | 84.16 | 100.99 | 134.66 | 168.32 | 201.99 | 235.65 | 269.32 | 302.98 | 336.65 |
| $5^{\prime}-0^{\prime \prime}$ | 37.41 | 56.11 | 74.81 | 93.51 | 112.22 | 149.62 | 187.03 | 224.43 | 261.84 | 299.24 | 336.65 | 374.05 |
| $6^{\prime}-0^{\prime \prime}$ | 44.89 | 67.33 | 89.77 | 112.22 | 134.66 | 179.54 | 224.43 | 269.32 | 314.20 | 359.09 | 403.97 | 448.86 |

Basis: Inside Measure, 1 cubic foot $=7.481$ gallons (U.S.) $\bullet$ Table Gives Gallons Per Foot of Depth

### 3.10.0 Testing for Hardness in Metal—Mohs, Brinell, Rockwell, Scleroscope, Durometer

## Metals

Metals account for about two-thirds of all the elements and about $24 \%$ of the mass of the planet. Metals have useful properties, including strength, ductility, high melting points, thermal and electrical conductivity, and toughness. As shown in the periodic table, a large number of the elements are classified as being a metal. A few of the common metals and their typical uses are presented below.

## Common Metallic Materials

- Iron/Steel—Steel alloys are used for strength-critical applications.
- Aluminum-Aluminum and its alloys are used because they are easy to form, readily available, inexpensive, and recyclable.
- Copper-Copper and copper alloys have a number of properties that make them useful, including high electrical and thermal conductivity, high ductility, and good corrosion resistance.
- Titanium-Titanium alloys are used for strength in higher temperature ( $\sim 1000^{\circ} \mathrm{F}$ ) applications, when component weight is a concern, or when good corrosion resistance is required.
- Nickel-Nickel alloys are used for still higher temperatures ( $\sim 1500-2000^{\circ} \mathrm{F}$ ) applications or when good corrosion resistance is required.
- Refractory materials are used for the highest temperature ( $>2000^{\circ} \mathrm{F}$ ) applications.



## Hardness

Hardness is the resistance of a material to localized deformation. The term can apply to deformation from indentation, scratching, cutting, or bending. In metals, ceramics, and most polymers, the deformation considered is plastic deformation of the surface. For elastomers and some polymers, hardness is defined as the resistance to elastic deformation of the surface. The lack of a fundamental definition indicates that hardness is not a basic property of a material, but rather a composite one with contributions from yield strength, work hardening, true tensile strength, modulus, and others factors. Hardness measurements are widely used for the quality control of materials because they are quick and are considered to be nondestructive tests when the marks or indentations produced by the test are in low-stress areas.

A large variety of methods are used for determining the hardness of a substance. A few of the more common methods are introduced next.

## Mohs Hardness Test

One of the oldest ways of measuring hardness was devised by the German mineralogist Friedrich Mohs in 1812. The Mohs hardness test involves observing whether the surface of a material is scratched by a substance of known or defined hardness. To give numerical values to this physical property, minerals are ranked along the Mohs scale, which is composed of 10 minerals that have been given arbitrary hardness values. The Mohs hardness test, while greatly facilitating the identification of minerals in the field, is not suitable for accurately gauging the hardness of industrial materials such as steel or ceramics. For engineering materials, a variety of instruments have been developed over the years to provide a precise measure of hardness. Many apply a load and measure the depth or size of the resulting indentation. Hardness can be measured on the macro-, micro- or nanoscale.

## Brinell Hardness Test

The oldest of the hardness test methods in common use on engineering materials today is the Brinell hardness test. Dr. J. A. Brinell invented this test in Sweden in 1900. The Brinell test uses a desktop machine to apply a specified load to a hardened sphere of a specified diameter. The Brinell hardness number, or simply the Brinell number, is obtained by dividing the load used, in kilograms, by the measured surface area of the indentation, in square millimeters, left on the test surface. The Brinell test is frequently used to determine the hardness metal forgings and castings that have large grain structures. The Brinell test provides a measurement over a fairly large area that is less affected by the course grain structure of these materials than are Rockwell or Vickers tests.

A wide range of materials can be tested using a Brinell test simply by varying the test load and indenter ball size. In the United States, Brinell testing is typically done on iron and steel castings using a 3000 kg test force and a 10 mm diameter ball. A 1500 kg load is usually used for aluminum castings. Copper, brass, and thin stock are frequently tested using a 500 kg test force and a 10 or 5 mm ball. In Europe Brinell testing is done using a much wider range of forces and ball sizes; it is common to perform Brinell tests on small parts using a 1 mm carbide ball and a test force as low as 1 kg . These low-load tests are commonly referred to as baby Brinell tests. The test conditions should be reported along with the Brinell hardness number. A value reported as $60 \mathrm{HB} 10 / 1500 / 30$ means that a Brinell hardness of 60 was obtained using a 10 mm diameter ball with a 1500 kg load applied for 30 seconds.

## Rockwell Hardness Test

The Rockwell hardness test also uses a machine to apply a specific load and then measure the depth of the resulting impression. The indenter may either be a steel ball of some specified diameter or a spherical diamond-tipped cone of $120^{\circ}$ angle and 0.2 mm tip radius, called a brale. A minor load of 10 kg is first applied, which causes a small initial penetration to seat the indenter and remove the effects of any surface irregularities. Then, the dial is
set to zero and the major load is applied. Upon removal of the major load, the depth reading is taken while the minor load is still on. The hardness number may then be read directly from the scale. The indenter and the test load used determine the hardness scale that is used (A, B, C, etc).

For soft materials such as copper alloys, soft steel, and aluminum alloys, a $1 / 16^{\prime \prime}$ diameter steel ball is used with a 100 kg load, and the hardness is read on the B scale. In testing harder materials, hard cast iron, and many steel alloys, a 120 degrees diamond cone is used with up to a 150 kg load, and the hardness is read on the C scale. There are several Rockwell scales other than the B and C scales (which are called the common scales). A properly reported Rockwell value will have the hardness number followed by HR (Hardness Rockwell) and the scale letter. For example, 50 HRB indicates that the material has a hardness reading of 50 on the B scale.

A -Cemented carbides, thin steel, and shallow case-hardened steel
B -Copper alloys, soft steels, aluminum alloys, malleable iron, etc.
C-Steel, hard cast irons, pearlitic malleable iron, titanium, deep case-hardened steel and other materials harder than B 100
D -Thin steel and medium case-hardened steel and pearlitic malleable iron
E -Cast iron, aluminum and magnesium alloys, bearing metals
F -Annealed copper alloys, thin soft sheet metals
G -Phosphor bronze, beryllium copper, malleable irons
H -Aluminum, zinc, lead
K, L, M, P, R, S, V—Bearing metals and other very soft or thin materials, including plastics.

## Rockwell Superficial Hardness Test

The Rockwell Superficial Hardness Tester is used to test thin materials, lightly carburized steel surfaces, or parts that might bend or crush under the conditions of the regular test. This tester uses the same indenters as the standard Rockwell tester, but the loads are reduced. A minor load of 3 kg is used, and the major load is either 15 or 45 kg depending on the indenter used. Using the $1 / 16^{\prime \prime}$ diameter, steel ball indenter, a " T " is added (meaning thin sheet testing) to the superficial hardness designation. An example of a superficial Rockwell hardness is 23 HR15T, which indicates the superficial hardness as 23 , with a load of 15 kg using the steel ball.

## Vickers and Knoop Microhardness Tests

The Vickers and Knoop hardness tests are a modification of the Brinell test and are used to measure the hardness of thin film coatings or the surface hardness of case-hardened parts. With these tests, a small diamond pyramid is pressed into the sample under loads that are much less than those used in the Brinell test. The difference between the Vickers and the Knoop Tests is simply the shape of the diamond pyramid indenter. The Vickers test uses a square pyramidal indenter, which is prone to crack brittle materials. Consequently, the Knoop test using a rhombic-based (diagonal ratio 7.114:1) pyramidal indenter was developed that produces longer but shallower indentations. For the same load, Knoop indentations are about 2.8 times longer than Vickers indentations.

An applied load ranging from 10 g to 1000 g is used. This low amount of load creates a small indent that must be measured under a microscope. The measurements for hard coatings like TiN must be taken at very high magnification (i.e., 1000X) because the indents are so small. The surface usually needs to be polished. The diagonals of the impression are measured, and these values are used to obtain a hardness number (VHN), usually from a lookup table or chart. The Vickers test can be used to characterize very hard materials, but the hardness is measured over a very small region.

The values are expressed like 2500 HK25 (or HV25), meaning 2500 Hardness Knoop at 25 gram force load. The Knoop and Vickers hardness values differ slightly, but for hard coatings, the values are close enough to be within the measurement error and can be used interchangeably.

## Scleroscope and Rebound Hardness Tests

The Scleroscope test is a very old test that involves dropping a diamond-tipped hammer, which falls inside a glass tube under the force of its own weight from a fixed height, onto the test specimen. The height of the rebound travel of the hammer is measured on a graduated scale. The scale of the rebound is arbitrarily chosen and consists of Shore units, divided into 100 parts, which represent the average rebound from pure hardened highcarbon steel. The scale is continued higher than 100 to include metals having greater hardness. The Shore Scleroscope measures hardness in terms of the elasticity of the material, and the hardness number depends on the height to which the hammer rebounds: the harder the material, the higher the rebound.

The Rebound hardness test method is a recent advancement that builds on the Scleroscope test. A variety of electronic instruments are on the market that measure the loss of energy of the impact body. These instruments typically use a spring to accelerate a spherical, tungsten carbide-tipped mass toward the surface of the test object. When the mass contacts the surface, it has a specific kinetic energy, and the impact produces an indentation (plastic deformation) on the surface that takes some of this energy from the impact body. The impact body will lose more energy, and its rebound velocity will be less when a larger indentation is produced on softer material. The velocities of the impact body before and after impact are measured, and the loss of velocity is related to Brinell, Rockwell, or other common hardness values.

## Durometer Hardness Test

A Durometer is an instrument that is commonly used for measuring the indentation hardness of rubbers/elastomers, and soft plastics such as polyolefin, fluoropolymer, and vinyl. A Durometer simply uses a calibrated spring to apply a specific pressure to an indenter foot. The indenter foot can be either cone- or sphere-shaped. An indicating device measures the depth of indentation. Durometers are available in a variety of models, and the most popular testers are the Model A used for measuring softer materials and the Model D for harder materials.

## Barcol Hardness Test

The Barcol hardness test obtains a hardness value by measuring the penetration of a sharp steel point under a spring load. The specimen is placed under the indenter of the Barcol hardness tester, and uniform pressure is applied until the dial indication reaches a maximum. The Barcol hardness test method is used to determine the hardness of both reinforced and nonreinforced rigid plastics, as well as to determine the degree of cure of resins and plastics.

### 3.11.0 Comparison of Hardness Scales Shown Graphically

## Approximate Comparison of Hardness Scales



| Vickers <br> Hardness <br> (DPH) | Brinell Hardness 10mm Diam Ball Load: 3000 kg |  |  | Rockwell Hardness (2) |  |  |  | Rockwell Special Hardness Special Brale Equipment |  |  | Shore <br> Hardness | Tensile <br> Strength <br> (100sq/ $\mathrm{m}^{2}$ <br> approx.) | Vickers <br> Hardness <br> Load: <br> 50 kg |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Std <br> Ball | Hultgren Ball | Tungstic Carbide Ball | A Scale <br> Load: 60 <br> kg Brale Equip. | B Scale <br> Load: 100 <br> kg 1/16in. <br> Diam Ball | C Scale <br> Load: <br> 150 kg <br> Brale <br> Equip. | D Scale <br> Load 100 <br> kg Brale Equip. | 15-N <br> Scale <br> Load: <br> 15 kg | $30-\mathrm{N}$ <br> Scale <br> Load: <br> 30 kg | 45-N <br> Scale <br> Load: <br> 45 kg |  |  |  |
| 410 | 388 | 388 | 388 | 71.4 | - | 41.8 | 56.8 | 81.4 | 61.1 | 45.3 | - | 195 | 410 |
| 400 | 379 | 379 | 379 | 70.8 | - | 40.8 | 56.0 | 81.0 | 60.2 | 44.1 | 55 | 190 | 400 |
| 390 | 369 | 369 | 369 | 70.3 | - | 39.8 | 55.2 | 80.3 | 59.3 | 42.9 | - | 185 | 390 |
| 380 | 360 | 360 | 360 | 69.8 | (110.0) | 38.8 | 54.4 | 79.8 | 58.4 | 41.7 | 52 | 180 | 380 |
| 370 | 350 | 350 | 350 | 69.2 | - | 37.7 | 53.6 | 79.2 | 57.4 | 40.4 | - | 175 | 370 |
| 360 | 341 | 341 | 341 | 68.7 | (109.0) | 36.6 | 52.8 | 78.6 | 56.4 | 39.1 | 50 | 170 | 360 |
| 350 | 331 | 331 | 331 | 68.1 | - | 35.5 | 51.9 | 78.0 | 55.4 | 37.8 | - | 166 | 350 |
| 340 | 322 | 322 | 322 | 67.6 | (108.0) | 34.4 | 51.1 | 77.4 | 54.4 | 36.5 | 47 | 161 | 340 |
| 330 | 313 | 313 | 313 | 67.0 | - | 33.3 | 50.2 | 76.8 | 53.6 | 35.2 | - | 156 | 330 |
| 320 | 303 | 303 | 303 | 66.4 | (107.0) | 32.2 | 49.4 | 76.2 | 52.3 | 33.9 | 45 | 151 | 320 |
| 310 | 294 |  | 294 | 65.8 | - | 31.0 | 48.4 | 75.6 | 51.3 | 32.5 | - | 146 | 310 |
| 300 | 284 | 284 | 284 | 65.2 | (105.5) | 29.8 | 47.5 | 74.9 | 50.2 | 31.1 | 42 | 141 | 300 |
| 295 | 280 |  | 280 | 64.8 | - | 29.2 | 47.1 | 74.6 | 49.7 | 30.4 | - | 139 | 295 |
| 290 | 275 | 275 | 275 | 64.5 | (104.5) | 28.5 | 46.5 | 74.2 | 49.0 | 29.5 | 41 | 136 | 290 |
| 285 | 270 | 270 | 270 | 64.2 | - | 27.8 | 46.0 | 73.8 | 48.4 | 28.7 | - | 134 | 285 |
| 280 | 265 | 265 | 265 | 63.8 | (103.5) | 27.1 | 45.3 | 73.4 | 47.8 | 27.9 | 40 | 131 | 280 |
| 275 | 261 | 261 | 261 | 63.5 | - | 26.4 | 44.9 | 73.0 | 47.2 | 27.1 | - | 129 | 275 |
| 270 | 256 | 256 | 256 | 63.1 | (102.0) | 25.4 | 44.3 | 72.6 | 46.4 | 26.2 | 38 | 126 | 270 |
| 265 | 252 | 252 | 252 | 62.7 | - | 24.8 | 43.7 | 72.1 | 45.7 | 25.2 | - | 124 | 265 |


| 260 | 247 | 247 | 247 | 62.4 | (101.0) | 24.0 | 43.1 | 71.6 | 45.0 | 24.3 | 37 | 121 | 260 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 255 | 243 | 243 | 243 | 62.0 | - | 23.1 | 42.2 | 71.1 | 44.2 | 23.2 | - | 119 | 255 |
| 250 | 238 | 238 | 238 | 61.6 | 99.5 | 22.2 | 41.7 | 70.6 | 43.4 | 22.2 | 36 | 116 | 250 |
| 245 | 233 | 233 | 233 | 61.2 | - | 21.3 | 41.1 | 70.1 | 42.5 | 21.1 | - | 114 | 245 |
| 240 | 228 | 228 | 228 | 60.7 | 98.1 | 20.3 | 40.3 | 69.6 | 41.7 | 19.9 | 34 | 11 | 240 |
| 230 | 219 | 219 | 219 | - | 96.7 | (18.0) | - | - | - |  | 33 | 106 | 230 |
| 220 | 209 | 209 | 209 | - | 95.0 | (15.7) | - | - | - | - | 32 | 101 | 220 |
| 210 | 200 | 200 | 200 | - | 93.4 | (13.4) | - | - | - | - | 30 | 97 | 210 |
| 200 | 190 | 190 | 190 | - | 91.5 | (11.0) | - | - | - | - | 29 | 92 | 200 |
| 190 | 181 | 181 | 181 | - | 89.4 | (8.5) | - | - | - | - | 28 | 88 | 190 |
| 180 | 171 | 171 | 171 | - | 87.1 | (6.0) | - | - | - | - | 26 | 84 | 180 |
| 170 | 162 | 162 | 162 | - | 85.0 | (3.0) | - | - | - | - | 25 | 79 | 170 |
| 160 | 152 | 152 | 152 | - | 81.7 | 0.0) | - | - | - | - | 24 | 75 | 160 |
| 150 | 143 | 143 | 143 | - | 78.7 | - | - | - | - | - | 22 | 71 | 150 |
| 140 | 133 | 133 | 133 | - | 75.0 | - | - | - | - | - | 21 | 66 | 140 |
| 130 | 124 | 124 | 124 | - | 71.2 | - | - | - | - | - | 20 | 62 | 130 |
| 120 | 114 | 114 | 114 | - | 66.7 | - | - | - | - | - |  | 57 | 120 |
| 110 | 105 | 105 | 105 | - | 62.3 | - | - | - | - | - | - | - | 110 |
| 100 | 95 | 95 | 95 | - | 56.2 | - | - | - | - | - | - | - | 100 |
| 95 | 90 | 90 | 90 | - | 52.0 | - | - | - | - | - | - | - | 95 |
| 90 | 86 | 86 | 86 | - | 48.0 | - | - | - | - | - | - | - | 90 |
| 85 | 81 | 81 | 81 | - | 41.0 | - | - | - | - | - | - | - | 85 |

## Section 4

## Site Work

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### 4.0.0 The Rudiments of Excavation-Classification, Use of Materials, Measurement, and Payment

## 1. SCOPE

The work shall consist of the excavation required by the drawings and specifications and disposal of the excavated materials.

## 2. CLASSIFICATION

Excavation will be classified as common excavation or rock excavation in accordance with the following definitions or will be designated as unclassified.

Common excavation shall be defined as the excavation of all materials that can be excavated, transported, and unloaded by the use of heavy ripping equipment and wheel tractor-scrapers with pusher tractors or that can be excavated and dumped into place or loaded onto hauling equipment by means of excavators having a rated capacity of one cubic yard or larger and equipped with attachments (such as shovel, bucket, backhoe, dragline or clam shell) appropriate to the material type, character, and nature of the materials.

Rock excavation shall be defined as the excavation of all hard, compacted or cemented materials that require blasting or the use of ripping and excavating equipment larger than defined for common excavation. The excavation and removal of isolated boulders or rock fragments larger than one (1) cubic yard encountered in materials otherwise conforming to the definition of common excavation shall be classified as rock excavation. The presence of isolated boulders or rock fragments larger than one (1) cubic yard will not in itself be sufficient cause to change the classification of the surrounding material.

For the purpose of these classifications, the following definitions shall apply:
Heavy ripping equipment shall be defined as a rear-mounted, heavy duty, single-tooth, ripping attachment mounted on a track type tractor having a power rating of at least 250 flywheel horsepower unless otherwise specified in Section 11.

Wheel tractor-scraper shall be defined as a self-loading (not elevating) and unloading scraper having a struck bowl capacity of at least twelve (12) cubic yards.

Pusher tractor shall be defined as a track type tractor having a power rating of at least 250 flywheel horsepower equipped with appropriate attachments.

## 3. UNCLASSIFIED EXCAVATION

Excavation designated as "Unclassified Excavation" shall include all materials encountered regardless of their nature or the manner in which they are removed. When excavation is unclassified, none of the definitions or classifications stated in Section 2, CLASSIFICATION, shall apply.

## 4. BLASTING

The transportation, handling, storage, and use of dynamite and other explosives shall be directed and supervised by person(s) of proven experience and ability who are authorized and qualified to conduct blasting operations.

Source: U.S. Department of Agriculture-Natural Resources Conservation Service.

Blasting shall be done in such a manner as to prevent damage to the work or unnecessary fracturing of the foundation and shall conform to any special requirements in Section 11 of this specification. When specified in Section 11, the Contractor shall furnish the Engineer in writing, a blasting plan prior to blasting operations.

## 5. USE OF EXCAVATED MATERIALS

Method 1 To the extent they are needed, all suitable materials from the specified excavations shall be used in the construction of required permanent earthfill or rockfill. The suitability of materials for specific purposes will be determined by the Engineer. The Contractor shall not waste or otherwise dispose of suitable excavated materials.

Method 2 Suitable materials from the specified excavations may be used in the construction of required earthfill or rockfill. The suitability of materials for specific purposes will be determined by the Engineer.

## 6. DISPOSAL OF WASTE MATERIALS

Method 1 All surplus or unsuitable excavated materials will be designated as waste and shall be disposed of at the locations shown on the drawings.

Method 2 All surplus or unsuitable excavated materials will be designated as waste and shall be disposed of by the Contractor at sites of his own choosing away from the site of the work in an environmental acceptable manner and that does not violate local rules and regulations.

## 7. EXCAVATION LIMITS

Excavations shall comply with OSHA Construction Industry Standards (29CFR Part 1926) Subpart P, Excavations, Trenching, and Shoring. All excavations shall be completed and maintained in a safe and stable condition throughout the total construction phase. Structure and trench excavations shall be completed to the specified elevations and to the length and width required to safely install, adjust, and remove any forms, bracing, or supports necessary for the installation of the work. Excavations outside of the lines and limits shown on the drawings or specified herein required to meet safety requirements shall be the responsibility of the Contractor in constructing and maintaining a safe and stable excavation.

## 8. BORROW EXCAVATION

When the quantities of suitable materials obtained from specified excavations are insufficient to construct the specified earthfills and earth backfills, additional materials shall be obtained from the designated borrow areas. The extent and depth of borrow pits within the limits of the designated borrow areas shall be as specified in Section 11 or as approved by the Engineer.

Borrow pits shall be excavated and finally dressed to blend with the existing topography and sloped to prevent ponding and to provide drainage.

## 9. OVER-EXCAVATION

Excavation in rock beyond the specified lines and grades shall be corrected by filling the resulting voids with portland cement concrete made of materials and mix proportions approved by the Engineer. Concrete that will be exposed to the atmosphere when construction is completed shall meet the requirements of concrete selected for use under Construction Specification 31, Concrete for Major Structures, or 32, Structure Concrete, as appropriate.

Concrete that will be permanently covered shall contain not less than five (5) bags of cement per cubic yard. The concrete shall be placed and cured as specified by the Engineer.

Excavation in earth beyond the specified lines and grades shall be corrected by filling the resulting voids with approved compacted earthfill, except that, if the earth is to become the subgrade for riprap, rockfill, sand or gravel bedding, or drainfill, the voids may be filled with material conforming to the specifications for the riprap,
rockfill, bedding or drainfill. Prior to correcting an over-excavation condition, the Contractor shall review the planned corrective action with the Engineer and obtain approval of the corrective measures.

## 10. MEASUREMENT AND PAYMENT

For items of work for which specific unit prices are established in the contract, the volume of each type and class of excavation within the specified pay limits will be measured and computed to the nearest cubic yard by the method of average cross-sectional end areas or by methods outlined in Section 11 of this specification. Regardless of quantities excavated, the measurement for payment will be made to the specified pay limits, except that excavation outside the specified lines and grades directed by the Engineer to remove unsuitable material will be included. Excavation required because unsuitable conditions result from the Contractor's improper construction operations, as determined by the Engineer, will not be included for measurement and payment.

Method 1 The pay limits shall be as designated on the drawings.
Method 2 The pay limits shall be defined as follows:
a. The upper limit shall be the original ground surface as it existed prior to the start of construction operations except that where excavation is performed within areas designated for previous excavation or earthfill the upper limit shall be the modified ground surface resulting from the specified previous excavation or earthfill.
b. The lower and lateral limits shall be the neat lines and grades shown on the drawings.

Method 3 The pay limits shall be defined as follows:
a. The upper limit shall be the original ground surface as it existed prior to the start of construction operations except that where excavation is performed within areas designated for previous excavation or earthfill the upper limit shall be the modified ground surface resulting from the specified previous excavation or earthfill.
b. The lower and lateral limits shall be the true surface of the completed excavation as directed by the Engineer.
Method 4 The pay limits shall be defined as follows:
a. The upper limit shall be the original ground surface as it existed prior to the start of construction operations except that where excavation is performed within areas designated for previous excavation or earthfill the upper limit shall be the modified ground surface resulting from the specified previous excavation or earthfill.
b. The lower limit shall be at the bottom surface of the proposed structure.
c. The lateral limits shall be 18 -inches outside of the outside surfaces of the proposed structure or shall be vertical planes 18 -inches outside of and parallel to the footings, whichever gives the larger pay quantity, except as provided in d, below.
d. For trapezoidal channel linings or similar structures that are to be supported upon the sides of the excavation without intervening forms, the lateral limits shall be at the under side of the proposed lining or structure.
e. For the purposes of the definitions in b , c , and d, above, any specified bedding or drainfill directly beneath or beside the structure will be considered to be a part of the structure.

All Methods The following provisions apply to all methods of measurement and payment.
Payment for each type and class of excavation will be made at the contract unit price for that type and class of excavation. Such payment will constitute full compensation for all labor, materials, equipment, and all other items necessary and incidental to the performance of the work, except that extra payment for backfilling over-excavation will be made in accordance with the following provisions:

Payment for backfilling over-excavation, as specified in Section 9 of this specification, will be made only if the excavation outside specified lines and grades is directed by the Engineer to remove unsuitable material and if
the unsuitable condition is not a result of the Contractor's improper construction operations as determined by the Engineer.

Compensation for any item of work described in the contract but not listed in the bid schedule will be included in the payment for the item of work to which it is made subsidiary. Such items and the items to which they are made subsidiary are identified in Section 11 of this specification.

### 4.1.0 The Unified Soil Classification and Constituents-Explained

## Uniform Field Soil Classification System

## (Modified Unified Description)

## Introduction April 6, 2009

The purpose of this system is to establish guidelines for the uniform classification of soils by inspection for MDOT Soils Engineers and Technicians. It is the intent of this system to describe only the soil constituents that have a significant influence on the visual appearance and engineering behavior of the soil. This system is intended to provide the best word description of the sample to those involved in the planning, design, construction, and maintenance processes. A method is presented for preparing a "word picture" of a sample for entering on a subsurface exploration log or other appropriate data sheet. The classification procedure involves visually and manually examining soil samples with respect to texture (grain-size), plasticity, color, structure, and moisture. In addition to classification, this system provides guidelines for assessment of soil strength (relative density for granular soils, consistency for cohesive soils), which may be included with the field classification as appropriate for engineering requirements. A glossary of terms is included at the end of this document for convenient reference.

It should be understood that the soil descriptions are based upon the judgment of the individual making the description. Laboratory classification tests are not intended to be used to verify the description, but to further determine the engineering behavior for geotechnical design and analysis, and for construction.

## Primary Soil Constituents

The primary soil constituent is defined as the material fraction which has the greatest impact on the engineering behavior of the soil, and which usually represents the soil type found in the largest percentage. To determine the primary constituent, it must first be determined whether the soil is "Fine-Grained" or "Coarse-Grained" or "Organic" as defined below. The field soil classification "word picture" will be built around the primary constituent as defined by the soil types described below.

Coarse-Grained Soils: More than $50 \%$ of the soil is RETAINED on the $(0.075 \mathrm{~mm}) \# 200$ sieve. A good rule of thumb to determine if particles will be retained or pass the \#200 sieve: If individual particles can be distinguished by the naked eye, then they will likely be retained. Also, the finest sand particles often can be identified by their sparkle or glassy quality.

Source: Michigan Department of Transportation
Gravel Identified by particle size, gravel consists of rounded, partially angular, or angular (crushed faces) particles of rock. Gravel size particles usually occur in varying combinations with other particle sizes. Gravel is subdivided into particle size ranges as follows: (Note that particles $>$ ( 75 mm ) 3 inches are cobbles or boulders, as defined in the Glossary of Terms.)
Coarse - Particles passing the ( 75 mm ) 3 inch sieve, and retained on the ( 19 mm ) $3 / 4$ inch sieve.
Fine - Gravel particles passing the $(19 \mathrm{~mm}) 3 / 4$ inch sieve, and retained on the $(4.76 \mathrm{~mm})$ \#4 U.S. standard sieve.

Note: The term "gravel" in this system denotes a particle size range and should not be confused with "gravel" used to describe a type of geologic deposit or a construction material.

Sand Identified by particle size, sand consists of rock particles, usually silicate (quartz) based, ranging between gravel and silt sizes. Sand has no cohesion or plasticity. Its particles are gritty grains that can easily be seen and felt, and may be rounded (natural) or angular (usually manufactured). Sand is subdivided into particle size ranges as follows:
Coarse - $\quad$ Particles that will pass the ( 4.76 mm ) \#4 U.S. Standard sieve and be retained on the ( 2 mm ) \# 10 U.S. Standard sieve.
Medium - Particles that will pass the ( 2 mm ) \#10 U.S. Standard sieve and be retained on the ( 0.425 mm ) \# 40 U.S. Standard sieve.
Fine - $\quad$ Particles that will pass the $(0.425 \mathrm{~mm})$ \#40 U.S. Standards sieve and be retained on the $(0.075 \mathrm{~mm})$ \# 200 U.S. Standard sieve.
Well-Graded - Indicates relatively equal percentages of Fine, Medium, and Coarse fractions are present.
Note: The particle size of coarse-grained primary soils is important to the Soil Engineer! Always indicate the particle size or size range immediately before the primary soil constituent.

Exception: The use of 'Gravel' alone will indicate both coarse and fine gravel are present.
Examples: Fine \& Medium Sand; Coarse Gravel.
Include the particle shape (angular, partially angular, or rounded) when appropriate, such as for aggregates or manufactured sands.

Example: Rounded Gravel.
Fine-Grained Soils: More than $50 \%$ of the soil PASSES the $(0.075 \mathrm{~mm})$ \#200 sieve.
Silt Identified by behavior and particle size, silt consists of material passing the ( 0.075 mm ) \#200 sieve that is nonplastic (no cohesion) and exhibits little or no strength when dried. Silt can typically be rolled into a ball or strand, but it will easily crack and crumble. To distinguish silt from clay, place material in one hand and make 10 brisk blows with the other; if water appears on the surface, creating a glossy texture, then the primary constituent is silt.
Clay Identified by behavior and particle size, clay consists of material passing the ( 0.075 mm ) \#200 sieve AND exhibits plasticity or cohesion (ability of particles to adhere to each other, like putty) within a wide range of moisture contents. Moist clay can be rolled into a thin ( 3 mm ) $1 / 8$ inch thread that will not crumble. Also, clay will exhibit strength increase with decreasing moisture content, retaining considerable strength when dry.
Clay is often encountered in combination with other soil constituents such as silt and sand. If a soil exhibits plasticity, it contains clay. The amount of clay can be related to the degree of plasticity; the higher the clay content, the greater the plasticity.

Note: When applied to laboratory gradation tests, silt size is defined as that portion of the soil finer than the $(0.075 \mathrm{~mm})$ \# 200 U.S. Standard sieve and coarser than the 0.002 mm . Clay size is that portion of soil finer than 0.002 mm . For field classification, the distinction will be strictly based upon cohesive characteristics.

## Organic Soils:

Peat Highly organic soil, peat consists primarily of vegetable tissue in various stages of decomposition, accumulated under excessive moisture conditions, with texture ranging from fibrous to amorphous. Peat is usually black or dark brown in color, and has a distinct organic odor. Peat may have minor amounts of sand, silt, and clay in various proportions.
Fibrous Peat - Slightly or undecomposed organic material having identifiable plant forms. Peat is relatively very lightweight and usually has spongy, compressible consistency.


#### Abstract

Amorphous Peat (Muck) - Organic material which has undergone substantial decomposition such that recognition of plant forms is impossible. Its consistency ranges from runny paste to compact rubbery solid. Marl Marl consists of fresh water sedimentary deposits of calcium carbonate, often with varying percentages of calcareous fine sand, silt, clay and shell fragments. These deposits are unconsolidated, so marl is usually lightweight. Marl is white or light-gray in color with consistency ranging from soft paste to spongy. It may also contain granular spheres, organic material, or inorganic soils. Note that marl will react (fizz) with weak hydrochloric acid due to the carbonate content.


## Secondary Soil Constituents

Secondary soil constituents represent one or more soil types other than the primary constituent which appear in the soil in significant percentages sufficient to readily affect the appearance or engineering behavior of the soil. To correlate the field classification with laboratory classification, this definition corresponds to amounts of secondary soil constituents $>12 \%$ for fine-grained and $>30 \%$ for coarse-grained secondary soil constituents. The secondary soil constituents will be added to the field classification as an adjective preceding the primary constituent. Two or more secondary soil constituents should be listed in ascending order of importance. Examples: Silty Fine Sand; Peaty Marl; Gravelly, Silty Medium Sand; Silty, Sandy Clay.

## Tertiary Soil Constituents

Tertiary soil constituents represent one or more soil types that are present in a soil in quantities sufficient to readily identify, but NOT in sufficient quantities to significantly affect the engineering behavior of the soil. The tertiary constituent will be added to the field classification with the phrase "with $\qquad$ " at the end, following the primary constituent and all other descriptors. This definition corresponds to approximately $5-12 \%$ for fine-grained and $15-29 \%$ for coarse-grained tertiary soil constituents. Example: Silty Fine to Coarse Sand with Gravel and Peat.

Soil types that appear in the sample in percentages below tertiary levels need not be included in the field classification. However, the slight appearance of a soil type may be characteristic of a transition in soil constituents (more significant deposits nearby), or may be useful in identifying the soil during construction. These slight amounts can be included for descriptive purposes at the end of the field classification as "Trace of $\qquad$ ."

## Additional Soil Descriptors

Additional descriptors should be added as needed to adequately describe the soil for the purpose required. These descriptors should typically be added to the field classification before the primary and secondary constituents, in ascending order of significance (Exceptions noted below). Definitions for several descriptive terms can be found in the Glossary of Terms below. Other terms may be used as appropriate for descriptive purposes, but not for soil constituents.

| Color | Brown, Gray, Yellow, Red, Black, Light-, Dark-, Pale-, etc. |
| :--- | :--- |
| Moisture Content | Dry, Moist, Saturated. Judge by appearance of sample before manipulating. <br> Structure |
| $\underline{\text { Fissured, Friable, Blocky, Varved, Laminated, Lenses, Layers, etc. }}$ |  |
| $\underline{\text { Examples: }}$ | Gray-Brown Laminated Silty Clay; Light-Brown Saturated Fine \& Medium Sand. |
| Certain descriptive terms such as "Fill" may be more appropriate after the primary <br> constituent or at the end of the field classification. Also, the description of distinct <br> soils (inclusions) within a larger stratum should be added after the complete field <br> classification of the predominant soil. |  |

Examples of exceptions: Stiff Brown Sandy Clay Fill, with Coarse Angular Gravel and Asphalt; Gray Silty Clay with Saturated Marl, Lenses of Saturated Fine Sand.

## Soil Strength Assessment

Soil strength refers to the degree of load-carrying capacity and resistance to deformation which a particular soil may develop. For cohesionless granular soils (sand, gravel, and silt) the relative in-place density is a measure of strength. The in-place consistency for cohesionless soils can be estimated by the Standard Penetration Test (SPT-Blow counts) and by resistance to drilling equipment or "pigtail" augers as described below. For cohesive soils, "consistency" is a measure of cohesion, or shear strength. The shear strength of clay soils can be estimated in the field using the manual methods described below, the SPT, or resistance to drilling equipment. Note that for clay soils, loss of moisture will result in increased strength; therefore, consistency of clay soils should be estimated at the natural moisture content.

The soil consistency, when appropriate and available, should be added to the field classification at the very beginning, using the terminology described below. Examples: Loose Brown Rounded Fine Gravel; Medium Stiff Gray Moist Sandy Clay.

## Cohesionless Soil

| Classification | Standard <br> Penetration, N | Relative <br> Density, \% | Resistance to Advancement of a ( 1.2 m ) $\mathbf{4} \mathrm{ft}$. Long, ( $\mathbf{3 8} \mathbf{~ m m}$ ) <br> $\mathbf{1 . 5}$ inch Diameter Spiral (Pigtail) Auger |
| :--- | :---: | :--- | :--- |
| Very Loose | $<4$ | $0-15$ | The auger can be forced several inches into the soil, without turning, <br> under the bodyweight of the technician. |
| Loose | $4-10$ | $15-35$ | The auger can be turned into the soil for its full length without difficulty. <br> It can be chugged up and down after penetrating about $(1 / 3 \mathrm{~m}) 1 \mathrm{ft}$, so that <br> it can be pushed down $(25 \mathrm{~mm}) 1$ inch into the soil. |
| Medium <br> Dense | $10-30$ | $35-65$ | The auger cannot be advanced beyond $\pm(3 / 4 \mathrm{~m}) 2.5 \mathrm{ft}$ without great <br> difficulty. Considerable effort by chugging required to advance further. |
| Dense | $30-50$ | $65-85$ | The auger turns until tight at $\pm(1 / 3 \mathrm{~m}) 1 \mathrm{ft}$; cannot be advanced further. |
| Very Dense | $>50$ | $85-100$ | The auger can be turned into the soil only to about the length of its spiral <br> section. |


| Cohesive Soil |  |  |  |  |
| :--- | :--- | :--- | :---: | :---: |
| Classification | Manual Index for Consistency | Cohesion <br> (psf) | Cohesion <br> $(\mathbf{k P a})$ | Standard <br> Penetration, N |
| Very Soft | Extrudes between fingers when squeezed | $0-250$ | $0-12$ | $<2$ |
| Soft | Molded by light to moderate finger pressure | $250-500$ | $12-24$ | $2-4$ |
| Medium Stiff | Molded by moderate to firm finger pressure | $500-1000$ | $24-48$ | $4-8$ |
| Stiff | Readily indented by thumb, difficult to penetrate | $1000-2000$ | $48-96$ | $8-15$ |
| Very Stiff | Readily indented by thumbnail | $2000-4000$ | $96-192$ | $15-30$ |
| Hard | Indented with difficulty by thumbnail | $4000-8000$ | $192-384$ | $>30$ |
|  |  |  |  |  |

## Glossary of Terms

Blocky Cohesive soil that can be broken down into small angular lumps that resist further breakdown.
Boulder A rock fragment, usually rounded by weathering or abrasion, with average dimension of ( 300 mm ) $12^{\prime \prime}$ or more.
Calcareous Soil containing calcium carbonate, either from limestone deposits or shells. The carbonate will react (fizz) with weak hydrochloric acid.
Cemented The adherence or bonding of coarse soil grains due to presence of a cementicious material. May be weak (readily fragmented), firm (appreciable strength), or indurated (very hard, water will not soften, rock-like)
Cobble A rock fragment, usually rounded or partially angular, with an average dimension ( 75 to 300 mm ) $3^{\prime \prime}-12^{\prime \prime}$.
Dry No appreciable moisture is apparent in the soil.
Fat Clay Fine-grained soil with very high plasticity and dry strength. Usually has a sticky or greasy texture due to very high affinity for water. Remains plastic at very high water contents (Liquid Limit $>50$ ).
Fill Man-made deposits of natural soils and/or waste materials. Document the components carefully since presence and depth of fill are important engineering considerations.
Fissured The soil breaks along definite planes of weakness with little resistance to fracturing.
Frequent Occurring more than one per $(300 \mathrm{~mm}) 1 \mathrm{ft}$ thickness.
Friable A soil that is easily crumbled or pulverized into smaller, nonuniform fragments or clumps.
Laminated Alternating horizontal strata of different material or color, usually in increments of ( 6 mm ) $1 / 4^{\prime \prime}$ or less.
Layer Horizontal inclusion or stratum of sedimentary soil greater than ( 100 mm ) $4^{\prime \prime}$ thick.
Lens Inclusion of a small pocket of a sedimentary soil between $(10 \mathrm{~mm}) 3 / 8^{\prime \prime}$ and $(100 \mathrm{~mm}) 4^{\prime \prime}$ thick, often with tapered edges.
Moist Describes the condition of a soil with moderate to water content relative to the saturated condition (near optimum). Moisture is readily discernible but not in sufficient content to adversely affect the soil behavior.
Mottled Irregularly marked soil, usually clay, with spots of different colors.
Muck See Amorphous Peat, under Primary Soil Constituents heading.
Occasional Occurring once or less per ( 300 mm ) 1 ft thickness.
Organic Indicates the presence of material that originated from living organisms, usually vegetative, undergoing some stage of decay. May range from microscopic size matter to fibers, stems, leaves, wood pieces, shells, etc. Usually dark brown or black in color, and accompanied by a distinct odor.
Parting A very thin soil inclusion of up to $(10 \mathrm{~mm}) 3 / 8^{\prime \prime}$ thickness.
Saturated All of the soil voids are filled with water (zero air voids). Practically speaking, the condition where the moisture content is sufficient to substantially affect the soil behavior.
Trace Indicates appearance of a slight amount of a soil type, which may be included in the classification for descriptive or identification purposes only. The Trace soil would have no effect on the soil behavior. Other modifiers such as "Slight" or "Heavy" should not be used with "Trace."
Varved The paired arrangement of laminations in glacial sediments that reflect seasonal changes during deposition; Fine sand and silt are deposited in the glacial lake during summer, and finer particles are usually deposited in thinner laminations in winter.

## 4．1．0．1 The＂Word Picture＂of Soil Grain Size

| UNIFIED SOIL CLASSIFICATION AND SYMBOL CHART |  |  |  |
| :---: | :---: | :---: | :---: |
| COARSE－GRAINED SOILS <br> （more than $50 \%$ of material is larger than No． 200 sieve size．） |  |  |  |
| Clean Gravels（Less than 5\％fines） |  |  |  |
| GRAVELS <br> More than 50\％ of coarse fraction larger than No． 4 sieve size | $0$ | GW | Well－graded gravels，gravel－sand mixtures，little or no fines |
|  |  | GP | Poorly－graded gravels，gravel－sand mixtures，little or no fines |
|  | Gravels with fines（More than 12\％fines） |  |  |
|  | 时景 | GM | Silty gravels，gravel－sand－silt mixtures |
|  | $5$ | GC | Clayey gravels，gravel－sand－clay mixtures |
| Clean Sands（Less than 5\％fines） |  |  |  |
| SANDS <br> 50\％or more of coarse fraction smaller than No． 4 sieve size |  | SW | Well－graded sands，gravelly sands， little or no fines |
|  |  | SP | Poorly graded sands，gravelly sands， little or no fines |
|  |  | Sand | with fines（More than 12\％fines） |
|  |  | SM | Silty sands，sand－silt mixtures |
|  | 緂运 | SC | Clayey sands，sand－clay mixtures |
| FINE－GRAINED SOILS <br> （ $50 \%$ or more of material is smaller than No． 200 sieve size．） |  |  |  |
| SILTS <br> AND <br> CLAYS <br> Liquid limit less than 50\％ |  | ML | Inorganic silts and very fine sands，rock flour，silty of clayey fine sands or clayey silts with slight plasticity |
|  |  | CL | Inorganic clays of low to medium plasticity，gravelly clays，sandy clays， silty clays，lean clays |
|  |  | OL | Organic silts and organic silty clays of low plasticity |
| SILTS <br> AND <br> CLAYS <br> Liquid limit 50\％ or greater |  | MH | Inorganic silts，micaceous or diatomaceous fine sandy or silty soils， elastic silts |
|  |  | CH | Inorganic clays of high plasticity，fat clays |
|  |  | OH | Organic clays of medium to high plasticity，organic silts |
| HIGHLY ORGANIC SOILS |  | PT | Peat and other highly organic soils |



Source:Virginia Department of Transportation

### 4.1.1 ASTM and AASHTO Aggregate and Soil Terminology

## ASTM Aggregate and Soil Terminology

## Terms

The basic reference for the Unified Soil Classification System is ASTM D 2487. Terms include:

- Coarse-Grained Soils: More than $50 \%$ retained on a 0.075 mm (No. 200) sieve
- Fine-Grained Soils: $50 \%$ or more passes a 0.075 mm (No. 200) sieve
- Gravel: Material passing a $75-\mathrm{mm}$ ( 3 -inch) sieve and retained on a $4.75-\mathrm{mm}$ (No. 4) sieve.
- Coarse Gravel: Material passing a $75-\mathrm{mm}$ ( 3 -inch) sieve and retained on a $19.0-\mathrm{mm}$ ( $3 / 4-\mathrm{inch}$ ) sieve.
- Fine Gravel: Material passing a $19.0-\mathrm{mm}$ (3/4-inch) sieve and retained on a $4.75-\mathrm{mm}$ (No. 4) sieve.
- Sand: Material passing a $4.75-\mathrm{mm}$ sieve (No. 4) and retained on a $0.075-\mathrm{mm}$ (No. 200) sieve.
- Coarse Sand: Material passing a $4.75-\mathrm{mm}$ sieve (No. 4) and retained on a $2.00-\mathrm{mm}$ (No. 10) sieve.
- Medium Sand: Material passing a $2.00-\mathrm{mm}$ sieve (No. 10) and retained on a $0.475-\mathrm{mm}$ (No. 40) sieve.
- Fine Sand: Material passing a $0.475-\mathrm{mm}$ (No. 40) sieve and retained on a $0.075-\mathrm{mm}$ (No. 200) sieve.
- Clay: Material passing a $0.075-\mathrm{mm}$ (No. 200) that exhibits plasticity and strength when dry $\left(\mathrm{PI}^{3} 4\right)$.
- Silt: Material passing a $0.075-\mathrm{mm}$ (No. 200) that is nonplastic and has little strength when dry (PI $<4$ ).
- Peat: Soil of vegetable matter.

Note that these definitions are Unified Soil Classification system definitions and are slightly different from those of AASHTO.

Source: pavementinteractive.org
AASHTO soil terminology comes from AASHTO M 145, "Classification of Soils and Soil-Aggregate Mixtures for Highway Construction Purposes." Aggregate terminology comes from AASHTO M 147, "Materials for Aggregate and Soil-Aggregate Subbase, Base and Surface Courses." Basic terms include:

- Boulders \& Cobbles: Material retained on a $75-\mathrm{mm}$ (3-inch) sieve.
- Gravel: Material passing a $75-\mathrm{mm}$ (3-inch) sieve and retained on a $2.00-\mathrm{mm}$ (No. 10) sieve.
- Coarse Sand: Material passing a $2.00-\mathrm{mm}$ sieve (No. 10) and retained on a $0.475-\mathrm{mm}$ (No. 40) sieve.
- Fine Sand: Material passing a $0.475-\mathrm{mm}$ (No. 40) sieve and retained on a $0.075-\mathrm{mm}$ (No. 200) sieve.
- Silt-Clay: Material passing a $0.075-\mathrm{mm}$ (No. 200) sieve.
- Silt Fraction: Material passing the 0.075 mm sieve and larger than 0.002 mm .
- Clay Fraction: Material smaller than 0.002 mm .
- Silty: Material passing a $4.75-\mathrm{mm}$ (No. 4) sieve with a PI $£ 10$
- Clayey: Material passing a $4.75-\mathrm{mm}$ (No. 4) sieve with a $\mathrm{PI}^{3} 11$
- Coarse Aggregate: Aggregate retained on the 2.00 mm sieve and consisting of hard, durable particles or fragments of stone, gravel or slag. A wear requirement (AASHTO T 96) is normally required.
- Fine Aggregate: Aggregate passing the 2.00 mm (No. 10) sieve and consisting of natural or crushed sand, and fine material particles passing the 0.075 mm (No. 200) sieve. The fraction passing the 0.075 mm (No. 200) sieve shall not be greater than two-thirds of the fraction passing the 0.425 mm (No. 40) sieve. The portion passing the 0.425 mm (No. 40) sieve shall have a $\mathrm{LL} \leq 25$ and a PI $\leq 6$. Fine aggregate shall be free from vegetable matter and lumps or balls of clay.

Note that these definitions are AASHTO definitions and are slightly different from those of the Unified Soil Classification system (ASTM).

### 4.1.2 Field Method of Classification

## Procedure

## Field Classification Technique for Coarse-Grained Soils

1. Take a representative sample of soil (excluding particles $>75 \mathrm{~mm}$ ) (see Note 1 ) and classify the soil as coarse-grained or fine-grained by estimating whether $50 \%$, by weight, of the particles can be seen individually by the naked eye. Soils containing $>50 \%$ of particles that can be seen are coarse-grained soils; soils containing $<50 \%$ of particles smaller than the eye can see are fine-grained soils. If the soil is predominantly coarse-grained, identify as being a gravel or a sand by estimating whether $50 \%$ or more, by weight, of the coarse grains are larger or smaller than 4.75 mm (No. 4 sieve size).
2. If the soil is a gravel, identify as being "clean" (containing little or no fines, $<5 \%$ ) or "dirty" (containing an appreciable amount of fines, $>12 \%$ ). For clean gravels final classification is made by estimating the gradation: the well-graded gravels belong to the GW groups and uniform and gap-graded gravels belong to the GP group. Dirty gravels are of two types: those with nonplastic (silty) fines (GM) and those with plastic (clayey) fines (GC). The determination of whether the fines are silty or clayey is made by the three manual tests for fine-graded soils.
3. If a soil is a sand, the same steps and criteria are used as for gravels in order to determine whether the soil is a well-graded clean sand (SW), poorly graded clean sand (SP), sand with silty fines (SM) or sand with clayey fines (SC).
4. If a material is predominantly ( $>50 \%$ by weight) fine-grained, it is classified into one of six groups (ML, CL, $\mathrm{OL}, \mathrm{MH}, \mathrm{CH}, \mathrm{OH}$ ) by estimating its dilatancy (reaction to shaking), dry strength (crushing characteristics), and toughness (consistency near the plastic limit) and by identifying it as being organic or inorganic. (See Note 2.)

Source: Florida International University, Miami, FL

### 4.1.3 Sediment Classification According to United Soil Classification System

|  |  |
| :--- | :--- |
| GW | Well-graded gravels, gravel/sand mixture, little or no fines |
| GP | Poorly graded gravels, gravel/sand mixture, little or no fines |
| GM | Gray calcareous gravel, sand/silt mixture |
| GC | Gray calcareous gravel/sand/clay mixtures |
| SW | Well-graded sands, gravelly sands, little or no fines |
| SP | Poorly graded sands, gravelly sands, little or no fines |
| SM | Silty sands, sand/silt mixtures |
| SC | Clayey sands, sand/clay mixtures |
| ML | Inorganic silts and very fine sands, silty/clayey fine sand |
| CL | Inorganic clays of low to medium plasticity, sandy silty or lean clays |
| OL | Organic silts and organic silty clays of low plasticity |
| MH | Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts |
| CH | Fat clays |
| OH | Fat organic clays |
| PT | Peat, humus, and other organic swamp soils |
| SP-SM | Sandy, gravelly, silty mixtures |
| L | Limestone |
| S | Sandstone |

Source: Florida International University, Miami, FL

### 4.1.4 USCS Classification Flowline



Source: Florida International University, Miami, FL

### 4.1.5 Group Names for Gravelly Soils

| Group Symbol | Criteria for Group Name: SF |  |
| :--- | :--- | :--- |
| GW | $<15$ | Group Name |
|  | $\geq 15$ | Well-graded gravel |
| GP | $<15$ | Well-graded gravel with sand |
|  | $\geq 15$ | Poorly graded gravel |
| GM | $<15$ | Poorly graded gravel with sand |
|  | $\geq 15$ | Silty gravel |
| GC | $<15$ | Silty gravel with sand |
| GC-GM | $\geq 15$ | Clayey gravel |
|  | $\geq 15$ | Clayey gravel with sand |
| GW-GM | $<15$ | Silty, clayey gravel |
|  | $\geq 15$ | Silty, clayey gravel with sand |
| GW-GC | $<15$ | Well-graded gravel with silt |
|  | $\geq 15$ | Well-graded gravel with silt and sand |
| GP-GM | $<15$ | Well-graded gravel with clay |
|  | $\geq 15$ | Poorly graded gravel with silt |
| GP-GC | $<15$ | Poorly graded gravel with silt and sand |

### 4.1.6 Group Names for Sandy Soils

| Group Symbol | Criteria for Group Name: $\mathrm{SF}^{\text {a }}$ | Group Name |
| :---: | :---: | :---: |
| GW | $<15$ | Well-graded sand |
|  | $\geq 15$ | Well-graded sand with gravel |
| GP | < 15 | Poorly graded sand |
|  | $\geq 15$ | Poorly graded sand with gravel |
| GM | $<15$ | Silty sand |
|  | $\geq 15$ | Silty sand with gravel |
| GC | < 15 | Clayey sand |
|  | $\geq 15$ | Clayey sand with gravel |
| GC-GM | $<15$ | Silty, clayey sand |
|  | $\geq 15$ | Silty, clayey sand with gravel |
| GW-GM | $<15$ | Well-graded sand with silt |
|  | $\geq 15$ | Well-graded sand with silt and gravel |
| GW-GC | $<15$ | Well-graded sand with clay |
|  | $\geq 15$ | Well-graded sand with clay and gravel |
| GP-GM | < 15 | Poorly graded sand with silt |
|  | $\geq 15$ | Poorly graded sand with silt and gravel |
| GP-GC | $<15$ | Poorly graded sand with clay |
|  | $\geq 15$ | Poorly graded sand with clay and gravel |
| Based on ASTM D-2487 |  |  |
| ${ }^{\text {a }}$ SF $=$ gravel fraction $=R 4$ |  |  |
| Source: Florida International University, Miami, FL |  |  |

### 4.1.7 Calculations to Determine USCS Classification Based upon Percentage Passing through Sieve

Classify the soil by using the Unified Soil Classification System if the percentage passing No. 4 sieve $=70$, percentage passing No. 200 sieve $=30$, the Liquid limit $=33$ and the Plasticity index $=12$. Give the group symbol and the name.

## Solution:

$$
\begin{aligned}
& \text { F200 }=30 \\
& \text { R200 }=100-\text { F200 }=100-30=70
\end{aligned}
$$

Because R200 is greater than 50, the soil is coarse grained.

$$
\begin{aligned}
& \text { R4 } \quad=100-\mathrm{F} 4=100-70=30 \\
& \text { For this soil, R4/R200 }=30 / 70<0.5
\end{aligned}
$$

This soil is sandy. The group symbol is $\mathbf{S C}$. For the group name,

$$
\mathrm{GF}=\text { gravel fraction }=\mathrm{R} 4=30 \text { (i.e } .,>15 \text { ) }
$$

Therefore, the group name is clayey sand with gravel.
Source: Florida International University, Miami, FL

### 4.1.8 Correlation between AASHTO and USCS Systems

| AASHTO | Unified |
| :--- | :--- |
| A-2-6 | GC, SC |
| A-2-7 | GC, SC |
| A-3 | SP |
| A-4 | $\mathrm{ML}, \mathrm{OL}$ |
| A-5 | MH |
| A-6 | CL |
| A-7-5 | $\mathrm{CL}, \mathrm{OL}$ |
| A-7-6 | $\mathrm{CH}, \mathrm{OH}$ |

Source: Florida International University, Miami, FL

### 4.1.9 Classification of Soil and Soil-Aggregate Mixtures for Highway Construction Purposes AASHTO M-145-91 (2000) (Modified)

This practice describes a procedure for classifying soils into seven groups based on laboratory determination of particle-size distribution, liquid limit, and plasticity index. The group classification should be useful in determining the relative quality of the soil material for use in embankments, subgrades, and backfills. For detailed design of important structures, additional data concerning strength or performance characteristics of the soil under field conditions will usually be required.

Modification: Determination of Group Index will not be a part of certification, but taught as a useful tool for more accurate determination of soil classification.

## Key Elements

1. Determine sieve analysis. Determine sieve analysis using AASHTO T-11 and AASHTO T-27 test procedures (Note 1). The 2.00 mm (No. 10) sieve, $425-\mu \mathrm{m}$ (No. 40) sieve, and $75-\mu \mathrm{m}$ (No. 200) sieve must be included to determine the particle size distribution as a basis for classification.
2. Determine the liquid limit. Determine the liquid limit of the material using AASHTO T-89 test procedures.
3. Determine the plastic limit. Determine the plastic limit and plasticity index of the material using AASHTO T-90 test procedures.
4. Determine classification of material. Using the test limits shown in Table 1 of AASHTO M-145, make the classification of the material. If a more detailed classification is desired, a further subdivision of the groups may be made using Table 2 of AASHTO M-145 (3.1). With required test data available, proceed from left to right in Table 1 or Table 2 and the correct group will be found by process of elimination (3.2). The first group from the left into which the test data will fit is the correct classification (3.2).
5. Report classification. All limiting test values are shown as whole numbers. If fractional numbers appear on test reports, convert to the nearest whole number for purposes of classification (3.2).

## Description of Soil Classification Groups:

Soil Fractions: According to the AASHTO system, soils are divided into two major groups as shown in Table 1 of AASHTO M-145. These are the granular materials with $35 \%$ or less passing the $75-\mu \mathrm{m}$ (No. 200) sieve (5.1, Note 2) and the silt-clay materials with more than $35 \%$ passing the $75-\mu \mathrm{m}$ (No. 200) sieve (5.2). Moreover, five soil fractions are recognized and often used in word descriptions of a material. These five fractions are defined as follows:

Boulders and Cobbles - material retained on the 75 mm (3 in.) sieve. They should be excluded from the portion of a sample to which the classification is applied, but the percentage of such material should be recorded (4.1.5).

Gravel - materials passing sieve with 75 mm ( 3 in .) square openings and retained on the 2.0 mm (No. 10) sieve (4.1.1).

Coarse Sand - materials passing the 2.0 mm (No. 10) sieve and retained on the $425-\mu \mathrm{m}$ (No. 40) sieve (4.1.2).
Fine Sand - materials passing the $425-\mu \mathrm{m}$ (No. 40) sieve and retained on the $75-\mu \mathrm{m}$ (No. 200) sieve (4.1.3).
Combined Silt and Clay - materials passing the $75-\mu \mathrm{m}$ (No. 200) sieve. The word "silty" is applied to a fine material having a Plasticity Index of $\mathbf{1 0}$ or less, and the term "clayey" is applied to fine material having a PI of more than 10 (4.1.6).

## Granular Materials:

Group A-1: Well-graded mixtures of stone fragments or gravel ranging from coarse to fine with a nonplastic or slightly plastic soil binder (5.1.1). However, this group also includes coarse materials without soil binder.

Subgroup A-1-a: Materials consisting predominantly of stone fragments or gravel, either with or without a well-graded soil binder (5.1.1.1).
Subgroup A-1-b: Materials consisting predominantly of coarse sand either with or without a well-graded soil binder (5.1.1.2).
Group A-3: Material consisting of sands deficient in coarse material and soil binder. Typical is fine beach sand or fine desert blow sand, without silt or clay fines or with a very small amount of nonplastic silt. This group also includes stream-deposited mixtures of poorly graded fine sand and limited amounts of coarse sand and gravel (5.1.2). These soils make suitable subgrades for all types of pavements when confined and damp. They are subject to erosion and have been known to pump and blow under rigid pavements. (Information: They can be compacted by vibratory, pneumatic-tired, and steel-wheeled rollers but not with a sheepsfoot roller.)

Group A-2: This group includes a wide variety of "granular" materials that are borderline between the materials falling in Groups A-1 and A-3 and silt-clay materials of Groups A-4, A-5, A-6, and A-7. It includes all materials containing $35 \%$ or less passing the $75-\mu \mathrm{m}$ (No. 200) sieve that cannot be classified as A-1 or A-3 (5.1.3).

Subgroups A-2-4 and A-2-5: Include various granular materials containing 35\% or less passing the $75-\mu \mathrm{m}$ (No. 200) sieve, and with that portion passing $425-\mu \mathrm{m}$ (No. 40) sieve having the characteristics of the A-4 and A-5 groups. These groups include such materials as gravel and coarse sand with silt contents or Plasticity Indexes in excess of the limitations of Group A-1, and fine sand with nonplastic silt content in excess of the limitations of Group A-3 (5.1.3.1).
Subgroups A-2-6 and A-2-7: Include materials similar to those described under Subgroups A-2-4 and A-2-5, except that the fine portion contains plastic clay having the characteristics of the A-6 or A-7 group (5.1.3.2).

A-2 soils are given a poorer rating than A-1 soils because of inferior binder, poor grading, or a combination of the two. Depending on the character and amount of binder, A-2 soils may become soft during wet weather and loose and dusty in dry weather when used as a road surface. If, however, they are protected from these extreme changes in moisture content, they may be quite stable. The A-2-4 and A-2-5 soils are satisfactory as base materials when properly compacted and drained. A-2-6 and A-2-7 soils with low percentages of minus $75-\mu \mathrm{m}$ (no. 200) sieve material are classified as good bases, whereas these same soils with high percentages of minus $75-\mu \mathrm{m}$ (No. 200) sieve and PI's of 10 or higher are questionable as a base material. Frequently, the A-2 soils are employed as a cover material for very plastic subgrades.

## Silt-Clay Materials

Group A-4: The typical material of this group is a nonplastic or moderately plastic silty soil usually having $75 \%$ or more passing the $75 \mu \mathrm{~m}$ (No. 200) sieve. The group includes also mixtures of fine silty soil and up to $64 \%$ of sand and gravel retained on the $75-\mu \mathrm{m}$ (No. 200) sieve (5.2.1). These predominantly silty soils are quite common in occurrence. Their texture varies from sandy loams to silty and clayey loams. With the proper amount of moisture present, they may perform well as a pavement component. However, they frequently have an affinity for water and will swell and lose much of their stability unless properly compacted and drained. Moreover, they are subject to frost heave. These soils do not drain readily and may absorb water by capillary action with resulting loss in strength. The silty loams are often difficult to compact properly. Careful field control of moisture content and pneumatic tired rollers are normally required for proper compaction.
Group A-5: The typical material of this group is similar to that described under Group A-4, except that it is usually of diatomaceous or micaceous character and may be highly elastic as indicated by the high liquid limit (5.2.2). These soils do not occur as widely as the A-4 soils. They are normally elastic or resilient in both the damp and semi-dry conditions. They are subject to frost heave, erosion, and loss of stability if not properly drained. Since these soils do not drain readily, they may absorb water by capillary action with resulting loss in strength. Careful control of moisture content is normally required for proper compaction.
Group A-6: The typical material of this group is plastic clay soil usually having $75 \%$ or more passing the $75-\mu \mathrm{m}$ (No. 200) sieve. The group also includes mixtures of fine clayey soil and up to $64 \%$ of sand and gravel retained on the $75-\mu \mathrm{m}$ (No. 200) sieve. Materials of this group usually have high-volume change between wet and dry states (5.2.3). These soils are quite common in occurrence and are widely used in fills. When moisture content is properly controlled, they compact quite readily with either a sheepsfoot or pneumatic-tired roller. They have high dry strength but lose much of this strength upon absorbing water. The A-6 soils will compress when wet and shrink and swell with changes in moisture content. When placed in the shoulders adjacent to the pavement, they tend to shrink away from the pavement edge upon drying and thereby provide an access route to the underside of the pavement for surface water. The A-6 soils do not drain readily and may absorb water by capillary action with resulting loss in strength.

Group A-7: The typical materials and problems of this group are similar to those described under Group A-6, except that they have the high liquid limits characteristic of the A-5 group and may be elastic as well as subject to high-volume change (5.2.4).

Subgroup A-7-5: Includes those materials with moderate Plasticity Indexes in relation to Liquid Limit and which may be highly elastic as well as subject to considerable volume change (5.2.4.1).
Subgroup A-7-6: Includes those materials with high Plasticity Indexes in relation to Liquid Limit and which are subject to extremely high volume change (5.2.4.2).

Highly organic soils such as peat or muck are not included in this classification. Because of their many undesirable properties, their use should be avoided, if possible, in all types of construction.

### 4.1.10 USDA Soil Textural Classification Chart



Source: U.S. Department of Agriculture

### 4.2.0 Soil Taxonomy—Formative Elements and Names of Soil Suborders

## Soil Formation and Classification

The National Cooperative Soil Survey identifies and maps over 20,000 different kinds of soil in the United States. Most soils are given a name, which generally comes from the locale where the soil was first mapped. Named soils are referred to as soil series.

Soil survey reports include the soil survey maps and the names and descriptions of the soils in a report area. These soil survey reports are published by the National Cooperative Soil Survey and are available to everyone.

Soils are named and classified on the basis of physical and chemical properties in their horizons (layers). "Soil Taxonomy" uses color, texture, structure, and other properties of the surface 2 meters deep to key the soil into a classification system to help people use soil information. This system also provides a common language for scientists.

Soils and their horizons differ from one another, depending on how and when they formed. Soil scientists use five soil factors to explain how soils form and to help them predict where different soils may occur. The scientists also allow for additions and removal of soil material and for activities and changes within the soil that continue each day.

## Soil-Forming Factors

Parent material. Few soils weather directly from the underlying rocks. These "residual" soils have the same general chemistry as the original rocks. More commonly, soils form in materials that have moved in from elsewhere. Materials may have moved many miles or only a few feet. Windblown "loess" is common in the Midwest. It buries "glacial till" in many areas. Glacial till is material ground up and moved by a glacier. The material in which soils form is called "parent material." In the lower part of the soils, these materials may be relatively unchanged from when they were deposited by moving water, ice, or wind.

Sediments along rivers have different textures, depending on whether the stream moves quickly or slowly. Fast-moving water leaves gravel, rocks, and sand. Slow-moving water and lakes leave fine-textured material (clay and silt) when sediments in the water settle out.

Climate. Soils vary, depending on the climate. Temperature and moisture amounts cause different patterns of weathering and leaching. Wind redistributes sand and other particles especially in arid regions. The amount, intensity, timing, and kind of precipitation influence soil formation. Seasonal and daily changes in temperature affect moisture effectiveness, biological activity, rates of chemical reactions, and kinds of vegetation.

Topography. Slope and aspect affect the moisture and temperature of soil. Steep slopes facing the sun are warmer, just like the south-facing side of a house. Steep soils may be eroded and lose their topsoil as they form. Thus, they may be thinner than the more nearly level soils that receive deposits from areas upslope. Deeper, darker colored soils may be expected on the bottom land.

Biological factors. Plants, animals, microorganisms, and humans affect soil formation. Animals and microorganisms mix soils and form burrows and pores. Plant roots open channels in the soils. Different types of roots have different effects on soils. Grass roots are "fibrous" near the soil surface and easily decompose, adding organic matter. Taproots open pathways through dense layers. Microorganisms affect chemical exchanges between roots and soil. Humans can mix the soil so extensively that the soil material is again considered parent material.

The native vegetation depends on climate, topography, and biological factors plus many soil factors such as soil density, depth, chemistry, temperature, and moisture. Leaves from plants fall to the surface and decompose on the soil. Organisms decompose these leaves and mix them with the upper part of the soil. Trees and shrubs have large roots that may grow to considerable depths.

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Time. Time for all these factors to interact with the soil is also a factor. Over time, soils exhibit features that reflect the other forming factors. Soil formation processes are continuous. Recently deposited material, such as the deposition from a flood, exhibits no features from soil development activities. The previous soil surface and underlying horizons become buried. The time clock resets for these soils. Terraces above the active floodplain, while genetically similar to the floodplain, are older land surfaces and exhibit more development features.

These soil-forming factors continue to affect soils even on "stable" landscapes. Materials are deposited on their surface, and materials are blown or washed away from the surface. Additions, removals, and alterations are slow or rapid, depending on climate, landscape position, and biological activity.

When mapping soils, a soil scientist looks for areas with similar soil-forming factors to find similar soils. The colors, texture, structure, and other properties are described. Soils with the same kind of properties are given taxonomic names. A common soil in the Midwest reflects the temperate, humid climate and native prairie vegetation with a thick, nearly black surface layer. This layer is high in organic matter from decomposing grass. It is called a "mollic epipedon." It is one of several types of surface horizons that we call "epipedons." Soils in the desert commonly have an "ochric" epipedon that is light colored and low in organic matter. Subsurface horizons also are used in soil classification. Many forested areas have a subsurface horizon with an accumulation of clay called an "argillic" horizon.

## Soil Orders

Soil taxonomy at the highest hierarchical level identifies 12 soil orders. The names for the orders and taxonomic soil properties relate to Greek, Latin, or other root words that reveal something about the soil. Sixty-four suborders are recognized at the next level of classification. There are about 300 great groups and more than 2400 subgroups. Soils within a subgroup that have similar physical and chemical properties that affect their responses to management and manipulation are families. The soil series is the lowest category in the soil classification system.

| Soil Order | Formative Terms | Pronunciation |
| :--- | :--- | :--- |
| Alfisols | Alf, meaningless syllable | Pedalfer |
| Andisols | Modified from ando | $\overline{\text { Ando }}$ |
| Aridisols | Latin, aridies, dry | Arid |
| Entisols | Ent, meaningless | Recent |
| Gelisols | Latin gelare, to freeze | $\underline{\text { Jell }}$ |
| Histosols | Greek, histos, tissue | Histology |
| Inceptisols | Latin, incepum, beginning | Inception |
| Mollisols | Latin, mollis, soft | Mollify |
| $\underline{\text { Oxisols }}$ | French oxide | $\underline{\text { Oxide }}$ |
| Spodosols | Greek spodos, wood ash | $\underline{\text { Odd }}$ |
| $\frac{\text { Ultisols }}{\text { Vertisols }}$ | Latin ultimus, last | $\underline{\text { Ultimate }}$ |
|  | Latin verto, turn |  |

## Maps

The distribution of these soil orders in the United States corresponds with the general patterns of the soil-forming factors across the country. A map of soil orders is useful in understanding broad areas of soils.

Detailed soil maps found in soil survey reports, however, should be used for local decision making. Soil maps are like roadmaps; for a very general overview, a small-scale map in an atlas is helpful, but for finding a location of a house in a city, a large-scale detailed map should be used.

## Formative Elements in Names of Soil Suborders

| Formative Element | Derivation | Sounds Like | Connotation |
| :---: | :---: | :---: | :---: |
| Alb | L, albus, white | Albino | Presence of albic horizon |
| Anthr | Modified from Gr. anthropes, human | Anthropology | Modified by humans |
| Aqu | L. aqua, water | Aquifer | Aquic conditions |
| Ar | L. Arare, to plow | Arable | Mixed horizons |
| Arg | Modified from argillic horizon; <br> L. argilla, white clay | Argillite | Presence of argillic horizon |
| Calc | L. calcis, lime | Calcium | Presence of a calcic horizon |
| Camb | L. cambiare, to exchange | Am | Presence of a cambic horizon |
| Cry | G. kryos, icy cold | Cry | Cold |
| Dur | L. durus, hard | Durable | Presence of a duripan |
| Fibr | L. fibra, fiber | Fibrous | Least decomposed stage |
| Fluv | L. fluvius, river | Fluvial | Flood plain |
| Fol | L. folia, leaf | Foliage | Mass of leaves |
| Gyps | L. gypsum, gypsum | Gypsum | Presence of a gypsic horizon |
| Hem | Gr hemi, half | Hemisphere | Intermediate stage of decomposition |
| Hist | Gr. histos, tissue | Histology | Presence of organic materials |
| Hum | L. humus, earth | Humus | Presence of organic matter |
| Orth | Gr. orthos, true | Orthodox | The common ones |
| Per | L. Per, throughout in time | Perennial | Perudic moisture regime |
| Psamm | Gr. psammos, sand | Sam | Sandy texture |
| Rend | Modified from Rendzina | End | High carbonate content |
| Sal | L. base of sal, salt | Saline | Presence of a salic horizon |
| Sapr | Gr. sapros, rotten | Sap | Most decomposed stage |
| Torr | L. torridus, hot and dry | Or | Torric moisture regime |
| Turb | L. Turbidis, disturbed | Turbulent | Presence of cryoturbation |
| Ud | L. udus, humid | You | Udic moisture regime |
| Vitr | L. vitrum, glass | It | Presence of glass |
| Ust | L. ustus, burnt | Combustion | Ustic moisture regime |
| Xer | Gr. xeros, dry | Zero | Xeric moisture regime |

Formative Elements in Names of Soil Great Groups

| Formative Element | Derivation | Sounds Like | Connotation |
| :---: | :---: | :---: | :---: |
| Acr | Modified from Gr. Akros, at the end | Act | Extreme weathering |
| Al | Modified from aluminum | Algebra | High aluminum, low iron |
| Alb | L. Albus, white | Albino | An albic horizon |
| Anhy | Gr. anydros, waterless | Anhydrous | Very dry |
| Anthr | Modified from Gr. anthropos, human | Anthropology | An anthropic epipedon |
| Aqu | L. aqua, water | Aquifer | Aquic conditions |
| Argi | Modified from argillic horizon; L. argilla, white clay | Argillite | Presence of an argillic horizon |
| Calci, calc | L. calcis, lime | Calcium | A calcic horizon |
| Cry | Gr. kryos, icy cold | Cry | Cold |
| Dur | L. durus, hard | Durable | A duripan |
| Dystr, dys | Modified from Gr. dys, ill; dystrophic infertile | Distant | Low base saturation |
| Endo | Gr. endon, endo, within | Endothermic | Implying a ground water table |
| Epi | Gr. epi, on, above | Epidermis | Implying a perched water table |
| Eutr | Modified from Gr. eu, good; euthrophic, fertile | You | High base saturation |
| Ferr | L. ferrum, iron | Fair | Presence of iron |
| Fibr | L. fibra, fiber | Fibrous | Least decomposed stage |
| Fluv | L. fluvius, river | Fluvial | Flood plain |
| Fol | L. folia, leaf | Foliage | Mass of leaves |
| Fragi | Modified from L. fragilis, brittle | Fragile | Presence of fragipan |
| Fragloss | Compound of fra (g) and gloss |  | See the formative elements "frag" and "gloss" |
| Fulv | L. fulvus, dull brownish yellow | Full | Dark brown color, presence of organic carbon |
| Glac | L. glacialis, icy | Glacier | Ice lenses or wedges |
| Gyps | L. gypsum, gypsum | Gypsum | Presence of gypsic horizon |
| Gloss | Gr. glossa, tongue | Glossary | Presence of glossic horizon |
| Hal | Gr. hals, salt | Halibut | Salty |
| Hapl | Gr. haplous, simple | Haploid | Minimum horizon development |
| Hem | G. hemi, half | Hemisphere | Intermediate stage of decomposition |
| Hist | Gr. histos, tissue | History | Presence of organic materials |
| Hum | L. humus, earth | Humus | Presence of organic matter |
| Hydr | Gr. hydo, water | Hydrophobia | Presence of water |
| Kand, kan | Modified from kandite | Can | 1:1 layer silicate clays |

## Formative Elements in Names of Soil Great Groups-Cont'd

| Formative Element | Derivation | Sounds Like | Connotation |
| :---: | :---: | :---: | :---: |
| Luv | Gr. louo, to wash | Ablution | Illuvial |
| Melan | Gr. melasanos, black | $\mathrm{Me}+$ Land | Black, presence of organic carbon |
| Moll | L. mollis, soft | Mollusk | Presence of a mollic epipedon |
| Natr | Modified from natrium, sodium | Date | Presence of natric horizon |
| Pale | Gr. paleos, old | Paleontology | Excessive development |
| Petr | Gr. comb. form of petra, rock | Petrified | A cemented horizon |
| Plac | Gr. base of plax, flat stone | Placard | Presence of thin pan |
| Plagg | Modified from Ger. plaggen, sod | Awe | Presence of plaggen epipedon |
| Plinth | Gr. plinthos, brick | In | Presence of plinthite |
| Psamm | Gr. psammos, sand | Sam | Sandy texture |
| Quartz | Ger. quarz, quartz | Quarter | High quartz content |
| Rhod | Gr. base of rhodon, rose | Rhododendron | Dark red color |
| Sal | L. base of sal, salt | Saline | Presence of salic horizon |
| Sapr | Gr. saprose, rotten | Sap | Most decomposed stage |
| Somb | F. sombre, dark | Somber | Presence of sombric horizon |
| Sphagn | Gr. sphagnos, bog | Sphagnum | Presence of Sphagnum |
| Sulf | L. sulfur, sulfur | Sulfur | Presence of sulfides or their oxidation products |
| Torr | L. torridus, hot and dry | $\underline{\text { Torrid }}$ | Torric moisture regime |
| Ud | L. udus, humid | You | Udic moisture regime |
| Umbr | L. umbra, shade | Umbrella | Presence of umbric epipedon |
| Ust | L. ustus, burnt | Combustion | Ustic moisture regime |
| Verm | L. base of vermes, worm | Vermilion | Wormy, or mixed by animals |
| Vitr | L. vitrum, glass | It | Presence of glass |
| Xer | Gr. xeros, dry | Zero | Xeric moisture regime |

soils.usda.gov/. . ./formation.html

### 4.3.0 Calculating Soil Compaction Utilizing Various Methods Soil Compaction Tests

## 1) The Sand-Cone Method

The sand-cone method is used to determine in the field the density of compacted soils in earth embankments, road fill, and structure backfill, as well as the density of natural soil deposits, aggregates, soil mixtures, or other similar materials. It is not suitable, however, for soils that are saturated, soft, or friable (crumble easily).

Characteristics of the soil are computed from
Volume of soil, $\mathrm{ft}^{3}\left(\mathrm{~m}^{3}\right)=[$ weight of sand-filling hole, $\mathrm{lb}(\mathrm{kg})] /\left[\right.$ Density of sand, $\left.\mathrm{lb} / \mathrm{ft}^{3}\left(\mathrm{~kg} / \mathrm{m}^{3}\right)\right]$
$\%$ Moisture $=100$ (weight of moist soil—weight of dry soil)/weight of dry soil
Field density, $\mathrm{lb} / \mathrm{ft}^{3}\left(\mathrm{~kg} / \mathrm{m}^{3}\right)=$ weight of soil, $\mathrm{lb}(\mathrm{kg}) /$ volume of soil, $\mathrm{ft}^{3}\left(\mathrm{~m}^{3}\right)$
Dry density $=$ field density $/(1+\%$ moisture/100 $)$
\% Compaction $=100($ dry density $) /$ max dry density
Maximum density is found by plotting a density-moisture curve.
2) Load-Bearing Test

One of the earliest methods for evaluating the in situ deformability of coarse-grained soils is the small-scale load-bearing test. Data developed from these tests have been used to provide a scaling factor to express the settlement $r$ of a full-size footing from the settlement $r 1$ of a $1-\mathrm{ft}^{2}\left(0.0929-\mathrm{m}^{2}\right)$ plate. This factor $\mathrm{r} / \mathrm{r} 1$ is given as a function of the width $B$ of the full-size bearing plate as

$$
\mathrm{r} / \mathrm{r} 1=(2 \mathrm{~B} / 1+\mathrm{B})^{2}
$$

From an elastic half-space solution, E's can be expressed from results of a plate load test in terms of the ratio of bearing pressure to plate settlement $k_{v}$ as

$$
\begin{aligned}
& \mathrm{K}_{\mathrm{v}}\left(1-\mathrm{m}^{2}\right) \mathrm{p} / 4 \\
& \mathrm{E}^{\prime} \mathrm{s}= \\
& 4 \mathrm{~B} /(1+\mathrm{B})^{2}
\end{aligned}
$$

where $m$ represents Poisson's ratio, which is usually considered to range between 0.30 and 0.40 . The E's equation assumes that r 1 is derived from a rigid, $1-\mathrm{ft}(0.3048-\mathrm{m})$-diameter circular plate and that B is the equivalent diameter of the bearing area of a full-scale footing. Empirical formulations, such as the $\mathrm{r} / \mathrm{r} 1$ equation, may be significantly in error because of the limited footing-size range used and the large scatter of the database. Furthermore, consideration is not given to variations in the characteristics and stress history of the bearing soils.

## 3) California Bearing Ratio

The California bearing ratio (CBR) is often used as a measure of the quality of strength of a soil that underlies a pavement, for determining the thickness of the pavement, its base, and other layers.

$$
\mathrm{CBR}=\mathrm{F} / \mathrm{F}_{\mathrm{o}}
$$

where
$\mathrm{F}=$ force per unit area required to penetrate a soil mass with a $3-\mathrm{in}^{2}\left(1935.6-\mathrm{mm}^{2}\right)$ circular piston (about 2 in $(50.8 \mathrm{~mm})$ in diameter) at the rate of $0.05 \mathrm{in} / \mathrm{min}(1.27 \mathrm{~mm} / \mathrm{min})$;
$\mathrm{F}_{\mathrm{v}}=$ force per unit area required for corresponding penetration of a standard material.
By permission: engineering civil.com
Typically, the ratio is determined at $0.10-\mathrm{in}$. ( $2.54-\mathrm{mm}$ ) penetration, although other penetrations sometimes are used. An excellent base course has a CBR of $100 \%$. A compacted soil may have a CBR of $50 \%$, whereas a weaker soil may have a CBR of 10 .
4) Soil Permeability

The coefficient of permeability k is a measure of the rate of flow of water through saturated soil under a given hydraulic gradient $\mathrm{i}, \mathrm{cm} / \mathrm{cm}$, and is defined in accordance with Darcy's law as

$$
\mathrm{V}=\mathrm{kiA}
$$

where $\mathrm{V}=$ rate of flow, $\mathrm{cm}^{3} / \mathrm{s}$, and $\mathrm{A}=$ cross-sectional area of soil conveying flow, $\mathrm{cm}^{2}$.

Coefficient k is dependent on the grain-size distribution, void ratio, and soil fabric and typically may vary from as much as $10 \mathrm{~cm} / \mathrm{s}$ for gravel to less than 10-7 for clays. For typical soil deposits, k for horizontal flow is greater than k for vertical flow, often by an order of magnitude.

### 4.3.0.1 Soil Testing-Types—Hand, Proctor, Nuclear Density, Sand Cone

## Moisture versus Soil Density

The moisture content of the soil is vital to proper compaction. Moisture acts as a lubricant within soil, sliding the particles together. Too little moisture means inadequate compaction-the particles cannot move past each other to achieve density. Too much moisture leaves water-filled voids and subsequently weakens the load-bearing ability. The highest density for most soils is at a certain water content for a given compaction effort. The drier the soil, the more resistant it is to compaction. In a water-saturated state the voids between particles are partially filled with water, creating an apparent cohesion that binds them together. This cohesion increases as the particle size decreases (as in clay-type soils).


## Soil Density Tests

To determine if proper soil compaction is achieved for any specific construction application, several methods were developed. The most prominent by far is soil density.

## Why Test?

Soil testing accomplishes the following:

- Measures density of soil for comparing the degree of compaction versus specs
- Measures the effect of moisture on soil density versus specs
- Provides a moisture density curve identifying optimum moisture


## Types of Tests

Tests to determine optimum moisture content are done in the laboratory. The most common is the Proctor Test, or Modified Proctor Test. A particular soil needs to have an ideal (or optimum) amount of moisture to achieve maximum density. This is important not only for durability, but will save money because less compaction effort is needed to achieve the desired results.

## The Hand Test

A quick method of determining moisture is known as the "Hand Test." Pick up a handful of soil. Squeeze it in your hand. Open your hand. If the soil is powdery and will not retain the shape made by your hand, it is too dry. If it shatters when dropped, it is too wet. It should mold like clay.

Source: concrete-catalog.com

### 4.3.1 Relative Desirability of Soils as Compacted Fill

RELATIVE DESIRABILITY OF SOILS AS COMPACTED FILL

| Group <br> Symbol |  | (NAVFAC DM-7.2, MAY 1982) <br> * if gravelly <br> ** erosion critical <br> ** volume change critical <br> -- not appropriate for this type of use <br> Soil Type | Relative Desirability for Various Uses <br> (1=best, 14=least desirable) |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Rolled Earth Fill Dams | Canal Sections |  | Foundations |  | Roadways |  |  |
|  |  |  | ¢ | $\begin{aligned} & \overline{\bar{\omega}} \\ & \stackrel{\rightharpoonup}{\omega} \end{aligned}$ |  |  |  |  | Fills |  |  |
|  |  |  |  |  |  |  |  |  |  |  |
|  | GW |  | Well-graded gravels, gravel/ sand mixtures, little or no fines | -- | -- | 1 | 1 | -- | -- | 1 | 1 | 1 | 3 |
|  | GP | Poorly-graded gravels, gravel/sand mixtures, little or no fines | -- | -- | 2 | 2 | -- | -- | 3 | 3 | 3 | -- |
|  | GM | Silty gravels, poorly-graded gravel/sand/silt mixtures | 2 | 4 | -- | 4 | 4 | 1 | 4 | 4 | 9 | 5 |
|  | GC | Clay-like gravels, poorly graded gravel/sand/clay mixtures | 1 | 1 | -- | 3 | 1 | 2 | 6 | 5 | 5 | 1 |
| $\begin{aligned} & \infty \\ & \stackrel{n}{2} \\ & \text { 心 } \end{aligned}$ | SW | Well-graded sands, gravelly sands, little or no fines | -- | -- | $3^{*}$ | 6 | -- | -- | 2 | 2 | 2 | 4 |
|  | SP | Poorly-graded sands, gravelly sands, little or no fines | -- | -- | 4* | 7* | -- | -- | 5 | 6 | 4 | -- |
|  | SM | Silty sands, poorly-graded sand/ silt mixtures | 4 | 5 | -- | 8* | 5** | 3 | 7 | 6 | 10 | 6 |
|  | SC | Clay-like sands, poorly-graded sand/clay mixtures | 3 | 2 | -- | 5 | 2 | 4 | 8 | 7 | 6 | 2 |
|  | ML | Inorganic silts and very fine sands, rock flour, silty or clay-like fine sands with slight plasticity | 6 | 6 | -- | -- | 6** | 6 | 9 | 10 | 11 | -- |
|  | CL | Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays | 5 | 3 | -- | 9 | 3 | 5 | 10 | 9 | 7 | 7 |
| $\frac{\curvearrowleft}{\frac{1}{\omega}}$ | OL | Organic silts and organic silt-clays of low plasticity | 8 | 8 | -- | -- | 7** | 7 | 11 | 11 | 12 | -- |
| $$ | MN | Organic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts | 9 | 9 | -- | -- | -- | 8 | 12 | 12 | 13 | -- |
| 安 | CH | Inorganic clays of high plasticity, fat clays | 7 | 7 | -- | 10 | $8^{* * *}$ | 9 | 13 | 13 | 8 | -- |
|  | OH | Organic clays of medium high plasticity | 10 | 10 | -- | -- | -- | 10 | 14 | 14 | 14 | -- |

FIGURE 5

### 4.3.2 Calculating the Bearing Capacity of Soils

## Bearing Capacity of Soils

The approximate ultimate bearing capacity under a long footing at the surface of a soil is given by Prandtl's equation:

$$
q_{n}=\left(\frac{c}{\tan \phi}\right)+\frac{1}{2} \gamma_{\mathrm{dry}} b \sqrt{K_{p}}\left(K_{p} e^{\pi \tan \phi}-1\right)
$$

where
$q_{u}=$ ultimate bearing capacity of soil, $\mathrm{lb} / \mathrm{ft}^{2}\left(\mathrm{~kg} / \mathrm{m}^{2}\right)$
$c=$ cohesion, $\mathrm{lb} / \mathrm{ft}^{2}\left(\mathrm{~kg} / \mathrm{m}^{2}\right)$
$\phi=$ angle of internal friction, degree
$\gamma_{\text {dry }}=$ unit weight of dry soil, $\mathrm{lb} / \mathrm{ft}^{3}\left(\mathrm{~kg} / \mathrm{m}^{3}\right)$
$b=$ width of footing, $\mathrm{ft}(\mathrm{m})$
$d=$ depth of footing below surface $\mathrm{ft}(\mathrm{m})$
$K_{p}=$ coefficient of passive pressure

$$
\begin{aligned}
& =\left[\tan \left(45+\frac{\phi}{2}\right)\right]^{2} \\
e & =2.718
\end{aligned}
$$

For footings below the surface, the ultimate bearing capacity of the soil may be modified by the factor $1+$ $\mathrm{Cd} / \mathrm{b}$. The coefficient C is about 2 for cohesionless soils and about 0.3 for cohesive soils. The increase in bearing capacity with depth for cohesive soils is often neglected.

By Permission: lengineeringcivil.com
Typically, the ratio is determined at $0.10-\mathrm{in}$. $(2.54-\mathrm{mm})$ penetration, although other penetrations sometimes are used. An excellent base course has a CBR of $100 \%$. A compacted soil may have a CBR of $50 \%$, whereas a weaker soil may have a CBR of 10 .

## 4) Soil Permeability

The coefficient of permeability $k$ is a measure of the rate of flow of water through saturated soil under a given hydraulic gradient $\mathrm{i}, \mathrm{cm} / \mathrm{cm}$, and is defined in accordance with Darcy's law as

$$
\mathrm{V}=\mathrm{kiA}
$$

where
$\mathrm{V}=$ rate of flow, $\mathrm{cm}^{3} / \mathrm{s}$, and $\mathrm{A}=$ cross-sectional area of soil conveying flow, $\mathrm{cm}^{2}$.
Coefficient k is dependent on the grain-size distribution, void ratio, and soil fabric and typically may vary from as much as $10 \mathrm{~cm} / \mathrm{s}$ for gravel to less than $10-7$ for clays. For typical soil deposits, k for horizontal flow is greater than k for vertical flow, often by an order of magnitude.

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## Settlement under Foundations

The approximate relationship between loads on foundations and settlement is

$$
\mathrm{q} / \mathrm{P}=\mathrm{C}_{1}(1+2 \mathrm{~d} / \mathrm{b})+\mathrm{C}_{2} / \mathrm{b}
$$

where
$\mathrm{q}=$ load intensity, $\mathrm{lb} / \mathrm{ft}^{2}\left(\mathrm{~kg} / \mathrm{m}^{2}\right)$
$\mathrm{P}=$ settlement, in (mm)
$\mathrm{d}=$ depth of foundation below ground surface, $\mathrm{ft}(\mathrm{m})$
$\mathrm{b}=$ width of foundation, $\mathrm{ft}(\mathrm{m})$
$\mathrm{C}_{1}=$ coefficient dependent on internal friction
$\mathrm{C}_{2}=$ coefficient dependent on cohesion
The coefficients $C_{1}$ and $C_{2}$ are usually determined by bearing plate loading tests.
Posted in Soil Engineering I 0 Comments

## Bearing Capacity of Soils

The approximate ultimate bearing capacity under a long footing at the surface of a soil is given by Prandtl's equation:

$$
q_{u}=\left(\frac{c}{\tan \phi}\right)+\frac{1}{2} \gamma_{\text {dry }} b \sqrt{K_{p}}\left(K_{p} e^{\pi \tan \phi}-1\right)
$$

### 4.3.3 Calculating Vibration Control

Explosive users should take steps to minimize vibration and noise from blasting and protect themselves against damage claims.

Vibrations caused by blasting are propagated with a velocity $\mathrm{V}, \mathrm{ft} / \mathrm{s}(\mathrm{m} / \mathrm{s})$, frequency $\mathrm{f}, \mathrm{Hz}$, and wavelength L , $\mathrm{ft}(\mathrm{m})$, related by

$$
\mathbf{L}=\mathbf{V} / \mathbf{f}
$$

Velocity v , in/s (mm/s), of the particles disturbed by the vibrations depends on the amplitude of the vibrations A, in (mm):

$$
\mathbf{v}=\mathbf{2 p f A}
$$

If the velocity $v_{1}$ at a distance $D_{1}$ from the explosion is known, the velocity $v_{2}$ at a distance $D_{2}$ from the explosion may be estimated from

$$
\mathrm{v}_{2} ? \mathrm{v}_{1}\left(\mathrm{D}_{1} / \mathrm{D}_{2}\right)^{1.5}
$$

The acceleration $\mathrm{a}, \mathrm{in} / \mathrm{s}^{2}\left(\mathrm{~mm} / \mathrm{s}^{2}\right)$, of the particles is given by

$$
\mathbf{a}=4 \mathbf{p}^{2} \mathbf{f}^{2} \mathbf{A}
$$

For a charge exploded on the ground surface, the overpressure $\mathrm{P}, \mathrm{lb} / \mathrm{in}^{2}(\mathrm{kPa})$, may be computed from

$$
\mathbf{P}=226.62\left(\mathbf{W}^{1 / 3} / D\right)^{1.407}
$$

where
$\mathrm{W}=$ maximum weight of explosives, $\mathrm{lb}(\mathrm{kg})$ per delay
$\mathrm{D}=$ distance, $\mathrm{ft}(\mathrm{m})$, from explosion to exposure.
The sound pressure level, decibels, may be computed from

$$
\mathrm{dB}=\left(\mathrm{P} /\left(6.95 \times 10^{-28}\right)\right)^{0.084}
$$

For vibration control, blasting should be controlled with the scaled-distance formula:

$$
\mathrm{V}=\mathrm{H}(\mathrm{D} / \ddot{\mathrm{O}} \mathrm{~W})^{-\mathrm{b}}
$$

where
$\mathrm{b}=$ constant (varies for each site)
$\mathrm{H}=$ constant (varies for each site).
Distance to exposure, $\mathrm{ft}(\mathrm{m})$, divided by the square root of maximum pounds $(\mathrm{kg})$ per delay, is known as scaled distance.

Most courts have accepted the fact that a particle velocity not exceeding $2 \mathrm{in} / \mathrm{s}(50.8 \mathrm{~mm} / \mathrm{s})$ does not damage any part of any structure. This implies that, for this velocity, vibration damage is unlikely at scaled distances larger than 8

### 4.3.4 Calculating Earth-Moving Equipment Production

Production is measured in terms of tons or bank cubic yards (cubic meters) of material a machine excavates and discharges, under given job conditions, in 1 h .

Production, bank $\mathrm{yd}^{3} / \mathrm{h}\left(\mathrm{m}^{3} / \mathrm{h}\right)=$ load, $\mathrm{yd}^{3}\left(\mathrm{~m}^{3}\right) \times$ trips per hour
Trips per hour $=$ working time, $\mathrm{min} / \mathrm{h} /$ cycle time, $\min$
The load, or amount of material a machine carries, can be determined by weighing or estimating the volume. Payload estimating involves determining the bank cubic yards (cubic meters) being carried, whereas the excavated material expands when loaded into the machine. For determination of bank cubic yards (cubic meters) from loose volume, the amount of swell or the load factor must be known.

Weighing is the most accurate method of determining the actual load. This is normally done by weighing one wheel or axle at a time with portable scales, adding the wheel or axle weights, and subtracting the weight empty. To reduce error, the machine should be relatively level. Enough loads should be weighed to provide a good average:

Bank $\mathrm{yd}^{3}=$ weight of load, $\mathrm{lb}(\mathrm{kg}) /$ density of material, $\mathrm{lb} / \mathrm{bank} \mathrm{yd}^{3}\left(\mathrm{~kg} / \mathrm{m}^{3}\right)$

## Equipment Required

To determine the number of scrapers needed on a job, required production must first be computed:
Production required, $\mathrm{yd}^{3} / \mathrm{h}\left(\mathrm{m}^{3} / \mathrm{h}\right)=$ quantity, bank $\mathrm{yd}^{3}\left(\mathrm{~m}^{3}\right) /$ working time, h
No. of scrapers needed $=$ production required, $\mathrm{yd}^{3} / \mathrm{h}\left(\mathrm{m}^{3} / \mathrm{h}\right) /$ production per unit, $\mathrm{yd}^{3} / \mathrm{h}\left(\mathrm{m}^{3} / \mathrm{h}\right)$
No. of scrapers a pusher can load $=$ scraper cycle time, min/pusher cycle time, min
Because speeds and distances may vary on haul and return, haul and return times are estimated separately.
Variable time, $\min =($ haul distance, $\mathrm{ft} / 88 \mathrm{Xspeed}, \mathrm{mi} / \mathrm{h}$ ) + (return distance, $\mathrm{ft} / 88 \mathrm{Xspeed}, \mathrm{mi} / \mathrm{h}$ )
Or
$=($ haul distance, $\mathrm{m} / 16.7 \times$ speed, $\mathrm{km} / \mathrm{h})+($ return distance, $\mathrm{m} / 16.7 \times$ speed, $\mathrm{km} / \mathrm{h})$
Haul speed may be obtained from the equipment specification sheet when the drawbar pull required is known.

Posted in Soil Engineering | 0 Comments

## Earth Quantities Hauled

When soils are excavated, they increase in volume, or swell, because of an increase in voids:

$$
\mathbf{V}_{\mathbf{b}}=\mathbf{V}_{\mathbf{b}} \mathrm{L}=(\mathbf{1 0 0} /(\mathbf{1 0 0}+\% \text { swell })) \mathbf{V}_{\mathbf{L}}
$$

where
$\mathrm{V}_{\mathrm{b}}=$ original volume, yd3 (m3), or bank yards
$\mathrm{V}_{\mathrm{L}}=$ loaded volume, yd3 (m3), or loose yards
$\mathrm{L}=$ load factor
When soils are compacted, they decrease in volume:

$$
\mathbf{V}_{\mathbf{c}}=\mathbf{V}_{\mathbf{b}} \mathbf{S}
$$

where
$\mathrm{V}_{\mathrm{c}}=$ compacted volume, $\mathrm{yd}^{3}\left(\mathrm{~m}^{3}\right)$
$\mathrm{S}=$ shrinkage factor.
Bank yards moved by a hauling unit equals weight of load, $\mathrm{lb}(\mathrm{kg})$, divided by density of the material in place, $\mathrm{lb}(\mathrm{kg})$, per bank yard $\left(\mathrm{m}^{3}\right)$.

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## Formulas for Earth Moving

External forces offer rolling resistance to the motion of wheeled vehicles, such as tractors and scrapers. The engine has to supply power to overcome this resistance; the greater the resistance is, the more power needed to move a load. Rolling resistance depends on the weight on the wheels and the tire penetration into the ground:

$$
\mathrm{R}=\mathrm{R}_{\mathrm{f}} \mathrm{~W}+\mathrm{R}_{\mathrm{p}} \mathrm{PW}
$$

where
$\mathrm{R}=$ rolling resistance, $\mathrm{lb}(\mathrm{N})$
$\mathrm{R}_{\mathrm{f}}=$ rolling-resistance factor, lb/ton (N/tonne)
$\mathrm{W}=$ weight on wheels, ton (tonne)
$\mathrm{R}_{\mathrm{p}}=$ tire-penetration factor, $\mathrm{lb} /$ ton in ( $\mathrm{N} /$ tonne mm ) penetration
$\mathrm{p}=$ tire penetration, in (mm)
$\mathrm{R}_{\mathrm{f}}$ usually is taken as $40 \mathrm{lb} /$ ton (or $2 \% \mathrm{lb} / \mathrm{lb}$ ) ( $173 \mathrm{~N} / \mathrm{t}$ ) and $\mathrm{R}_{\mathrm{p}}$ as $30 \mathrm{lb} / \mathrm{ton}$ in $(1.5 \% \mathrm{lb} / \mathrm{lb} \mathrm{in})(3288 \mathrm{~N} / \mathrm{mm})$.
Hence, the above equation can be written as

$$
\mathrm{R}=(2 \%+1.5 \% \mathrm{p}) \mathrm{W}^{\prime}=\mathrm{R}^{\prime} \mathrm{W}
$$

where

$$
\begin{aligned}
& \mathrm{W}^{\prime}=\text { weight on wheels, } \mathrm{lb}(\mathrm{~N}) \\
& \mathrm{R}^{\prime}=2 \%+1.5 \% \mathrm{p} .
\end{aligned}
$$

Additional power is required to overcome rolling resistance on a slope. Grade resistance also is proportional to weight:

$$
\mathrm{G}=\mathrm{R}_{\mathrm{g}} \mathrm{~s} \mathrm{~W}
$$

where
$\mathrm{G}=$ grade resistance, $\mathrm{lb}(\mathrm{N})$
$\mathrm{R}_{\mathrm{g}}=$ grade-resistance factor $=20 \mathrm{lb} /$ ton $(86.3 \mathrm{~N} / \mathrm{t})=1 \% \mathrm{lb} / \mathrm{lb}(\mathrm{N} / \mathrm{N})$
$\mathrm{s}=$ percent grade, positive for uphill motion. Negative for downhill
Thus, the total road resistance is the algebraic sum of the rolling and grade resistances, or the total pull, $\mathrm{lb}(\mathrm{N})$, required:

$$
\mathrm{T}=\left(\mathrm{R}^{\prime}+\mathrm{R}_{\mathrm{g}} \mathrm{~s}\right) \mathrm{W}^{\prime}=(2 \%+1.5 \% \mathrm{p}+1 \% \mathrm{~s}) \mathrm{W}^{\prime}
$$

In addition, an allowance may have to be made for loss of power with altitude. If so, allow $3 \%$ pull loss for each $1000 \mathrm{ft}(305 \mathrm{~m})$ above $2500 \mathrm{ft}(762 \mathrm{~m})$.

Usable pull P depends on the weight W on the drivers:

$$
\mathrm{P}=\mathrm{fW}
$$

where
$\mathrm{f}=$ coefficient of traction.

### 4.3.5 Calculating Production of Roller-Type Compaction Equipment

A wide variety of equipment is used to obtain compaction in the field. Sheepsfoot rollers generally are used on soils that contain high percentages of clay. Vibrating rollers are used on more granular soils.

To determine maximum depth of lift, make a test fill. In the process, the most suitable equipment and pressure to be applied, $\mathrm{lb} / \mathrm{in}^{2}(\mathrm{kPa})$, for ground contact also can be determined. Equipment selected should be able to produce desired compaction with four to eight passes. Desirable speed of rolling also can be determined.

Average speeds, $\mathrm{mi} / \mathrm{h}(\mathrm{km} / \mathrm{h})$, under normal conditions, are given in the table below.

| Type | $\mathbf{m i} / \mathbf{h}$ | $\mathbf{k m} / \mathbf{h}$ |
| :--- | :---: | ---: |
| Grid rollers | 12 | 19.3 |
| Sheepsfoot rollers | 3 | 4.8 |
| Tamping rollers | 10 | 16.1 |
| Pneumatic rollers | 8 | 12.8 |

Compaction production can be computed from

$$
\mathrm{yd}^{3} / \mathrm{h}\left(\mathrm{~m}^{3} / \mathrm{h}\right)=16 \mathrm{WSLFE} / \mathrm{P}
$$

where
$\mathrm{W}=$ width of roller, $\mathrm{ft}(\mathrm{m})$
$\mathrm{S}=$ roller speed, $\mathrm{mi} / \mathrm{h}(\mathrm{km} / \mathrm{h})$
$\mathrm{L}=$ lift thickness, in (mm)
$\mathrm{F}=$ ratio of pay $\mathrm{yd}^{3}\left(\mathrm{~m}^{3}\right)$ to loose $\mathrm{yd}^{3}\left(\mathrm{~m}^{3}\right)$
$\mathrm{E}=$ efficiency factor (allows for time losses, such as those due to turns); 0.90 , excellent; 0.80 , average; 0.75 , poor
$\mathrm{P}=$ number of passes

### 4.3.6 Compaction Equipment TypesApplications and Illustrations

## Equipment Types

## Rammers

Rammers deliver a high-impact force (high amplitude), making them an excellent choice for cohesive and semicohesive soils. Frequency range is 500 to 750 blows per minute. Rammers get compaction force from a small gasoline or diesel engine powering a large piston set with two sets of springs. The rammer is inclined at a forward angle to allow forward travel as the machine jumps. Rammers cover three types of compaction: impact, vibration, and kneading.

## Vibratory Plates

Vibratory plates are low amplitude and high frequency, designed to compact granular soils and asphalt. Gasoline or diesel engines drive one or two eccentric weights at a high speed to develop compaction force. The resulting vibrations cause forward motion. The engine and handle are vibration-isolated from the vibrating plate. The heavier the plate, the more compaction force it generates. Frequency range is usually 2500 vpm to 6000 vpm . Plates used for asphalt have a water tank and sprinkler system to prevent asphalt from sticking to the bottom of the baseplate. Vibration is the one principal compaction effect.

## Reversible Vibratory Plates

In addition to some of the standard vibratory plate features, reversible plates have two eccentric weights that allow smooth transition for forward or reverse travel,


EQUIPMENT APPLICATIONS

|  | Granular Soils | Sand and Clay | Cohesive Clay | Asphalt |
| :---: | :---: | :---: | :---: | :---: |
| Rammers |  | B | A |  |
| Vibratory Plates | A | B |  | A |
| Reversible Plates | B | A | C | C |
| Vibratory Rollers | B | A | C | A |
| Rammax Rollers | C | A | A |  |

A-Provides optimum performance for most applications.
B-Provides acceptable performance for most applications.
C-Limited performance for most applications. Testing required.

[^7]plus increased compaction force as the result of dual weights. Due to their weight and force, reversible plates are ideal for semicohesive soils.

A reversible is possibly the best compaction buy dollar for dollar. Unlike standard plates, the reversible's forward travel may be stopped, and the machine will maintain its force for "spot" compaction.

## Rollers

Rollers are available in several categories: walk-behind and ride-on, which are available as smooth drum, padded drum, and rubber-tired models; and are further divided into static and vibratory subcategories.


## Walk-behind

## Smooth

A popular design for many years, smooth-drum machines are ideal for both soil and asphalt. Dual steel drums are mounted on a rigid frame and powered by gasoline or diesel engines. Steering is done by manually turning the machine handle.

Frequency is around 4000 vpm , and amplitudes range from .018 to .020 . Vibration is provided by eccentric shafts placed in the drums or mounted on the frame.

## Padded

Padded rollers are also known as trench rollers owing to their effective use in trenches and excavations. These machines feature hydraulic or hydrostatic steering and operation. Powered by diesel engines, trench rollers are built to withstand the rigors of confined compaction. Trench rollers are either skid-steer or equipped with articulated steering. Operation can be by manual or remote control. Large eccentric units provide high-impact force and high amplitude (for rollers) that are appropriate for cohesive soils. The drum pads provide a kneading action on soil. Use these machines for high productivity.

## Ride-on

Configured as static steel-wheel rollers, ride-ons are used primarily for asphalt surface sealing and finishing work in the larger ( 8 to 15 ton) range. Small ride-on units are used for patch jobs with thin lifts.

The trend is toward vibratory rollers. Tandem vibratory rollers are usually found with drum widths of $30^{\prime \prime}$ up to $110^{\prime \prime}$, with the most common being $48^{\prime \prime}$.

Suitable for soil, sub-base, and asphalt compaction, tandem rollers use the dynamic force of eccentric vibrator assemblies for high-production work. Single-drum machines feature a single vibrating drum with pneumatic drive wheels. The drum is available as smooth for sub-base or rock fill, or padded for soil compaction. In addition, a ride-on version of the pad foot trench roller is available for very high productivity in confined areas, with either manual or remote control operation.

## Rubber-tire

These rollers are equipped with 7 to 11 pneumatic tires, with the front and rear tires overlapping. A static roller by nature, compaction force is altered by the addition or removal of weight added as ballast in the form of water or sand. Weight ranges vary from 10 to 35 tons. The compaction effort is pressure and kneading, primarily with asphalt finish rolling. Tire pressures on some machines can be decreased while rolling to adjust ground contact pressure for different job conditions.

## Safety and General Guidelines

As with all construction equipment, many safety practices should be followed while using compaction equipment. While this handbook is not designed to cover all aspects of job site safety, we wish to mention some of the more obvious items in regard to compaction equipment. Ideally, equipment operators should familiarize themselves with all of their company's safety regulations, as well as any OSHA, state agency, or local agency regulations pertaining to job safety. Basic personal protection, consisting of durable work gloves, eye protection, ear protection, and approved hard hat and work clothes, should be standard issue on any job and available for immediate use.

In the case of walk-behind compaction equipment, additional toe protection devices should be available, depending on applicable regulations. All personnel operating powered compaction equipment should read all operating and safety instructions for each piece of equipment. In addition, training should be provided so that the operator is aware of all aspects of operation.

No minors should be allowed to operate construction equipment. No operator should run construction equipment when under the influence of medication, illegal drugs, or alcohol. Serious injury or death could occur as a result of improper use or neglect of safety practices and attitudes. This applies to both the new worker and the seasoned professional.

## Shoring

Trench work brings a new set of safety practices and regulations for the compaction equipment operator. This section does not intend to cover the regulations pertaining to trench safety (OSHA Part 1926, Subpart P). The operator should have knowledge of what is required before compacting in a trench or confined area. Be certain a "competent person"' (as defined by OSHA in Part 1926.650 revised July 1, 1998) has inspected the trench and follows the OSHA guidelines for inspection during the duration of the job. Besides the obvious danger of a trench cave-in, the worker must also be protected from falling objects. Unshored (or shored) trenches can be compacted with the use of remote control compaction equipment. This allows the operator to stay outside the trench while operating the equipment.

Safety first!

### 4.3.6.1 List of Compaction Measuring Devices by Type and Manufacturer

|  | Device | Distributor/Manufacturer |
| :---: | :---: | :---: |
| 1 | Nuclear Density Gauge-[Troxler Model 3440] | Troxler Electronic Laboratories Contact: Michael Dixon 1430 Brook Dr. <br> Downers Grove, IL 60515 Phone: 630-261-9304 |
| 2 | Sand-Cone Density Apparatus | Humboldt Mfg. Co. 7300 West Agatite Ave. <br> Norridge, IL 60706 <br> Phone: 800-544-7220 |
| 3 | Soil Compaction Supervisor [SCS] | MBW Incorporated Contact: Frank Multerer P.O. Box 440 Slinger, WI 53086 Phone: 800-678-5237 |
| 4 | Dynamic Cone Penetrometer [Utility DCP] | SGS Manufacturing Contact: Sandy Golgart 4391 Westgrove Dr. <br> Addison, TX 75001 <br> Phone: 800-526-0747 |
| 5 | Dynamic Cone Penetrometer [Standard DCP] | Kessler Instruments, Inc. 160 Hicks St. <br> Westbury, NY 11590 <br> Phone: 516-334-4063 |
| 6 | Geogauge | Humboldt Mfg. Co. 7300 West Agatite Ave. <br> Norridge, IL 60706 <br> Phone: 800-544-7220 |
| 7 | Clegg Hammer [10-kg \& 20-kg Hammers] | Lafayette Instruments Contact: Paul Williams P.O. Box 5729 Lafayette, IN 47903 Phone: 765-423-1505 |
| 8 | PANDA | Sol Solution <br> Contact: www.sol-solution.com 115 Old Short Hills Rd., Apt. 306 West Orange, NJ 07052 Phone: 973-243-7237 |

[^8]
### 4.3.6.2 Moisture Density Relation—Compaction Test—Proctor and Modified Proctor

## Purpose

- This laboratory test is performed to determine the relationship between the moisture content and the dry density of a soil for a specified compactive effort. The compactive effort is the amount of mechanical energy that is applied to the soil mass. Several different methods are used to compact soil in the field; some examples include tamping, kneading, vibration, and static load compaction. This laboratory will employ the tamping or impact compaction method using the type of equipment and methodology developed by R. R. Proctor in 1933. Therefore, the test is also known as the Proctor test.
- Two types of compaction tests are routinely performed: (1) the Standard Proctor Test, and (2) the Modified Proctor Test. Each of these tests can be performed by three different methods as outlined in the attached table. In the Standard Proctor Test, the soil is compacted by a $5.5-\mathrm{lb}$ hammer falling a distance of 1 foot into a soil-filled mold. The mold is filled with three equal layers of soil, and each layer is subjected to 25 drops of the hammer. The Modified Proctor Test is identical to the Standard Proctor Test except that it employs a $10-\mathrm{lb}$ hammer falling a distance of 18 inches and uses five equal layers of soil instead of three. Two types of compaction molds are used for testing. The smaller type is 4 in . in diameter and has a volume of about $1 / 30 \mathrm{ft}^{3}\left(944 \mathrm{~cm}^{3}\right)$, and the larger type is 6 in . in diameter and has a volume of about $1 / 13.333 \mathrm{ft}^{3}$ $\left(2123 \mathrm{~cm}^{3}\right)$. If the larger mold is used, each soil layer must receive 56 blows instead of 25 (see table).

Source: Prof. Krishna Reddy, University of Illinois, Chicago

## Alternative Proctor Test Methods

|  | Standard Proctor ASTM 698 |  |  | Modified Proctor ASTM 1557 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Method A | Method B | Method C | Method A | Method B | Method C |
| Material | $\leq 20 \%$ <br> Retained on No. 4 Sieve | $>20 \%$ Retained on No. $4 \leq 20 \%$ <br> Retained on 3/8" Sieve | $>20 \%$ Retained on No.3/8" $<30 \%$ Retained on $3 / 4^{\prime \prime}$ Sieve | $\leq 20 \%$ <br> Retained on No. 4 Sieve | $>20 \%$ Retained on No. $4 \leq 20 \%$ Retained on 3/8" Sieve | $>20 \%$ Retained on No.3/8" $<30 \%$ Retained on $3 / 4^{\prime \prime}$ Sieve |
| For test sample, use soil passing | Sieve No. 4 | $3 / 8^{\prime \prime}$ Sieve | $3 / 4^{\prime \prime}$ Sieve | Sieve No. 4 | $3 / 8^{\prime \prime}$ Sieve | $3 / 4^{\prime \prime}$ Sieve |
| Mold | $4^{\prime \prime}$ DIA | $4^{\prime \prime}$ DIA | $6^{\prime \prime}$ DIA | $4^{\prime \prime}$ DIA | $4^{\prime \prime}$ DIA | $6^{\prime \prime}$ DIA |
| No. of Layers | 3 | 3 | 3 | 5 | 5 | 5 |
| No. of blows/layer | 25 | 25 | 56 | 25 | 25 | 56 |

Note: Volume of $4^{\prime \prime}$ diameter mold $=944 \mathrm{~cm}^{3}$, Volume of $6^{\prime \prime}$ diameter mold $=2123 \mathrm{~cm}^{3}$ (verify these values prior to testing)

### 4.4.0 Calculating the Maximum Dry Density and Optimum Moisture Content of Soil

This test is done to determine the maximum dry density and the optimum moisture content of soil using heavy compaction as per IS: 2720 (Part 8)-983. The apparatus used is
i) Cylindrical metal mould-it should be either of 100 mm dia. and 1000 cc volume or 150 mm dia. and 2250 cc volume and should conform to IS: 10074-1982.
ii) Balances-one of 10 kg capacity, sensitive to 1 g and the other of 200 g capacity, sensitive to 0.01 g .
iii) Oven-thermostatically controlled with an interior of noncorroding material to maintain temperature between 105 and $110^{\circ} \mathrm{C}$.
iv) Steel straightedge- 30 cm long.
v) IS Sieves of sizes- $4.75 \mathrm{~mm}, 19 \mathrm{~mm}$, and 37.5 mm .

## Preparation of Sample

A representative portion of air-dried soil material, large enough to provide about 6 kg of material passing through a 19 mm IS Sieve (for soils not susceptible to crushing during compaction) or about 15 kg of material passing through a 19 mm IS Sieve (for soils susceptible to crushing during compaction) should be taken. This portion should be sieved through a 19 mm IS Sieve, and the coarse fraction rejected after its proportion of the total sample has been recorded. Aggregations of particles should be broken down so that if the sample was sieved through a 4.75 mm IS Sieve, only separated individual particles would be retained.

## Procedure to Determine the Maximum Dry Density and the Optimum Moisture Content of Soil

A) Soil not susceptible to crushing during compaction
i) A 5 kg sample of air-dried soil passing through the 19 mm IS Sieve should be taken. The sample should be mixed thoroughly with a suitable amount of water depending on the soil type (for sandy and gravelly soil- 3 to $5 \%$ and for cohesive soil- 12 to $16 \%$ below the plastic limit). The soil sample should be stored in a sealed container for a minimum period of 16 hrs .
ii) The mold of 1000 cc capacity with base plate attached should be weighed to the nearest $1 \mathrm{~g}\left(\mathrm{~W}_{1}\right)$. The mold should be placed on a solid base, such as a concrete floor or plinth, and the moist soil should be compacted into the mold, with the extension attached, in five layers of approximately equal mass, each layer being given 25 blows from the 4.9 kg rammer dropped from a height of 450 mm above the soil. The blows should be distributed uniformly over the surface of each layer. The amount of soil used should be sufficient to fill the mold, leaving not more than about 6 mm to be struck off when the extension is removed. The extension should be removed, and the compacted soil should be leveled off carefully to the top of the mold by means of the straightedge. The mold and soil should then be weighed to the nearest gram $\left(W_{2}\right)$.
iii) The compacted soil specimen should be removed from the mold and placed onto the mixing tray. The water content ( w ) of a representative sample of the specimen should be determined.
iv) The remaining soil specimen should be broken up, rubbed through 19 mm IS Sieve, and then mixed with the remaining original sample. Suitable increments of water should be added successively and mixed into the sample, and the above operations (i.e., ii to iv) should be repeated for each increment of water added. The total number of determinations made should be at least five, and the moisture content should be such that the optimum moisture content at which the maximum dry density occurs lies within that range.
B) Soil susceptible to crushing during compaction

Five or more 2.5 kg samples of air-dried soil passing through the 19 mm IS Sieve should be taken. The samples should each be mixed thoroughly with different amounts of water and stored in a sealed container as mentioned in Part A.
C) Compaction in large size mold

For compacting soil containing coarse material up to 37.5 mm size, the 2250 cc mold should be used. A sample weighing about 30 kg and passing through the 37.5 mm IS Sieve is used for the test. Soil is compacted in five layers, each layer being given 55 blows of the 4.9 kg rammer. The rest of the procedure is the same as above.

## Reporting of Results

Bulk density $\mathrm{Y}(\mathrm{gamma})$ in $\mathrm{g} / \mathrm{cc}$ of each compacted specimen should be calculated from the equation,

$$
\mathbf{Y}(\text { gamma })=\left(\mathbf{W}_{2}-\mathbf{W}_{1}\right) / \mathbf{V}
$$

where
$\mathrm{V}=$ volume in cc of the mold.
The dry density Yd in $\mathrm{g} / \mathrm{cc}$

$$
\mathbf{Y d}=\mathbf{1 0 0} Y /(\mathbf{1 0 0}+\mathbf{w})
$$

The dry densities Yd obtained in a series of determinations should be plotted against the corresponding moisture contents, w. A smooth curve should be drawn through the resulting points, and the position of the maximum on the curve should be determined. A sample graph is shown below:


The dry density in $\mathrm{g} / \mathrm{cc}$ corresponding to the maximum point on the moisture content/dry density curve should be reported as the maximum dry density to the nearest 0.01 . The percentage moisture content corresponding to the maximum dry density on the moisture content/dry density curve should be reported as the optimum moisture content and quoted to the nearest 0.2 for values below $5 \%$, to the nearest 0.5 for values from 5 to $10 \%$, and to the nearest whole number for values exceeding $10 \%$.

## Water Pressure

The total thrust from water retained behind a wall is

$$
P=1 / 2 g_{o} H^{2}
$$

where
$\mathrm{H}=$ height of water above bottom of wall, $\mathrm{ft}(\mathrm{m})$; and
$\mathrm{g}_{\mathrm{o}}=$ unit weight of water, $\mathrm{lb} / \mathrm{ft}^{3}\left(62.4 \mathrm{lb} / \mathrm{ft}^{3}(1001 \mathrm{~g} / \mathrm{m})\right.$ for fresh water and $64 \mathrm{lb} / \mathrm{ft}^{3}\left(1026.7 \mathrm{~kg} / \mathrm{m}^{3}\right)$ for saltwater)

The thrust is applied at a point $\mathrm{H} / 3$ above the bottom of the wall, and the pressure distribution is triangular, with the maximum pressure of $2 \mathrm{P} / \mathrm{H}$ occurring at the bottom of the wall. Regardless of the slope of the surface behind the wall, the thrust from water is always horizontal.

## Lateral Pressures in Cohesive Soils

For walls that retain cohesive soils and are free to move a considerable amount over a long period of time, the total thrust from the soil (assuming a level surface) is

$$
P=1 / 2 g H^{2} K_{A}-2 c H K_{A}^{1 / 2}
$$

or because highly cohesive soils generally have small angles of internal friction,

$$
P=1 / 2 g H^{2} K_{A}-2 c H K_{A}^{1 / 2}
$$

The thrust is applied at a point somewhat below $\mathrm{H} / 3$ from the bottom of the wall, and the pressure distribution is approximately triangular.

For walls that retain cohesive soils and are free to move only a small amount or not at all, the total thrust from the soil is

$$
P=1 / 2 g H^{2} K_{P}
$$

because the cohesion would be lost through plastic flow.

## Lateral Pressures in Cohesionless Soils

For walls that retain cohesionless soils and are free to move an appreciable amount, the total thrust from the soil is measurable.

## Lateral Pressures in Soils

## Lateral Pressures in Soils, Forces on Retaining Walls

The Rankine theory of lateral earth pressures, used for estimating approximate values for lateral pressures on retaining walls, assumes that the pressure on the back of a vertical wall is the same as the pressure that would exist on a vertical plane in an infinite soil mass. Friction between the wall and the soil is neglected. The pressure on a wall consists of (1) the lateral pressure of the soil held by the wall, (2) the pressure of the water (if any) behind the wall, and (3) the lateral pressure from any surcharge on the soil behind the wall.

### 4.4.1 Calculating the In Situ Dry Density of Soil by the Sand Replacement Method

This test is done to determine the in situ dry density of soil by the sand replacement method as per IS: 2720 (Part XXVIII)—1974. The apparatus needed is
i) Sand-pouring cylinder conforming to IS: 2720 (Part XXVIII)—1974
ii) Cylindrical calibrating container conforming to IS: 2720 (Part XXVIII)—1974
iii) Soil-cutting and excavating tools such as a scraper tool, bent spoon
iv) Glass plate- 450 mm square and 9 mm thick or larger
v) Metal containers to collect excavated soil
vi) Metal tray- 300 mm square and 40 mm deep with a 100 mm hole in the center
vii) Balance, with an accuracy of 1 g

## Procedure to Determine the In Situ Dry Density of Soil by the Sand Replacement Method

A. Calibration of apparatus
a) The method given below should be followed for determining the weight of sand in the cone of the pouring cylinder:
i) The pouring cylinder should be filled so that the level of the sand in the cylinder is within about 10 mm of the top. Its total initial weight $\left(\mathrm{W}_{1}\right)$ should be maintained constant throughout the tests for which the calibration is used. A volume of sand equivalent to that of the excavated hole in the soil (or equal to that of the calibrating container) should be allowed to run out of the cylinder under gravity. The shutter of the pouring cylinder should then be closed and the cylinder placed on a plain surface, such as a glass plate.
ii) The shutter of the pouring cylinder should be opened and sand allowed to run out. When no further movement of sand takes place in the cylinder, the shutter should be closed and the cylinder removed carefully.
iii) The sand that had filled the cone of the pouring cylinder (that is, the sand that is left on the plain surface) should be collected and weighed to the nearest gram.
iv) These measurements should be repeated at least three times and the mean weight $\left(\mathrm{W}_{2}\right)$ taken.
b) The method described below should be followed for determining the bulk density of the sand (Ys):
i) The internal volume $(\mathrm{V})$ in ml of the calibrating container should be determined from the weight of water contained in the container when filled to the brim. The volume may also be calculated from the measured internal dimensions of the container.
ii) The pouring cylinder should be placed concentrically on the top of the calibrating container after being filled to the constant weight $\left(\mathrm{W}_{1}\right)$. The shutter of the pouring cylinder should be closed during the operation. The shutter should be opened and sand allowed to run out. When no further movement of sand takes place in the cylinder, the shutter should be closed. The pouring cylinder should be removed and weighed to the nearest gram.
iii) These measurements should be repeated at least three times and the mean weight $\left(\mathrm{W}_{3}\right)$ taken.
B. Measurement of soil density

The following method should be followed for the measurement of soil density:
i) A flat area, approximately 450 sq mm of the soil to be tested, should be exposed and trimmed down to a level surface, preferably with the aid of the scraper tool.
ii) The metal tray with a central hole should be laid on the prepared surface of the soil with the hole over the portion of the soil to be tested. The hole in the soil should then be excavated using the hole in the tray as a pattern, to the depth of the layer to be tested up to a maximum of 150 mm . The excavated soil should be carefully collected, leaving no loose material in the hole and weighed to the nearest gram ( $\mathrm{W}_{\mathrm{w}}$ ). The metal tray should be removed before the pouring cylinder is placed in position over the excavated hole.
iii) The water content (w) of the excavated soil should be determined as discussed in earlier posts. Alternatively, the whole of the excavated soil should be dried and weighed $\left(\mathrm{W}_{\mathrm{d}}\right)$.
iv) The pouring cylinder, filled to the constant weight $\left(\mathrm{W}_{1}\right)$, should be so placed that the base of the cylinder covers the hole concentrically. The shutter should then be opened and sand allowed to run out into the hole. The pouring cylinder and the surrounding area should not be vibrated during this period. When no further movement of sand takes place, the shutter should be closed. The cylinder should be removed and weighed to the nearest gram $\left(W_{4}\right)$.

## Calculations

i) The weight of sand $\left(\mathrm{W}_{\mathrm{a}}\right)$ in grams required to fill the calibrating container should be calculated from the formula:

$$
\mathrm{W}_{\mathrm{a}}=\mathrm{W}_{1}-\mathrm{W}_{3}-\mathrm{W}_{2}
$$

ii) The bulk density of the sand $\left(\gamma_{\mathrm{s}}\right)$ in $\mathrm{kg} / \mathrm{m}^{3}$ should be calculated from the formula:

$$
\gamma_{s}=\frac{\mathrm{W}_{\mathrm{a}}}{\mathrm{~V}} \times 1000
$$

iii) The weight of sand $\left(\mathrm{W}_{\mathrm{b}}\right)$ in grams required to fill the excavated hole should be calculated from the formula:

$$
\mathrm{W}_{\mathrm{b}}=\mathrm{W}_{1}-\mathrm{W}_{4}-\mathrm{W}_{2}
$$

iv) The bulk density $\left(\gamma_{\mathrm{b}}\right)$, that is, the weight of the wet soil per cubic meter should be calculated from the formula:

$$
\gamma_{\mathrm{b}}=\frac{\mathrm{W}_{\mathrm{w}}}{\mathrm{~W}_{\mathrm{b}}} \times \gamma_{\mathrm{a}} \mathrm{~kg} / \mathrm{m}^{3}
$$

v) The dry density $\left(\gamma_{d}\right)$, that is, the weight of dry soil per cubic meter should be calculated from the formula:

$$
\begin{aligned}
& \gamma_{\mathrm{d}}=\frac{100 \gamma_{\mathrm{b}}}{100-\mathrm{w}} \mathrm{~kg} / \mathrm{m}^{3} \\
& \gamma_{\mathrm{d}}=\frac{\mathrm{W}_{\mathrm{d}}}{\mathrm{~W}_{\mathrm{b}}} \times \gamma_{\mathrm{b}} \mathrm{~kg} / \mathrm{m}^{3}
\end{aligned}
$$

## Reporting of results

The following values should be reported:
i) dry density of soil in $\mathrm{kg} / \mathrm{m}^{3}$ to the nearest whole number, also to be calculated and reported in $\mathrm{g} / \mathrm{cc}$ correct to the second place of decimal
ii) water content of the soil in percent reported to two significant figures

A sample pro forma for the record of the test results is given below.

## In Situ Dry Density of Soil by Sand Replacement Method

Calibration of Apparatus

| S. No. | Description | Determination |
| :--- | :--- | :---: |
| 1 | Mean weight of sand in cone (of pouring cylinder) $\left(\mathrm{W}_{2}\right)$ in g | 450 |
| 2 | Volume of calibrating container $(\mathrm{V})$ in ml | 980 |
| 3 | Weight of sand + Cylinder, before pouring $\left(\mathrm{W}_{1}\right)$ in g | 11040 |
| 4 | Mean weight of sand + Cylinder, after pouring $\left(\mathrm{W}_{3}\right)$ in g | 9120 |
| 5 | Weight of sand to fill calibrating container $\left(\mathrm{W}_{\mathrm{a}}=\mathrm{W}_{1}-\mathrm{W}_{3}-\mathrm{W}_{2}\right)$ in g | 1470 |
| 6 | Bulk density of sand $\gamma_{\mathrm{s}}=\frac{\mathrm{W}_{\mathrm{a}}}{\mathrm{V}} \times 1000 \mathrm{~kg} / \mathrm{m}^{3}$ | $=$ |

Source: www.engineeringcivil.com

| Measurement of Soil Density |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| S. No. | Observation and Calculations | Determination No. |  |  |
|  |  | 1 | II | III |
| 1 | Weight of wet soil from the hole ( $\mathrm{W}_{\mathrm{w}}$ ) in g | 2310 | 2400 | 2280 |
| 2 | Weight of sand + Cylinder, before pouring ( $\mathrm{W}_{1}$ ) in g | 11040 | 11042 | 11037 |
| 3 | Weight of sand + Cylinder, after pouring ( $\mathrm{W}_{4}$ ) in g | 8840 | 8752 | 8882 |
| 4 | Weight of sand in the hole $\left(W_{b}=W_{1}-W_{4}-W_{2}\right)$ in $g$ | 1750 | 1840 | 1705 |
| 5 | Bulk density $\gamma_{\mathrm{b}}=\frac{\mathrm{W}_{\mathrm{w}}}{\mathrm{W}_{\mathrm{b}}} \times \gamma_{\mathrm{s}} \mathrm{kg} / \mathrm{m}^{3}$ | 1980 | 1956.5 | 2005.8 |
| 6 | Water content (w) in \% | 18.48 | 18.81 | 19.26 |
|  | Dry density $\gamma_{d}=\frac{100 \gamma_{b}}{100+w} \mathrm{~kg} / \mathrm{m}^{3}$ | 1671.17 | 1646.75 | 1681.87 |
| Dry density (Average value) |  |  | $1667 \mathrm{~kg} / \mathrm{m}$ |  |

Note: The figures given in the above tables are for illustration purposes only.

### 4.5.0 Calculating the Percent of Slope

Purpose: To apply measuring and math skills in determining the steepness of a slope.
Materials: Yard (meter) stick or measuring tape, a straight stick or board, carpenters level, or flat bottle half filled with colored water
TEKS: $\quad$ 5.1A 5.2A,B,C,D 5.4A,B 5.8B
Procedure: Slope is expressed in percent, meaning the number of units the land falls (or rises) in 100 units of horizontal distance. The higher the percent, the steeper the slope. Example: A slope that drops 10 vertical feet in 100 horizontal feet is a $10 \%$ slope (vertical drop/horizontal distance times 100).
A transit is the most accurate instrument for measuring slope.
A transit is a telescopic sighting instrument mounted on a tripod that has adjustable legs and gears for leveling the telescope. Usually a transit is not available for student use. You can measure how steep a slope is using some simple materials.
Place the 50 -inch stick horizontally on the ground. Put the level on the 50 -inch stick, and move the lower end of the stick up until the bubble shows that the stick is level. Measure the distance from the ground to the end of the level 50 -inch stick in inches. To determine percent on slope, you may divide the distance from ground to end of level stick by 50 inches and multiply by 100 , or do it the simple way and multiply the distance from ground to end of level stick by two. Slope is a very important land feature. It often determines whether a piece of land should be used for grass, trees, or cultivated crops. Slope also determines the rate at which water flows downhill. Water flows slowly over a gentle slope and rapidly over a steep one. The steepness of a slope can be evaluated as follows, according to the United States Department of Agriculture's Soil Conservation Service:
Nearly Level ( $0-2 \%$ ). Has no limitation on its uses. Any limitations are the result of other factors, such as drainage.

Gently Sloping (3-6\%). Desirable for almost any type of development; may have erosion problems; limitations are due mostly to factors other than slope.
Moderately Sloping (6-12\%). May have severe erosion problems and has a strong appeal for single-family development.
Strongly Sloping (12-18\%). Has severe limitations for all
Source: www.co.bell.tx.us

### 4.5.0.1 Calculating Grade from a Map

One way to calculate the grade of a hill is with a map that shows the altitudes of locations.
For example, you've measured out a distance of 3 miles (run) with a change in altitude of 396 feet (rise). First, the units must be made consistent, so we convert 3 miles to 15,840 feet.

$$
\begin{aligned}
& \text { grade }=(\text { rise } \div \text { run }) * 100 \\
& \text { grade }=(396 \div 15,840) * 100=2.5 \%
\end{aligned}
$$

### 4.5.0.2 Calculating Grade by Measuring the Road Distance

With an altimeter and an odometer, we travel the exact route we measured on the map, and our altimeter indicates a change in altitude of 396 feet which, not surprisingly, is precisely what we had already measured on the map. However, there is a small difference between the 3-mile distance measured on the map and the 3.0009375 miles $(15,844.95$ feet $)$ we just traveled on the road. The map distance is the true horizontal distance, but the travel distance of 3.0009375 miles is the slope length or slope distance. To calculate the true run, we need to use the Pythagorean Theorem.

$$
\begin{aligned}
& \text { run }=\text { Square Root }\left(15,844.95^{2}-396^{2}\right) \\
& \text { run }=15,840 \text { feet }
\end{aligned}
$$

Now we can calculate the grade $=(396 \div 15840) * 100=2.5 \%$
The slope angle exactly equals what we previously calculated because instead of using the slope length as the run, we used it to calculate the true horizontal distance.

### 4.5.0.3 Calculating Grade by Using Slope Distance



Bicyclists, motorists, carpenters, roofers, and others either need to calculate slope or at least must have some understanding of it.

Slope, tilt, or inclination can be expressed in three ways:

1) As a ratio of the rise to the run (for example, 1 in 20)
2) As an angle (almost always in degrees)
3) As a percentage called the "grade" which is the (rise $\div$ run) * 100

Of these three ways, slope is expressed as a ratio or a grade much more often than an actual angle, and here's the reason why:

Stating a ratio such as 1 in 20 tells you immediately that for every 20 horizontal units traveled, your altitude increases 1 unit.

Stating this as a percentage, whatever horizontal distance you travel, your altitude increases by $5 \%$ of that distance.

Stating this as an angle of 2.8624 degrees doesn't give you much of an idea how the rise compares to the run.


By permisssion: wolf@1728.com:

### 4.5.0.4 Formulas Showing Grade, Ratio, and Angle Relationships

1) If we know the ratio of a road or highway (for example, 1 in 20), then angle $A=\operatorname{arctangent}$ (rise $\div$ run) which equals

- arctangent $(1 \div 20)=$
- arctangent $(.05)=$
- 2.8624 degrees and the
- grade $=($ rise $\div$ run $) * 100$ which equals
- $(1 \div 20) * 100=$
- $5 \%$.

2) If we know the angle of a road or highway (for example, 3 degrees), then the ratio $=1$ in $(1 \div \tan (\mathrm{A})$ ), which equals

- 1 in $(1 \div \tan (3))=$
- 1 in $(1 \div .052408)=$
- 1 in 19.081 and the
- grade $=($ rise $\div$ run $) * 100$ which equals
- $(1 \div 19.081) * 100=$
- $5.2408 \%$

3) If we know the grade of a road (for example, $3 \%$ ), then angle $A=\operatorname{arctangent}$ (rise $\div$ run), which equals

- arctangent $(.03)=$
- 1.7184 degrees and the
- ratio $=1$ in $(1 \div \tan (\mathrm{A}))$ which equals
- 1 in $(1 \div \tan (1.7184))=$
- 1 in $(1 \div .03)=$
- 1 in 33.333


### 4.5.0.5 Chart Showing Slope Angles-0 Degrees to 80 Degrees

The graph toward the top of the page shows a small range of angles from 0 to 20 degrees.
This chart covers a wider range:


### 4.5.1 Illustration of Slope Layback

## Layback

Layback for excavation is given in units of run to rise. However, it can be specified in terms of either variable run to a constant rise of one (1), or it can be given in a constant run (1) to variable rise.

A layback of zero ( 0 ) is simply an excavation with vertical walls (no slope to it).


Here are some common laybacks and their equivalents:
$\mathbf{1} / \mathbf{2}: 1$ is the same as $\mathbf{1 : 2}$
3/4:1 is the same as $\mathbf{1 : 1} 1 / 3$
1:1 is a $45^{\circ}$ slope
$\mathbf{1 - 1 / 2}: \mathbf{1}$ is the same as $\mathbf{1 :} \mathbf{2 / 3}$

By permission: www.mc2-ice.com

### 4.5.2 Common Stable Slope Ratios for Varying Soil/Rock Conditions

Low-Volume Roads BMPs: 105

TABLE 11.1 Common Stable Slope Ratios for Varying Soil/Rock Conditions

| Soil/Rock Condition | Slope Ratio (Hor:Vert) |
| :--- | :--- |
| Most rock | $1 / 4: 1$ to $1 / 2: 1$ |
| Very well cemented soils | $1 / 4: 1$ to $1 / 2: 1$ |
| Most in-place soils | $3 / 4: 1$ to $1: 1$ |
| Very fractured rock | $1: 1$ to $11 / 2: 1$ |
| Loose coarse granular soils | $11 / 2: 1$ |
| Heavy clay soils | $2: 1$ to $3: 1$ |
| Soft clay rich zones or wet seepage areas | $2: 1$ to $3: 1$ |
| Fills of most soils | $11 / 2: 1$ to $2: 1$ |
| Fills of hard, angular rock | $11 / 3: 1$ |
| Low cuts and fills $(<2-3 \mathrm{~m}$. high) | $2: 1$ or flatter (for revegetation) |

### 4.5.3 Illustrations of Various Cut/Fill Configurations-Typical Fill, Benched Fill, Reinforced Fill



Source: U.S. Bureau of Land Management

### 4.5.4 Illustrationsn of Various Cut/Fill Configurations-Balanced Cut-Fill, Full Cut, Through Cut



Source:U.S. Bureau of Land Management

### 4.5.5 Calculating the Design of Gabion Retaining Walls to 20 Feet (6 Meters) in Height



Flat Backfill (smooth face)


| No. of <br> levels | H | B | No. of <br> gabions <br> (per <br> width) |
| :---: | :---: | :---: | :---: |
| 1 | $3^{\prime} 3^{\prime \prime}$ | $3^{\prime} 3^{\prime \prime}$ | 1 |
| 2 | $6^{\prime} 6^{\prime \prime}$ | $4^{\prime} 11^{\prime \prime}$ | $11 / 2$ |
| 3 | $9^{\prime} 9^{\prime \prime}$ | $6^{\prime} 6^{\prime \prime}$ | 2 |
| 4 | $13^{\prime} 1^{\prime \prime}$ | $8^{\prime} 2^{\prime \prime}$ | $21 / 2$ |
| 5 | $16^{\prime} 4^{\prime \prime}$ | $9^{\prime} 9 \prime$ | 3 |
| 6 | $19^{\prime \prime} 7 \prime \prime$ | $11^{\prime \prime} 5^{\prime \prime}$ | $31 / 2$ |

Fill at 1 1/2:1 (face with steps)

Note: Loading conditions are for silty sand to sand and gravel backfill. For finer or clay rich soils, earth pressure on the wall will increase, and the wall base width (B) will have to increase for each height. Backfill weight $=$ $110 \mathrm{pcf}\left(1.8\right.$ tons $\left./ \mathrm{m}^{3}\right)\left(1,762 \mathrm{~kg} / \mathrm{m}^{3}\right)$

- Safe against overturning for soils with a minimum bearing capacity of $2 \mathrm{Tons} / \mathrm{foot}^{2}\left(19,500 \mathrm{~kg} / \mathrm{m}^{2}\right)$
- For flat or sloping backfills, either a flat or stepped face may be used.

Standard design for Gabion Retaining Structures up to 20 feet in height (6 meters) with flat or sloping backfill.

Source: U.S. Bureau of Land Management

### 4.5.6 Calculating the Design of Common Types of Retaining Structures


a. Common Types of Retaining Structures.


Source:U.S. Bureau of Land Management

### 4.7.0 Material Density Chart—Ashes to Wood

| Material | Density (Loose) |
| :--- | :--- |
| Ashes | $1100 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Caliche | $2100 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Cement-Portland | $2550 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Cereals-Wheat, Bulk | $1300 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Clay (natural bed) | $2800 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Clay (Dry Lumps) | $1820 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Clay (Wet Lumps) | $2700 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Clay with Gravel (Dry) | $2700 \mathrm{lb} / \mathrm{yd}^{3}$ |


| Material | Density (Loose) |
| :--- | :---: |
| Clay with Gravel (Wet) | $3080 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Coal-Anthracite (Broken) | $1850 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Coal-Bituminous (Broken) | $1400 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Coke | $875 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Earth-Dry, Packed | $2550 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Earth-Wet, Excavated | $2700 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Earth, Loam | $2100 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Garbage-Wet | $1350 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Granite-Broken | $2800 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Gravel-Dry | $2550 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Graveled Sand | $3250 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Gravel-Dry $\left(1 / 2^{\prime \prime}\right.$ to 2") | $2850 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Gravel-Wet (1/2" to 2") | $3375 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Gypsum-Crushed | $2700 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Kaolin | $1730 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Limestone-Broken | $2600 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Salt-Coarse | $1350 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Sand-Dry | $2700 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Sand-Wet | $3500 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Sand with gravel-Dry | $2900 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Sand with gravel-Wet | $3400 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Sandstone-Broken | $2550 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Sawdust | $550 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Shale | $2100 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Silage | $865 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Slag-Broken | $2950 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Snow-Dry | $400 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Snow-Wet | $600 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Stone-Crushed | $2700 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Sugar-Raw, Refined | $1750 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Topsoil | $1600 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Wood Chips (Dry) | $600 \mathrm{lb} / \mathrm{yd}^{3}$ |
| Wood Chips (Wet) | $900 \mathrm{lb} / \mathrm{yd}^{3}$ |

Actual material density may vary from these standard values.

## Source: American Copier Systems

### 4.8.0 Calculating the Density of Rock, Sand, Till Measuring the Density of Rock, Sand, Till, etc.

## 1 Summary

For measuring the density of a variety of geological materials, in particular oddly shaped samples of relatively consolidated material; density is important in the context of cosmogenic-nuclide measurements because the cosmic ray flux is attenuated according to mass depth below the surface; that is, it's necessary to think of depth of overburden or sample thickness in $\mathrm{g} \mathrm{cm}^{-3}$, a unit of mass per square area, rather than simply in length. This quantity is generally called mass depth and is equal to $z \rho$, where $z$ is depth below the surface and $\rho$ is the integrated density of overlying material between the surface and depth $z$. In order to compute this, you need to measure the density of your sample and/or its overburden.

### 1.1 References

Most of these methods are standard and can be found in any geological or soil science manual.
If you use the data, measurements, or conclusions in this document, however, please cite it as follows:
Balco, G., Stone, J.O., 2003. Measuring the density of rock, sand, till, etc. UW Cosmogenic Nuclide Laboratory, methods and procedures. http://depts.washington.edu/cosmolab/chem.html.
The glass bead method is not commonly described in manuals. We got the idea from Sheldrick (see below) and adapted it for our purposes. If you use it, please also cite:
Sheldrick, B.H., (Ed.), 1984. Analytical methods manual 1984. Land Resource Research Institute, Research Branch, Agriculture Canada. http://sis.agr.gc.ca/cansis/publications/manuals/analytical.html.

Source: University of Washington, Cosmogenic Nuclide Lab

## 2 Methods

### 2.1 A note on collecting samples

The idea of this whole procedure is to determine the density of the material in its natural condition. Thus, the most important thing is to try to get the sample to the measurement without disturbing it too much. If you can, measure the density in the field. For weakly consolidated material, try to collect the sample in some sort of rigid holder so that it won't be crushed during transport. For wet material, seal samples in something watertight so that water will not evaporate before you measure their density. Try to collect multiple samples, and try to make the samples representative. In general, larger samples are better.

### 2.2 Collecting a known volume in the field-unconsolidated sediment

In principle, it should be easy to measure the density of any material by cutting out a cube of the material, measuring the size of the cube to determine its volume, and then weighing the cube. In practice, it's nearly impossible to cut out a regular cube of any natural geological deposit.

For some unconsolidated sediments, it is possible to collect a known volume in the field. This works well for wet sand and silt. It sometimes works for glaciolacustrine sediment and wet, clay-rich glacial till. It generally doesn't work for gravel, dry sand and silt, or anything cemented. The preferred device is a section of aluminum pipe several inches long. It's helpful if the edges of the ends of the pipe have been beveled on the outside to make a sharp edge. The procedure is as follows:

1. Determine the volume of the pipe section by accurately measuring the inside diameter and length of the pipe. Measure as accurately as possible using calipers. If the pipe was cut by hand, measure the length at several locations around the circumference of the pipe and take the average. Determine the weight of the pipe.
2. In the field, push the pipe into the outcrop face until material starts to extrude out the near end. Hammer it as necessary. Be careful to ensure that there is no air space inside the pipe. Dig the pipe out and carefully slice the protruding sediment away from each end.
3. Weigh the pipe and sediment. It's best to bring the el-cheapo balance into the field with you and do this on site. If this is not possible, wrap the sample by placing something hard over either end (proper pipe caps are best) and then saran-wrapping and taping the whole thing to minimize water loss during transport. When disassembling it in the lab, make sure that all the sediment in the tube gets weighed and doesn't fall out during cap removal, or that you weigh the tube, sediment, and caps together and then the caps separately.
4. Subtract the weight of the pipe (and caps) from the total weight to determine the sample weight. Divide by the pipe volume to get the density.

It's good to do this a few times for each unit. The most important thing is to get the pipe entirely full of sediment.

### 2.3 Nonporous samples-weighing in water

The second easiest way to measure the density of material is to weigh it in air and then in water. If $W_{a}$ is the weight of the sample in air and $W_{w}$ is its weight when immersed in water, then its density is:

$$
\begin{equation*}
\rho=\frac{W_{a}}{W_{a}-W_{w}} \tag{1}
\end{equation*}
$$

assuming, of course, that your water is pure $\mathrm{H}_{2} \mathrm{O}$ at $25^{\circ} \mathrm{C}$.
This is easy to do with most modern analytical balances, which generally have a hook on the bottom connected to the load cell so that one can weigh suspended objects. In our lab, use the Scout balance. Place it over one of the strategically located holes in the lab bench. Obtain a 1-meter length of thin steel wire. Weigh it. Affix the sample to one end of the wire. This may require some creativity. Make a loop in the other end of the wire. Fish it up through the hole and place it on the hook on the balance. Read off the weight. The sample weight in air is this weight less the weight of the wire. Fill a large beaker with DI water. Raise the beaker up from beneath the sample so that the sample is immersed about 1 inch below the surface of the water. Prop the beaker on something of the appropriate height. Read off the weight. The immersed sample weight is this weight less the weight of the wire (relatively little of the wire is immersed). Calculate the density with equation (1). Remember to dry the sample thoroughly before repeating the exercise, so as not to change the dry weight.

Obviously, this method is restricted to samples that are nonporous and will not absorb any of the water in which they are immersed. In practice, this means igneous and metamorphic rocks, and some limestones.

### 2.4 Oddly shaped and porous, but well-consolidated samples-glass bead method

This method is designed for oddly shaped samples of at-least-somewhat-consolidated material-for example, glacial till, compacted loess, saprolite, cemented sand, sandstone, shale. The procedure is as follows:

1. From the top left drawer in the sediment lab, select a stainless-steel tin slightly larger than your sample. Place it in one of the aluminum baking pans. Record the tare weight $W_{T}$ and volume $V_{T}$ of the tin (written on the side of most of the tins).
2. Using the Scout balance, weigh your sample and record the weight $W_{S}$.
3. Pour out enough beads to fill the tin about 5 mm deep. Bed a flat side of your sample in the beads. Make sure the sample does not stick out past the rim. Fill the rest of the tin with beads, making sure to tap the tin to settle the beads into all the nooks and crannies of the sample.
4. Overfill the tin with beads, then take the steel spatula and scrape the excess beads away, filling in gaps around the edges, until the surface of the beads is precisely flat with the rim. It's important to do this exactly the same way every time. Make sure to catch all the loose beads in the baking pan.
5. Weigh tin, sample, and packed beads and record the weight $W_{T S P}$. Dump the beads back into the baking pan. Remove the sample, trying not to break it up too much and get crud in the beads. Pour the beads from the baking pan back into the storage tin.
6. Calculate the weight of packing material, $W_{P}=W_{T S P}-W_{T}-W_{S}$. Calculate the volume of packing. $V_{P}=W_{P} / \rho_{P} . \rho_{P}$ is the density of the packing material (see below). Calculate the sample density $\rho_{S}=W_{S} /\left(V_{T}-V_{P}\right)$.
7. Do the measurement a couple of times to make sure you really did fill all nooks and crannies with the beads the first time.

## Notes:

- Metal vessels work best for this. Anything plastic will cause trouble with the beads owing to static electricity.
- We measure the volume of the tins by measuring the weight of water that will fit in them. It's important to make sure that the water surface is close to the actual top of the tin (to which you will grade the beads). It's possible to overfill due to surface tension.
- We use 1 mm glass beads (available from chemical supply companies). 0.5 mm beads also work OK but are a bit messier. We determine the density of packed beads by filling a tin of known volume and measuring the weight of the beads. The density of our 1 mm beads $\rho_{P}$ is $1.53 \mathrm{~g} \mathrm{~cm}^{-3}$. It's probably a good idea for each person to independently determine bead density with their own particular scraping/compacting technique.
- Occasionally it's necessary to clean the beads. We do this by sonicating them in water, rinsing thoroughly, and drying. If a lot of large chunks of foreign material build up in the beads, sieving might be needed.
- We also use clean beach sand (mostly quartz) in the $0.5-0.85 \mathrm{~mm}$ size range. We prepare it by sieving the sand to this size, sonicating it in water for approximately 1 hr , then rinsing it thoroughly and drying in the oven. The advantage of sand is that it is inexpensive and can be used in sacrificial applications, such as the wet density determination method described below, or when samples are very poorly compacted and are likely to break up during the process and make a mess. Sand does not compact as readily as glass beads (more angular grains), so it's very important to repeatedly tap the tin containing sand and sample on the bench as you are filling it, to make sure the sand is fully compacted. Also, each batch of sand will have a slightly different density that will need to be measured before starting. Our sand has a density $\rho_{P}$ of $1.45-1.47 \mathrm{~g} \cdot \mathrm{~cm}^{-3}$.
- By repeatedly measuring the density of a variety of samples, including large quartz crystals, whose density, of course, we know exactly, we've determined the accuracy/precision of this technique to be $\pm 0.08 \mathrm{~g} \cdot \mathrm{~cm}^{-3}$ for typical materials with densities of $1.2-2.7 \mathrm{~g} \cdot \mathrm{~cm}^{-3}$.


### 2.5 Unconsolidated samples-stuffing into a vial

If the sample is completely unconsolidated, for example, dry sand, there is one method remaining. Take a vial of known volume, for example, one of the small metal tins or plastic vials in the drawer. Using your fingers, press the sample into the vial, attempting to duplicate the natural compaction of the material. For most sands, this means squishing it in with some authority to ensure that the sand grains are well packed. Overfill the vial and blade off the excess with the steel spatula. Weigh the sample and vial, subtract the tare weight of the vial, and divide by the volume of the vial.

Despite the ad hoc nature of this technique, it probably does a fairly good job of measuring the density of sandy surficial sediments, because relatively well-sorted sand reaches its maximum compaction quickly and then does not compact any further until buried really deeply, like kilometers. This would also be the only way to measure the density of unconsolidated fluvial gravels, but the vessel would have to be much bigger, in keeping with the grain size of the gravel, to ensure a representative sample.

### 2.6 Notes on wet versus dry density

In reality, most geological materials are water-saturated below a few meters depth in most environments. In many cases, however, especially when working on drill core, the only samples available to measure overburden density have been dried during storage. Thus, we need some means of converting dry to wet density. The wet density of samples which are collected dry can be approximately measured by the following procedure:

1. For unconsolidated samples, pack a vial of known weight and volume with the sample as described in 2.5 . For consolidated samples, pack a tin of known weight and volume with sample and packing material as described in 2.4. The packing material will be inseparable from the sample at the end of this procedure, so we suggest using sand as described in the notes to 2.4 above. Record the relevant weights.
2. Add distilled water to the vial or tin slowly and carefully until everything in the tin is completely saturated. Leave the sample to soak for at least 24 hours to ensure that well-compacted samples become fully saturated. For large samples of glacial till or the like, you may want to let them soak for a couple of days. Periodically add water to keep the sample fully saturated. When you are satisfied that the sample is fully saturated, record the total weight of vessel and contents.
3. For unconsolidated samples, subtract the weight of the vial from the total weight to obtain the wet weight of the sample, then divide by the volume of the vial to determine the wet density.
4. For consolidated samples, you have just measured $W_{T S P W}$. Calculate the total weight of water added $W_{W T}=W_{T S P W}-W_{T S P}$. Calculate the weight of water incorporated in the packing material $W_{W P}=V_{P} f_{W P} \rho_{W}$, where $f_{W P}$ is the water content of saturated packing material (see below) and $\rho_{W}$ is the density of water, i.e., $1 \mathrm{~g} \cdot \mathrm{~cm}^{-3}$. Calculate the wet weight of the sample $W_{W S}=W_{S}+$ $\left(W_{W T}-W_{W P}\right)$. The wet density of the sample $\rho_{w e t}$ is then $W_{W S} / V_{S}$.

Our quartz sand has a saturated water content $f_{W P} 0.45$ by volume ( 0.24 by weight), which equates to a wet density of $1.90 \mathrm{~g} \cdot \mathrm{~cm}^{-3}$.

This method is not always accurate, primarily because of the tendency of many materials that contain clays to expand when wet, but it is often the only option for determining the wet density of dry material obtained from old, cruddy drill core. It is always better to collect samples at natural moisture conditions in the field.


FIGURE 1 Measured wet densities compared with those calculated from dry densities using 2. Circles, wet density determined by saturation of dry sand, and triangles, of dry till, as described in 2.6 . Diamonds, samples of glacial till collected wet and then oven-dried. Error-bars reflect what we believe to be measurement precision as described in 2.4.

In addition, this method is somewhat time consuming. A simple alternative is to assume that all of the grains in the sample are composed of quartz $\left(\rho=2.65 \mathrm{~g} \cdot \mathrm{~cm}^{-3}\right)$ and that all the pore space is filled when wet. Under these assumptions:

$$
\begin{equation*}
\rho_{\text {wet }}=\left(1-\frac{\rho_{d r y}}{\rho_{\text {quartz }}}\right)+\rho_{d r y} \tag{2}
\end{equation*}
$$

We tested this approximation with samples of unconsolidated sand and glacial till that we obtained from dried drillcore and whose wet density we measured as described above, as well as with samples of glacial till that were collected wet and whose density we measured before and after oven-drying (Figure 1). The results show that this approximation seems to be adequate within the resolution of our measurement technique.

## Calculations Relating to Concrete and Masonry

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### 5.0.0 Standard American Concrete Institute (ACI) and Portland Cement Association (PCA) Divide the Production of Concrete into Seven (7) Basic Components and Ingredients

The standard ACI mix design procedure can be divided up into seven basic steps:

1. Choice of slump
2. Maximum aggregate size selection
3. Mixing water and air content selection
4. Water-cement ratio
5. Cement content
6. Coarse aggregate content
7. Fine aggregate content

Source: Washington,edu

### 5.0.1 Chemical Additives Provide Characteristics not Obtainable When Utilizing the Seven Basic Components

## Types of Concrete Admixtures

## Posted by Civil Engineer

Chemical concrete admixtures are material in the form of powder or fluids that are added to concrete to give it certain characteristics not obtainable with plain concrete mixes. In normal use, admixture dosages are less than $5 \%$ by mass of cement and are added to the concrete at the time of batching/mixing. The most common types of concrete admixtures are:

1. Accelerators speed up the hydration (hardening) of the concrete.
2. Retarders slow the hydration (hardening) of the concrete and are used in large or difficult pours where partial setting before the pour is complete is undesirable.
3. Air-entrainers add and distribute tiny air bubbles in the concrete, which will reduce damage during freezethaw cycles, thereby increasing the concrete's durability.
4. Plasticizers (water-reducing admixtures) increase the workability of plastic of fresh concrete, allowing it to be placed more easily with less consolidating effort.
5. Superplasticizers (high-range water-reducing admixtures) are a class of plasticizers that have fewer deleterious effects when used to significantly increase workability. Alternatively; plasticizers can be used to reduce the water content of concrete (and have been called water reducers due to this application) while maintaining workability. This improves its strength and durability characteristics.
6. Pigments can be used to change the color of concrete, for aesthetics. Mainly they are ferrous oxides.
7. Corrosion inhibitors are used to minimize the corrosion of steel and steel bars in concrete.
8. Bonding agents are used to create a bond between old and new concrete.
9. Pumping aids improve pumpability, thicken the paste, and reduce dewatering of the paste.

Thus, chemical admixture is one ingredient creating concrete that provides the differentiation of concrete types.

Source: civilengineeringblog.com

### 5.0.1.1 Slump

The choice of slump is actually a choice of mix workability. Workability can be described as a combination of several different, but related, properties of portland cement concrete (PCC) related to its rheology:

- Ease of mixing
- Ease of placing
- Ease of compaction
- Ease of finishing

Generally, mixes of the stiffest consistency that can still be placed adequately should be used. Typically, slump is specified, but Table 5.14 shows general slump ranges for specific applications. Slump specifications are different for fixed-form paving and slip-form paving. Table 5.15 shows typical and extreme state Department of Transportation (DOT) slump ranges.

Source: washington.edu/PGI/html

TABLE 5.14 Slump Ranges for Specific Applications

|  | Slump |  |
| :--- | :---: | :---: |
| Type of Construction | $(\mathbf{m m})$ | (inches) |
| Reinforced foundation walls and footings | $25-75$ | $1-3$ |
| Plain footings, caissons, and substructure walls | $25-75$ | $1-3$ |
| Beams and reinforced walls | $25-100$ | $1-4$ |
| Building columns | $25-100$ | $1-4$ |
| Pavements and slabs | $25-75$ | $1-3$ |
| Mass concrete | $25-50$ | $1-2$ |

TABLE 5.15 Typical State DOT Slump Specifications

| Specifications | Fixed Form |  | Slip Form |  |
| :---: | :---: | :---: | :---: | :---: |
|  | (mm) | (inches) | (mm) | (inches) |
| Typical Extremes | $25-75$ <br> as low as 25 as high as 175 | $1-3$ <br> as low as 1 as high as 7 | $0-75$ <br> as low as 0 as high as $125$ | $0-3$ <br> as low as 0 as high as 5 |

### 5.0.1.2 Maximum Aggregate Size

Maximum aggregate size will affect such PCC parameters as amount of cement paste, workability, and strength. In general, ACI (American Concrete Institute) recommends that maximum aggregate size be limited to one-third of the slab depth and three-fourths of the minimum clear space between reinforcing bars. Aggregate larger than these dimensions may be difficult to consolidate and compact, resulting in a honeycombed structure or large air pockets. Pavement PCC maximum aggregate sizes are on the order of 25 mm ( 1 in .) to 37.5 mm ( 1.5 in .).

### 5.0.1.3 Mxing Water and Air Content Estimation

Slump is dependent on nominal maximum aggregate size, particle shape, aggregate gradation, PCC temperature, amount of entrained air, and certain chemical admixtures. It is not generally affected by the amount of cementitious material. Therefore, ACI provides a table relating nominal maximum aggregate size, air entrainment, and desired slump to the desired mixing water quantity. Table 5.16 is a partial reproduction of ACI Table 6.3 .3 (keep in mind that pavement PCC is almost always air-entrained, so air-entrained values are most appropriate). Typically, state agencies specify between about 4 and $8 \%$ air by total volume.

Note that the use of water-reducing and/or set-controlling admixtures can substantially reduce the amount of mixing water required to achieve a given slump.

Source: washington.edu/PGI/html

TABLE 5.16 Approximate Mixing Water and Air Content Requirements for Different Slumps and Maximum Aggregate Sizes

|  | Mixing Water Quantity in $\mathrm{kg} / \mathrm{m}^{3}\left(\mathrm{lb} / \mathrm{yd}^{3}\right)$ for the listed Nominal Maximum Aggregate Size |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Slump | $\begin{aligned} & 9.5 \mathrm{~mm} \\ & \text { (0.375 in.) } \end{aligned}$ | $\begin{aligned} & 12.5 \mathrm{~mm} \\ & \text { (0.5 in.) } \end{aligned}$ | $\begin{aligned} & 19 \mathrm{~mm} \\ & (0.75 \mathrm{in} .) \end{aligned}$ | $\begin{aligned} & 25 \mathrm{~mm} \\ & \text { (1 in.) } \end{aligned}$ | $\begin{aligned} & 37.5 \mathrm{~mm} \\ & \text { (1.5 in.) } \end{aligned}$ | $\begin{aligned} & 50 \mathrm{~mm} \\ & \text { (2 in.) } \end{aligned}$ | $\begin{aligned} & 75 \mathrm{~mm} \\ & \text { (3 in.) } \end{aligned}$ | $\begin{aligned} & 100 \mathrm{~mm} \\ & \text { (4 in.) } \end{aligned}$ |

Non-Air-Entrained PCC

| $25-50$ | 207 | 199 | 190 | 179 | 166 | 154 | 130 | 113 |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $(1-2)$ | $(350)$ | $(335)$ | $(315)$ | $(300)$ | $(275)$ | $(260)$ | $(220)$ | $(190)$ |
| $75-100$ | 228 | 216 | 205 | 193 | 181 | 169 | 145 | 124 |
| $(3-4)$ | $(385)$ | $(365)$ | $(340)$ | $(325)$ | $(300)$ | $(285)$ | $(245)$ | $(210)$ |
| $150-175$ | 243 | 228 | 216 | 202 | 190 | 178 | 160 | - |
| $(6-7)$ | $(410)$ | $(385)$ | $(360)$ | $(340)$ | $(315)$ | $(300)$ | $(270)$ |  |
| Typical entrapped <br> air (percent) | 3 | 2.5 | 2 | 1.5 | 1 | 0.5 | 0.3 | 0.2 |


| Air-Entrained PCC | 181 | 175 | 168 | 160 | 148 | 142 | 122 | 107 |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $25-50$ | $(305)$ | $(295)$ | $(280)$ | $(270)$ | $(250)$ | $(240)$ | $(205)$ | $(180)$ |
| $(1-2)$ | 202 | 193 | 184 | 175 | 165 | 157 | 133 | 119 |
| $75-100$ | $(340)$ | $(325)$ | $(305)$ | $(295)$ | $(275)$ | $(265)$ | $(225)$ | $(200)$ |
| $(3-4)$ | 216 | 205 | 197 | 184 | 174 | 166 | 154 | - |
| $150-175$ | $(365)$ | $(345)$ | $(325)$ | $(310)$ | $(290)$ | $(280)$ | $(260)$ |  |
| $(6-7)$ |  |  |  |  |  |  |  |  |


|  | Recommended Air Content (percent) |  |  |  |  |  |  |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Mild Exposure | 4.5 | 4.0 | 3.5 | 3.0 | 2.5 | 2.0 | 1.5 | 1.0 |
| Moderate Exposure | 6.0 | 5.5 | 5.0 | 4.5 | 4.5 | 4.0 | 3.5 | 3.0 |
| Severe Exposure | 7.5 | 7.0 | 6.0 | 6.0 | 5.5 | 5.0 | 4.5 | 4.0 |

### 5.0.1.4 Water-Cement Ratio

The water-cement ratio is a convenient measurement whose value is well correlated with PCC strength and durability. In general, lower water-cement ratios produce stronger, more durable PCC. If natural pozzolans are used in the mix (such as fly ash), then the ratio becomes a water-cementitious material ratio (cementitious material $=$ portland cement + pozzolonic material). The ACI method bases the water-cement ratio selection on desired compressive strength and then calculates the required cement content based on the selected watercement ratio. Table 5.17 is a general estimate of 28 -day compressive strength versus water-cement ratio (or water-cementitious ratio). Values in this table tend to be conservative. Most state DOTs tend to set a maximum water-cement ratio between 0.40 and 0.50 .

### 5.0.1.5 Cement Content

Cement content is determined by comparing the following two items:

- The calculated amount based on the selected mixing water content and water-cement ratio.
- The specified minimum cement content, if applicable. Most state DOTs specify minimum cement contents in the range of $300-360 \mathrm{~kg} / \mathrm{m}^{3}\left(500-600 \mathrm{lbs} / \mathrm{yd}^{3}\right)$.

An older practice used to be to specify the cement content in terms of the number of 94 lb sacks of portland cement per cubic yard of PCC. This resulted in specifications such as a " 6 sack mix" or a " 5 sack mix." While these specifications are quite logical to a small contractor or an individual who buys portland cement in 94 lb sacks, they do not have much meaning to the typical pavement contractor or batching plant who buys portland cement in bulk. As such, specifying cement content by the number of sacks should be avoided.

Source: washington.edu/PGI/html

### 5.0.1.6 Adjustments for Aggregate Moisture

Unlike HMA (Hot Mix Asphalt), PCC batching does not require dried aggregate. Therefore, aggregate moisture content must be accounted for. Aggregate moisture affects the following parameters:

1. Aggregate weights. Aggregate volumes are calculated based on oven dry unit weights, but aggregate is typically batched based on actual weight. Therefore, any moisture in the aggregate will increase its weight, and stockpiled aggregates almost always contain some moisture. Without correcting for this, the batched aggregate volumes will be incorrect.
2. Amount of mixing water. If the batched aggregate is anything but saturated, surface drying it will absorb water (if oven dry or air dry) or give up water (if wet) to the cement paste. This causes a net change in the amount of water available in the mix and must be compensated for by adjusting the amount of mixing water added.
Source: washington.edu/PGI/html

TABLE 5.17 Water-Cement Ratio and Compressive Strength Relationship

|  | Water-Cement Ratio by Weight |  |
| :--- | :---: | :---: |
| 28-Day Compressive Strength in MPa (psi) | Non-Air-Entrained | Air-Entrained |
| $41.4(6000)$ | 0.41 | - |
| $34.5(5000)$ | 0.48 | 0.40 |
| $27.6(4000)$ | 0.57 | 0.48 |
| $20.7(3000)$ | 0.68 | 0.59 |
| $13.8(2000)$ | 0.82 | 0.74 |

### 5.1.0 Portland Cement—ASTM Types

The properties of concrete depend on the quantities and qualities of its components. Because cement is the most active component of concrete and usually has the greatest unit cost, its selection and proper use are important in obtaining most economically the balance of properties desired for any particular concrete mixture.

Type I/II portland cements, which can provide adequate levels of strength and durability, are the most popular cements used by concrete producers. However, some applications require the use of other cements to provide higher levels of properties. The need for high-early strength cements in pavement repairs and the use of blended cements with aggregates susceptible to alkali-aggregate reactions are examples of such applications.

It is essential that highway engineers select the type of cement that will obtain the best performance from the concrete. This choice involves correct knowledge of the relationship between cement and performance and, in particular, between type of cement and durability of concrete.

## Portland Cement (ASTM Types)

ASTM C 150 defines portland cement as "hydraulic cement (cement that not only hardens by reacting with water but also forms a water-resistant product) produced by pulverizing clinkers consisting essentially of hydraulic calcium silicates, usually containing one or more of the forms of calcium sulfate as an inter ground addition." Clinkers are nodules (diameters, $0.2-1.0 \mathrm{in} .[5-25 \mathrm{~mm}]$ ) of a sintered material that is produced when a raw mixture of predetermined composition is heated to high temperature. The low cost and widespread availability of the limestone, shales, and other naturally occurring materials make portland cement one of the lowest-cost materials widely used over the last century throughout the world. Concrete becomes one of the most versatile construction materials available in the world.

The manufacture and composition of portland cements, hydration processes, and chemical and physical properties have been repeatedly studied and researched, with innumerable reports and papers written on all aspects of these properties.

## Types of Portland Cement

Different types of portland cement are manufactured to meet different physical and chemical requirements for specific purposes, such as durability and high-early strength. Eight types of cement are covered in ASTM C 150 and AASHTO M 85.

More than $92 \%$ of portland cement produced in the United States is Type I and II (or Type I/II); Type III accounts for about $3.5 \%$ of cement production. Type IV cement is only available on special request, and Type V may also be difficult to obtain (less than $0.5 \%$ of production).

Although IA, IIA, and IIIA (air-entraining cements) are available as options, concrete producers prefer to use an air-entraining admixture during concrete manufacture, where they can get better control in obtaining the desired air content. However, this kind of cement can be useful under conditions in which quality control is poor, particularly when no means of measuring the air content of fresh concrete is available.

If a given type of cement is not available, comparable results can frequently be obtained by using modifications of available types. High-early strength concrete, for example, can be made by using a higher content of Type I when Type III cement is not available, or by using admixtures such as chemical accelerators or high-range water reducers (HRWR). The availability of portland cements will be affected for years to come by energy and pollution requirements. In fact, the increased attention to pollution abatement and energy conservation has already greatly influenced the cement industry, especially in the production of low-alkali cements. Using high-alkali raw materials in the manufacture of low-alkali cement requires bypass systems to avoid

## Portland Cement Types and Their Uses

| Cement Type | Use |
| :--- | :--- |
| $I^{1}$ | General-purpose cement, when there are no extenuating conditions |
| $I I^{2}$ | Aids in providing moderate resistance to sulfate attack |
| $I I I$ | When high-early strength is required |
| $I V^{3}$ | When a low heat of hydration is desired (in massive structures) |
| $V^{4}$ | When high sulfate resistance is required |
| $I A^{4}$ | A type I cement containing an integral air-entraining agent |
| $I A^{4}$ | A type II cement containing an integral air-entraining agent |
| $I I I A^{4}$ | A type III cement containing an integral air-entraining agent |

${ }^{1}$ Cements that simultaneously meet requirements of Type I and Type II are also widely available.
${ }^{2}$ Type Ill low alkali (total alkali as $\mathrm{Na} 2 \mathrm{O}<0.6 \%$ ) is often specified in regions where aggregates susceptible to alkali-silica reactivity are employed.
${ }^{3}$ Type IV cements are only available on special request.
${ }^{4}$ These cements are in limited production and not widely available.
concentrating alkali in the clinkers, which consumes more energy. It is estimated that $4 \%$ of energy used by the cement industry could be saved by relaxing alkali specifications. Limiting use of low-alkali cement to cases in which alkali-reactive aggregates are used could lead to significant improvement in energy efficiency.

Source: U.S. Department of Transportation-FHWA (Federal Highway Administration)

### 5.1.1 Cement Composition

Cement Composition. The composition of portland cements is what distinguishes one type of cement from another. ASTM C 150 and AASHTO M 85 present the standard chemical requirements for each type. The phase compositions in portland cement are denoted by ASTM as tricalcium silicate ( $\mathrm{C}_{3} \mathrm{~S}$ ), dicalcium silicate $\left(\mathrm{C}_{2} \mathrm{~S}\right)$, tricalcium aluminate $\left(\mathrm{C}_{3} \mathrm{~A}\right)$, and tetracalcium aluminoferrite $\left(\mathrm{C}_{4} \mathrm{AF}\right)$. However, it should be noted that these compositions would occur at a phase equilibrium of all components in the mix and do not reflect the effects of burn temperatures, quenching, oxygen availability, and other real-world kiln conditions. The actual components are often complex chemical crystalline and amorphous structures, denoted by cement chemists as "elite" $\left(C_{3} S\right)$, "belite" $\left(C_{2} S\right)$, and various forms of aluminates. The behavior of each type of cement depends on the content of these components. Characterization of these compounds, their hydration, and their influence on the behavior of cements are presented in full detail in many texts. Different analytical techniques such as X-ray diffraction and analytical electron microscopy are used by researchers in order to understand fully the reaction of cement with water (hydration process) and to improve its properties.

In simplest terms, results of these studies have shown that early hydration of cement is principally controlled by the amount and activity of $\mathrm{C}_{3} \mathrm{~A}$, balanced by the amount and type of sulfate interground with the cement. $\mathrm{C}_{3} \mathrm{~A}$ hydrates very rapidly and will influence early bonding characteristics. Abnormal hydration of $\left(\mathrm{C}_{3} \mathrm{~A}\right)$ and poor control of this hydration by sulfate can lead to such problems as flash set, false set, slump loss, and cement-admixture incompatibility.

Development of the internal structure of hydrated cement (referred to by many researchers as the microstructure) occurs after the concrete has set and continues for months (and even years) after placement. The microstructure of the cement hydrates will determine the mechanical behavior and durability of the concrete. In terms of cement composition, the $\mathrm{C}_{3} \mathrm{~S}$ and $\mathrm{C}_{2} \mathrm{~S}$ will have the primary influence on long-term development of structure, although aluminates may contribute to formation of compounds such as ettringite (sulfoaluminate hydrate), which can cause expansive disruption of concrete. Cements high in $\mathrm{C}_{3} \mathrm{~S}$ (especially those that are finely ground)
will hydrate more rapidly and lead to higher early strength. However, the hydration products formed will, in effect, make it more difficult for hydration to proceed at later ages, leading to an ultimate strength lower than desired in some cases. Cements high in $\mathrm{C}_{2} \mathrm{~S}$ will hydrate much more slowly, leading to a denser ultimate structure and a higher long-term strength. The relative ratio of $\mathrm{C}_{3} \mathrm{~S}$ to $\mathrm{C}_{2} \mathrm{~S}$, and the overall fineness of cements, have been steadily increasing over the past few decades. This ability to achieve desired strengths at a higher workability (and hence a higher w/c) may account for many durability problems, as it is now established that higher $\mathrm{w} / \mathrm{c}$ invariably leads to higher permeability in the concrete.

One of the major aspects of cement chemistry that concern cement users is the influence of chemical admixtures on portland cement. Since the early 1960s, most states have permitted or required the use of waterreducing and other admixtures in highway pavements and structures. A wide variety of chemical admixtures have been introduced to the concrete industry over the last three decades, and engineers are increasingly concerned about the positive and negative effects of these admixtures on cement and concrete performance.

Considerable research dealing with admixtures has been conducted in the United States. Air-entraining agents are widely used in the highway industry in North America, where concrete will be subjected to repeated freezethaw cycles. Air-entraining agents have no appreciable effect on the rate of hydration of cement or on the chemical composition of hydration products. However, an increase in cement fineness or a decrease in cement alkali content generally increases the amount of an admixture required for a given air content. Water reducers or retarders influence cement compounds and their hydration. Lignosulfonate-based admixtures affect the hydration of $\mathrm{C}_{3} \mathrm{~A}$, which controls the setting and early hydration of cement. $\mathrm{C}_{3} \mathrm{~S}$ and $\mathrm{C}_{4} \mathrm{AF}$ hydration is also influenced by water reducers.

Test results showed that alkali and $\mathrm{C}_{3} \mathrm{~A}$ contents influence the required admixtures to achieve the desired mix. It appears that set retarders, for example, are more effective with cement of low alkali and low $\mathrm{C}_{3} A$ content, and that water reducers seem to improve the compressive strength of concrete-containing cements of low alkali content more than that of the concrete-containing cements of high alkali content.

Source: U.S. Department of Transportation-FHWA

### 5.1.2 Physical Properties of Portland Cement

Physical Properties of Portland Cements. ASTM C 150 and AASHTO M 85 have specified certain physical requirements for each type of cement. These properties include (1) fineness, (2) soundness, (3) consistency, (4) setting time, (5) compressive strength, (6) heat of hydration, (7) specific gravity, and (8) loss of ignition. Each of these properties has an influence on the performance of cement in concrete. The fineness of the cement, for example, affects the rate of hydration. Greater fineness increases the surface available for hydration, causing greater early strength and more rapid generation of heat (the fineness of Type III is higher than that of Type I cement).

ASTM C 150 and AASHTO M 85 specifications are similar except with regard to fineness of cement. AASHTO M 85 requires coarser cement, which will result in higher ultimate strengths and lower early-strength gain. The Wagner Turbidimeter and the Blaine air permeability test for measuring cement fineness are both required by the American Society for Testing Materials (ASTM) and the American Association for State Highway Transportation Officials (AASHTO). Average Blaine fineness of modern cement ranges from 3,000 to $5,000 \mathrm{~cm}^{2} / \mathrm{g}$ ( 300 to $500 \mathrm{~m}^{2} / \mathrm{kg}$ ).

Soundness, which is the ability of hardened cement paste to retain its volume after setting, can be characterized by measuring the expansion of mortar bars in an autoclave (ASTM C 191, AASHTO T 130). The compressive strength of $2-\mathrm{in}$. ( $50-\mathrm{mm}$ ) mortar cubes after 7 days (as measured by ASTM C 109) should not be less than $2800 \mathrm{psi}(19.3 \mathrm{MPa})$ for Type I cement. Other physical properties included in both ASTM C 150 and AASHTO M 95 are specific gravity and false set. False set is a significant loss of plasticity shortly after mixing

TABLE 1.2 Effects of cements on concrete properties.

| Cement Property | Cement Effects |
| :--- | :--- |
| Placeability | Cement amount, fineness, setting characteristics |
| Strength | Cement composition $\left(\mathrm{C}_{3} \mathrm{~S}, \mathrm{C}_{2} \mathrm{~S}\right.$ and $\left.\mathrm{C}_{3} \mathrm{~A}\right)$, loss on ignition, fineness |
| Drying Shrinkage | $\mathrm{SO}_{3}$ content, cement composition |
| Permeability | Cement composition, fineness |
| Resistance to sulfate | $\mathrm{C}_{3} \mathrm{~A}$ content |
| Alkali Silica Reactivity | Alkali content |
| Corrosion of embedded steel | Cement Composition (esp. $\mathrm{C}_{3} \mathrm{~A}$ content) |

due to the formation of gypsum or the formation of ettringite after mixing. In many cases, workability can be restored by remixing concrete before it is cast.

The effects of cement on the most important concrete properties are presented in Table 1.2.
Cement composition and fineness play a major role in controlling concrete properties. Fineness of cement affects the placeability, workability, and water content of a concrete mixture much like the amount of cement used in concrete does.

Cement composition affects the permeability of concrete by controlling the rate of hydration. However, the ultimate porosity and permeability are unaffected. The coarse cement tends to produce pastes with higher porosity than that produced by finer cement. Cement composition has only a minor effect on freeze-thaw resistance. Corrosion of embedded steel has been related to $\mathrm{C}_{3} \mathrm{~A}$ content. The higher the $\mathrm{C}_{3} \mathrm{~A}$, the more chloride can be tied into chloroaluminate complexes-and thereby be unavailable for catalysis of the corrosion process.

Source: U.S. Department of Transportation-FHWA

### 5.1.3 Blended Portland Cement

Blended cement, as defined in ASTM C 595, is a mixture of portland cement and blast-furnace slag (BFS) or a "mixture of portland cement and a pozzolan (most commonly fly ash)."

The use of blended cements in concrete reduces mixing water and bleeding, improves finishability and workability, enhances sulfate resistance, inhibits the alkali-aggregate reaction, and lessens heat evolution during hydration, thus moderating the chances for thermal cracking on cooling.

Blended cement types and blended ratios are presented in Table 1.3.

TABLE 1.3 Blended cement types and blended ratios.

| Type | Blended Ingredients |
| :--- | :--- |
| IP | $15-40 \%$ by weight of pozzolan (fly ash) |
| I(PM) | $0-15 \%$ by weight of Pozzolan (fly ash) (modified) |
| P | $15-40 \%$ by weitht of pozzolan (fly ash) |
| IS | $25-70 \%$ by weight of blast-furnace slag |
| I(SM) | $0-25 \%$ by weight of blast-furnace slag (modified) |
| S | $70-100 \%$ by weight of blast-furnace slag |

The advantages to using mineral admixtures added at the batch plant are as follows.

- Mineral admixture replacement levels can be modified on a day-to-day and job-to-job basis to suit project specifications and needs.
- Cost can be decreased substantially while performance is increased (taking into consideration the fact that the price of blended cement is at least $10 \%$ higher than that of Type I/II cement [U.S. Dept. Int. 1989]).
- GGBFS can be ground to its optimum fineness.
- Concrete producers can provide specialty concretes in the concrete product markets.

At the same time, several precautions must be considered when mineral admixtures are added at the batch plant.

- Separate silos are required to store the different hydraulic materials (cements, pozzolans, slags). This might slightly increase the initial capital cost of the plant.
- There is a need to monitor variability in the properties of the cementitious materials, often enough to enable operators to adjust mixtures or obtain alternate materials if problems arise.
- Possibilities of cross-contamination or batching errors are increased as the number of materials that must be stocked and controlled is increased.


## Source: U.S.Department of Transportation-FHWA

### 5.1.4 Modified Portland Cement (Expansive Cement)

Expansive cement, as well as expansive components, is a cement-containing hydraulic calcium silicate (such as those characteristic of portland cement) that, upon being mixed with water, forms a paste. During the early hydrating period occurring after setting, it increases in volume significantly more than does portland cement paste. Expansive cement is used to compensate for volume decrease due to shrinkage and to induce tensile stress in reinforcement.

Expansive cement concrete used to minimize cracking caused by drying shrinkage in concrete slabs, pavements, and structures is termed shrinkage-compensating concrete.

Self-stressing concrete is another expansive cement concrete in which the expansion, if restrained, will induce a compressive stress high enough to result in a significant residual compression in the concrete after drying shrinkage has occurred.

Types of Expansive Cements. Three kinds of expansive cement are defined in ASTM C 845.

- Type K: Contains anhydrous calcium aluminate
- Type M: Contains calcium aluminate and calcium sulfate
- Type S: Contains tricalcium aluminate and calcium sulfate

Only Type K is used in any significant amount in the United States.
Concrete placed in an environment where it begins to dry and lose moisture will begin to shrink. The amount of drying shrinkage that occurs in concrete depends on the characteristics of the materials, mixture proportions, and placing methods. When pavements or other structural members are restrained by subgrade friction, reinforcement, or other portions of the structure, drying shrinkage will induce tensile stresses. These drying shrinkage stresses usually exceed the concrete tensile strengths, causing cracking. The advantage of using expansive cements is to induce stresses large enough to compensate for drying shrinkage stresses and minimize cracking.

Physical and mechanical properties of shrinkage compensating concrete are similar to those of portland cement concrete (PCC). Tensile, flexural, and compressive strengths are comparable to those in PCC. Airentraining admixtures are as effective with shrinkage-compensating concrete as with portland cement in improving freeze-thaw durability.

Some water-reducing admixtures may be incompatible with expansive cement. Type A water-reducing admixture, for example, may increase the slump loss of shrinkage-compensating concrete. Fly ash and other pozzolans may affect expansion and may also influence strength development and other physical properties.

Structural design considerations and mix proportioning and construction procedures are available in ACI 223-83. This report contains several examples of using expansive cements in pavements.

In Japan, admixtures containing expansive compounds are used instead of expansive cements. Described using expansive admixtures in building chemically prestressed precast concrete box culverts. Bending characteristics of chemically prestressed concrete box culverts were identical to those of reinforced concrete units of greater thickness. Expansive compounds are also available in the United States. They can be added to the mix in a way similar to how fly ash is added to concrete mixes.

Source: U.S. Department of Transportation-FHWA

### 5.2.0 Types of Cement and What They Do

Portland cement is a type of cement, not a brand name. Many cement manufacturers make portland cement. To find what concrete is made of and to learn about concrete mix designs, admixtures, and water-to-cement ratios, read our section "What Is Concrete".

Type 1—Normal portland cement. Type 1 is a general use cement.
Type 2-Used for structures in water or soil containing moderate amounts of sulfate, or when heat buildup is a concern.
Type 3-High-early strength. Used when high strengths are desired at very early periods.
Type 4-Low-heat portland cement. Used where the amount and rate of heat generation must be kept to a minimum.
Type 5-Sulfate-resistant portland cement. Used where the water or soil is high in alkali.
Types IA, IIA, and IIIA are cements used to make air-entrained concrete. They have the same properties as types I, II, have small quantities of air-entrained materials combined with them.

These are very short descriptions of the basic types of cement. There are other types for various purposes such as masonry cements, just to name two examples.

Your ready-mix company will know what the requirements are for your area and for your particular use. Simply ask the of cement is and if that will work for your conditions.

Source: concretenet work.com

### 5.3.0 Concrete Compressive Strengths-U.S. and Metric

## Nominal MPa Values of Equivalent psi Concrete Strengths

In metric, concrete strength is denominated in megapascals (MPa).
In imperial, concrete strength is denominated in pounds per square inch (psi).

$$
\begin{aligned}
& 2500 \mathrm{psi}=18 \mathrm{MPa}(17.23 \mathrm{MPa} \text { exact }) \\
& 3000 \mathrm{psi}=20 \mathrm{MPa}(20.67 \mathrm{MPa} \text { exact }) \\
& 3500 \mathrm{psi}=25 \mathrm{MPa}(24.12 \mathrm{MPa} \text { exact }) \\
& 4000 \mathrm{psi}=30 \mathrm{mpa}(27.57 \mathrm{mpa} \text { exact }) \\
& 5000 \mathrm{psi}=35 \mathrm{mpa}(34.46 \mathrm{mpa} \text { exact }) \\
& 6000 \mathrm{psi}=40 \mathrm{mpa}(41.35 \mathrm{mpa} \text { exact })
\end{aligned}
$$

Use 0.0068915 to convert psi to MPa.

## Newtons, psi, Concrete Strength, and Prestressed Slabs

Concrete strengths are customarily denominated in psi (pounds per square inch) in the imperial system and in MPa (megapascals) in metric. These are units of pressure.

The newton ( N ) is a measure of force. 1 newton is that force which pushes 1 gram of matter with an acceleration of 1 centimeter per second per second (or per second ${ }^{2}$ ) or, equivalently, the force that accelerates 1 kilogram of matter to 1 meter per second ${ }^{2}$.

$$
\text { Force }=\text { mass } \times \text { acceleration }
$$

Velocity is a measure of constant speed (i.e., meters per second, miles per hour, furlongs per fortnight)
Velocity is speed in a certain direction
Acceleration is the rate of change in velocity over time
Acceleration can be either positive or negative (deceleration)

$$
\begin{aligned}
& 1 \mathrm{~N}=1 \mathrm{~kg} \times\left(1 \mathrm{~meter} / \text { second }^{2}\right) \longrightarrow 1 \mathrm{~N}=1 \mathrm{~kg} \cdot \mathrm{~meter} / \text { second }^{2} \\
& 1 \mathrm{~N}=1 \mathrm{~g} \times\left(1 \mathrm{~cm} / \text { second }^{2}\right)
\end{aligned}
$$

When you apply the force of 1 newton to a 1 meter ${ }^{2}$ area, you have pressure.

$$
\text { Pressure }=\text { force per area }
$$

By permission: mc2-ice.com

### 5.4.0 Sieve Analysis Defining Coarse and Fine Aggregates

| Sieve Analysis Table |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Sieve Size | Coarse <br> Aggregate |  |  | Fine <br> Aggregate |  | Cumulative Combined |  | Combined |
|  | 1\% <br> Passing | $2 \%$ <br> Passing | 3\% <br> Passing | 1\% <br> Passing | $2 \%$ <br> Passing | $\%$ <br> Passing | \% <br> Retained | \% <br> Retained |
| 2 in . | 100 | 100 | - | 100 | - | 100 | 0 | 0 |
| 1-1/2 in. | 100 | 100 | - | 100 | - | 100 | 0 | 0 |
| 1 in . | 95 | 100 | - | 100 | - | 98 | 2 | 2 |
| 3/4 in. | 62 | 100 | - | 100 | - | 81 | 19 | 17 |
| 1/2 in. | 35 | 100 | - | 100 | - | 67 | 33 | 14 |
| 3/8 in. | 20 | 95 | - | 100 | - | 59 | 41 | 8 |
| No. 4 | 1 | 65 | - | 100 | - | 46 | 54 | 13 |
| No. 8 | - | 1 | - | 96 | - | 36 | 64 | 10 |
| No. 16 | - | - | - | 79 | - | 29 | 71 | 7 |
| No. 30 | - | - | - | 45 | - | 17 | 83 | 12 |
| No. 50 | - | - | - | 17 | - | 6 | 94 | 11 |
| No. 100 | - | - | - | 7 | - | 3 | 97 | 3 |
| No. 200 | - | - | - | 2 | - | 1 | 99 | 3 |
| Pan | - | - | - | 0 | - | 0 | 100 | 1 |
| \% of Aggregate | 50\% | 13\% | 0\% | 37\% | 0\% | - | - | - |

## Charts

Coarseness Factor Chart-Use the cumulative combined sieve analysis to determine the coarseness and workability factors. Plot the coarseness and workability factors on the Coarseness Factor Chart.

Determine the coarseness factor using the following equation:

$$
\text { Coarseness Factor }=\left(\frac{\mathrm{S}}{\mathrm{~T}}\right) \times 100
$$

where:
$\mathrm{S}=\%$ Cumulative Retained on $3 / 8 \mathrm{in}$. Sieve and
$\mathrm{T}=\%$ Cumulative Retained on No. 8 Sieve.

The workability factor is the cumulative combined percentage passing the No. 8 sieve.
Increase the workability factor by 2.5 percentage points for every 94 lb per cubic yard of cementitious material used in excess of 564 lb per cubic yard in the mix design.
Decrease the workability factor by 2.5 percentage points for every 94 lb per cubic yard
Source: Texas DOT

### 5.4.1 Air-Entraining Admixtures

## Air Entrainment

Air entrainment is the process whereby many small air bubbles are incorporated into concrete and become part of the matrix that binds the aggregate together in the hardened concrete. These air bubbles are dispersed throughout the hardened cement paste but are not, by definition, part of the paste (Dolch 1984). Air entrainment has now been an accepted fact in concrete technology for more than 45 years. Although historical references indicate that certain archaic and early twentieth-century concretes were indeed inadvertently air entrained, the New York State Department of Public Works and the Universal Atlas Cement Company were among the first to recognize that controlled additions of certain naturally occurring organic substances derived from animal and wood by-products could materially increase the resistance of concrete in roadways to attack brought on by repeated freeze-thaw cycles and the application of deicing agents.

Extensive laboratory testing and field investigation concluded that the formation of minute air bubbles dispersed uniformly through the cement paste increased the freeze-thaw durability of concrete. This formation can be achieved through the use of organic additives, which enable the bubbles to be stabilized or entrained within the fresh concrete. These additives are called air-entraining agents.

Besides the increase in freeze-thaw and scaling resistances, air-entrained concrete is more workable than nonentrained concrete. The use of air-entraining agents also reduces bleeding and segregation of fresh concrete.

Materials and Specifications. The most commonly used chemical surfactants can be categorized into four groups: (1) salts of wood resins, (2) synthetic detergents, (3) salts of petroleum acids, and (4) fatty and resinous acids and their salts (Dolch 1984).

Until the early 1980s, the majority of concrete air entrainers were based solely on salts of wood resins or neutralized Vinsol resin (Edmeades and Hewlett 1986), and most concrete highway structures and pavements were air entrained by Vinsol resin. Today, a wider variety of air-entraining agents is available and competes with Vinsol resins.

Requirements and specifications of air-entraining agents to be used in concrete are covered in ASTM C 260 and AASHTO M 154. According to these specifications, each admixture to be used as an air-entraining agent should cause a substantial improvement in durability, and none of the essential properties of the concrete should be seriously impaired. This provides a means to evaluate air-entraining admixtures on a performance basis.

Factors Affecting Air Entrainment. The air-void system created by using air-entraining agents in concrete is also influenced by concrete materials and construction practice. Concrete materials such as cement, sand, aggregates, and other admixtures play an important role in maintaining the air-void system in concrete. It has been found that air content will increase as cement alkali levels increase and decrease as cement fineness increases significantly.

Fine aggregate serves as a three-dimensional screen and traps the air; the more median sand there is in the total aggregate, the greater the air content of the concrete will be (Dolch 1984). Gradation has more influence in leaner mixes. Median sand ranging from the No. 30 sieve to the No. 100 is the most effective at entraining air. Excessive fines, minus No. 100 material, causes a reduction in air entrainment.

Because the use of chemical and mineral admixtures in addition to air-entraining agents has become common practice, concrete users are always concerned about the effects of these admixtures on the air-void system and durability of concrete. The effects of water reducers, retarders, and accelerators were widely investigated by many researchers. In regards to gross air content obtained when water-reducing and retarding admixtures are used in concrete, numerous studies have shown that for most of the materials, less air-entraining agent is needed to achieve a given specified air content. Chemical admixtures should be added separately from air-entraining additives.

Source: U.S. Department of Transportation-FHWA
When lignosulfonate water reducers are used, less air-entraining agent is required because the lignosulfonates have a moderate air-entraining capacity, although alone they do not react as air-entraining agents (Dolch 1984). For a fixed amount of air-entraining agent, the effect of added calcium chloride is to slightly increase the air content (Edmeades and Hewlett 1986). The effect is more pronounced as amounts greater than $1 \%$ of the weight of cement are used. Some HRWR (superplasticizers) interact with cements and air-entraining agents, resulting in reductions in specific surfaces and increases in air-void spacing factors.

Mineral admixtures such as fly ash and silica fume also affect the formation of void systems in concrete. In their study on the effect of fly ash on air-void stability of concrete, showed that concretes containing fly ash produced relatively stable air-void systems. However, the volume of air retained is affected by fly ash types. In mixtures containing fly ashes, the amount of air-entraining agent required to produce a given percentage of entrained air is higher, and sometimes much higher, than it is in comparable mixtures without fly ash. In a series of papers, researchers presented the results of a study on factors that affect the air-void stability in concretes (Pigeon, Aitcin, and LaPlante 1987; Pigeon and Plante 1989). They found that silica fume has no significant influence on the production and stability of the air-void system during mixing and agitation. Also indicated that silica fume has no detrimental effects on the air-void system.

Temperature can also have a significant effect on air entrainment. Air entrainment varies inversely with temperature. The same mix will entrain more air at $50^{\circ} \mathrm{F}\left(10^{\circ} \mathrm{C}\right)$ than at $100^{\circ} \mathrm{F}\left(38^{\circ} \mathrm{C}\right)$.

Air Content Control. Measurement of air content is an important checking "sensor" for the concrete user to know whether concrete will resist freeze-thaw damage. Because average void spacing decreases as air content increases, an "optimum" air content at which void spacing will prevent the development of excessive pressure due to freezing and thawing will exist.

It is important to check the air content of fresh concrete regularly for control purposes. Air content should be tested not only at the mixer but also at the point of discharge into the forms, because of losses of air content due to handling and transportation.

## Recommendations

1. Air-entraining admixtures should be specified when concrete will be exposed to freeze-thaw conditions, deicing salt applications, or sulfate attack.
2. Although air-entraining admixtures are compatible with most other admixtures, care should be taken to prevent them from coming in contact during the mixing process.

## References

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www.fhwa.dot.gov/. . ./airentr.htm.

### 5.4.2 Superplasticizers

The use of superplasticizers (high-range water reducer) has become a quite common practice. This class of water reducers was originally developed in Japan and Germany in the early 1960s; it was introduced in the United States in the mid-1970s.

Superplasticizers are linear polymers containing sulfonic acid groups attached to the polymer backbone at regular intervals. Most of the commercial formulations belong to one of four families:

- Sulfonated melamine-formaldehyde condensates (SMF)
- Sulfonated naphthalene-formaldehyde condensates (SNF)
- Modified lignosulfonates (MLS)
- Polycarboxylate derivatives

The sulfonic acid groups are responsible for neutralizing the surface charges on the cement particles and causing dispersion, thus releasing the water tied up in the cement particle agglomerations and thereafter reducing the viscosity of the paste and concrete.

ASTM C 494 was modified to include high-range water-reducing admixtures in the edition published in July 1980. The admixtures were designated Type F water-reducing, high-range admixtures and Type G water-reducing, high-range, and retarding admixtures.

Effect of Superplasticizers on Concrete Properties. The main purpose of using superplasticizers is to produce flowing concrete with very high slump in the range of $7-9 \mathrm{in}$. ( $175-225 \mathrm{~mm}$ ) to be used in heavily reinforced structures and in placements where adequate consolidation by vibration cannot be readily achieved. The other major application is the production of high-strength concrete at w/c's ranging from 0.3 to 0.4 .

The ability of superplasticizers to increase the slump of concrete depends on such factors as the type, dosage, and time of addition of superplasticizer; w/c; and the nature or amount of cement. It has been found that for most types of cement, superplasticizer improves the workability of concrete. For example, incorporation of $1.5 \%$ SMF to a concrete containing Type I, II, and V cements increases the initial slump of 3 in . ( 76 mm ) to 8.7, 8.5 , and 9 in . (222, 216, and 229 mm ), respectively.

The capability of superplasticizers to reduce water requirements $12-25 \%$ without affecting the workability leads to production of high-strength concrete and lower permeability. Compressive strengths greater than $14,000 \mathrm{psi}(96.5 \mathrm{mpa})$ at 28 days have been attained. Use of superplasticizers in air-entrained concrete can produce coarser-than-normal air-void systems. The maximum recommended spacing factor for air-entrained concrete to resist freezing and thawing is $0.008 \mathrm{in} .(0.2 \mathrm{~mm})$. In superplasticized concrete, spacing factors in many cases exceed this limit. Even though the spacing factor is relatively high, the durability factors are above 90 after 300 freeze-thaw cycles for the same cases. A study indicated that high-workability concrete containing superplasticizer can be made with a high freeze-thaw resistance, but air content must be increased relative to concrete without superplasticizer. This study also showed that the type of superplasticizer has nearly no influence on the air-void system.

One problem associated with using a high-range water reducer in concrete is slump loss. In a study of the behavior of fresh concrete containing conventional water reducers and high-range water reducer, found that with time slump loss is very rapid in spite of the fact that second-generation high-range water reducers are claimed not to suffer as much from the slump loss phenomenon as the first-generation conventional water reducers do. However, slump loss of flowing concrete was found to be less severe, especially for newly developed admixtures based on copolymeric formulations.

The slump loss problem can be overcome by adding the admixture to the concrete just before the concrete is placed.

Source: U.S. Department of Transportation-FHWA
However, there are disadvantages to such a procedure. The dosage control, for example, might not be adequate, and it requires ancillary equipment such as truck-mounted admixture tanks and dispensers. Adding admixtures at the batch plant, besides dosage control improvement, reduces wear of truck mixers and lessens the tendency to add water onsite. New admixtures now being marketed can be added at the batch plant and can hold the slump above 8 in . ( 204 mm ) for more than 2 hours.

## Recommendations

1. Verification tests should be performed on liquid admixtures to confirm that the material is the same as that which was approved. The identifying tests include chloride and solids content, ph, and infrared spectrometry.
2. If transit mix trucks are used to mix high-slump concrete, it is recommended that a 75 mm slump concrete be used at a full mixing capacity to ensure uniform concrete properties.
3. If transit mix trucks are used to mix low $\mathrm{w} / \mathrm{c}$ ratio concrete, it is recommended that the load size be reduced to $1 / 2$ to $2 / 3$ of the mixing capacity to ensure uniform concrete properties.
4. If freeze-thaw testing as described by ASTM C 666 indicates this to be a problem, it is recommended that the air content be increased by $1.5 \%$.

### 5.4.3 Fly Ash

## Infrastructure

Fly ashes are finely divided residue resulting from the combustion of ground or powdered coal. They are generally finer than cement and consist mainly of glassy-spherical particles as well as residues of hematite and magnetite, char, and some crystalline phases formed during cooling. Use of fly ash in concrete started in the United States in the early 1930s. The first comprehensive study was that described in 1937, by R. E. Davis at the University of California. The major breakthrough in using fly ash in concrete was the construction of Hungry Horse Dam in 1948, utilizing 120,000 metric tons of fly ash. This decision by the U.S. Bureau of Reclamation paved the way for using fly ash in concrete constructions.

In addition to economic and ecological benefits, the use of fly ash in concrete improves its workability; reduces segregation, bleeding, heat evolution, and permeability; inhibits alkali-aggregate reaction; and enhances sulfate resistance. Even though the use of fly ash in concrete has increased in the last 20 years, less than $20 \%$ of the fly ash collected was used in the cement and concrete industries.

One of the most important fields of application for fly ash is PCC pavement, where a large quantity of concrete is used and economy is an important factor in concrete pavement construction. The FHWA has been encouraging the use of fly ash in concrete. When the price of fly ash concrete is equal to, or less than, the price of mixes with only portland cement, fly ash concretes are given preference if technically appropriate under FHWA guidelines.

## Classifications and Specifications

Two major classes of fly ash are specified in ASTM C 618 on the basis of their chemical composition resulting from the type of coal burned; these are designated Class F and Class C. Class F is fly ash normally produced from burning anthracite or bituminous coal, and Class C is normally produced from the burning of subbituminous coal and lignite (as are found in some of the western states of the United States) (Halstead 1986). Class C fly ash usually has cementitious properties in addition to pozzolanic properties due to free lime, whereas Class F is rarely cementitious when mixed with water alone. All fly ashes used in the United States before 1975 were Class F.

Fly ash that is produced at base-loaded electric generating plants is usually very uniform. Base-loaded plants are those plants that operate continuously. The only exception to uniformity is in the start-up and the shutdown of these plants. Contamination may occur from using other fuels to start the plant, and inconsistencies in carbon content occur until the plant reaches full operating efficiency. The ash produced from the start-up and shutdown must be separated from what is produced when the plant is running efficiently. In addition, when sources of coal are changed, it is necessary to separate the two types of fly ashes. Peak-load plants are subjected to many start-up and shutdown cycles. Because of this, these plants may not produce much uniform fly ash.

The most often used specifications for fly ash are ASTM C 618 and AASHTO M 295. While some differences exist, these two specifications are essentially equivalent. Some state transportation agencies have specifications that differ from the standards. The general classification of fly ash by the type of coal burned does not adequately define the type of behavior to be expected when the materials are used in concrete.

There are also wide differences in characteristics within each class. Despite the reference in ASTM C 618 to the classes of coal from which Class F and Class C fly ashes are derived, there was no requirement that a given class of fly ash must come from a specific type of coal. For example, Class F ash can be produced from coals that are not bituminous, and bituminous coals can produce ash that is not Class F (Halstead 1986). It should be noted that current standards contain numerous physical and chemical requirements that do not serve a useful purpose. Whereas some requirements are needed for ensuring batch-to-batch uniformity, many are unnecessary.

Source: U.S. Department of Transportation-FHWA

## Mix Design

The substitution rate of fly ash for portland cement will vary depending on the chemical composition of both the fly ash and the portland cement. The rate of substitution typically specified is a minimum of 1 to $1 \frac{1}{2}$ pounds of fly ash to 1 pound of cement. It should be noted that the amount of fine aggregate will have to be reduced to accommodate the additional volume of fly ash. This is due to fly ash being lighter than the cement.

The amount of substitution is also dependent on the chemical composition of the fly ash and the portland cement. Currently, states allow a maximum substitution in the range of 15 to $25 \%$.

The effects of fly ash, especially Class F, on fresh and hardened concrete properties have been extensively studied by many researchers in different laboratories, including the U.S. Army Corps of Engineers, PCA (Portland Cement Association), and the Tennessee Valley Authority. The two properties of fly ash that are of most concern are its carbon content and fineness. Both of these properties will affect the air content and water demand of the concrete.

The finer the material, the higher the water demand due to the increase in surface area. The finer material requires more air-entraining agent to give the mix the desired air content. The important thing to remember is uniformity. If fly ash is uniform in size, the mix design can be adjusted to give a good uniform mix.

The carbon content, which is indicated by the loss of ignition, also affects the air-entraining agents and reduces the entrained air for a given amount of air-entraining agent. More air-entraining agent will need to
be added to get the desired air content. The carbon content will also affect water demand since the carbon will absorb water. Again, uniformity is important since the differences from non-fly ash concrete can be adjusted in the mix design.

Fresh Concrete Workability. Use of fly ash increases the absolute volume of cementitious materials (cement plus fly ash) compared to non-fly-ash concrete. Therefore, the paste volume is increased, leading to a reduction in aggregate particle interference and enhancement in concrete workability. The spherical particle shape of fly ash also participates in improving the workability of fly ash concrete because of the so-called ball-bearing effect. It has been found that both classes of fly ash improve concrete workability.

Bleeding. Using fly ash in air-entrained and non-air-entrained concrete mixtures usually reduces bleeding by providing greater fines volume and lower water content for a given workability. Although increased fineness usually increases the water demand, the spherical particle shape of the fly ash lowers particle friction and offsets such effects. Concrete with relatively high fly ash content will require less water than non-fly-ash concrete of equal slump.

Time of Setting. All Class F and most Class C fly ashes increase the time of setting of concrete. Time of setting of fly ash concrete is influenced by the characteristics and amounts of fly ash used in concrete. For highway construction, changes in time of setting of fly ash concrete from non-fly-ash concrete using similar materials will not usually introduce a need for changes in construction techniques; the delays that occur may be considered advantageous.

Strength and Rate of Strength of Hardened Concrete. Strength of fly ash concrete is influenced by type of cement, quality of fly ash, and curing temperature compared to that of non-fly-ash concrete proportioned for equivalent 28 -day compressive strength. Concrete containing typical Class F fly ash may develop lower strength at 3 or 7 days of age when tested at room temperature. However, fly ash concretes usually have higher ultimate strengths when properly cured. The slow gain of strength is the result of the relatively slow pozzolanic reaction of fly ash. In cold weather, the strength gain in fly ash concretes can be more adversely affected than the strength gain in non-fly-ash concrete. Therefore, precautions must be taken when fly ash is used in cold weather.

Freeze-Thaw Durability of Hardened Concrete. On the basis of a comparative experimental study of freezethaw durability of conventional and fly ash concrete, it has been observed that the addition of fly ash has no major effect on the freeze-thaw resistance of concrete if the strength and air content are kept constant. The addition of fly ash may have a negative effect on the freeze-thaw resistance of concrete when a major part of the cement is replaced by it. The use of fly ash in air-entrained concrete will generally require an increase in the dosage rate of the air-entraining admixture to maintain constant air. Air-entraining admixture dosage depends on carbon content, loss of ignition, fineness, and amount of organic material in the fly ash. ACI Carbon content of fly ash, which is related to the coal burned by the producing utility of the type and condition of furnaces in the production process of fly ash, influences the behavior of admixtures in concrete. It has been found that high-carbon-content fly ash reduces the effectiveness of admixtures such as air-entraining agents.

Alkali-Silica Reaction of Hardened Concrete. One important reason for using fly ash in highway construction is to inhibit the expansion resulting from alkali-silica reaction (ASR). It has been found that (1) the alkalies released by the cement preferentially combine with the reactive silica in the fly ash rather than in the aggregate, and (2) the alkalies are tied up in nonexpansive calcium-alkali-silica gel. Thus hydroxyl ions remaining in the solution are insufficient to react with the material in the interior of the larger reactive aggregate particles, and disruptive osmotic forces are not generated.

In a paper presented at the 8th International Conference on alkali-aggregate reactivity held in Japan in 1989, Swamy and Al-Asali indicated that ASR expansion is generally not proportional to the percentage of cement replacement by fly ash. The rate of reactivity, the replacement level, the method of replacement, and the environment all have a profound influence on the protection against ASR afforded by fly ash. Several investigators have stated that ASR expansions correlated better with water-soluble alkali-silica content than with total
alkali content. The addition of some high-calcium fly ash containing large amounts of soluble alkali sulfate might increase rather than decrease the alkali-aggregate reactivity. The effectiveness of different fly ashes in reducing long-term expansion varied widely; for each fly ash, this may be dependent on its alkali content or fineness.

## Blended Cements

The following will discuss the Type "IP," "P," and "I(PM)" cements. The specifications for these cements are in AASHTO M-240 and ASTM C-595. Blended cements can be manufactured by either intimate blending of portland cement and pozzolan or intergrinding of the pozzolan with the cement clinker in the kiln. Type "I(PM)" (pozzolan-modified cement) allows up to $15 \%$ replacement of cement with fly ash. The Type "IP" and Type"P" are pozzolan-modified portland cements that allow $15-40 \%$ replacement with pozzolans. The difference in the two types of cements is in the ultimate strength and the rate of strength gain of the concretes. Most states specify limits on the pozzolanic content on Type "IP" cement. These limits are between 15 and $25 \%$.

## Restraints on the Use of Fly Ash Concrete in Highway Construction

It is well known now that both classes of fly ash improve the properties of concrete, but several factors and cautions should be considered when using fly ashes, especially in highway construction, where fly ash is heavily used. A report prepared by the Virginia Highway and Transportation Research Council (VHTRC) and discussed several restraints relating to the use of fly ash concrete for construction of highways and other highway structures. These restraints include the following: (1) special precautions may be necessary to ensure that the proper amount of entrained air is present; (2) not all fly ashes have sufficient pozzolanic activity to provide good results in concrete; (3) suitable fly ashes are not always available near the construction site, and transportation costs may nullify any cost advantage; and (4) mix proportions might have to be modified for any change in the fly ash composition.

Since the cement-fly ash reaction is influenced by the properties of the cement, it is important for a transportation agency not only to test and approve each fly ash source but also to investigate the properties of the specific fly ash-cement combination to be used for each project.

### 5.4.4 Silica Fume

Silica fume, also known as microsilica, is a by-product of the reduction of high-purity quartz with coal in electric furnaces in the production of silicon and ferrosilicon alloys. Silica fume is also collected as a by-product of other silicon alloys such as ferrochromium, ferromanganese, ferromagnesium, and calcium silicon. Before the mid-1970s, nearly all silica fume was discharged into the atmosphere. After environmental concerns necessitated the collection and landfilling of silica fume, it became economically justified to use it in various applications.

Silica fume consists of very fine vitreous particles with a surface area ranging from 60,000 to $150,000 \mathrm{ft}^{2} / \mathrm{lb}$ or 13,000 to $30,0000 \mathrm{~m}^{2} / \mathrm{kg}$ when measured by nitrogen absorption techniques, with particles approximately 100 times smaller than the average cement particle. Because of its extreme fineness and high silica content, silica fume is a highly effective pozzolanic material. Silica fume is used in concrete to improve its properties. It has been found that it improves compressive strength, bond strength, and abrasion resistance; reduces permeability; and therefore helps in protecting reinforcing steel from corrosion.

## Specifications

The first national standard for use of silica fume. The AASHTO and ASTM C 1240 covers microsilica for use as a mineral admixture in PCC and mortar to fill small voids and in cases in which pozzolanic action is desired. It provides the chemical and physical requirements, specific acceptance tests, and packaging and package marking.

## Mix Design

Silica fume has been used as an addition to concrete up to $15 \%$ by weight of cement, although the normal proportion is 7 to $10 \%$. With an addition of $15 \%$, the potential exists for very strong, brittle concrete. It increases the water demand in a concrete mix; however, dosage rates of less than $5 \%$ will not typically require a water reducer. High replacement rates will require the use of a high-range water reducer.

Effects on Air Entrainment and Air-Void System of Fresh Concrete. The dosage of air-entraining agents needed to maintain the required air content when using silica fume is slightly higher than that for conventional concrete because of the high surface area and the presence of carbon. This dosage is increased with increasing amounts of silica fume content in concrete.

Effects on Water Requirements of Fresh Concrete. Silica fume added to concrete by itself increases water demands, often requiring one additional pound of water for every pound of added silica fume. This problem can be easily compensated for by using HRWR.

Effects on Consistency and Bleeding of Fresh Concrete. Concrete incorporating more than $10 \%$ silica fume becomes sticky; in order to enhance workability, the initial slump should be increased. It has been found that silica fume reduces bleeding because of its effect on rheologic properties.

Effects on Strength of Hardened Concrete. Silica fume has been successfully used to produce very high-strength, low-permeability, and chemically resistant concrete. Addition of silica fume by itself, with other factors being constant, increases the concrete strength.

Incorporation of silica fume into a mixture with HRWR also enables the use of a lower water-to-cementitiousmaterials ratio than may have been possible otherwise. The modulus of rupture of silica fume concrete is usually either about the same as or somewhat higher than that of conventional concrete at the same level of compressive strength.

Effects on Freeze-thaw Durability of Hardened Concrete. Air-void stability of concrete incorporating silica fume was studied by Pigeon, Aitcin, and LaPlante (1987) and Pigeon and Plante (1989). Their test results indicated that the use of silica fume has no significant influence on the production and stability of the air-void system. Freeze-thaw testing (ASTM C 666) on silica fume concrete showed acceptable results; the average durability factor was greater than $99 \%$.

Source: U.S. Department of Transportation-FHWA
Effects on Permeability of Hardened Concrete. It has been shown by several researchers that addition of silica fume to concrete reduces its permeability. Rapid chloride permeability testing (AASHTO 277) conducted on silica fume concrete showed that addition of silica fume ( $8 \%$ silica fume) significantly reduces the chloride permeability. This reduction is primarily the result of the increased density of the matrix due to the presence of silica fume.

Effects on ASR of Hardened Concrete. Silica fume, like other pozzolans, can reduce ASR and prevent deleterious expansion due to ASR.

## Availability and Handling

Silica fume is available in two conditions: dry and wet. Dry silica can be provided as produced or densified with or without dry admixtures and can be stored in silos and hoppers. Silica fume slurry with low or high dosages of chemical admixtures is available. Slurried products are stored in tanks with capacities ranging from a few thousand to 400,000 gallons $\left(15100 \mathrm{~m}^{3}\right)$.

### 5.4.5 Ground Granulated Blast-Furnace Slag

Although portland blast-furnace slag cement, which is made by intergrinding the granulated slag with portland cement clinker (blended cement), has been used for more than 60 years, the use of separately ground slag combined with portland cement at the mixer as a mineral admixture did not start until the late 1970s. Ground granulated blast-furnace slag is the granular material formed when molten iron blast-furnace slag is rapidly chilled (quenched) by immersion in water. It is a granular product with very limited crystal formation, is highly cementitious in nature and, ground to cement fineness, and hydrates like portland cement.

## Specifications

ASTM C 989-82 and AASHTO M 302 were developed to cover ground granulated blast-furnace slag for use in concrete and mortar. The three grades are 80,100 , and 120.

## Mix Design

The use of grade 80 ground granulated blast-furnace slag should be avoided unless warranted in special circumstances. The grade of a ground granulated blast-furnace slag is based on its activity index, which is the ratio of the compressive strength of a mortar cube made with a $50 \%$ ground granulated blast-furnace slag-cement blend to that of a mortar cube made with a reference cement. For a given mix, the substitution of grade 120 ground granulated blast-furnace slag for up to $50 \%$ of the cement will generally yield a compressive strength at 7 days and beyond equivalent to or greater than that of the same concrete made without ground granulated blast-furnace slag. Substitution of grade 100 ground granulated blast-furnace slag will generally yield an equivalent or greater strength at 28 days. However, concrete made with grade 80 ground granulated blastfurnace slag will have a lower compressive strength at all ages. To provide a product with equivalent or greater compressive strengths, only grades 100 and 120 ground granulated blast-furnace slag should be used. However, in mass concrete, the heat of hydration may be an overriding factor, and the use of grade 80 slag may be appropriate.

Ground granulated blast-furnace slag is a cementitious material and can be substituted for cement on a 1:1 basis. In the absence of special circumstances or mix specific data, the substitution of ground granulated blastfurnace slag should be limited to $50 \%$ for areas not exposed to deicing salts and to $25 \%$ for concretes that will be exposed to deicing salts. While substitution of ground granulated blast-furnace slag for up to $70 \%$ of the portland cement in a mix has been used, there appears to be an optimum substitution percentage that produces the greatest 28 -day strength. This is typically $50 \%$ of the total cementitious material but depends on the grade of ground granulated blast-furnace slag used. Also, research has shown that the scaling resistance of concretes decreases with ground granulated blast-furnace slag substitution rates greater than $25 \%$.

These guidelines on ground granulated blast-furnace slag substitution rates are intended to provide a starting point for designers with little or no experience in the use of cement and concrete containing ground granulated blast-furnace slag. If local data shows good performance at greater percentages, this information can be used in lieu of the recommended guidelines. Section 4.2.3.2 of ACI 318-89, "Building Code Requirements for

Reinforced Concrete," indicates that substitution rates of up to $50 \%$ may be acceptable for concretes exposed to deicing chemicals. In addition, in mass concreting operations, the heat of hydration may be an overriding factor, and substitution rates greater than $50 \%$ may be appropriate.

Source: U.S. Department of Transportation-FHWA
Effects of Slags on Properties of Fresh Concrete. Use of slag or slag cements usually improves workability and decreases the water demand due to the increase in paste volume caused by the lower relative density of slag. The higher strength potential of Grade 120 slag may allow for a reduction of total cementitious material. In such cases, further reductions in water demand may be possible.

The setting times of concretes containing slag increase as the slag content increases. An increase of slag content from 35 to $65 \%$ by mass can extend the setting time by as much as 60 minutes. This delay can be beneficial, particularly in large pours and in hot weather conditions in which this property prevents the formation of "cold joints" in successive pours.

The rate and quantity of bleeding in concrete containing slag or slag cements are usually less than that in concrete containing no slag because of the relatively higher fineness of slag. The higher fineness of slag also increases the air-entraining agent required, compared to conventional concrete. However, unlike fly ash, slag does not contain carbon, which may cause instability and air loss in concrete.

Effect on Strength of Hardened Concrete. The compressive strength development of slag concrete depends primarily on the type, fineness, activity index, and proportions of slag used in concrete mixtures. In general, the strength development of concrete incorporating slags is slow at $1-5$ days compared with that of the control concrete. Between 7 and 28 days, the strength approaches that of the control concrete; beyond this period, the strength of the slag concrete exceeds the strength of control concrete (Admixtures and ground slag 1990). Flexural strength is usually improved by the use of slag cement, which makes it beneficial to concrete paving applications where flexural strengths are important. It is believed that the increased flexural strength is the result of the stronger bonds in the cement-slag-aggregate system because of the shape and surface texture of the slag particles.

Problems occur when slag concrete is used in cold weather applications. At low temperatures, the strengths are substantially reduced up to 14 days, and the percentage of slag is usually reduced to $25-30 \%$ of replacement levels; when saw cutting of joints is required, the use of slag is discontinued.

Effects on Permeability of Hardened Concrete. Incorporation of granulated slags in cement paste helps in the transformation of large pores in the paste into smaller pores, resulting in decreased permeability of the matrix and of the concrete. Indicated that significant reduction in permeability is achieved as the replacement level of the slag increases from 40 to $65 \%$ of total cementitious material by mass. Because of the reduction in permeability, concrete containing granulated slag may require less depth of cover than conventional concrete requires to protect the reinforcing steel.

Effects on Freeze-Thaw Durability of Hardened Concrete. Freeze-thaw durability of slag concrete has been studied by many researchers. It has been reported that resistance of air-entrained concrete incorporating ground granulated blast-furnace slag is comparable to that of conventional concrete. Reported results of freeze-thaw tests on concrete incorporating $25-65 \%$ slag. Test results indicate that regardless of the water-to-(cement + slag) ratio, air-entrained slag concrete specimens performed excellently in freeze-thaw tests, with relative durability factors greater than $91 \%$.

Effect on ASR of Hardened Concrete. Effectiveness of slag in preventing damage due to ASR is attributed to the reduction of total alkalies in the cement-slag blend, the lower permeability of the system, and the tying up of the alkalies in the hydration process. There have been many studies of ground granulated blast-furnace slag that has been used as partial replacement for portland cement in concrete to reduce expansion caused by alkali-aggregate reaction.

## Handling, Storage, and Batching

Ground granulated blast-furnace slag should be stored in separate watertight silos (such as those used for cement) and should be clearly marked to avoid confusion with cement. In batching, it is recommended that portland cement be weighed first and then followed by the slag. Slag is like cement in that normal valves are adequate to stop the flow of material.

### 5.5.0 Structural Concrete Components-Calculations to Achieve High-Strength Concrete

| Concrete Class | Required Field <br> Compressive Strength | Cement Content: Minimum or Range | Air Content: Percent Range (Total) | Water Cement Ratio: Maximum or Range |
| :---: | :---: | :---: | :---: | :---: |
| B | 3000 psi at 28 days | $565 \mathrm{lbs} / \mathrm{cu} \mathrm{yd}$ | 5-8 | N/A |
| BZ | 4000 psi at 28 days | $610 \mathrm{lbs} / \mathrm{cu} \mathrm{yd}$ | N/A | N/A |
| D | 4500 psi at 28 days | 615 to $660 \mathrm{lbs} . / \mathrm{cu} \mathrm{yd}$ | 5-8 | 0.44 |
| DT | 4500 psi at 28 days | $700 \mathrm{lbs} / \mathrm{cu} \mathrm{yd}$ | 5-8 | 0.44 |
| E | 4200 psi at 28 days | $660 \mathrm{lbs} / \mathrm{cu} \mathrm{yd}$ | 4-8 | 0.44 |
| H | 4500 psi at 56 days | 580 to $640 \mathrm{lbs} / \mathrm{cu} \mathrm{yd}$ | 5-8 | 0.38-0.42 |
| HT | 4500 psi at 56 days | 580 to $640 \mathrm{lbs} / \mathrm{cu} \mathrm{yd}$ | 5-8 | 0.38-0.42 |
| P | 4200 psi at 28 days | $660 \mathrm{lbs} / \mathrm{cu} \mathrm{yd}$ | 4-8 | 0.44 |
| S35 | 5000 psi at 28 days | 615 to $720 \mathrm{lbs} / \mathrm{cu} \mathrm{yd}$ | 5-8 | 0.42 |
| S40 | 5800 psi at 28 days | 615 to $760 \mathrm{lbs} / \mathrm{cu} \mathrm{yd}$ | 5-8 | 0.40 |
| S50 | 7250 psi at 28 days | 615 to $800 \mathrm{lbs} / \mathrm{cu} \mathrm{yd}$ | 5-8 | 0.38 |

Class B concrete is an air-entrained concrete for general use. Class D or H concrete may be substituted for Class B concrete. Additional requirements for Class B concrete are: Class B concrete shall have a nominal coarse aggregate size of 37.5 mm ( $11 / 2 \mathrm{in}$.) or smaller. Approved fly ash may be substituted for portland cement up to a maximum of $20 \%$ Class C or $30 \%$ Class F by weight.

Class BZ concrete is concrete for drilled piers. Additional requirements for class BZ concrete are: Entrained air is not required unless specified in the contract. High-range water reducers may be added at the job site to obtain desired slump and retardation. Slump shall be a minimum of 5 in . and a maximum of 8 in . Class BZ caisson concrete shall be made with $19.0 \mathrm{~mm}(3 / 4 \mathrm{in}$.) nominal-sized coarse aggregate. Approved fly ash may be substituted for portland cement up to a maximum of $20 \%$ Class C or $30 \%$ Class F by weight.

Class D concrete is a dense medium strength structural concrete. Class H may be substituted for Class D concrete. Additional requirements for Class D concrete are: An approved water-reducing admixture shall be incorporated in the mix. Class D concrete shall be made with $19.0 \mathrm{~mm}(3 / 4 \mathrm{in}$.) nominal-sized coarse aggregate. When placed in a bridge deck, Class D concrete shall contain a minimum of 55\% AASHTO M 43 size No. 67 coarse aggregate. Approved fly ash may be substituted for portland cement up to a maximum of $20 \%$ Class C or $30 \%$ Class F by weight.

Class DT concrete may be used for deck resurfacing and repairs. Class HT may be substituted for Class DT concrete. Additional requirements for Class DT concrete are: An approved water-reducing admixture shall be incorporated in the mix. Class DT concrete shall contain a minimum of $50 \%$ AASHTO M 43 size No. 7 or No. 8 coarse aggregate. Approved fly ash may be substituted for portland cement up to a maximum of $20 \%$ Class C or 30\% Class F by weight.

Class E concrete may be used for fast-track pavements needing early strength in order to open a pavement to service soon after placement. Additional requirements for Class E concrete are: Type III cement may be used. Class E concrete shall contain a minimum of $55 \%$ AASHTO M 43 size No. 357 or No. 467 coarse aggregate. If all transverse joints are doweled, then Class E concrete shall contain a minimum of $55 \%$ AASHTO M 43 sizes No. 57 , No. 67 , No. 357 , or No. 467 coarse aggregate. Unless stated otherwise on the plans, Class E concrete shall achieve a field compressive strength of 2500 psi within 12 hours. Laboratory trial mix for Class E concrete must produce an average 28-day flexural strength of at least 650 psi . Approved fly ash may be substituted for portland cement up to a maximum of $30 \%$ Class F by weight.

Class H concrete is used for bare concrete bridge decks that will not receive a waterproofing membrane. Additional requirements for Class H concrete are: An approved water-reducing admixture shall be incorporated in the mix. Class H concrete shall contain a minimum of $55 \%$ AASHTO M 43 size No. 67 coarse aggregate. Class H concrete shall contain cementitious materials in the following ranges: 450 to 500 pounds per cubic yard Type II portland cement, 90 to 125 pounds per cubic yard fly ash, and 20 to 30 pounds per cubic yard silica fume. The total content of Type II portland cement, fly ash, and silica fume shall be 580 to 640 pounds per cubic yard. Laboratory trial mix for Class H concrete must not exceed permeability of 2000 coulombs at 56 days (ASTM C 1202). Laboratory trial mix for Class H concrete must not exhibit a crack at or before 14 days in the cracking tendency test (AASHTO PP 34).

Class HT concrete is used as the top layer for bare concrete bridge decks that will not receive a waterproofing membrane. Additional requirements for Class HT concrete are: An approved water-reducing admixture shall be incorporated in the mix. Class HT concrete shall contain a minimum of $50 \%$ AASHTO M 43 size No. 7 or No. 8 coarse aggregate. Class HT concrete shall contain cementitious materials in the following ranges: 450 to 500 pounds per cubic yard Type II portland cement, 90 to 125 pounds per cubic yard fly ash, and 20 to 30 pounds per cubic yard silica fume. The total content of Type II portland cement, fly ash, and silica fume shall be 580 to 640 pounds per cubic yard. Laboratory trial mix for Class HT concrete must not exceed permeability of 2000 coulombs at 56 days (ASTM C 1202). Laboratory trial mix for Class HT concrete must not exhibit a crack at or before 14 days in the cracking tendency test (AASHTO PP 34).

Class $\mathbf{P}$ concrete is used in pavements. Additional requirements for Class P concrete are: Class P concrete shall contain a minimum of $55 \%$ AASHTO M 43 size No. 357 or No. 467 coarse aggregate. If all transverse joints are doweled, then Class $P$ concrete shall contain a minimum of $55 \%$ AASHTO M 43 sizes No. 57, No. 67, No. 357, or No. 467 coarse aggregate. Laboratory trial mix for Class $P$ concrete must produce an average 28 -day flexural strength of at least 650 psi. Class $P$ concrete shall contain 70 to $80 \%$ portland cement and 20 to $30 \%$ Class F fly ash in the total weight of cement plus fly ash. Unless acceptance is based on flexural strength, the total weight of cement plus Class F fly ash shall not be less than 660 pounds per cubic yard. If acceptance is based on flexural strength, the total weight of cement plus Class F fly ash shall not be less than 520 pounds per cubic yard.

Class $\mathbf{S 3 5}$ concrete is a dense high-strength structural concrete. Additional requirements for Class S35 concrete are: An approved water-reducing admixture shall be incorporated in the mix. Class S 35 concrete shall be made with $19 \mathrm{~mm}(3 / 4 \mathrm{inch})$ nominal-sized coarse aggregate, that is, $100 \%$ passing the 25.0 mm ( 1 in .) sieve and 90 to $100 \%$ passing the $19 \mathrm{~mm}(3 / 4 \mathrm{in}$.) sieve. When placed in a bridge deck, Class S 35 concrete shall contain a minimum of $55 \%$ AASHTO M 43 size No. 67 coarse aggregate. Approved fly ash may be substituted for portland cement up to a maximum of $20 \%$ Class C or $30 \%$ Class F by weight.

Class $\mathbf{S 4 0}$ concrete is a dense high-strength structural concrete. Additional requirements for Class S 40 concrete are: An approved water-reducing admixture shall be incorporated in the mix. Class S 40 concrete shall be made with $19 \mathrm{~mm}(3 / 4 \mathrm{in}$.) nominal-sized coarse aggregate. When placed in a bridge deck, Class S 40 concrete shall contain a minimum of $55 \%$ AASHTO M 43 size No. 67 coarse aggregate. Approved fly ash may be substituted for portland cement up to a maximum of $20 \%$ Class C or $30 \%$ Class F by weight.

Class S50 concrete is a dense high-strength structural concrete. Additional requirements for Class S50 concrete are: An approved water-reducing admixture shall be incorporated in the mix. Class S 50 concrete shall be made with $19 \mathrm{~mm}(3 / 4 \mathrm{in}$.) nominal-sized coarse aggregate. When placed in a bridge deck, Class S 50 concrete shall contain a minimum of $55 \%$ AASHTO M 43 size No. 67 coarse aggregate. Approved fly ash may be substituted for portland cement up to a maximum of $20 \%$ Class C or $30 \%$ Class F by weight. Laboratory trial mix for Class S50 concrete must not exhibit a crack at or before 14 days in the cracking tendency test (AASHTO PP 34).

### 5.6.0 Lightweight Concrete Mix Design-Calculations Utilizing Perlite

## Perlite product guide

## Perlite Concrete

## Lightweight/Insulating/Fireproof

This product guide contains various mix designs for lightweight concrete, utilizing perlite as the primary aggregate. The basic mix designs presented may be used as stated or as a starting point for your own custom mixes.

Perlite lightweight concrete is used in many different applications, including lightweight tile mortar, garden sculpture, decorative brick, gas-fireplace logs and floor fills.

Some Perlite Concrete Applications
Chimney Lining Statuary
Floor Systems Tank Bases
Fuel Tanks Tank Insulation
Pool Base Tile Mortars
Sound/Firewalls Underground Pipe
Perlite concrete, while not usually suited for structural or load bearing uses, offers many advantages beyond its lightweight. Perlite concrete provides sound deadening properties and is thermal insulating as well, depending on mix design. Generally speaking, the lighter the weight, the greater the insulative properties.

| Mix Designs |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Cement <br> (sack) | Perlite <br> (ft ${ }^{3}$ ) | Expanded <br> Shale | Washed <br> Concrete <br> Sand $\left(\mathrm{ft}^{3}\right)$ | Water (Gal) | Admix <br> (FI. Oz.) | Dry Density | Wet Density | Compressive <br> Strength <br> (lb/in ${ }^{2}$ ) | Thermal Conductivity ("k") | Yield <br> (Cu. Ft.) |
| 1 | 8 | - | - | 16 | $28^{\text {A }}$ | 22 | 37 | 90-125 | 0.54 | 8 |
| 1 | 6 | - | - | 13 | $24^{\text {A }}$ | 27 | 42 | 125-200 | 0.64 | 6 |
| 1 | 5 | - | - | 11 | $20^{\text {A }}$ | 30 | 46 | 230-300 | 0.71 | 5 |
| 1 | 4 | - | - | 10 | $16^{\text {A }}$ | 36 | 50 | 350-500 | 0.83 | 4 |
| 1 | 3 | $2^{1}$ | - | 9 | $1^{\text {A }} \& 3^{\text {B }}$ | 54 | 72 | 1400-1700 | n/a | 3.8 |
| 1 | 3 | $2^{2}$ | - | 10 | $2^{B} \& 3^{\text {B }}$ | 62 | 78 | 2000-2100 | n/a | 3.5 |
| 1 | 3 | $2^{2}$ | - | 10 | $3^{\text {B }}$ | 65 | 90 | 2500-2800 | n/a | 3.2 |
| 1 | 3 | - | - | 7.5 | 7 | 45 | 58 | 800-1100 | n/a | 3 |
| 1 | 1.6 | - | 2.5 | 9.2 | $3^{\text {A }}$ | 82 | 98 | 1100-1300 | n/a | 5.1 |
| 1 | 2 | - | - | 5.5 | $3^{\text {A }}$ | 60 | 74 | 1600-1900 | n/a | 2 |
| 1 | 1.1 | - | 2.1 | 7.8 | $3^{\text {A }}$ | 88 | 105 | 2300-2500 | n/a | 3.5 |

## EXPANDED SHALE <br> ${ }^{1} 5 / 16^{\prime \prime}$ <br> ${ }^{2} 1 / 2^{\prime \prime}$

ADMIXTURE
$\overline{\text { A Air Entrainment }}$
${ }^{\text {B }}$ Pozzolith 300-N

## Mix Instructions

Proper mixing will assure the maximum yield and uniformity. Low shear, low RPM mixers (similar to plaster mixers) are recommended for best results.

1. Add all materials except perlite to mixer; then mix until this slurry is fairly uniform. Two minutes will usually suffice.
2. Add all perlite; then mix again only long enough for a uniform mix, probably another 2 to 3 minutes. Excess water and undermixing may reduce yield and workability. Overmixing may degrade the perlite and increase concrete density, reducing yield. Optimum mixing cycle can usually be determined with one or two trial batches.

Source: Perlite Institute, Harrisburg, PA

## General Considerations

- Addition of limited mason sand to a perlite/cement mix increases the compressive strength (to a point) and also the weight by approximately 100 lbs per cubic foot of sand.
- Addition of expanded shale also increases the compressive strength (to a point) and weight, but at about one-third the weight of sand, at higher cost.
- Addition of fibers increases the tensile and flexural strength of perlite concrete, thereby reducing shrink cracking.
- Addition of air-entraining agents reduces the weight and compressive strength of the mix and improves freeze-thaw performance.
- A range of aggregate size is desirable for increasing compressive strength. Superplasticizers and water reducers can also be used to increase strength.
- For detailed product finishes, finer aggregate particles can be used.

| Typical Mix Data for Perlite Sand Concrete |  |  |
| :--- | :--- | :--- |
| Cement (sacks) | 1 | 1 |
| Perlite $\left(\mathrm{ft}^{3}\right)$ | 3 | 2.4 |
| Sand ( $\mathrm{ft}^{3}$ ) | 2 | 1.5 |
| AEA (oz) |  |  |
| Water (gal/sack) | 8.2 | 8.1 |
| Cement (factor/yd-100\% yield) | 5.87 | 7.44 |
| Density (Wet) | 83 | 84 |
| Density (Dry) | 69 | 74 |
| Compressive Strength (28-day, lb/in ${ }^{2}$ ) | 1000 | 1200 |

*Air Entraining Agent as recommended by perlite manufacturer.

## AMERICAN SOCIETY FOR TESTING AND MATERIALS

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Grading Requirements for Lightweight Aggregates for Insulating Concrete

| Size | Weight \% Passing Sieves |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Designation |  |  |  |  |  |  |  |  |  |
|  | $\begin{aligned} & 19.0-\mathrm{mm} \\ & (3 / 4-\mathrm{in} .) \end{aligned}$ | $\begin{aligned} & 12.5-\mathrm{mm} \\ & (1 / 2-\mathrm{in} .) \end{aligned}$ | $9.5-\mathrm{mm}$ <br> (3/8-in.) | $\begin{aligned} & 4.75-\mathrm{mm} \\ & \text { (No. 4) } \end{aligned}$ | $\begin{aligned} & 2.36-\mathrm{mm} \\ & \text { (No. 8) } \end{aligned}$ | $\begin{aligned} & 1.18-\mathrm{mm} \\ & \text { (No. 16) } \end{aligned}$ | $\begin{aligned} & 600-\mu \mathrm{m} \\ & \text { (No. 30) } \end{aligned}$ | $\begin{aligned} & 300-\mu \mathrm{m} \\ & \text { (No. 50) } \end{aligned}$ | $\begin{aligned} & 150-\mu \mathrm{m} \\ & \text { (No. } \\ & 100 \text { ) } \end{aligned}$ |
| Group 1 |  |  |  |  |  |  |  |  |  |
| Perlite | '" | '" | '" | 100 | 85 to 100 | 45 to 85 | 20 to 60 | 5 to 25 | 0 to 10 |

### 5.7.0 Set-Retarding Admixtures Delay Hydration of Cement

## Set-Retarding

Retarding admixtures (retarders) are known to delay hydration of cement without affecting the long-term mechanical properties. They are used in concrete to offset the effect of high temperatures, which decrease setting times, or to avoid complications when unavoidable delays between mixing and placing occur. Use of set retarders in concrete pavement construction (1) enables farther hauling, thus eliminating the cost of relocating central mixing plants; (2) allows more time for texturing or plastic grooving of concrete pavements; (3) allows more time for hand finishing around the headers at the start and end of the production day; and (4) helps eliminate cold joints in two-course paving and in the event of equipment breakdown. Retarders can also be used to resist cracking due to form deflection that can occur when horizontal slabs are placed in sections (Mindess and Young 1981). Because of these advantages, set retarders are considered the second most commonly used admixtures in the highway industry, especially in the construction of bridge decks (U.S. Dept. Trans. 1990).

Composition and Mechanism of Retardation. Many water reducers have a retarding tendency. Therefore, some of the ingredients in water reducers, such as lignosulfate acids and hydroxycarboxylic acids, are also a basis for set-retarding admixtures. Other important materials used in producing set retarders are sugars and their derivatives.

Mechanisms of set retardation were studied by many researchers. Several theories have been offered to explain this mechanism. The role of retarding admixtures can be explained in a simple way: the admixtures form a film around the cement compounds (e.g., by absorption), thereby preventing or slowing the reaction with water. The thickness of this film will dictate how much the rate of hydration is retarded. After a while, this film breaks down, and normal hydration proceeds. However, in some cases when the dosage of admixtures exceeds a certain critical point, hydration of cement compounds will never proceed beyond a certain stage, and the cement paste will never set. Thus, it is important to avoid overdosing a concrete with a retarding admixture.

Other factors influencing the degree of retardation include the w/c, cement content, C3A and alkali contents in cement, the type and dosage of the admixture, and the stage at which the retarder is added to the mix. The effectiveness of retarder is increased if its addition to the fresh concrete is delayed for a few minutes.

Effect on Concrete Properties and Application. In addition to their role in controlling setting time, retarders-like any other admixtures-influence the properties of fresh and hardened concrete. Air entrainment of concrete is affected and fewer air-entraining agents need to be used because some retarders entrain air (see water reducers). Slump loss might increase even when abnormal setting behavior does not occur.

Because of retarding action, the 1 -day strength of the concrete is reduced. However, ultimate strength is reported to be improved by using set-controlling admixtures. Rates of drying shrinkage and creep could increase by using retarders, but the ultimate values cannot increase.

One of the most important applications of retarding admixtures is hot-weather concreting, when delays between mixing and placing operations may result in early stiffening. Another important application is in prestressed concrete, where retarders prevent the concrete that is in contact with the strand from setting before vibrating operations are completed. Set retarders also allow use of high-temperature curing in prestressed concrete production without affecting the ultimate strength of the concrete.

## Recommendations

1. Verification tests should be performed on liquid admixtures to confirm that the material is the same as that which was approved. The identifying tests include chloride and solids content, ph, and infrared spectrometry.
2. Water reducers and retarders may be used in bridge deck concrete to extend the time of set. This is especially important when the length of placement may result in flexural cracks created by dead load deflections during placement.
3. Increased attention needs to be placed on curing and protection due to the potential for shrinkage cracks and bleeding when water reducers are used.

### 5.8.0 Calculations for Mixing Small Batches of Concrete

## Mixing Small Batches of Concrete on Site

Ready-mix concrete requires coarse aggregate (stone), fine aggregate (sand), cement, and water. These components can be measured by weight or by volume; the following charts provide the proportions.

To mix one cubic yard of concrete by volume

| Maximum Size Aggregate | Cement | Sand | Stone | Water |
| :---: | :---: | :---: | :---: | :---: |
| $3 / 8^{\prime \prime}(9.52 \mathrm{~mm})$ | 1 | $2^{1 / 2}$ | $11 / 2$ | 1/2 |
| $1 / 2^{\prime \prime}(12.6 \mathrm{~mm})$ | 1 | $2^{1 / 2}$ | 2 | 1/2 |
| $3^{3 / 1}{ }^{\prime \prime}(19.05 \mathrm{~mm})$ | 1 | $2^{1 / 2}$ | $21 / 2$ | 1/2 |
| $1^{\prime \prime}(25.39 \mathrm{~mm})$ | 1 | $2^{1 / 2}$ | $21 / 4$ | 1/2 |
| $11 / 2^{\prime \prime}(37.99 \mathrm{~mm})$ | 1 | $21 / 2$ | 3 | 1/2 |

To mix one cubic yard of concrete by weight

| Maximum size Aggregate | Cement | Sand | Stone | Water |
| :--- | :--- | :--- | :--- | :--- |
| $3 / 8^{\prime \prime}(9.52 \mathrm{~mm})$ | 29 lbs | 59 lbs | 46 lbs | 11 lbs |
|  | $(13.15 \mathrm{~kg})$ | $(26.76 \mathrm{~kg})$ | $(20.87 \mathrm{~kg})$ | $4.99 \mathrm{~kg})$ |
| $1 / 2^{\prime \prime}(12.6 \mathrm{~mm})$ | 27 lbs | 53 lbs | 55 lbs | 11 lbs |
|  | $(12.25 \mathrm{~kg})$ | $(24.04 \mathrm{~kg})$ | $(24.95 \mathrm{~kg})$ | $(4.99 \mathrm{~kg})$ |
| $3 / 4^{\prime \prime}(19.05 \mathrm{~mm})$ | 25 lbs | 47 lbs | 65 lbs | 10 lbs |
|  | $(11.34 \mathrm{~kg})$ | $(21.32 \mathrm{~kg})$ | $(29.66 \mathrm{~kg})$ | $4.54 \mathrm{~kg})$ |
| $\mathbf{1}^{\prime \prime}(25.39 \mathrm{~mm})$ | 24 lbs | 45 lbs | 70 lbs | 10 lbs |
|  | $(10.89 \mathrm{~kg})$ | $(20.41 \mathrm{~kg})$ | $(31.756 \mathrm{~kg})$ | $(4.54 \mathrm{~kg})$ |
| $1 \mathbf{1}^{\prime \prime \prime}$ | 23 lbs | 43 lbs | 75 lbs | 9 lbs |
|  | $(10.43 \mathrm{~kg})$ | $(19.50 \mathrm{~kg})$ | $(34.02 \mathrm{~kg})$ | $(4.08 \mathrm{~kg})$. |

## Other Simple Calculations for Small Batches

Cement-one bucket
Sand-two and one-quarter buckets
Stone-one and one-half buckets
Water-one-half bucket
Equivalents: One cubic foot of water is equal to 7.48 gallons
One gallon of water weighs 8.33 pounds
One gallon equals 3.787 liters
One liter equals 0.2642 gallons
One liter equals 1.0567 quarts

# 5.9.0 In-Depth Concrete Inspection Guide as Published by U.S. DOT—FHWA 

..................... . Division Office*
1999 Inspection Guide (Metric Version)
Inspection-In-Depth: Major Structures-Concrete
Plant and Placement
PROJECT DATA

Project Number:
County:
Inspection Made By:
In Company With:
Date of Inspection:
Percent Complete:
Percent Time:

## Scope of inspection:

The overall purpose of this inspection is to evaluate project enforcement of established concrete inspection procedures as it relates to Structural Concrete. The items of interest are concrete plant operations, form work, reinforcing steel, and concrete placement. As part of the inspection review, a determination should be conducted on the existing procedures and/or specifications as to whether they are practical and easily understood and implemented by construction personnel and the contractor and whether field personnel have sufficient training, ability, and interest to thoroughly understand and enforce existing procedures and specifications. The Area Engineer is provided the flexibility of using the guideline in its entirety or portions depending on job conditions and time limitations. This guide may be supplemented as deemed necessary by the Area Engineer for items distinct to the individual project. It is suggested that prior to the inspection, a review of all applicable provisions be made in order that a broad knowledge of the provisions can be achieved and utilized during the inspection.

## REFERENCES

- Standard Specifications and Supplemental Specifications,
- Special Provisions,
- Construction Directives,

[^9]
## - Construction Manual

I. Concrete Plant Operation

1. a. Are mix designs available for each class of concrete?
b. Are they approved?
c. Dates approved?
2. a. Is the concrete plant approval available?
b. Is the certification still valid?
c. Expiration date?
d. Is the plant inspection checklist used by $\qquad$
e. Plant Type? (Central Mix, Transit Mix, Automatic, Manual, Manufacturer)
f. Manufactures Rating?
g. Has the mixer been checked for condition and wear (Section 601.5.3)?
3. a. Have the scales been inspected and sealed?
b. Expiration date?
c. Are the Ten - 20 kg weights available?
d. Have the scales been zero balanced and sensitivity checks been conducted?
4. a. Is the Quality Control Plan available?
b. Is it approved?
c. Date approved?
d. Approved by whom?
5. a. Are the aggregates properly stored?
b. Are intermixing, segregation, and/or contamination problems present?
c. Where are the aggregate samples being taken (Section 601.5.2.7)?
d. Are moisture tests being performed?
e. Are the tests being documented?
6. a. What is the water source used?
b. Is the source approved?
c. Comment on the water quality. Is it clean and free of oil, salt, acid, alkali, sugar, vegetation, etc.?
d. Is the water added to the mix adjusted for moisture in aggregate?
e. Is water added at the project site, or at the plant?
f. Comment on the accuracy of the water measuring equipment (within $1 \%$ )?
g. Is additional water being added at the project site?
h. What is allowable Water/Cement (W/C) ratio from mix design?
i. What is the W/C ratio that is being used (Section 601.7)?
7. a. Is the cement bin and weight hopper properly sealed?
b. Is the cement bin clean and dry?
c. Is the fly ash in a separate bin?
d. Is a Material Certification for the fly ash available?
e. Are adequate records being kept of cement being delivered to the batch plant to properly track quantity received vs. quantity used for test coverage?
8. a. Note the Field Laboratory location.
b. Is the lab properly equipped?
c. Is the lab in a reasonable proximity of plant?
9. a. Are laboratory tests documented and available for water, cement, air entraining agent, retarder, curing compound, aggregate gradations, etc.?
b. Where are the control charts being maintained? Are they complete and up to date?

Are there any undesirable trends noted?
Comments:
10. a. Is the Plant Inspector's Diary up to date?
b. What entries are being made in the Plant Diary? (Materials received, tests, checks, etc.)
c. Does the State or Contractor's Quality Control Personnel maintain a Plant Diary?
d. Comment on the documentation of instructions to the contractor:
11. a. Comment on the knowledge of plant personnel:
b. Comment on Personnel Certification:
12. General/Additional Comments on the Concrete Plant Operation:
II. Concrete Placement:

1. Form Work (Section 601.8)
a. Type of form materials used?
b. Is the mortar tight, and clean?
c. What form treatment is being utilized (Section 601.8.5)?
d. Are "Telltales," used for settlement monitoring, in-place?
e. Is the form work adequately supported and true to line and grade?
f. Has the overhang support been sufficiently checked (SD-7)?
g. Is form removal (Section 601.8.7) based on early cylinder breaks or specification guidance?
h. Slip forming (Section 601.8.8)?
i. Any field welding noted?
2. Reinforcing Steel
a. Are rebars being properly stored at the job site?
b. Are the rebars clean?
c. In the case of epoxy coated rebars, is the coating sound?
d. Note bar sizes, spacing, clearances and general layout of steel mat.

| Rebar Size | As Built |
| :--- | :--- |
| Bar Spacing |  |
| Clearances |  |

Are they in accordance with the plans?
e. Is the reinforcing steel adequately supported?
f. Are bar splices in compliance with the 30 bar diameters minimum overlap?
g. Comment on welding (only if shown on the plans)?
h. Field bending of reinforcing bars noted (Section 602.5)?

## 3. Concrete Operation

a. Form work and reinforcing steel checked and approved to concrete placement?
b. Was a preplacement conference held?

Note items discussed (type of equipment, pour sequence, schedule, etc.)
c. Was a dry run prior to concrete placement conducted?
d. What is the minimum required placement rate for the operation observed?
$\qquad$ cubic meters/hr
e. Note the time placement commenced and was completed.
f. What placement rate was actually achieved?
$\qquad$ cubic meters/hr
g. How was the concrete delivered (Chute, bucket, trough)?
h. Was proper technique executed?
i. Was concrete dropped through the air less than 1.5 meters?
j. Is concrete rehandling being avoided as much as possible?
k. Is concrete placed against previously placed concrete at all times?
l. Are concrete trucks clean of build-up and agitator blades not worn?
$\mathbf{m}$. Note the condition of the contractor's equipment.
n. Were vibrators sufficient in quantity to do the work that was needed?
o. Were vibrators being used for a sufficient duration but not to the point of segregation?
p. Were vibrators being used to move concrete?
q. Note the type of placement/finishing machine used?
r. Was screed support adequate to maintain line and grade?
s. Was water applied to the surface to aid finishing?
t. Was a straight edge being used?
u. Was a rolling straight edge used?

Any surface deficiencies found? Corrective action taken?
v. Were texture grooves 3 mm to 5 mm deep? (Section 601.11.4)
w. Comment on curing (Section 601.12) as to type, adequacy, timeliness, cold weather conditions, and maintenance of curing?
x. Note general weather conditions during concrete placement operation.
y. Comment on the establishment of adverse weather condition plans (Section 601.9) and their applicability.
4. Field Testing
a. Observe State or Contractor's personnel conducting sample and field testing procedures. Comment on the following:

- sample source
- performance of tests in accordance with accepted test procedures
- equipment adequacy
b. Any material rejected?
c. Record a sampling of test results for specification compliance:

| $\begin{aligned} & \text { Air } \\ & \text { (\%) } \end{aligned}$ | Slump (mm) | Temperature (Celsius) |  | Depth Check |  | In Compliance (Yes/No) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Air | Mix | Full Depth of Steel | $\begin{aligned} & \text { To Top } \\ & \text { Mat } \end{aligned}$ |  |

5. General/additional comments on concrete placement:

## III. Project Records

1. Review inspectors daily reports (IDRs), supervisor's daily reports, HL-440s, and progress estimates. Comment on the documentation as to quantities used, work performed, test results recorded, problems encountered, and proper cross referencing.
2. a. How does the project monitor acceptance sampling and testing, independent assurance sampling and testing, concrete plant operations, manufactures certification of appropriate items (ex. curing compound)?
b. Are these materials records found to be orderly?
c. Review a sample of completed concrete cylinder strength test results:

| Class of <br> Concrete <br> $(\mathrm{A}, \mathrm{B}, \mathrm{K}, \mathrm{ETC})$, | Strength Tests <br> (MPa) | Minimum Specification <br> Requirements (MPa) | Meet Specs. <br> (Yes/No) |
| :--- | :--- | :--- | :--- |
|  |  |  |  |

d. Any problems noted?
3. Is the contractor meeting the requirements of the Quality Control Plan?
IV. Closeout Conference:

1. Discuss all findings and come to an agreement on corrective actions when required.
2. Any recommendations from the review or from the project personnel?

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Word files can be viewed with the Word Viewer 2003
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### 5.10.0 Calculate the Size and Weight of Concrete Reinforcing Bars

Concrete reinforcing bars are produced in straight lengths and in coils. The bars are referred to as "deformed' because they are made with ridges that allow them to "grip" the concrete into which they are embedded. Rebars are designated by size (diameter) in both U.S. and metric, and they are also designated by grade or minimum yield strength as set forth by ASTM standards.

Three ASTM grades are the most commonly manufactured

| Inch-pound grade | Metric Grade | Yield in Pounds per <br> Square Inch | Yield in Metric <br> (Megapascals) |
| :--- | :--- | :--- | :---: |
| Grade 40 | Grade 280 | 40,000 | 280 |
| Grade 60 | Grade 420 | 60,000 | 420 |
| Grade A706 | Grade 520 | 75,000 | 520 |

Coiled Rebar is produced in ASTM Grades 60 and A706 in the following diameters and coil size:

| Size-in Diameter—Inches | Size—in Diameter-Metric | Weight per Coil |
| :--- | :---: | :---: |
| $\# 3$ | 10 | 4500 lbs |
| $\# 4$ | 13 | 4500 lbs |
| $\# 5$ | 16 | 3800 lbs |
| $\# 6$ | 19 | 3800 lbs |

### 5.10.1 Reinforcing Bar Designations-Size and Diameter-U.S. and Metric

| Bar Size.U.S. | Bar Size-Metric | Diameter-inches | Diameter Metric | Weight/Ft. |
| :--- | :---: | :---: | :---: | :---: |
| $\# 3$ | $\# 10$ | 0.375 | 9.52 mm | 0.376 |
| $\# 4$ | $\# 13$ | 0.500 | 12.7 mm | 0.668 |
| $\# 5$ | $\# 16$ | 0.620 | 15.8 mm | 1.043 |
| $\# 6$ | $\# 19$ | 0.750 | 19.05 mm | 1.502 |
| $\# 7$ | $\# 22$ | 0.875 | 22.23 mm | 2.044 |
| $\# 8$ | $\# 25$ | 1.000 | 25.4 mm | 2.670 |
| $\# 9$ | $\# 29$ | 1.128 | 28.65 mm | 3.400 |
| $\# 10$ | $\# 32$ | 1.270 | 32.25 mm | 4.303 |
| $\# 11$ | $\# 36$ | 1.410 | 35.81 mm | 5.313 |
| $\# 14$ | $\# 43$ | 1.693 | 73.0 mm | 7.650 |
| $\# 18$ | $\# 57$ | 2.257 | 57.33 mm | 13.60 |
| $\# 2^{*}$ | $\# 6$ | 0.250 | 6.35 mm | 0.167 |

*Number 2 bars are often referred to as "temperature bars" and are rarely used as concrete reinforcement.

### 5.10.2 Welded Wire Mesh Designations-U.S. and Metric

## Welded Wire Mesh

Here are the metric equivalents of some common mesh sizes:
When you see a mesh size, what do the numbers mean?
$\mathbf{6} \times \mathbf{6} \mathbf{W 1 . 4} / 1.4$ is a designated mesh size. The $\mathbf{6} \times \mathbf{6}$ is the horizontal and vertical spacing of the strands in inches. This is the size of the squares of space bounded by the wire strands in the mesh (the equivalent in metric is in millimeters, and so $\mathbf{6} \times \mathbf{6}$ inches is $\mathbf{1 5 2} \times \mathbf{1 5 2} \mathrm{mm}$ ). The $\mathbf{1 . 4} / \mathbf{1 . 4}$ is the " $\mathbf{W}$ " number. This is the wire size (longitudinal/transverse) in cross-sectional area measured in $1 / 100$ 's of a square inch ( 1.4 hundredths of a square inch). (The equivalent in metric is in square millimeters, and so 1.4 hundredths of a square inch is $9.1 \mathrm{~mm}^{2}$.)

In earlier times, the wire size was given in gauge rather than in cross-sectional area by hundredths of a square inch. What is now called $6 \times 6 \mathrm{~W} 1.4 / 1.4$ used to be called $6 \times 6 \mathrm{~W} 10 / 10$.
"MW" is for "Metric W number."

| Current Mesh Name <br> (wire size) | Former Mesh Name <br> (wire gauge) | Metric Name |
| :--- | :--- | :--- |
| $2 \times 2 \mathrm{~W} 4.0 / 4.0$ | $2 \times 2-4 / 4$ | $50 \times 50 \mathrm{MW} 25.8 / 25.8$ |
| $2 \times 2 \mathrm{~W} 2.9 / 2.9$ | $2 \times 2-6 / 6$ | $50 \times 50 \mathrm{MW} 18.7 / 18.7$ |
| $2 \times 2 \mathrm{~W} 2.1 / 2.1$ | $2 \times 2-8 / 8$ | $50 \times 50 \mathrm{MW} 13.3 / 13.3$ |
| $2 \times 2 \mathrm{~W} 1.4 / 1.4$ | $2 \times 2-10 / 10$ | $50 \times 50 \mathrm{MW} 9.1 / 9.1$ |
| $2 \times 2 \mathrm{~W} 0.9 / 0.9$ | $2 \times 2-12 / 12$ | $50 \times 50 \mathrm{MW} 5.6 / 5.6$ |
| $2 \times 2 \mathrm{~W} 0.5 / 0.5$ | $2 \times 2-14 / 14$ | $50 \times 50 \mathrm{MW} 3.2 / 3.2$ |
| $2 \times 2 \mathrm{~W} 0.3 / 0.3$ | $2 \times 2-16 / 16$ | $50 \times 50 \mathrm{MW} 2.0 / 2.0$ |
| $3 \times 3 \mathrm{~W} 2.1 / 2.1$ | $3 \times 3-8 / 8$ | $76 \times 76 \mathrm{MW} 13.3 / 13.3$ |
| $3 \times 3 \mathrm{~W} 1.4 / 1.4$ | $3 \times 3-10 / 10$ | $76 \times 76 \mathrm{MW} 9.1 / 9.1$ |
| $3 \times 3 \mathrm{~W} 0.9 / 0.9$ | $3 \times 3-12 / 12$ | $76 \times 76 \mathrm{MW} 5.6 / 5.6$ |


| Current Mesh Name <br> (wire size) | Former Mesh Name <br> (wire gauge) | Metric Name |
| :--- | :--- | :--- |
| $3 \times 3$ W0.5/0.5 | $3 \times 3-14 / 14$ | $76 \times 76$ MW3.2/3.2 |
| $4 \times 4$ W4.0/4.0 MW25.8/25.8 | $4 \times 4-4 / 4$ | $102 \times 102$ |
| $4 \times 4$ W2.9/2.9 MW18.7/18.7 | $4 \times 4-6 / 6$ | $102 \times 102$ |
| $4 \times 4$ W2.1/2.1 MW13.3/13.3 | $4 \times 4-8 / 8$ | $102 \times 102$ |
| $4 \times 4$ Wh.7/1.7 MW11.1/11.1 | $4 \times 4-9 / 9$ | $102 \times 102$ |
| $4 \times 4$ W1.4/1.4 MW9.1/9.1 | $4 \times 4-10 / 10$ | $102 \times 102$ |
| $4 \times 4$ W0.9/0.9 MW5.6/5.6 | $4 \times 4-12 / 12$ | $102 \times 102$ |
| $4 \times 4$ W0.7/0.7 MW4.2/4.2 | $102 \times 102$ |  |
| $4 \times 4$ W0.5/0.5 MW3.2/3.2 | $4 \times 4-14 / 14$ | $102 \times 102$ |
| $6 \times 6$ W7.4/7.4 MW47.6/47.6 | $6 \times 6-0 / 0$ | $152 \times 152$ |
| $6 \times 6$ W6.3/6.3 MW40.6/40.6 | $6 \times 6-1 / 1$ | $152 \times 152$ |
| $6 \times 6$ W5.4/5.4 MW34.9/34.9 | $6 \times 6-2 / 2$ | $152 \times 152$ |
| $6 \times 6$ W4.7/4.7 MW30.1/30.1 | $6 \times 6-3 / 3$ | $152 \times 152$ |
| $6 \times 6$ W4.0/4.0 MW25.8/25.8 | $6 \times 6-4 / 4$ | $152 \times 152$ |
| $6 \times 6$ W4.0/2.9 MW25.8/18.7 | $6 \times 6-4 / 6$ | $152 \times 152$ |
| $6 \times 6$ W3.4/3.4 MW21.7/21.7 | $6 \times 6-5 / 5$ | $152 \times 152$ |
| $6 \times 6$ WW2.9/2.9 MW18.7/18.7 | $6 \times 6-6 / 6$ | $152 \times 152$ |
| $6 \times 6$ W2.5/2.5 MW15.9/15.9 | $6 \times 6-7 / 7$ | $152 \times 152$ |
| $6 \times 6$ W2.1/2.1 MW13.3/13.3 | $6 \times 6-8 / 8$ | $152 \times 152$ |
| $6 \times 6$ W1.7/1.7 MW11.1/11.1 | $6 \times 6-9 / 9$ | $152 \times 152$ |
| $6 \times 6$ W1.4/1.4 MW9.1/9.1 | $6 \times 6-10 / 10$ | $152 \times 152$ |
| $12 \times 12$ W5.4/5.4 MW34.9/34.9 | $12 \times 12-2 / 2$ | $305 \times 305$ |
|  |  |  |

### 5.11.0 Embedded Anchor Bolt—Diameter, Length, Hook, and Thread Sizes—Plain Finish



## Anchor Bolts in Stock Plain Finish

## Specifications: Steel to Astm F1554 (A36, GR. 55 \& 105) (New Anchor Bolt Specification)

| DIA. (D) | Length $(\mathbf{A})$ | Hook $(\mathbf{B})$ | Thread (C) |
| :--- | :---: | :---: | :---: |
| $1 / 2^{\prime \prime}$ | $6^{\prime \prime}$ | $1-1 / 2^{\prime \prime}$ | $1-1 / 2^{\prime \prime}$ |
| $1 / 2^{\prime \prime}$ | $8^{\prime \prime}$ | $1-1 / 2^{\prime \prime}$ | $1-1 / 2^{\prime \prime}$ |


| DIA. (D) | Length $(\mathbf{A})$ | Hook (B) | Thread (C) |
| :--- | :---: | :---: | :---: |
| $1 / 2^{\prime \prime}$ | $10^{\prime \prime}$ | $1-1 / 2^{\prime \prime}$ | $1-1 / 2^{\prime \prime}$ |
| $1 / 2^{\prime \prime}$ | $12^{\prime \prime}$ | $1-1 / 2^{\prime \prime}$ | $1-1 / 2^{\prime \prime}$ |
| $1 / 2^{\prime \prime}$ | $14^{\prime \prime}$ | $1-1 / 2^{\prime \prime}$ | $1-1 / 2^{\prime \prime}$ |
| $1 / 2^{\prime \prime}$ | $16^{\prime \prime}$ | $1-1 / 2^{\prime \prime}$ | $1-1 / 2^{\prime \prime}$ |
| $1 / 2^{\prime \prime}$ | $18^{\prime \prime}$ | $1-1 / 2^{\prime \prime}$ | $1-1 / 2^{\prime \prime}$ |
| $1 / 2^{\prime \prime}$ | $20^{\prime \prime}$ | $1-1 / 2^{\prime \prime}$ | $1-1 / 2^{\prime \prime}$ |


| DIA. (D) | Length $(\mathbf{A})$ | Hook (B) | Thread (C) |
| :--- | :---: | :---: | :---: |
| $5 / 8^{\prime \prime}$ | $8^{\prime \prime}$ | $3^{\prime \prime}$ | $3^{\prime \prime}$ |
| $5 / 8^{\prime \prime}$ | $10^{\prime \prime}$ | $3^{\prime \prime}$ | $3^{\prime \prime}$ |
| $5 / 8^{\prime \prime}$ | $12^{\prime \prime}$ | $3^{\prime \prime}$ | $3^{\prime \prime}$ |
| $5 / 8^{\prime \prime}$ | $14^{\prime \prime}$ | $3^{\prime \prime}$ | $3^{\prime \prime}$ |
| $5 / 8^{\prime \prime}$ | $15^{\prime \prime}$ | $3^{\prime \prime}$ | $3^{\prime \prime}$ |
| $5 / 8^{\prime \prime}$ | $16^{\prime \prime}$ | $3^{\prime \prime}$ | $3^{\prime \prime}$ |
| $5 / 8^{\prime \prime}$ | $18^{\prime \prime}$ | $3^{\prime \prime}$ | $3^{\prime \prime}$ |
| $5 / 8^{\prime \prime}$ | $20^{\prime \prime}$ | $3^{\prime \prime}$ | $3^{\prime \prime}$ |
| $5 / 8^{\prime \prime}$ | $21^{\prime \prime}$ | $3^{\prime \prime}$ | $3^{\prime \prime}$ |
| $5 / 8^{\prime \prime}$ | $24^{\prime \prime}$ | $3^{\prime \prime}$ | $3^{\prime \prime}$ |
| $5 / 8^{\prime \prime}$ | $30^{\prime \prime}$ | $3^{\prime \prime}$ | $3^{\prime \prime}$ |
|  |  |  |  |


| DIA. (D) | Length $(\mathbf{A})$ | Hook (B) | Thread (C) |
| :--- | :---: | :---: | :---: |
| $3 / 4^{\prime \prime}$ | $8^{\prime \prime}$ | $3^{\prime \prime}$ | $4^{\prime \prime}$ |
| $3 / 4^{\prime \prime}$ | $10^{\prime \prime}$ | $3^{\prime \prime}$ | $4^{\prime \prime}$ |
| $3 / 4^{\prime \prime}$ | $12^{\prime \prime}$ | $3^{\prime \prime}$ | $4^{\prime \prime}$ |
| $3 / 4^{\prime \prime}$ | $14^{\prime \prime}$ | $3^{\prime \prime}$ | $4^{\prime \prime}$ |
| $3 / 4^{\prime \prime}$ | $15^{\prime \prime}$ | $3^{\prime \prime}$ | $4^{\prime \prime}$ |
| $3 / 4^{\prime \prime}$ | $16^{\prime \prime}$ | $3^{\prime \prime}$ | $4^{\prime \prime}$ |
| $3 / 4^{\prime \prime}$ | $18^{\prime \prime}$ | $3^{\prime \prime}$ | $4^{\prime \prime}$ |
| $3 / 4^{\prime \prime}$ | $20^{\prime \prime}$ | $3^{\prime \prime}$ | $4^{\prime \prime}$ |
| $3 / 4^{\prime \prime}$ | $21^{\prime \prime}$ | $3^{\prime \prime}$ | $4^{\prime \prime}$ |
| $3 / 4^{\prime \prime}$ | $24^{\prime \prime}$ | $3^{\prime \prime}$ | $4^{\prime \prime}$ |
| $3 / 4^{\prime \prime}$ | $30^{\prime \prime}$ | $3^{\prime \prime}$ | $4^{\prime \prime}$ |

## More Anchor Bolts in Stock Plain Finish

Specifications: Steel to Astm F1554 (A36, GR. 55 \& 105)
(New Anchor Bolt Specification)

| DIA. (D) | Length $(\mathbf{A})$ | Hook $(\mathbf{B})$ | Thread $(\mathbf{C})$ |
| :--- | :---: | :---: | :---: |
| $7 / 8^{\prime \prime}$ | $12^{\prime \prime}$ | $4^{\prime \prime}$ | $4^{\prime \prime}$ |
| $7 / 8^{\prime \prime}$ | $14^{\prime \prime}$ | $4^{\prime \prime}$ | $4^{\prime \prime}$ |
| $7 / 8^{\prime \prime}$ | $16^{\prime \prime}$ | $4^{\prime \prime}$ | $4^{\prime \prime}$ |


| DIA. (D) | Length (A) | Hook (B) | Thread (C) |
| :--- | :---: | :---: | :---: |
| $7 / 8^{\prime \prime}$ | $18^{\prime \prime}$ | $4^{\prime \prime}$ | $4^{\prime \prime}$ |
| $7 / 8^{\prime \prime}$ | $20^{\prime \prime}$ | $4^{\prime \prime}$ | $4^{\prime \prime}$ |
| $7 / 8^{\prime \prime}$ | $22^{\prime \prime}$ | $4^{\prime \prime}$ | $4^{\prime \prime}$ |
| $7 / 8^{\prime \prime}$ | $24^{\prime \prime}$ | $4^{\prime \prime}$ | $4^{\prime \prime}$ |
| $7 / 8^{\prime \prime}$ | $30^{\prime \prime}$ | $4^{\prime \prime}$ | $4^{\prime \prime}$ |
|  |  |  |  |


| DIA. (D) | Length (A) | Hook (B) | Thread (C) |
| :---: | :---: | :---: | :---: |
| $1^{\prime \prime}$ | $12^{\prime \prime}$ | 4 " | 4 " |
| $1^{\prime \prime}$ | $14^{\prime \prime}$ | $4 \prime$ | 4 " |
| 1 " | $15^{\prime \prime}$ | 4 " | 4 " |
| $1^{\prime \prime}$ | $16^{\prime \prime}$ | $4 \prime$ | $4 \prime$ |
| $1^{\prime \prime}$ | $18^{\prime \prime}$ | 4 " | 4 " |
| $1^{\prime \prime}$ | $20^{\prime \prime}$ | 4 " | $4 \prime$ |
| $1^{\prime \prime}$ | $22^{\prime \prime}$ | 4 " | 4 " |
| $1^{\prime \prime}$ | $24^{\prime \prime}$ | 4 " | 4 " |
| 1 " | $30^{\prime \prime}$ | $4 \prime$ | $6{ }^{\prime \prime}$ |
| 1 " | $36^{\prime \prime}$ | $6^{\prime \prime}$ | $7{ }^{\prime \prime}$ |
|  |  |  |  |
| DIA. (D) | Length (A) | Hook (B) | Thread (C) |
| 1-1/4" | 24 " | $6^{\prime \prime}$ | $6{ }^{\prime \prime}$ |
| 1-1/4" | $30^{\prime \prime}$ | $6^{\prime \prime}$ | $6^{\prime \prime}$ |
| 1-1/4" | $36^{\prime \prime}$ | $6^{\prime \prime}$ | $6^{\prime \prime}$ |
| 1-1/4' | $44^{\prime \prime}$ | $6^{\prime \prime}$ | $8^{\prime \prime}$ |

Source: St. Louis Screw and Bolt

### 5.11.1 Embedded Anchor Bolts—Diameter, Length, Hook, and Thread Sizes—Galvanized

Anchor Bolts in Stock Hot Dip Galvanized Specifications: Steel to Astm F1554 (A36, GR. 55 \& 105) (New Anchor Bolt Specification)

| DIA. (D) | Length $(\mathbf{A})$ | Hook $(\mathbf{B})$ | Thread (C) |
| :--- | :---: | :---: | :---: |
| $1 / 2^{\prime \prime}$ | $6^{\prime \prime}$ | $1-1 / 2^{\prime \prime}$ | $1-1 / 2^{\prime \prime}$ |
| $1 / 2^{\prime \prime}$ | $8^{\prime \prime}$ | $1-1 / 2^{\prime \prime}$ | $1-1 / 2^{\prime \prime}$ |
| $1 / 2^{\prime \prime}$ | $10^{\prime \prime}$ | $1-1 / 2^{\prime \prime}$ | $1-1 / 2^{\prime \prime}$ |
| $1 / 2^{\prime \prime}$ | $12^{\prime \prime}$ | $1-1 / 2^{\prime \prime}$ | $1-1 / 2^{\prime \prime}$ |
| $1 / 2^{\prime \prime}$ | $14^{\prime \prime}$ | $1-1 / 2^{\prime \prime}$ | $1-1 / 2^{\prime \prime}$ |
| $1 / 2^{\prime \prime}$ | $16^{\prime \prime}$ | $1-1 / 2^{\prime \prime}$ | $1-1 / 2^{\prime \prime}$ |
| $1 / 2^{\prime \prime}$ | $18^{\prime \prime}$ | $1-1 / 2^{\prime \prime}$ | $1-1 / 2^{\prime \prime}$ |
| $1 / 2^{\prime \prime}$ | $20^{\prime \prime}$ | $1-1 / 2^{\prime \prime}$ | $1-1 / 2^{\prime \prime}$ |


| DIA. (D) | Length (A) | Hook (B) | Thread (C) |
| :--- | :---: | :---: | :---: |
| $5 / 8^{\prime \prime}$ | $8^{\prime \prime}$ | $3^{\prime \prime}$ | $3^{\prime \prime}$ |
| $5 / 8^{\prime \prime}$ | $10^{\prime \prime}$ | $3^{\prime \prime}$ | $3^{\prime \prime}$ |
| $5 / 8^{\prime \prime}$ | $12^{\prime \prime}$ | $3^{\prime \prime}$ | $3^{\prime \prime}$ |
| $5 / 8^{\prime \prime}$ | $14^{\prime \prime}$ | $3^{\prime \prime}$ | $3^{\prime \prime}$ |
| $5 / 8^{\prime \prime}$ | $15^{\prime \prime}$ | $3^{\prime \prime}$ | $3^{\prime \prime}$ |
| $5 / 8^{\prime \prime}$ | $16^{\prime \prime}$ | $3^{\prime \prime}$ | $3^{\prime \prime}$ |
| $5 / 8^{\prime \prime}$ | $18^{\prime \prime}$ | $3^{\prime \prime}$ | $3^{\prime \prime}$ |
| $5 / 8^{\prime \prime}$ | $20^{\prime \prime}$ | $3^{\prime \prime}$ | $3^{\prime \prime}$ |
| $5 / 8^{\prime \prime}$ | $21^{\prime \prime}$ | $3^{\prime \prime}$ | $3^{\prime \prime}$ |
| $5 / 8^{\prime \prime}$ | $24^{\prime \prime}$ | $3^{\prime \prime}$ | $3^{\prime \prime}$ |
| $5 / 8^{\prime \prime}$ | $30^{\prime \prime}$ | $3^{\prime \prime}$ | $3^{\prime \prime}$ |
|  |  |  |  |


| DIA. (D) | Length $(\mathbf{A})$ | Hook (B) | Thread (C) |
| :--- | :---: | :---: | :---: |
| $3 / 4^{\prime \prime}$ | $8^{\prime \prime}$ | $3^{\prime \prime}$ | $4^{\prime \prime}$ |
| $3 / 4^{\prime \prime}$ | $10^{\prime \prime}$ | $3^{\prime \prime}$ | $4^{\prime \prime}$ |
| $3 / 4^{\prime \prime}$ | $12^{\prime \prime}$ | $3^{\prime \prime}$ | $4^{\prime \prime}$ |
| $3 / 4^{\prime \prime}$ | $14^{\prime \prime}$ | $3^{\prime \prime}$ | $4^{\prime \prime}$ |
| $3 / 4^{\prime \prime}$ | $15^{\prime \prime}$ | $3^{\prime \prime}$ | $4^{\prime \prime}$ |
| $3 / 4^{\prime \prime}$ | $16^{\prime \prime}$ | $3^{\prime \prime}$ | $4^{\prime \prime}$ |
| $3 / 4^{\prime \prime}$ | $18^{\prime \prime}$ | $3^{\prime \prime}$ | $4^{\prime \prime}$ |
| $3 / 4^{\prime \prime}$ | $20^{\prime \prime}$ | $3^{\prime \prime}$ | $4^{\prime \prime}$ |
| $3 / 4^{\prime \prime}$ | $21^{\prime \prime}$ | $3^{\prime \prime}$ | $4^{\prime \prime}$ |
| $3 / 4^{\prime \prime}$ | $24^{\prime \prime}$ | $3^{\prime \prime}$ | $4^{\prime \prime}$ |
| $3 / 4^{\prime \prime}$ | $30^{\prime \prime}$ | $3^{\prime \prime}$ | $4^{\prime \prime}$ |

### 5.12.0 Brick Sizes—Nominal versus Actual Size

## Standard and modular brick dimensions

Face Brick

|  | Nominal Size | Actual Size | No. per sq ft* |
| :---: | :---: | :---: | :---: |
| Standard | $4^{\prime \prime} \times 22 / 3^{\prime \prime} \times 8^{\prime \prime}$ | $35 / 8^{\prime \prime} \times 21 / 4^{\prime \prime} \times 8^{\prime \prime}$ | 6.27 |
| Modular | $4^{\prime \prime} \times 22 / 3^{\prime \prime} \times 8^{\prime \prime}$ | $35 / 8^{\prime \prime} \times 21_{4}^{\prime \prime \prime} \times 75 / 8^{\prime \prime}$ | 6.86 |
| King | $33 / 8^{\prime \prime} \times 3^{\prime \prime} \times 10^{\prime \prime}$ | $3^{\prime \prime} \times 25 / 8^{\prime \prime} \times 95 / 8^{\prime \prime}$ | 4.80 |
| Queen | $23 / 4^{\prime \prime} \times 3^{\prime \prime} \times 10^{\prime \prime}$ | $31 / 8^{\prime \prime} \times 23 / 4^{\prime \prime} \times 95 / 8^{\prime \prime}$ | 4.61 |
| Engineer | $4^{\prime \prime} \times 31 / 5^{\prime \prime} \times 8^{\prime \prime}$ | $35 / 8^{\prime \prime} \times 213 / 16^{\prime \prime} \times 75 / 8^{\prime \prime}$ | 5.65 |
| Economy | $4^{\prime \prime} \times 4^{\prime \prime} \times 8^{\prime \prime}$ | $35 / 8^{\prime \prime} \times 35 / 8^{\prime \prime} \times 75 / 8^{\prime \prime}$ | 4.50 |
| Utility | $4^{\prime \prime} \times 4^{\prime \prime} \times 12^{\prime \prime}$ | $35 / 8^{\prime \prime} \times 35 / 8^{\prime \prime} \times 11^{1 / 2 \prime}{ }^{\prime \prime}$ | 3.03 |
| Jumbo | $4^{\prime \prime} \times 3^{\prime \prime} \times 8^{\prime \prime}$ | $35 / 8^{\prime \prime} \times 23 / 4^{\prime \prime} \times 8^{\prime \prime}$ | 5.50 |
| Norman | $4^{\prime \prime} \times 22 / 3^{\prime \prime} \times 12^{\prime \prime}$ | $35 / 8^{\prime \prime} \times 21 / 4^{\prime \prime} \times 115 / 8^{\prime \prime}$ | 4.57 |
| Norwegian | $31 / 2^{\prime \prime} \times 3^{\prime \prime} \times 12^{\prime \prime}$ | $31 / 2^{\prime \prime} \times 23 / 4^{\prime \prime} \times 115 / 8^{\prime \prime}$ | 3.84 |

[^10]There is no true standard of face brick sizes versus names. The same-name brick may vary in size among different manufacturers. It is best to specify face bricks by size first and then by name.

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### 5.12.1 Diagrams of Modular and Nonmodular Bricks

## Modular Brick Sizes (Nominal Dimensions)



FIG. 1

## Nonmodular Brick Sizes (Specified Dimensions)



FIG. 2

### 5.12.2 Brick Positions in a Wall

## Brick Positions



### 5.12.3 Calculate the Number of Bricks in a Wall

No. of Standard Bricks in One Sq Ft of Brick Wall

| Wall Thickness |  | No. of Bricks Thick | Thickness of Horizontal Mortar Joint |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 5/8 ${ }^{\prime \prime}$ | $3 / 4^{\prime \prime}$ |  | 1/8" | 1/4 ${ }^{\prime \prime}$ | $3 / 8^{\prime \prime}$ | 1/2" | 5/8' ${ }^{\prime \prime}$ | $3 / 4^{\prime \prime}$ |
| $4^{\prime \prime}$ | $41 / 2^{\prime \prime}$ | 1 | 7.33 | 7.00 | 6.67 | 6.33 | 6.08 | 5.80 |
| $8^{\prime \prime}$ | $9 \prime$ | 2 | 14.67 | 14.00 | 13.33 | 12.67 | 12.17 | 11.60 |
| $12^{\prime \prime}$ | $13^{\prime \prime}$ | 3 | 22.00 | 21.00 | 20.00 | 19.00 | 18.25 | 17.40 |
| $16^{\prime \prime}$ | $17^{\prime \prime}$ | 4 | 29.33 | 28.00 | 26.67 | 25.33 | 24.33 | 23.20 |
| $20^{\prime \prime}$ | $21^{\prime \prime}$ | 5 | 36.67 | 35.00 | 33.33 | 31.67 | 30.42 | 29.00 |
| $24^{\prime \prime}$ | $25^{\prime \prime}$ | 6 | 44.00 | 42.00 | 40.00 | 38.00 | 36.50 | 34.80 |

[^11]No. of square inches per standard brick with mortar joints
$8^{\prime \prime} \times 21 / 4^{\prime \prime}$ face with $14^{\prime \prime}$ vertical mortar joints
Thickness of horizontal mortar joints

| $1 / 8^{\prime \prime}$ | $1 / 4^{\prime \prime}$ | $3 / 8^{\prime \prime}$ | $1 / 2^{\prime \prime}$ | $5 / 8^{\prime \prime}$ | $3 / 4^{\prime \prime}$ | $7 / 8^{\prime \prime}$ | $1^{\prime \prime}$ |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| 19.30 | 20.625 | 21.656 | 22.69 | 23.72 | 24.75 | 25.78 | 26.81 |

### 5.12.3.1 Calculate the Number of Bricks for Your Project

1. Add up the total square footage of brick wall area.
(Multiply height $\times$ length of brick wall area.)
2. Add up the total square footage of masonry openings.
(Window and door areas not requiring brick.)
3. Subtract masonry opening square footage from brick wall area square footage.
4. Multiply this number of 7 if using modular size brick.
5. Multiply this number by 5.8 if using engineer size brick.

## Example:

1530 square feet of wall area. 154 square feet of windows and doors (Masonry Openings)
1530 minus 154 - 1376 square feet of brick wall
$1376 \times 7=9976$ modular size brick
$1376 \times 5.8-8,394$ engineer size brick

## How to Figure Pavers:

1. Add up the total square footage of the horizontal area.
2. Multiply this number by 5 .

## Example:

$10 \times 10$ area -100 square feet of paving area.
100 square feet $\times 5=500$ pavers needed for a 100 square foot area.

### 5.12.4 Percentages to Add for Various Bond Patterns

## Bonds

Face brick quantities

## Percentages to add to numbers calculated for running bond walls

English bond (with a full header course every other course) $50.000 \%$
English bond (with a full header course every 6th course) 16.667
English cross bond (with a full header course every other course) 50.000
English cross bond (with a full header course every 6th course) 16.667
Common bond* (with a full header course every 5th course) 20.000
Common bond (with a full header course every 6th course) 16.667
Common bond (with a full header course every 7th course) 14.333
Dutch bond (with a full header course every other course) 50.000

[^12]Dutch bond (with a full header course every 6th course) 16.667
Dutch cross bond (with a full header course every other course) 50.000
Dutch cross bond (with a full header course every 6th course) 16.667
Double header bond (two headers and a stretcher every 5th course) 10.000
Double header bond (two headers and a stretcher every 6th course) 8.333
Flemish bond (with a full header course every other course) 33.333
Flemish bond (with a full header course every 6th course) 5.600
Double Flemish bond (with a full header course every other course) $\quad 10.000$
Double Flemish bond (with a full header course every 3rd course) 6.667
3 stretcher Flemish bond (with a full header course every other course) 7.143
3 stretcher Flemish bond (with a full header course every 3rd course) 4.800
4 stretcher Flemish bond (with a full header course every other course) 5.600
4 stretcher Flemish bond (with a full header course every 3rd course) 3.704

### 5.12.5 Types of Brick—by Material

Adobe
Kiln burned (your common red brick)
Sand lime
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### 5.12.6 Chart Reflecting Nominal Size, Joint Thickness, Actual Size—Modular/Nonmodular Bricks

| MODULAR BRICK SIZES |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Unit Designation | Nominal Dimensions, in. |  |  | Joint Thickness ${ }^{1}$, in. | Specified Dimensions ${ }^{2}$, in. |  |  | Vertical Coursing |
|  | w | $h$ | I |  | w | $h$ | I |  |
| Modular | 4 | $22 / 3$ | 8 | 3/8 | 3 \% | $21 / 4$ | 75\% | $3 \mathrm{C}=8 \mathrm{in}$. |
|  |  |  |  | 1/2 | $31 / 2$ | 21/4 | $71 / 2$ |  |
| Engineer Modular | 4 | $31 / 5$ | 8 | $3 / 8$ | $35 / 8$ | $2^{3 / 4}$ | 75/8 | $5 C=16 \mathrm{in}$. |
|  |  |  |  | 1/2 | $31 / 2$ | $2^{13 / 18}$ | $71 / 2$ |  |
| Closure Modular | 4 | 4 | 8 | $3 / 8$ | 3 /8 | $35 / 8$ | 75/8 | $1 \mathrm{C}=4 \mathrm{in}$. |
|  |  |  |  | 1/2 | $31 / 2$ | $31 / 2$ | 71/2 |  |
| Roman | 4 | 2 | 12 | $3 / 8$ | $35 / 8$ | 15/8 | 115/8 | $2 \mathrm{C}=4 \mathrm{in}$. |
|  |  |  |  | 1/2 | $31 / 2$ | 11/2 | 111/2 |  |
| Norman | 4 | $2^{2 / 3}$ | 12 | 3/8 | $35 / 8$ | $21 / 4$ | 115/8 | $3 \mathrm{C}=8 \mathrm{in}$. |
|  |  |  |  | 1/2 | $31 / 2$ | $21 / 4$ | 111/2 |  |
| Engineer Norman | 4 | $31 / 5$ | 12 | 3/8 | $35 / 8$ | $2^{3 / 4}$ | 115/8 | $5 \mathrm{C}=16 \mathrm{in}$. |
|  |  |  |  | 1/2 | $31 / 2$ | 213/18 | 111/2 |  |
| Utility | 4 | 4 | 12 | 3/8 | $35 / 8$ | $35 / 8$ | 115/8 | $1 \mathrm{C}=4 \mathrm{in}$. |
|  |  |  |  | 1/2 | $31 / 2$ | $31 / 2$ | 111/2 |  |


| Unit Designation | Nominal Dimensions, in. |  |  | Joint Thickness, in. | Specified Dimensions, in. |  |  | Vertical Coursing |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | w | $h$ | I |  | w | $h$ | I |  |
| NONMODULAR BRICK SIZES |  |  |  |  |  |  |  |  |
| Standard |  |  |  | $3 / 8$ | 35/8 | $2^{1 / 4}$ | 8 | $3 \mathrm{C}=8 \mathrm{in}$. |
|  |  |  |  | 1/2 | $31 / 2$ | 21/4 | 8 |  |
| Engineer Standard |  |  |  | $3 / 8$ | $35 / 8$ | $2^{3 / 4}$ | 8 | $5 C=16 \mathrm{in}$. |
|  |  |  |  | 1/2 | $31 / 2$ | $2{ }^{13 / 18}$ | 8 |  |
| Closure Standard |  |  |  | $3 / 8$ | $35 / 8$ | 35/8 | 8 | $1 \mathrm{C}=4 \mathrm{in}$. |
|  |  |  |  | 1/2 | $31 / 2$ | $31 / 2$ | 8 |  |
| King |  |  |  | 3/8 | 3 | $2^{3 / 4}$ | 95/8 | $5 C=16 \mathrm{in}$. |
|  |  |  |  |  | $2^{3 / 4}$ | 25/8 | 95/8 |  |
| Queen |  |  |  | 3/8 | 3 | $2^{3 / 4}$ | 8 | $5 C=16 \mathrm{in}$. |
|  |  |  |  |  | $2^{3 / 4}$ | $2^{3 / 4}$ | 8 |  |
| ${ }^{1}$ Common joint sizes used with length and width dimensions. Joint thicknesses of bed joints vary based on vertical coursing and specified unt height. <br> ${ }^{2}$ Specified dimensions may vary within this range from manufacturer to manufacturer. |  |  |  |  |  |  |  |  |

## Brick dimensions

- Brick are identified by three dimensions: width, height, and length. Height and length are sometimes called face dimensions, for these are the dimensions showing when the brick is laid as a stretcher. The shaded areas indicate the surfaces of the brick that are exposed. Specifications and purchase orders should list brick dimensions in the standard order of width first, followed by height, then length.
- When specifying or designing with brick, it is important to understand the difference between nominal, specified, and actual dimensions. Nominal dimensions are most often used by the architect in modular construction. In modular construction, all dimensions of the brick and other building elements are multiples of a given module. Such dimensions are known as nominal dimensions. For brick masonry the nominal dimension is equal to the specified unit dimension plus the intended mortar joint thickness. The intended mortar joint thickness is the thickness required so that the unit plus joint thickness match the coursing module. In the inch-pound system of measurement, nominal brick dimensions are based on multiples (or fractions) of 4 in . In the SI (metric) system, nominal brick dimensions are based on multiples of 100 mm . For more information on modular construction see Technical Notes 10A Revised.
- As the name implies, the specified dimension is the anticipated manufactured dimension. It should be stated in project specifications and purchase orders. Specified dimensions are used by the structural engineer in the rational design of brick masonry.

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### 5.12.7 Chart Reflecting Nominal Size, Joint Thickness, Actual Size-Other Brick Sizes

| MODULAR BRICK SIZES |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Nominal Dimensions, in. |  |  | Joint Thickness ${ }^{1}$, in. | Specified Dimensions ${ }^{2}$, in. |  |  | Vertical Coursing |
| w | $h$ | I |  | w | $h$ | $I$ |  |
| 4 | 6 | 8 | $\begin{aligned} & 3 / 8 \\ & 1 / 2 \end{aligned}$ | $\begin{aligned} & 3^{5 / 8} \\ & 3^{1 / 2} \end{aligned}$ | $\begin{aligned} & 5^{5 / 8} \\ & 5^{1 / 2} \end{aligned}$ | $\begin{aligned} & 7^{5 / 8} \\ & 7^{1 / 2} \end{aligned}$ | $2 \mathrm{C}=12 \mathrm{in}$. |
| 4 | 8 | 8 | $\begin{aligned} & 3 / 8 \\ & 1 / 2 \end{aligned}$ | $\begin{aligned} & 35 / 8 \\ & 31 / 2 \end{aligned}$ | $\begin{aligned} & 75 / 8 \\ & 71 / 2 \end{aligned}$ | $\begin{aligned} & 7^{5 / 8} \\ & 71 / 2 \end{aligned}$ | $1 \mathrm{C}=8 \mathrm{in}$. |
| 6 | $31 / 3$ | 12 | $\begin{aligned} & 3 / 8 \\ & 1 / 2 \end{aligned}$ | $\begin{aligned} & 5 / 8 \\ & 51 / 2 \end{aligned}$ | $\begin{aligned} & 2^{3 / 4} \\ & 2^{13 / 18} \end{aligned}$ | $\begin{aligned} & 11^{5 / 8} \\ & 111 / 2 \end{aligned}$ | $5 \mathrm{C}=16 \mathrm{in}$. |
| 6 | 4 | 12 | $\begin{aligned} & 3 / 8 \\ & 1 / 2 \end{aligned}$ | $\begin{aligned} & 5^{5 / 8} \\ & 51 / 2 \end{aligned}$ | $\begin{aligned} & 35 / 8 \\ & 31 / 2 \end{aligned}$ | $\begin{aligned} & 115 / 8 \\ & 111 / 2 \end{aligned}$ | $1 \mathrm{C}=4 \mathrm{in}$. |
| 8 | 4 | 12 | $\begin{aligned} & 3 / 8 \\ & 1 / 2 \end{aligned}$ | $\begin{aligned} & 75 / 8 \\ & 71 / 2 \end{aligned}$ | $\begin{aligned} & 35 / 8 \\ & 31 / 2 \end{aligned}$ | $\begin{aligned} & 11^{5 / 8} \\ & 111 / 2 \end{aligned}$ | $1 \mathrm{C}=4 \mathrm{in}$. |
| 8 | 4 | 16 | $\begin{aligned} & 3 / 8 \\ & 1 / 2 \end{aligned}$ | $\begin{aligned} & 75 / 8 \\ & 71 / 2 \end{aligned}$ | $\begin{aligned} & 3^{5 / 8} \\ & 31 / 2 \end{aligned}$ | $\begin{aligned} & 15^{5 / 8} \\ & 15^{1 / 2} \end{aligned}$ | $1 \mathrm{C}=4 \mathrm{in}$. |
|  | UL | K SIZES | 3/8 | $\begin{aligned} & 3 \\ & 3 \end{aligned}$ | $\begin{aligned} & 2^{3 / 4} \\ & 2^{5 / 8} \end{aligned}$ | $\begin{aligned} & 8^{5 / 8} \\ & 8^{5 / 8} \end{aligned}$ | $5 \mathrm{C}=16 \mathrm{in}$. |

${ }^{1}$ Common joint sizes used with length and width dimensions. Joint thicknesses of bed joints vary based on vertical coursing and specified unit height.
${ }^{2}$ Specified dimensions may vary within this range from manufacturer to manufacturer.

### 5.12.8 Nominal Modular Size of Brick and Number of Courses in 16 Inches

|  |  | Face Dimensions |  |  |
| :--- | :---: | :---: | :---: | :---: |
| Unit Designation | Thickness in. | Height in. | Length in. | Number of Courses in 16 in. |
| Standard | 4 | $22 / 3$ | 8 | 6 |
| Engineer | 4 | $31 / 5$ | 8 | 5 |
| Economy 8 or Jumbo Closure | 4 | 4 | 8 | 4 |
| Double | 4 | $51 / 3$ | 8 | 3 |
| Roman | 4 | 2 | 12 | 8 |
| Norman | 4 | $22 / 3$ | 12 | 6 |
| Norwegian | 4 | $31 / 5$ | 12 | 5 |
| Economy 12 or Jumbo Utility | 4 | 4 | 12 | 4 |
| Triple | 4 | $51 / 3$ | 12 | 3 |
| SCR brick | 6 | $2 / 3$ | 12 | 6 |
| 6-in. Norwegian | 6 | $31 / 5$ | 12 | 5 |
| 6-in. Jumbo | 6 | 4 | 12 | 4 |
| 8-in. Jumbo | 8 | 4 | 12 | 4 |

Available as solid units conforming to ASTM C 216 or ASTM C 62, or, in some cases. as hollow brick conforming to ASTM C 652

By permission: Brick Industry Association, Reston VA

### 5.12.9 Calculate Vertical Coursing Height Based upon Number of Units

| No. of Courses | Vertical Coursing of Unit |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  | $2 \mathrm{C}=4 \mathrm{in}$. | $3 C=8 \mathrm{in}$. | $5 C=16 \mathrm{in}$. | $1 \mathrm{C}=4 \mathrm{in}$. |
| 1 | $0^{\prime}-2^{\prime \prime}$ | $0^{\prime}-22_{3}^{\prime \prime}$ | $0^{\prime}-31 / 5^{\prime \prime}$ | $0^{\prime}-4^{\prime \prime}$ |
| 2 | $0^{\prime}-4^{\prime \prime}$ | $0^{\prime}-5^{1 / 3}{ }^{\prime \prime}$ | $0^{\prime}-625^{\prime \prime}$ | $0^{\prime}-8^{\prime \prime}$ |
| 3 | $0^{\prime}-6^{\prime \prime}$ | $0^{\prime}-8^{\prime \prime}$ | $0^{\prime}-9^{3 / 5^{\prime \prime}}$ | $1^{\prime}-0^{\prime \prime}$ |
| 4 | $0^{\prime}-8^{\prime \prime}$ | $0^{\prime}-10^{2 / 3}{ }^{\prime \prime}$ | $1^{\prime}-0^{4 / 5}$ | $1^{\prime}-4^{\prime \prime}$ |
| 5 | $0^{\prime}-10^{\prime \prime}$ | $1^{\prime}-1 / 3^{\prime \prime}$ | $1^{\prime}-4^{\prime \prime}$ | $1^{\prime}-8^{\prime \prime}$ |
| 6 | $1^{\prime}-0^{\prime \prime}$ | $1^{\prime}-4^{\prime \prime}$ | $1^{\prime}-71_{5}^{\prime \prime}$ | $2^{\prime}-0^{\prime \prime}$ |
| 7 | $1^{\prime}-2^{\prime \prime}$ | $1^{\prime}-62_{3}^{\prime \prime}$ | $1^{\prime}-102 / 5^{\prime \prime}$ | $2^{\prime}-4^{\prime \prime}$ |
| 8 | $1^{\prime}-4^{\prime \prime}$ | $1^{\prime}-9^{1 / 3}{ }^{\prime \prime}$ | $2^{\prime}-13 / 5^{\prime \prime}$ | $2^{\prime}-8^{\prime \prime}$ |
| 9 | $1^{\prime}-6^{\prime \prime}$ | $2^{\prime}-0^{\prime \prime}$ | $2^{\prime}-4^{4 / 5^{\prime \prime}}$ | $3^{\prime}-0^{\prime \prime}$ |
| 10 | $1^{\prime}-8^{\prime \prime}$ | $2^{\prime}-2^{2} 3^{\prime \prime}$ | $2^{\prime}-8^{\prime \prime}$ | $3^{\prime}-4^{\prime \prime}$ |
| 11 | $1^{\prime}-10^{\prime \prime}$ | $2^{\prime}-5^{1 / 3}{ }^{\prime \prime}$ | $2^{\prime}-11 / 5^{\prime \prime}$ | $3^{\prime}-8^{\prime \prime}$ |
| 12 | $2^{\prime}-0^{\prime \prime}$ | $2^{\prime}-8^{\prime \prime}$ | $3^{\prime}-225^{\prime \prime}$ | $4^{\prime}-0^{\prime \prime}$ |
| 13 | $2^{\prime}-2^{\prime \prime}$ | $2^{\prime}-10^{2} / 3^{\prime \prime}$ | $3^{\prime}-5^{3 / 5^{\prime \prime}}$ | $4^{\prime}-4^{\prime \prime}$ |
| 14 | $2^{\prime}-4^{\prime \prime}$ | $3^{\prime}-1 / 3^{\prime \prime}$ | $3^{\prime}-8^{4 / 5^{\prime \prime}}$ | $4^{\prime}-8^{\prime \prime}$ |
| 15 | $2^{\prime}-6^{\prime \prime}$ | $3^{\prime}-4^{\prime \prime}$ | $4^{\prime}-0^{\prime \prime}$ | $5^{\prime}-0^{\prime \prime}$ |
| 16 | $2^{\prime}-8^{\prime \prime}$ | $3^{\prime}-62^{\prime \prime}{ }^{\prime \prime}$ | $4^{\prime}-31 / 5^{\prime \prime}$ | $5^{\prime}-4^{\prime \prime}$ |
| 17 | $2^{\prime}-10^{\prime \prime}$ | $3^{\prime}-91_{3}{ }^{\prime \prime}$ | $4^{\prime}-625^{\prime \prime}$ | $5^{\prime}-8^{\prime \prime}$ |
| 18 | $3^{\prime}$ - $0^{\prime \prime}$ | $4^{\prime}-0^{\prime \prime}$ | $4^{\prime}-9^{3 / 5^{\prime \prime}}$ | $6^{\prime}-0^{\prime \prime}$ |
| 19 | $3^{\prime}-2^{\prime \prime}$ | $4^{\prime}-2^{2} 3^{\prime \prime}$ | $5^{\prime}-045^{\prime \prime}$ | $6^{\prime}-4^{\prime \prime}$ |
| 20 | $3^{\prime}-4^{\prime \prime}$ | $4^{\prime}-5^{1 / 3}{ }^{\prime \prime}$ | $5^{\prime}-4^{\prime \prime}$ | $6^{\prime}-8^{\prime \prime}$ |
| 21 | $3^{\prime}-6^{\prime \prime}$ | $4^{\prime}-8^{\prime \prime}$ | $5^{\prime}-7{ }^{1 / 5^{\prime \prime}}$ | $7^{\prime}-0^{\prime \prime}$ |
| 22 | $3^{\prime}-8^{\prime \prime}$ | $4^{\prime}-102 / 3^{\prime \prime}$ | $5^{\prime}-102 / 5^{\prime \prime}$ | $7^{\prime}-4^{\prime \prime}$ |
| 23 | $3^{\prime}-10^{\prime \prime}$ | $5^{\prime}-1 / 3^{\prime \prime}$ | $6^{\prime}-13 / 5^{\prime \prime}$ | $7^{\prime}-8^{\prime \prime}$ |
| 24 | $4^{\prime}-0^{\prime \prime}$ | $5^{\prime}-4^{\prime \prime}$ | $6^{\prime}-4^{4 / 5^{\prime \prime}}$ | $8^{\prime}-0^{\prime \prime}$ |
| 25 | $4^{\prime}-2^{\prime \prime}$ | $5^{\prime}-6^{2 / 3 \prime}{ }^{\prime \prime}$ | $6^{\prime}-8^{\prime \prime}$ | $8^{\prime}-4^{\prime \prime}$ |
| 26 | $4^{\prime}-4^{\prime \prime}$ | $5^{\prime}-9{ }^{1 / 3^{\prime \prime}}$ | $6^{\prime}-11 / 5^{\prime \prime}$ | $8^{\prime}-8^{\prime \prime}$ |
| 27 | $4^{\prime}-6^{\prime \prime}$ | $6^{\prime}-0^{\prime \prime}$ | $7^{\prime}-2^{2 / 5} 5^{\prime \prime}$ | $9^{\prime}-0^{\prime \prime}$ |
| 28 | $4^{\prime}-8^{\prime \prime}$ | $6^{\prime}-22^{\prime \prime}{ }^{\prime \prime}$ | $7^{\prime}-5^{3 / 5^{\prime \prime}}$ | $9^{\prime}-4^{\prime \prime}$ |
| 29 | $4^{\prime}-10^{\prime \prime}$ | $6^{\prime}-5^{1 / 3^{\prime \prime}}$ | $7^{\prime}-8^{4 / 5^{\prime \prime}}$ | $9^{\prime}-8^{\prime \prime}$ |
| 30 | $5^{\prime}-0^{\prime \prime}$ | $6^{\prime}-8^{\prime \prime}$ | $8^{\prime}-0^{\prime \prime}$ | $10^{\prime}-0^{\prime \prime}$ |
| 31 | $5^{\prime}-2^{\prime \prime}$ | $6^{\prime}-103_{3}^{\prime \prime}$ | $8^{\prime}-31 / 5^{\prime \prime}$ | $10^{\prime}-4 \prime$ |
| 32 | $5^{\prime}-4^{\prime \prime}$ | $7^{\prime}-1 / 3^{\prime \prime}$ | $8^{\prime}-62^{\prime \prime \prime}$ | $10^{\prime}-8^{\prime \prime}$ |
| 33 | $5^{\prime}-6^{\prime \prime}$ | $7^{\prime}-4^{\prime \prime}$ | $8^{\prime}-9^{3 / 1 / 5}$ | $11^{\prime}-0^{\prime \prime}$ |
| 34 | $5^{\prime}-8^{\prime \prime}$ | $7^{\prime}-6{ }^{\prime \prime} 3^{\prime \prime}$ | $9^{\prime}-04 / 5^{\prime \prime}$ | $11^{\prime}-4^{\prime \prime}$ |
| 35 | $5^{\prime}-10^{\prime \prime}$ | $7^{\prime}-9^{1 / 3} 3^{\prime \prime}$ | $9^{\prime}-4^{\prime \prime}$ | $11^{\prime}-8^{\prime \prime}$ |
| 36 | $6^{\prime}-0^{\prime \prime}$ | $8^{\prime}-0^{\prime \prime}$ | $9^{\prime}-7^{1 / 5^{\prime \prime}}$ | $12^{\prime}-0^{\prime \prime}$ |
| 37 | $6^{\prime}-2^{\prime \prime}$ | $8^{\prime}-2^{2} / 3^{\prime \prime}$ | $9^{\prime}-102 / 5^{\prime \prime}$ | $12^{\prime}-4^{\prime \prime}$ |
| 38 | $6^{\prime}-4^{\prime \prime}$ | $8^{\prime}-5^{1 / 3}{ }^{\prime \prime}$ | $10^{\prime}-13 / 5^{\prime \prime}$ | $12^{\prime}-8^{\prime \prime}$ |
| 39 | $6^{\prime}-6^{\prime \prime}$ | $8^{\prime}-8^{\prime \prime}$ | $10^{\prime}-44 / 5^{\prime \prime}$ | $13^{\prime}-0^{\prime \prime}$ |
| 40 | $6^{\prime}-8^{\prime \prime}$ | $8^{\prime}-10^{2 / 3}{ }^{\prime \prime}$ | $10^{\prime}-8^{\prime \prime}$ | $13^{\prime}-4^{\prime \prime}$ |
| 41 | $6^{\prime}-10^{\prime \prime}$ | $9^{\prime}-1 / 3^{\prime \prime}$ | $10^{\prime}-111 / 5^{\prime \prime}$ | $13^{\prime}-8^{\prime \prime}$ |
| 42 | $7^{\prime}-0^{\prime \prime}$ | $9^{\prime}-4^{\prime \prime}$ | $11^{\prime}-22 / 5^{\prime \prime}$ | $14^{\prime}-0^{\prime \prime}$ |
| 43 | $7^{\prime}-2^{\prime \prime}$ | $9^{\prime}-62_{3}^{\prime \prime}$ | $11^{\prime}-5^{1 / 5^{\prime \prime}}$ | $14^{\prime}-4^{\prime \prime}$ |
| 44 | $7^{\prime}-4^{\prime \prime}$ | $9^{\prime}-913^{\prime \prime}$ | $11^{\prime}-8^{4 / 5^{\prime \prime}}$ | $14^{\prime}-8^{\prime \prime}$ |
| 45 | $7^{\prime}-6^{\prime \prime}$ | $10^{\prime}-0^{\prime \prime}$ | $12^{\prime}-0^{\prime \prime}$ | $15^{\prime}-0^{\prime \prime}$ |


| No. of Courses | Vertical Coursing of Unit |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  | $2 C=4 \mathrm{in}$. | $3 C=8 \mathrm{in}$. | $5 C=16$ in. | $1 \mathrm{C}=4 \mathrm{in}$. |
| 46 | $7^{\prime}-8^{\prime \prime}$ | $10^{\prime}-24^{\prime \prime}$ | $12^{\prime}-3^{1 / 5}{ }^{\prime \prime}$ | $15^{\prime}-4^{\prime \prime}$ |
| 47 | $7^{\prime}-10^{\prime \prime}$ | $10^{\prime}-5^{1 / 3 \prime}{ }^{\prime \prime}$ | $12^{\prime}-6^{2 / 51}$ | $15^{\prime}-8^{\prime \prime}$ |
| 48 | $8^{\prime}-0^{\prime \prime}$ | $10^{\prime}-8^{\prime \prime}$ | $12^{\prime}-9^{3 / 5}{ }^{\prime \prime}$ | $16^{\prime}-0^{\prime \prime}$ |
| 49 | $8^{\prime}-2^{\prime \prime}$ | $10^{\prime}-102_{3}^{\prime \prime}$ | $13^{\prime}-04 / 5^{\prime \prime}$ | $16^{\prime}-4^{\prime \prime}$ |
| 50 | $8^{\prime}-4^{\prime \prime}$ | $11^{\prime}-11^{\prime \prime}$ | $13^{\prime}-4^{\prime \prime}$ | $16^{\prime}-8^{\prime \prime}$ |
| 100 | $16^{\prime}-8^{\prime \prime}$ | $22^{\prime}-22^{\prime \prime}$ | $26^{\prime}-8^{\prime \prime}$ | $33^{\prime}-4^{\prime \prime}$ |

$1 \mathrm{in} .=25.4 \mathrm{~mm} ; 1 \mathrm{ft}=0.3 \mathrm{~m}$
Brick positioned in wall as stretchers or headers.

By permission: Brick Industry Association, Reston, VA

### 5.12.10 Calculate Horizontal Coursing Based upon Number of Units

| Number of Units | Unit Length |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Nominal Dimensions, in. |  | Specified Dimensions, in. |  |  |  |
|  |  |  | 8 |  | $8 \% / 8$ | 9\%/8 |
|  | 8 | 12 | 1/2 in. jt. | 3/8 in. jt. | $3 / 8 \mathrm{in} . j$ t. | $3 / 8 \mathrm{in} . j$ j. |
| 1 | $0^{\prime}-8^{\prime \prime}$ | $1^{\prime}-0^{\prime \prime}$ | $0^{\prime}-8^{1 / 2^{\prime \prime}}$ | $0^{\prime}-8^{3 / 8^{\prime \prime}}$ | $0^{\prime}-9^{\prime \prime}$ | $0^{\prime}-10^{\prime \prime}$ |
| 2 | $1^{\prime}-4^{\prime \prime}$ | $2^{\prime}-0^{\prime \prime}$ | $1^{\prime}-5^{\prime \prime}$ | $1^{\prime}-4^{3 / 4} 4^{\prime \prime}$ | $1^{\prime}-6^{\prime \prime}$ | $1^{\prime}-8^{\prime \prime}$ |
| 3 | $2^{\prime}-0^{\prime \prime}$ | $3^{\prime}-0^{\prime \prime}$ | $2^{\prime}-1 / 2^{\prime \prime}$ | $2^{\prime}-1{ }^{1 / 8^{\prime \prime}}$ | $2^{\prime}-3^{\prime \prime}$ | $2^{\prime}-6^{\prime \prime}$ |
| 4 | $2^{\prime}-8^{\prime \prime}$ | $4^{\prime}-0^{\prime \prime}$ | $2^{\prime}-10^{\prime \prime}$ | $2^{\prime}-9^{1 / 2}{ }^{\prime \prime}$ | $3^{\prime}-0^{\prime \prime}$ | $3^{\prime}-4^{\prime \prime}$ |
| 5 | $3^{\prime}-4^{\prime \prime}$ | $5^{\prime}-0^{\prime \prime}$ | $3^{\prime}-6^{1 / 2}{ }^{\prime \prime}$ | $3^{\prime}-5^{1 / 8^{\prime \prime}}$ | $3^{\prime}-9^{\prime \prime}$ | $4^{\prime}-2^{\prime \prime}$ |
| 6 | $4^{\prime}-0^{\prime \prime}$ | $6^{\prime}-0^{\prime \prime}$ | $4^{\prime}-3^{\prime \prime}$ | $4^{\prime}-2^{1 / 4^{\prime \prime}}$ | $4^{\prime}-6^{\prime \prime}$ | $5^{\prime}-0^{\prime \prime}$ |
| 7 | $4^{\prime}-8^{\prime \prime}$ | $7^{\prime}-0^{\prime \prime}$ | $4^{\prime}-11^{1 / 2}{ }^{\prime \prime}$ | $4^{\prime}-10^{5 / 8}{ }^{\prime \prime}$ | $5^{\prime}-3^{\prime \prime}$ | $5^{\prime}-10^{\prime \prime}$ |
| 8 | $5^{\prime}-4^{\prime \prime}$ | $8^{\prime}-0^{\prime \prime}$ | $5^{\prime}-8^{\prime \prime}$ | $5^{\prime}-7^{\prime \prime}$ | $6^{\prime}-0^{\prime \prime}$ | $6^{\prime}-8^{\prime \prime}$ |
| 9 | $6^{\prime}-0^{\prime \prime}$ | $9^{\prime}-0^{\prime \prime}$ | $6^{\prime}-4^{1 / 2}{ }^{\prime \prime}$ | $6^{\prime}-3^{3 / 8^{\prime \prime}}$ | $6^{\prime}-9^{\prime \prime}$ | $7^{\prime}-6^{\prime \prime}$ |
| 10 | $6^{\prime}-8^{\prime \prime}$ | $10^{\prime}-0^{\prime \prime}$ | $7^{\prime}-1^{\prime \prime}$ | $6^{\prime}-11^{3 / 4}{ }^{\prime \prime}$ | $7^{\prime}-6^{\prime \prime}$ | $8^{\prime}-4^{\prime \prime}$ |
| 11 | $7^{\prime}-4^{\prime \prime}$ | 11' - $0^{\prime \prime}$ | $7^{\prime}-9^{1 / 2^{\prime \prime}}$ | $7^{\prime}-8^{1 / 8^{\prime \prime}}$ | $8^{\prime}-3^{\prime \prime}$ | $9^{\prime}-2^{\prime \prime}$ |
| 12 | $8^{\prime}-0^{\prime \prime}$ | $12^{\prime}-0^{\prime \prime}$ | $8^{\prime}-6^{\prime \prime}$ | $8^{\prime}-4 \frac{1}{2} 2^{\prime \prime}$ | $9^{\prime}-0^{\prime \prime}$ | $10^{\prime}-0^{\prime \prime}$ |
| 13 | $8^{\prime}-8^{\prime \prime}$ | $13^{\prime}-0^{\prime \prime}$ | $9^{\prime}-2^{1 / 2}{ }^{\prime \prime}$ | $9^{\prime}-07 / 8^{\prime \prime}$ | $9^{\prime}-9^{\prime \prime}$ | $10^{\prime}-10^{\prime \prime}$ |
| 14 | $9^{\prime}-4^{\prime \prime}$ | $14^{\prime}-0^{\prime \prime}$ | $9^{\prime}-11^{\prime \prime}$ | $9^{\prime}-9{ }^{1 / 4^{\prime \prime}}$ | $10^{\prime}-6^{\prime \prime}$ | $11^{\prime}-8^{\prime \prime}$ |
| 15 | $10^{\prime}-0^{\prime \prime}$ | 15' - $0^{\prime \prime}$ | $10^{\prime}-712^{\prime \prime}$ | $10^{\prime}-5 / 8^{\prime \prime}$ | 11'-3" | $12^{\prime}-6^{\prime \prime}$ |
| 16 | $10^{\prime}-8^{\prime \prime}$ | 16' - $0^{\prime \prime}$ | 11'-4" | $11^{\prime}-2^{\prime \prime}$ | $12^{\prime}-0^{\prime \prime}$ | 13'-4" |
| 17 | $11^{\prime}-4^{\prime \prime}$ | $17^{\prime}-0^{\prime \prime}$ | $12^{\prime}-0{ }^{1 / 2}{ }^{\prime \prime}$ | $11^{\prime}-103 / 8^{\prime \prime}$ | $12^{\prime}-9^{\prime \prime}$ | $14^{\prime}-2^{\prime \prime}$ |
| 18 | $12^{\prime}-0^{\prime \prime}$ | 18' - $0^{\prime \prime}$ | 12' - 9' | $12^{\prime}-6{ }^{3 / 4}{ }^{\prime \prime}$ | $13^{\prime}-6^{\prime \prime}$ | $15^{\prime}-0^{\prime \prime}$ |
| 19 | $12^{\prime}-8^{\prime \prime}$ | 19'-0' ${ }^{\prime \prime}$ | $13^{\prime}-51_{2}^{\prime \prime}$ | 13' - 31/81 ${ }^{\prime \prime}$ | 14' - $3^{\prime \prime}$ | 15' - 10" |
| 20 | $13^{\prime}-4^{\prime \prime}$ | 20' - $0^{\prime \prime}$ | $14^{\prime}-2^{\prime \prime}$ | $13^{\prime}-11^{1 / 2}{ }^{\prime \prime}$ | $15^{\prime}-0^{\prime \prime}$ | $16^{\prime}-8^{\prime \prime}$ |
| 21 | $14^{\prime}-0^{\prime \prime}$ | 21' - $0^{\prime \prime}$ | $14^{\prime}-10^{1 / 2}{ }^{\prime \prime}$ | 14' ${ }^{\prime}$-7/8 ${ }^{\prime \prime}$ | 15' - $9^{\prime \prime}$ | $17^{\prime}-6^{\prime \prime}$ |
| 22 | $14^{\prime}-8^{\prime \prime}$ | $22^{\prime}-0^{\prime \prime}$ | 15' - 7' | $15^{\prime}-4{ }^{1 / 4 \prime}$ | $16^{\prime}-6^{\prime \prime}$ | 18'-4" |
| 23 | $15^{\prime}-4^{\prime \prime}$ | $23^{\prime}-0^{\prime \prime}$ | $16^{\prime}-31 / 2^{\prime \prime}$ | $16^{\prime}-05 / 8^{\prime \prime}$ | $17^{\prime}-3^{\prime \prime}$ | 19'-2" |
| 24 | $16^{\prime}-0^{\prime \prime}$ | $24^{\prime}-0^{\prime \prime}$ | $17^{\prime}-0^{\prime \prime}$ | $16^{\prime}-9^{\prime \prime}$ | $18^{\prime}-0^{\prime \prime}$ | $20^{\prime}-0^{\prime \prime}$ |
| 25 | $16^{\prime}-8^{\prime \prime}$ | $25^{\prime}-0^{\prime \prime}$ | $17^{\prime}-8^{1 / 2}{ }^{\prime \prime}$ | $17^{\prime}-53 / 8^{\prime \prime}$ | 18' - $9^{\prime \prime}$ | $20^{\prime}-10^{\prime \prime}$ |


| Number of Units | Unit Length - Cont'd |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Nominal Dimensions, in. |  | Specified Dimensions, in. |  |  |  |
|  |  |  | 8 |  | $8{ }^{5 / 8}$ | 9\%/8 |
|  | 8 | 12 | 1/2 in. jt. | 3/8 in. jt. | $3 / 8$ in. jt. | $3 / 8$ in. jt. |
| 26 | $17^{\prime}-4^{\prime \prime}$ | 26 ${ }^{\prime}$ - $0^{\prime \prime}$ | 18' - $5^{\prime \prime}$ | 18' - $13 / 4^{\prime \prime}$ | 19'-6" | 21' - $8^{\prime \prime}$ |
| 27 | $18^{\prime}-0^{\prime \prime}$ | $27^{\prime}-0^{\prime \prime}$ | $19^{\prime}-11 / 2^{\prime \prime}$ | $18^{\prime}-10^{1 / 8^{\prime \prime}}$ | $20^{\prime}-3^{\prime \prime}$ | $22^{\prime}-6^{\prime \prime}$ |
| 28 | $18^{\prime}-8^{\prime \prime}$ | 28' - $0^{\prime \prime}$ | 19'-10 ${ }^{\prime \prime}$ | $19^{\prime}-6^{1 / 2 \prime}$ | $21^{\prime}-0^{\prime \prime}$ | 23' ${ }^{\prime \prime} 4^{\prime \prime}$ |
| 29 | $19^{\prime}-4^{\prime \prime}$ | 29 ${ }^{\prime}-0^{\prime \prime}$ | 20' - $6^{1 / 2}{ }^{\prime \prime}$ | $20^{\prime}-27 / 8^{\prime \prime}$ | $21^{\prime}-9^{\prime \prime}$ | $24^{\prime}-2^{\prime \prime}$ |
| 30 | $20^{\prime}-0^{\prime \prime}$ | 30' - $0^{\prime \prime}$ | 21'-3" | 20' - 111/4" | $22^{\prime}-6^{\prime \prime}$ | 25' - $0^{\prime \prime}$ |
| 31 | $20^{\prime}-8^{\prime \prime}$ | $31^{\prime}-0^{\prime \prime}$ | 21 ${ }^{\prime}-11_{1 / 2}{ }^{\prime \prime}$ | 21 - $75 /{ }^{\prime \prime}{ }^{\prime \prime}$ | 23' - $3^{\prime \prime}$ | 25' - 10' ${ }^{\prime \prime}$ |
| 32 | $21^{\prime}-4^{\prime \prime}$ | $32^{\prime}-0^{\prime \prime}$ | $22^{\prime}-8^{\prime \prime}$ | 22' - $4^{\prime \prime}$ | 24' - $0^{\prime \prime}$ | $26^{\prime}-8^{\prime \prime}$ |
| 33 | $22^{\prime}-0^{\prime \prime}$ | $33^{\prime}-0^{\prime \prime}$ | $23^{\prime}-4{ }^{\prime \prime} 2^{\prime \prime}$ | $23^{\prime}-03^{\prime \prime} 8^{\prime \prime}$ | 24' - $9^{\prime \prime}$ | $27^{\prime}-6^{\prime \prime}$ |
| 34 | $22^{\prime}-8^{\prime \prime}$ | $34^{\prime}-0^{\prime \prime}$ | 24' - 1' | $23^{\prime}-8^{3 / 4^{\prime \prime}}$ | 25' - $6^{\prime \prime}$ | 28' $8^{\prime \prime}$ |
| 35 | $23^{\prime}-4^{\prime \prime}$ | $35^{\prime}-0^{\prime \prime}$ | $24^{\prime}-91^{\prime \prime}{ }^{\prime \prime}$ | $24^{\prime}-5^{1 / 8^{\prime \prime}}$ | $26^{\prime}-3^{\prime \prime}$ | 29'- $2^{\prime \prime}$ |
| 36 | 24 ${ }^{\prime}$ - $0^{\prime \prime}$ | $36^{\prime}-0^{\prime \prime}$ | 25' - $6^{\prime \prime}$ | 25' ${ }^{\prime}$ - $11 / 2^{\prime \prime}$ | $27^{\prime}-0^{\prime \prime}$ | $30^{\prime}-0^{\prime \prime}$ |
| 37 | $24^{\prime}-8^{\prime \prime}$ | $37^{\prime}-0^{\prime \prime}$ | $26^{\prime}-2^{1 / 2}{ }^{\prime \prime}$ | 25' - 97/ $\mathbf{8}^{\prime \prime}$ | $27^{\prime}-9^{\prime \prime}$ | $30^{\prime}-10^{\prime \prime}$ |
| 38 | $25^{\prime}-4^{\prime \prime}$ | $38^{\prime}-0^{\prime \prime}$ | 26'-11" | 26' - $61 / 4^{\prime \prime}$ | 28' - $6^{\prime \prime}$ | $31^{\prime}-8^{\prime \prime}$ |
| 39 | $26^{\prime}-0^{\prime \prime}$ | $39^{\prime}-0^{\prime \prime}$ | $27^{\prime}-71_{2}^{\prime \prime}$ | $27^{\prime}-25^{\prime \prime}{ }^{\prime \prime}$ | 29'-3" | $32^{\prime}-6^{\prime \prime}$ |
| 40 | $26^{\prime}-8^{\prime \prime}$ | $40^{\prime}-0^{\prime \prime}$ | 28'-4' | 27' - 11' ${ }^{\prime \prime}$ | $30^{\prime}-0^{\prime \prime}$ | $33^{\prime}-4^{\prime \prime}$ |
| 41 | $27^{\prime}-4^{\prime \prime}$ | $41^{\prime}-0^{\prime \prime}$ | 29 - $0^{1 / 2}{ }^{\prime \prime}$ | 28 ${ }^{\prime}-738^{\prime \prime}$ | $30^{\prime}-9^{\prime \prime}$ | 34' - $2^{\prime \prime}$ |
| 42 | $28^{\prime}-0^{\prime \prime}$ | $42^{\prime}-0^{\prime \prime}$ | 29'-9' | 29'-33/4" | $31^{\prime}-6^{\prime \prime}$ | 35' - $0^{\prime \prime}$ |
| 43 | $28^{\prime}-8^{\prime \prime}$ | $43^{\prime}-0^{\prime \prime}$ | 30' - $5^{1 / 2}{ }^{\prime \prime}$ | 30' - 0 $1_{8}^{\prime \prime \prime}$ | $32^{\prime}-3^{\prime \prime}$ | 35' - 10' ${ }^{\prime \prime}$ |
| 44 | $29^{\prime}-4^{\prime \prime}$ | $44^{\prime}-0^{\prime \prime}$ | $31^{\prime}-2^{\prime \prime}$ | $30^{\prime}-8^{1 / 2}{ }^{\prime \prime}$ | $33^{\prime}-0^{\prime \prime}$ | $36^{\prime}-8^{\prime \prime}$ |
| 45 | $30^{\prime}-0^{\prime \prime}$ | $45^{\prime}-0^{\prime \prime}$ | $31^{\prime}-10{ }^{1 / 2}{ }^{\prime \prime}$ | 31'-47/8' ${ }^{\prime \prime}$ | 33'-9' | $37^{\prime}-6^{\prime \prime}$ |
| 46 | $30^{\prime}-8^{\prime \prime}$ | $46^{\prime}-0^{\prime \prime}$ | 32' - ${ }^{\prime \prime}$ | 32' - $11 / 4^{\prime \prime}$ | $34^{\prime}-6^{\prime \prime}$ | $38^{\prime}-4^{\prime \prime}$ |
| 47 | $31^{\prime}-4^{\prime \prime}$ | $47^{\prime}-0^{\prime \prime}$ | $33^{\prime}-31 / 2^{\prime \prime}$ | 32' - 95 $8^{\prime \prime}$ | $35^{\prime}-3^{\prime \prime}$ | $39^{\prime}-2^{\prime \prime}$ |
| 48 | $32^{\prime}-0^{\prime \prime}$ | 48' - $0^{\prime \prime}$ | 34' ${ }^{\prime}$ - ${ }^{\prime \prime}$ | $33^{\prime}-6^{\prime \prime}$ | $36^{\prime}-0^{\prime \prime}$ | $40^{\prime}-0^{\prime \prime}$ |
| 49 | $32^{\prime}-8^{\prime \prime}$ | $49^{\prime}-0^{\prime \prime}$ | $34^{\prime}-81 / 2^{\prime \prime}$ | 34' - $3^{3} 3^{\prime \prime}$ | 36' - $9^{\prime \prime}$ | $40^{\prime}-10^{\prime \prime}$ |
| 50 | $33^{\prime}-4^{\prime \prime}$ | $50^{\prime}-0^{\prime \prime}$ | 35' - $5^{\prime \prime}$ | $34^{\prime}-10^{3 / 4}{ }^{\prime \prime}$ | $37^{\prime}-6^{\prime \prime}$ | $41^{\prime}-8^{\prime \prime}$ |
| 100 | $66^{\prime}-8^{\prime \prime}$ | $100^{\prime}-0^{\prime \prime}$ | $70^{\prime}-10^{\prime \prime}$ | $69^{\prime}-9^{1 / 2}{ }^{\prime \prime}$ | $75^{\prime}-0^{\prime \prime}$ | $83^{\prime}-4^{\prime \prime}$ |

$1 \mathrm{in} .=25.4 \mathrm{~mm} ; 1 \mathrm{ft}=0.3 \mathrm{~m}$

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### 5.12.11 Horizontal Coursing—Soft and Hard Metric Dimensioning

$$
\text { Hard metric equivalent of } \begin{aligned}
8^{\prime \prime} \times 8^{\prime \prime} \times 16^{\prime \prime} & =400 \mathrm{~mm} \\
& =390 \mathrm{~mm}+10 \mathrm{~mm} \text { mortar joint }
\end{aligned}
$$



Hard metric is based on 100 mm modules.

## Detail 1 - Hard Metric Dimensioning

$$
\text { Soft metric equivalent of } \begin{aligned}
8^{\prime \prime} \times 8^{\prime \prime} \times 16^{\prime \prime} & =406.4 \mathrm{~mm} \\
& =396.9 \mathrm{~mm}+9.5 \mathrm{~mm} \text { mortar joint }
\end{aligned}
$$



| $15 \frac{1 / 8}{}$ | $15^{5 / \prime}$ | $15^{5 / \prime}$ | $15^{5 / \prime}$ | $15 \frac{1}{8 \prime}$ |
| :---: | :---: | :---: | :---: | :---: |

Soft metric is based on $4^{\prime \prime}$ modules.

## Detail 2 - Soft Metric Dimensioning

### 5.13.0 Profiles and Dimensions of Typical Concrete Masonry Units (CMUs)




Source: Palestine Concrete Tile Co., Palestine, TX

### 5.14.0 Mortar Mixes—ASTM Minimums

| Type M | 2500 psi |
| :--- | ---: |
| Type $\mathbf{S}$ | 1800 psi |
| Type $\mathbf{N}$ | 750 psi |
| Type $\mathbf{O}$ | 350 psi |
| Type K | 75 psi |

But be aware that the mix listed for type N mortar typically achieves a 28 -day strength in the range of 1500 to 2400 psi. This meets and beats the ASTM requirement of 750 psi by a great deal.

Another example is the mix listed for type O mortar that provides a usual psi in the range of 750 to 1200 and higher, sometimes up to 2000. Again, this meets the minimum psi of 350 by a large percentage.

Typical type M mixes have strengths of 3000 to 3800 psi and so exceed the ASTM minimum compressive strength requirement of 2500 psi .

Type S mortars are required to have a minimum of 1800 psi , and their mixes usually give you strengths of from 2300 to 3000 psi .

### 5.14.1 Mixture Calculations for Types N, M, S, O, K Mortar

Here are seven common and uncommon mortar mixes. They are types N, M, S, and O. There is also mortar for glass block, straight lime mortar, and type K. Type K is used solely in historic preservation. Each one has a certain proportion of portland cement, hydrated lime, and sand. Mortar proportions are always expressed in that order. In addition, these proportions always refer to volumes, not to weight or a combination of volumes and weights. But then the components of these mixes are usually purchased by weight, but that's not how the mixes are measured.

A mix designated as $3 / 1 / 12$ has three parts of portland cement, 1 part hydrated lime, and 12 parts sand. Now let's say that you want to compute mortar by the cubic yard. So how much of each mortar component is in a cubic yard? Let's go through all seven of the mixes and see.

Be aware that the proportions of lime, cement, and sand in each mix type can vary a bit by geographic regions or by contractors within a region. However, we are showing you the commonly used proportions and if you are used to something a little different, then you are simply using a regional or personal variation on the standard.

Also, these amounts are designed to add up to exactly one cu yd of material. Field amounts can show other quantities of components due to the realities of outdoor mortar mixing. Much of the literature on mortar proportions and mixes show greater or different quantities due to the great amount of waste in the actual preparation, transportation within the job site, and handling during the use of a batch of mortar. The numbers shown here reflect computed amounts. These are exact mathematical measurements down to the spoonful (though we give you final amounts of sand in tons and the other parts in bags). The tons and bags are finely measured. The terms hydrated lime and lime putty mean the same thing since lime putty is simply wet hydrated lime (you added some water to it and stirred it up), whereas in hydrated lime all of the water molecules are stoichiometrically bonded to the calcium and magnesium in the lime and the lime remains a dry powder. Lime putty is just wet hydrated lime.

The mix calculations use densities set out by the ASTM. These are:

| Portland cement | $94 \mathrm{lbs} / \mathrm{cu} \mathrm{ft}$ |
| :--- | :--- |
| Hydrated lime | $40 \mathrm{lbs} / \mathrm{cu} \mathrm{ft}$ |
| Sand | $80 \mathrm{lbs} / \mathrm{cu} \mathrm{ft}$ |

The purchased items are by these:
Portland cement 94 lb bags

Hydrated lime $\quad 50 \mathrm{lb}$ bags
Sand by the ton
Component amounts
Type N mortar
This uses a $1 / 1 / 6 \mathrm{mix}$ and results in a mortar with a 750 psi compressive strength. Type N is the normal, general-purpose mortar mix and can be used in above-grade work in both exterior and interior load-bearing installations.

To get 1 cu yd of N mortar, you need 27 cubic feet of the components in a 1 to 1 to 6 proportion.
Portland cement $\quad 3.375 \mathrm{cu} \mathrm{ft}$
Hydrated lime $\quad 3.375 \mathrm{cu} \mathrm{ft}$
Sand $\quad 20.25 \mathrm{cu} \mathrm{ft}$
Total 27 cu ft
Based on the ASTM densities, this gives you 317.25 lbs of portland cement, 135 lbs of hydrated lime, and 1620 lbs of sand.

To put together a single cubic yard of type N mortar, you need to buy and mix together:
3.375 bags of portland cement ( 94 lb bags)
2.7 bags of hydrated lime ( 50 lb bags)
0.81 tons of sand

Type M mortar
This uses a $3 / 1 / 12 \mathrm{mix}$ and results in a mortar with a 2500 psi compressive strength. Type M is used for below-grade load-bearing masonry work and for chimneys and brick manholes.

To get $1 \mathrm{cu} y$ d of M mortar, you need 27 cubic feet of the components in a 3 to 1 to 12 proportion.
Portland cement $\quad 5.0625 \mathrm{cu} \mathrm{ft}$
Hydrated lime $\quad 1.6875 \mathrm{cu} \mathrm{ft}$
Sand $\quad 20.25 \mathrm{cu} \mathrm{ft}$
Total $\quad 27 \mathrm{cu} \mathrm{ft}$
Based on the ASTM densities, this gives you 475.875 lbs of portland cement, 67.5 lbs of hydrated lime, and 1620 lbs of sand.

To put together a single cubic yard of type M mortar, you need to buy and mix:
5.0625 bags of portland cement ( 94 lb bags)
1.35 bags of hydrated lime ( 50 lb bags)
0.81 tons of sand

Type S mortar
This uses a $2 / 1 / 9 \mathrm{mix}$ and results in a mortar with a 1800 psi compressive strength. Type S is used for belowgrade work and in such areas as masonry foundation walls, brick manholes, retaining walls, sewers, brick walkways, brick pavement, and brick patios.

To get 1 cu yd of S mortar, you need 27 cubic feet of the components in a 2 to 1 to 9 proportion.
Portland cement 4.5 cu ft
Hydrated lime $\quad 2.25 \mathrm{cu} \mathrm{ft}$
Sand $\quad 20.25 \mathrm{cu} \mathrm{ft}$
Total $\quad 27 \mathrm{cu} \mathrm{ft}$

Based on the ASTM densities, this gives you 423 lbs of portland cement, 90 lbs of hydrated lime, and 1620 lbs of sand.

To put together a single cubic yard of type $S$ mortar, you need to buy and mix:
4.5 bags of portland cement ( 94 lb bags)
1.8 bags of hydrated lime ( 50 lb bags)
0.81 tons of sand

Type O mortar
This uses a $1 / 2 / 9$ mix and results in a mortar with a 350 psi compressive strength. Type O is a lime-rich mortar and is also referred to as "pointing" mortar. It is used in above-grade, non-load-bearing situations in both interior and exterior environments.

To get 1 cu yd of O mortar, you need 27 cubic feet of the components in a 1 to 2 to 9 proportion.

| Portland cement | 2.25 cu ft |
| :--- | :--- |
| Hydrated lime | 4.5 cu ft |
| Sand | 20.25 cu ft |
| Total | 27 cu ft |

Based on the ASTM densities, this gives you 211.5 lbs of portland cement, 180 lbs of hydrated lime, and 1620 lbs of sand.

To put together a single cubic yard of type O mortar, you need to buy and mix together:
2.25 bags of portland cement ( 94 lb bags)
3.6 bags of hydrated lime ( 50 lb bags)
0.81 tons of sand

Type K mortar
This uses a $1 / 3 / 10 \mathrm{mix}$ and results in a mortar with but a 75 psi compressive strength. Type K is useful only in historic preservation situations where load-bearing strength is not of importance and the porous qualities of this mortar allows very little movement due to temperature and moisture fluctuations. This aids in prolonging the integrity of the old or even ancient bricks in historic structures.

To get 1 cu yd of K mortar, you need 27 cubic feet of the components in a 1 to 3 to 10 proportion.

| Portland cement | 1.93 cu ft |
| :--- | ---: |
| Hydrated lime | 5.79 cu ft |
| Sand | 19.29 cu ft |
| Total | 27 cu ft |

Based on the ASTM densities, this gives you 181.42 lbs of portland cement, 231.6 lbs of hydrated lime, and 1543.2 lbs of sand.

To put together a single cubic yard of type K mortar, you need to buy:
1.93 bags of portland cement
4.632 bags of hydrated lime
0.7716 tons of sand

### 5.14.2 Mixture Calculations for Straight Lime Mortar

This uses a $0 / 1 / 3$ mix and is used now only to re-create the construction and review the methods of times past or maybe for purely visual purposes. This mortar was made before portland cement was available in many areas, and so this is what was used. Sometimes you'll see straight lime mortar called "L" mortar (for
lime), but this is not designating it as "type L" mortar as in the MSNOK types. There is no "type L" mortar.

To get 1 cu yd of lime mortar, you need 27 cubic feet of the components in a 0 to 1 to 3 proportion.
Portland cement none
Hydrated lime $\quad 6.75 \mathrm{cu} \mathrm{ft}$
Sand $\quad 20.25 \mathrm{cu} \mathrm{ft}$
Total $\quad 27 \mathrm{cu} \mathrm{ft}$
Based on the ASTM densities, this gives you no portland cement, 270 lbs of hydrated lime, and 1620 lbs of sand.

To put together a single cubic yard of lime mortar, you need to buy:
No bags of portland cement
5.4 bags of hydrated lime ( 50 lb bags)
0.81 tons of sand

### 5.14.3 Mixture Calculations for Glass Block Mortar

This uses a $1 / 1 / 4 \mathrm{mix}$ and is used with as little water as possible. This is a mix designed specifically for glass block. Also, note that it uses waterproof Portland cement in place of "regular" Portland cement.

To get 1 cu yd of glass block mortar, you need 27 cubic feet of the components in a 1 to 1 to 4 proportion.

### 5.14.4 Mixture Calculations for Waterproof Portland Cement

| Waterproof Portland cement | 4.5 cu ft |
| :--- | ---: |
| Hydrated lime | 4.5 cu ft |
| Sand | 18 cu ft |
| Total | 27 cu ft |

Based on the ASTM densities, this gives you 423 lbs of waterproof portland cement, 180 lbs of hydrated lime, and 1,440 lbs of sand.

To put together a single cubic yard of glass block mortar, you need to buy and mix:
4.5 bags of portland cement ( 94 lb bags)
3.6 bags of hydrated lime ( 50 lb bags)
0.72 tons of sand

## Note

Lime Types versus Mortar Mix Designations
Limestone formed by nature contains varying proportions of calcium to magnesium. No large scientist with a giant beaker and a set of stoppered test tubes measured out the things that make up rocks beforehand. Some of it has more magnesium, while other limestone rock has more calcium. For making mortar, it is desirable to have from a third to a half of the rock from which the mortar lime is derived composed of magnesium carbonate. The remainder then would be from one-half to two-thirds calcium carbonate. A limestone whose composition falls within these percentages is dolomitic limestone, and from it is made Type $\mathbf{S}$ lime hydrate. Masonry lime made from limestone that is composed of less than $5 \%$ magnesium carbonate (called high calcium limestone since it is 95 to $99 \%$ calcium carbonate) is labeled Type N lime hydrate. Type S lime is used to make masonry mortar. Type $\mathbf{N}$ lime can be used only if it is tested and proven on a batch-by-batch basis. The type
$\mathbf{S}$ lime designation stands for Special and the type $\mathbf{N}$ stands for Normal. The special lime hydrate is the one normally used, and the normal lime hydrate is used only with special testing. These lime "types" have absolutely nothing to with mortar mixes type N and type S . You must never, ever confuse these lime hydrate types with mortar mixes. They have nothing to do with one another. Why "they" should label them with the same designations, we have no idea.

## Minimum compressive mortar strengths, ASTM and its psi requirements

The ASTM assigns minimum required compressive strengths to the various mortar types.

### 5.15.0 Typical Properties of Colorless Coatings for Brick Masonry

|  | Water Vapor Transmission | Water Repellency | Life-Span, Years | Available with Glossy Finish | Graffiti Resistance |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Film Formers Acrylics | Poor | Very good | 5 to 7 | Yes | Yes |
| Stearates | Poor | Varies | 1 | No | No |
| Mineral gum waxes | Poor | Good | Varies | No | No |
| Urethanes | Poor | Very good | 1 to 3 | Yes | Yes |
| Penetrants Siloxanes | Very good | Very good | 10+ | No | No |
| Silanes | Very good | Very good | 10+ | No | No |
| Silicates | Poor | Poor | Varies | No | No |
| Methyl siliconates | Good | Fair | Varies | No | No |
| Silicone resins | Fair | Varies | 1 | Yes | No |
| RTV silicone rubber | Good | Good | 5 to 10 | No | Yes |
| Blends | Varies | Varies | Varies | No | No |

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## Section 6

## Calculating the Size/Weight of Structural Steel and Miscellaneous Metals

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### 6.0.0 Ingredients of Steel

## Iron Ore

Iron ore is a rock that contains iron combined with oxygen. It is sourced from mines around the world. Some of the world's highest quality iron ore comes from Australia.

## Coke

Coke is made from coal. Once mined, the coal is crushed and washed. Coal is then baked in coke ovens for about 18 hours. During this process, by-products are removed and coke is produced.

## Flux

Flux is a term that describes minerals used to collect impurities during iron and steelmaking. Fluxes used by BHP Steel include limestone and dolomite. The flux causes a chemical reaction, and the elements not needed for steelmaking combine to form slag.

## Molten Iron

Iron is the main ingredient needed to make steel in the Basic Oxygen Steelmaking process. Molten iron is made from iron ore and other ingredients in a blast furnace.

## Scrap Steel

Scrap steel comes from many different sources because it is very easily recycled. Some scrap comes from within the steelworks, where it might have been damaged or is at the end of a batch of one type of steel. It also comes from old car bodies, old ship containers, and buildings that have been demolished.

Another source of scrap can be found in our homes. Steel cans (food cans, pet food cans, aerosols, paint cans, etc.) are collected as part of council curbside collections and can be recycled an infinite number of times.

## Alloying Materials

Alloying materials are used to give the steel special properties and make different types of steel. Alloying materials can be added as elements, like manganese, aluminium, and nickel, or as compounds of iron.

### 6.0.1 Structural Steel in the Construction Industry

There are four basic types of structural steel in general use in the construction industry today:

## Carbon Steel

- A36-Structural Shapes and Plates
- A53-Structural Pipe and Tubing
- A500-Structural Pipe and Tubing
- A501-Structural Pipe and Tubing
- A529-Structural Shapes and Plate


## High-Strength Low-Alloy Steel

- A441-Structural Shapes and Plates
- A572-Structural Shapes and Plates
- A618-Structural Plate and Tubing
- A992-W Shape Beams only
- A270-Structural Shapes and Plates


## Corrosion-Resistant High-Strength Low-Alloy Steel

- A242-Structural Shapes and Plates
- A588-Structural Shapes and Plates


## Quenched and Tempered Alloy Steel

- A514-Structural Shapes and Plates
- A517-Boiler and Pressure Vessel Steel


### 6.0.1.1 AISC Shape Designations-Old and New

Aisc Hot-Rolled Structural Designations Steel Shape

| New Designation | Type of Shape | Old Designation |
| :--- | :--- | :--- |
| W $24 \times 76$ | W shape | 24 WF 76 |
| W14 $\times 26$ | W shape | 14 B 26 |
| S $24 \times 100$ | S shape | 241100 |
| M $8 \times 18.5$ | M shape | 8 M 18.5 |
| M $10 \times 9$ | M shape | 10 JR 9.0 |
| M $8 \times 34.3$ | M shape | $8 \times 8$ M 34.3 |
| C $12 \times 20.7$ | American Std. Channel | 12 C 20.7 |
| MC $12 \times 45$ | Miscellaneous Channel | $12 \times 4$ C 45.0 |
| MC $12 \times 10.6$ | Miscellaneous Channel | 12 JR C 10.6 |
| HP $14 \times 73$ | HP shape | 14 BP 73 |
|  |  |  |


| New Designation | Type of Shape | Old Designation |
| :---: | :---: | :---: |
| $\mathrm{L} 6 \times 6 \times 3 / 4$ | Equal Leg Angle | L $6 \times 6 \times 3 / 4$ |
| L6 $\times 4 \times 5 / 8$ | Unequal Leg Angle | L $6 \times 4 \times 5 / 8$ |
| WT $12 \times 38$ | Structural Tee | ST 12 WF 38 |
| WT $7 \times 13$ | Cut from W shape | ST 7 B 13 |
| ST $12 \times 50$ | Cut from S shape | ST 12/50 |
| MT $4 \times 9.25$ | Cut from $M$ shape | ST 4 M 9.25 |
| MT $5 \times \mu 5$ | Cut from $M$ shape | ST 5 J $\mu 4.5$ |
| MT $4 \times 17.15$ | Cut from M shape | ST 4 M 17.15 |
| PL $1 / 2 \times 18$ | Plate | PL 18X7 ½ |
| Bar 1 | Square Bar | Bar 1 |
| Bar 1-1/4 O | Round Bar | Bar 1-1/4 O |
| Bar 2-1/2 $\times 1 / 2$ | Flat Bar | Bar $2-1 / 2 \times 1 / 2$ |
| Pipe 4 Std. | Pipe | Pipe 4 Std. |
| Pipe $4 \times$ Strong . | Pipe | Pipe $4 \times$-Strong |
| Pipe $4 \times \times$ Strong | Pipe | Pipe $4 \times \times$-Strong. |
| TS $4 \times 4 \times .375$ | Structural Tubing: Square | Tube $4 \times 4 \times .375$ |
| TS $5 \times 3 \times .375$ | Rectanqular | Tube $5 \times 3 \times .375$ |
| TS3 OD $\times .250$ | Circular | Tube 3 OD $\times .250$ |

### 6.0.2 ASTM Designations for Most Common Types of Steel in Construction

American Society for Testing and Materials (ASTM) specifications that are common in steel design and construction for materials, preparation, and testing are given below. Just click the name to see its scope and most current date of revision. The links below get you the scope statements from ASTM. You can get nearly every one of them, though, in AISC's 2001 Selected ASTM Standards for Structural Steel Fabrication.

- A6/A6M Standard Specification for General Requirements for Rolled Structural Steel Bars, Plates, Shapes, and Sheet Piling
- A27/A27M Standard Specification for Steel Castings, Carbon, for General Application
- A36/A36M Standard Specification for Carbon Structural Steel
- A53/A53M Standard Specification for Pipe, Steel, Black and Hot-Dipped, Zinc-Coated, Welded and Seamless
- A123/A123M Standard Specification for Zinc (Hot- Dip Galvanized) Coatings on Iron and Steel Products
- A148/A148M Standard Specification for Steel Castings, High Strength, for Structural Purposes
- A153/A153M Standard Specification for Zinc Coating (HotDip) on Iron and Steel Hardware
- A193/A193M Standard Specification for Alloy-Steel and Stainless Steel Bolting Materials for High- Temperature Service
- A194/A194M Standard Specification for Carbon and Alloy Steel Nuts for Bolts for High Pressure or High- Temperature Service, or Both
- A242/A242M Standard Specification for High- Strength Low-Alloy Structural Steel
- A276 Specification for Stainless Steel Bars and Shapes
- A283/A283M Standard Specification for Low and Intermediate Tensile Strength Carbon Steel Plates
- A307 Standard Specification for Carbon Steel Bolts and Studs, 60000 PSI Tensile Strength
- A325 Standard Specification for Structural Bolts, Steel, Heat Treated, 120/105 ksi Minimum Tensile Strength
- A325M Standard Specification for High- Strength Bolts for Structural Steel Joints (Metric)
- A354 Standard Specification for Quenched and Tempered Alloy Steel Bolts, Studs, and Other E×ternally Threaded Fasteners
- A370 Standard Test Methods and Definitions for Mechanical Testing of Steel Products
- A384 Standard Practice for Safeguarding Against Warpage and Distortion During Hot-Dip Galvanizing of Steel Assemblies
- A385 Standard Practice for Providing High-Quality Zinc Coatings (Hot-Dip)
- A435/A435M Standard Specification for Straight- Beam Ultrasonic Examination of Steel Plates
- A449 Standard Specification for Quenched and Tempered Steel Bolts and Studs
- A490 Standard Specification for Heat-Treated Steel Structural Bolts, 150 ksi Minimum Tensile Strength
- A490M Standard Specification for High- Strength Steel Bolts, Classes 10.9 and 10.9.3, for Structural Steel Joints (Metric)
- A500 Standard Specification for Cold-Formed Welded and Seamless Carbon Steel Structural Tubing in Rounds and Shapes
- A501 Standard Specification for Hot-Formed Welded and Seamless Carbon Steel Structural Tubing
- A514/A514M Standard Specification for High-Yield Strength, Quenched and Tempered Alloy Steel Plate, Suitable for Welding
- A529/A529M Standard Specification for High- Strength Carbon-Manganese Steel of Structural Quality
- A563 Standard Specification for Carbon and Alloy Steel Nuts
- A568/A568M Standard Specification for Steel, Sheet, Carbon, and High-Strength, Low-Alloy, Hot-Rolled and Cold-rolled
- A572/A572M Standard Specification for High-Strength LowAlloy Columbium-Vanadium Structural Steel
- A578/A578M Standard Specification for Straight- Beam Ultrasonic Examination of Plain and Clad Steel Plates for Special Applications
- A588/A588M Standard Specification for High- Strength Low-Alloy Structural Steel with 50 ksi [345MPa] Minimum Yield Point to 4 in . [100mm] Thick
- A606 Standard Specification $\overline{\text { for }}$ Steel, Sheet and Strip, High-Strength, Low-Alloy, Hot-Rolled and Cold-Rolled, with Improved Atmospheric Corrosion Resistance
- A618 Standard Specification for Hot-Formed Welded and Seamless High-Strength LowAlloy Structural Tubing
- A666 Standard Specification for Annealed or Cold- Worked Austenitic Stainless Steel Sheet, Strip, Plate, and Flat Bar
- A673/A673M Standard Specification for Sampling Procedure for Impact Testing of Structural Steel
- A706/A706M Standard Specification for Low-Alloy Steel Deformed and Plain Bars for Concrete Reinforcement
- A709/A709M Standard Specification for Carbon and HighStrength Low-Alloy Structural Steel Shapes, Plates, and Bars and Quenched-and- Tempered Alloy Structural Steel Plates for Bridges
- A759 Standard Specification for Carbon Steel Crane Rails
- A770/A770M Standard Specification for Through- Thickness Tension Testing of Steel Plates for Special Applications
- A780 Ptandard Sractice for Repair of Damaged and Uncoated Areas of Hot-Dip Galvanized Coatings
- A786/A786M Standard Specification for Rolled Steel Floor Plates
- A847 Standard Specification $\overline{\text { for Cold-Formed Welded and }}$ Seamless High-Strength, LowAlloy Structural Tubing with Improved Atmospheric Corrosion Resistance
- A852/A852M Standard Specification for Quenched and Tempered Low-Alloy Structural Steel Plate with 70 ksi [485 MPa] Minimum Yield Strength to 4 in. [100 mm] Thick
- A913/A913M Standard Specification for High- Strength LowAlloy Steel Shapes of Structural Quality, Produced by Quenching and Self-Tempering Process (QST)
- A931 Standard Test Method for Tension Testing of Wire Ropes and Strand
- A941 Standard Terminology Relating to Steel, Stainless Steel, Related Alloys, and Ferroalloys
- A949/A949M Standard Specification for Spray- Formed Seamless Ferritic/Austenitic Stainless Steel Pipe
- A992/A992M Standard Specification for Steel for Structural Shapes for Use in Building Framing
- A1011/A1011M Standard Specification for Steel, Sheet and Strip, Hot-Rolled, Carbon, Structural, High- Strength Low-Alloy and High-Strength Low-Alloy with Improved Formability
- B695 Standard Specification for Coatings of Zinc Mechanically Deposited on Iron and Steel
- D3359 Standard Test Methods for Measuring Adhesion by Tape Test
- D4541 Standard Test Method for Pull-Off Strength of

Coatings Using Portable Adhesion Testers

- D4752 Standard Test Method for Measuring MEK Resistance of Ethyl Silicate (Inorganic) Zinc-Rich Primers by Solvent Rub
- D5402 Standard Practice for Assessing the Solvent Resistance of Organic Coatings Using Solvent Rubs
- E94 Standard Guide for Radiographic Examination
- El65 Standard Test Method for Liquid Penetrant Examination
- E709 Standard Guide for Magnetic Particle Examination
- El032 Standard Test Method for Radiographic Examination of Weldments
- F436 Standard Specification for Hardened Steel Washers
- F606 Standard Test Methods for Determining the Mechanical Properties of Externally and Internally Threaded Fasteners, Washers, and Rivets
- F959 Standard Specification for Compressible-Washer- Type Direct Tension Indicators for Use with Structural Fasteners
- F1554 Standard Specification for Anchor Bolts, Steel, 36, 55, and 105 ksi Yield Strength
- F1852 Standard Specification for "Twist-Off" Type Tension Control Structural Bolt/Nut/ Washer Assemblies, Steel, Heat Treated, 120/105 ksi Minimum Tensile Strength
- Gl01 Standard Guide for Estimating the Atmospheric Corrosion Resistance of Low- Alloy Steels

Here are a few brand-new and newer ASTM specifications (and some discontinued and replaced ones, too).

- ASTM A992/A992M-The new 50 ksi steel for wide-flange shapes (only) that replaces ASTMA36, ASTM A572 grade 50 and the similar dual-certified products for wide-flange shapes (only). Read about this in three places: Are You Properly Specifying Materials? Part l-Structural Shapes from the January 1999 issue of AISC's Modern Steel Construction magazine, Steel Industry Embraces ASTM A292 from the April 1999 issue of AISC's Modern Steel Construction magazine and AISC Technical Bulletin No.3, which was AISC's announcement of ASTM A992 before it had an ASTM number. Also, note that 50 ksi W -shapes are now less expensive than 36 ksi W-shapes, as explained here.
- ASTM F1554 - The new ASTM specification for anchor rods (what used to be called anchor bolts). Read about this in Are You Properly Specifying Materials? Part 3-Fastening Products from the March 1999 issue of AISC's Modern Steel Construction magazine.
- ASTM F1852—The new ASTM specification for twist-off-type tension-control bolt assemblies that meet mechanical and chemical requirements similar to ASTM A325 high-strength bolts. Read about this in Are You Properly Specifying Materials? Part 3-Fastening Products from the March 1999 issue of AISC's Modern Steel Construction magazine.

And here are ASTM Specifications that have been discontinued or replaced:

- ASTM A687 was discontinued (1999) without replacement.
- ASTM A570/A570M was discontinued (2000) and replaced by ASTM A1011/A1011M.
- ASTM E142 was discontinued (2000) and replaced by ASTM E94.


## Home > Useful Information > Materials, Preparation, and Testing

Properly specify requirements for materials, preparation, and testing in your steel projects with the help in this feature. Here's a three-part article in AISC's Modern Steel Construction magazine that will help you to do so. Surely it's no coincidence that its title is "Are You Properly Specifying Materials?"

- Part 1—Structural Shapes (January 1999 issue)
- Part 2—Plate Products (February 1999 issue)
- Part 3-Fastening Products (March 1999 issue)

American Society for Testing and Materials (ASTM) specifications that are common in steel design and construction for materials, preparation, and testing are given below. Just click the name to see its scope and most current date of revision. The links below get you the scope statements from ASTM. You can get nearly every one of them, though, in AISC's 2001 Selected ASTM Standards Fabrication.

- A6/A6M Standard Specification for General Requirements for Rolled Structural Steel Bars, Plates, Shapes, and Sheet Piling
- A27/A27M Standard Specification for Steel Castings, Carbon, for General Application
- A36/A36M Standard Specification for Carbon Structural Steel
- A53/A53M Standard Specification for Pipe, Steel, Black and Hot-Dipped, Zinc-Coated, Welded and Seamless
- A123/A123M Standard Specification for Zinc (HotDip Galvanized) Coatings on Iron and Steel Products
- A148/A148M Standard Specification for Steel Castings, High Strength, for Structural Purposes
- Al53/A153M Standard Specification for Zinc Coating (Hot-Dip) on Iron and Steel Hardware
- A193/A193M Standard Specification for AlloySteel and Stainless Steel Bolting Materials for High-Temperature Service
- A194/A194M Standard Specification for Carbon and Alloy Steel Nuts for Bolts for High Pressure or High- Temperature Service, or Both
- A242/A242M Standard Specification for HighStrength Low-Alloy Structural Steel
- A276 Specification for Stainless Steel Bars and Shapes
- A283/A283M Standard Specification for Low and Intermediate Tensile Strength Carbon Steel Plates
- A307 Standard Specification for Carbon Steel Bolts and Studs, 60000 PSI Tensile Strength
- A325 Standard Specification for Structural Bolts, Steel, Heat Treated, 120/ 105 ksi Minimum Tensile Strength
- A325M Standard Specification for High-Strength Bolts for Structural Steel Joints (Metric)
- A354 Standard Specification for Quenched and Tempered Alloy Steel Bolts, Studs, and Other Externally Threaded Fasteners
- A501 Standard Specification for Hot-Formed Welded and Seamless Carbon Steel Structural Tubing
- A514/A514M Standard Specification for HighYield Strength, Quenched and Tempered Alloy Steel Plate, Suitable for Welding
- A529/A529M Standard Specification for HighStrength Carbon-Manganese Steel of Structural Quality
- A563 Standard Specification for Carbon and Alloy Steel Nuts
- A568/A568M Standard Specification for Steel, Sheet, Carbon, and HighStrength, Low-Alloy, HotRolled and Cold-rolled, General Requirements for steel
- A572/A572M Standard Specification for HighStrength Low-Alloy Colum-bium- Vanadium Structural Steel
- A578/A578M Standard Specification for StraightBeam Ultrasonic Examination of Plain and Clad Steel Plates for Special Applications
- A588/A588M Standard Specification for HighStrength Low-Alloy Structural Steel with 50 ksi [345MPa] Minimum Yield Point to 4 in. $[100 \mathrm{~mm}]$ Thick
- A606 Standard Specification for Steel, Sheet and Strip, High-Strength, LowAlloy, Hot-Rolled and Cold-Rolled, with Improved Atmospheric Corrosion Resistance
- A618 Standard Specification for Hot-Formed Welded and Seamless High-Strength Low-Alloy Structural Tubing
- A666 Standard Specification for Annealed or ColdWorked Austenitic Stainless Steel Sheet, Strip, Plate, and Flat Bar
- A673/A673M Standard Specification for Sampling Procedure for Impact Testing of Structural Steel
- A706/A1Q6M Standard Specification for LowAlloy Steel Deformed and Plain Bars for Concrete Reinforcement
- A709/A709M Standard Specification for Carbon and High-Strength Low-Alloy

Structural Steel Shapes, Plates, and Bars and Ferritic/ Austenitic Stainless Steel Pipe

- A852/A852M Standard Specification for Quenched and Tempered Low-Alloy Structural Steel Plate with 70 ksi [ 485 MPa ] Minimum Yield Strength to 4 in. [100 mm] Thick
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and Strip, Hot-Rolled, Carbon, Structural, HighStrength Low-Alloy and High-Strength Low-Alloy with Improved Formability
- B695 Standard Specification for Coatings of Zinc Mechanically Deposited on Iron and Steel
- D3359 Standard Test Methods for Measuring Adhesion by Tape Test
- D4541 Standard Test Method for Pull-Off Strength of Coatings Using Portable Adhesion Testers
- D4752 Standard Test Method for Measuring MEK Resistance of Ethyl Silicate (Inorganic) ZincRich Primers by Solvent Rub
- D5402 Standard Practice for Assessing the Solvent Resistance of Organic Coatings Using Solvent Rubs
- E94 Standard Guide for Radiographic Examination
- El65 Standard Test Method for Liquid Penetrant Examination
- E709 Standard Guide for Magnetic Particle Examination
- E1032 Standard Test Method for Radiographic Examination of Weldments

Here are a few brand-new and newer ASTM specifications (and some discontinued and replaced ones, too).

- ASTM A992/A992M - The new 50 ksi steel for wide-flange shapes (only) that replaces ASTM A36, ASTM A572 grade 50, and the similar dual-certified products for wide-flange shapes (only). Read about this in three places: Are You Properly Specifying Materials? Part 1 - Structural Shapes from the January 1999 issue of AISC's Modern Steel Construction magazine, Steel Industry Embraces ASTM A992 from the April 1999 issue of AISC's Modern Steel Construction magazine, and AISC Technical Bulletin No. 3, which was AISC's announcement of ASTM A992 before it had
an ASTM number. Also, note that 50 ksi W -shapes are now less expensive than 36 ksi W -shapes, as explained here.
- ASTM F1554 - The new ASTM specification for anchor rods (what used to be called "anchor bolts"). Read about this in Are You Properly Specifying Materials? Part 3 - Fastening Products from the March 1999 issue of AISC's Modern Steel Construction magazine.
- ASTM F1852 - The new ASTM specification for twist-off-type tension-control bolt assemblies that met mechanicals and chemical requirements similar to ASTM A325 high-strength bolts. Read about this in Are You Properly Specifying Materials? Part 3 - Fastening Products from the March 1999 issue of AISC's Modern Steel Construction magazine

And here are ASTM specifications that have been discontinued or replaced:

- ASTM A687 was discontinued (1999) without replacement.
- ASTM A570/A570M was discontinued (2000) and replaced by ASTM A1011/A1011M.
- ASTM E142 was discontinued (2000) and replaced by ASTM E94.
- A370 Standard Test Methods and Definitions for Mechanical Testing of Steel Products
- A384 Standard Practice for Safeguarding Against Warpage and Distortion During Hot-Dip Galvanizing of Steel Assemblies
- A385 Standard Practice for Providing High-Quality Zinc. Coatings (Hot-Dip)
- A435/A435M Standard Specification for StraightBeam Ultrasonic Examination of Steel Plates
- A449 Standard Specification for Quenched and Tempered Steel Bolts and Studs
- A490 Standard Specification for Heat-Treated Steel Structural Bolts, 150 ksi Minimum Tensile Strength
- A490M Standard Specification for High-Strength Steel Bolts, Classes 10.9 and 10.9.3, for Structural Steel Joints (Metric)
- A500 Standard Specification for Cold-Formed

Welded and Seamless Carbon Steel Structural Tubing in Rounds and Shapes Quenched-and-Tempered Alloy Structural Steel Plates for Bridges

- A759 Standard Specification for Carbon Steel Crane Rails
- A770/A770M Standard Specification for ThroughThickness Tension Testing of Steel Plates for Special Applications
- A780 Standard Practice for Repair of Damaged and Uncoated Areas for HotDip Galvanized Coatings
- A786/A786M Standard Specification for Rolled Steel Floor Plates
- A847 Standard Specification for Cold-Formed Welded and Seamless High-Strength, Low-Alloy Structural Tubing with Improved Atmospheric Corrosion Resistance
- F436 Standard Specification for Hardened Steel Washers
- F606 Standard Test Methods for Determining the Mechanical Properties of Externally and Internally Threaded Fasteners, Washers, and Rivets
- F959 Standard Specification for Compressible-Washer-Type Direct Tension Indicators for Use with Structural Fasteners
- F1554 Standard Specification for Anchor Bolts, Steel, 36, 55, and 105 ksi Yield Strength
- F1852 Standard Specification for "Twist-Off" type Tension Control Structural Bolt/Nut/Washer Asemblies, Steel, Heat Treated, 120/105 ksi Minimum Tensile Strength
- G101 Standard Guide for Estimating the Atmospheric Corrosion Resistance of Low-Alloy Steels

Can't find the specification you need in the above list? Give these materials web sites a try:

- ASTM - their standards search page.
- NSSN - "a national resource for global materials standards."
- Metal Basics - "digital solutions for marketing and sourcing metals."


### 6.0.3 Worldwide National Standards for Steel

Australia : AS standards
Austria : ONORM standards
Belgium : NBN standards
Bulgaria: BDS standards
Canada : CSA standards
China : GB standards
Czech/Slovak Republic : CSN standards
Finland : SFS standards
France : AFNOR standards
Great Britain : BS standards
Hungary : MSZ standards
Italy : UNI standards
Japan : JIS standards
Norway : NS standards
Poland PN standards
Romania : STAS standards
Russia : UNE standards
Spain : UNE standards
Sweden : SS standards
Switzerland : SNV/VSM standards
United States : ACI, AISI, AMS, ASME, ASTM, AWS, FED, MIL, SAE, UNS standards
European: Euronorm standards
Source: West Yorkshire Steel Ltd., Leeds, UK

### 6.0.3.1 Quick Review of U.S. Metric Conversions to Assist When Reviewing Steel Sizes

|  | $\mathbf{p s i}$ | $\mathbf{k s i}$ | $\mathbf{k s f}$ | $\mathbf{k P a}$ | atm | bar |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| $1 \mathrm{psi}=$ | 1 | 0.001 | 0.144 | 6.895 | 0.068 | 0.069 |
| $1 \mathrm{ksi}=$ | 1000 | 1 | 144.0 | 6894.757 | 68.045 | 68.948 |
| $1 \mathrm{ksf}=$ | 6.94 | 0.007 | 1 | 47.88 | 0.473 | 0.479 |
| $1 \mathrm{mPa}=$ | 145 | 0.145 | 20.885 | 1000 | 9.869 | 10 |
| $1 \mathrm{~atm}=$ | 14.696 | 0.015 | 2.116 | 101.325 | 1 | 1.013 |
| $1 \mathrm{bar}=$ | 14.504 | 0.015 | 2.089 | 100 | 0.987 | 1 |


| U.S. Customary Units | U.S. Units Equivalent | SI Units Equivalent |
| :---: | :---: | :---: |
| one square inch (sq in) | $=1 / 144 \mathrm{sq} \mathrm{ft}$ | $=645.16 \mathrm{~mm}^{2}$ |
| one square foot (sq ft) | $=144 \mathrm{sq}$ in $=1 / 9 \mathrm{sq} \mathrm{yd}$ | $=0.092903 \mathrm{~m}^{2}$ |
| one square yard (sq yd) | $=1296 \mathrm{sq} \mathrm{in}=9 \mathrm{sq} \mathrm{ft}$ | $=0.83613 \mathrm{~m}^{2}$ |
| one acre (ac) | $\begin{aligned} & =160 \mathrm{sq} \mathrm{rd}=43560 \mathrm{sq} \mathrm{ft} \\ & =1 / 640 \mathrm{sq} \mathrm{mi} \end{aligned}$ | $\begin{aligned} & =4046.85642 \mathrm{~m}^{2} \\ & =0.40469 \text { ha } \end{aligned}$ |
| one square rod (sq rd) | $=2721 / 4 \mathrm{sq} \mathrm{ft}$ | $=25.2929 \mathrm{~m}^{2}$ |
| one rood | $=40 \mathrm{sq} \mathrm{rd}=10890 \mathrm{sq} \mathrm{ft}$ | $=1011.7141 \mathrm{~m}^{2}$ |
| one square mile | $=640$ acres | $=2.58999 \mathrm{~km}^{2}$ |
| Metric Units | US units equivalent | SI units equivalent |
| one acre (a) | $=\sim 1076^{3} / 8 \mathrm{sq} \mathrm{ft}$ | $=100 \mathrm{~m}^{2}=0.01 \mathrm{ha}$ |
| one hectare (ha) | $=\sim 21 / 2$ acres | $=10000 \mathrm{~m}^{2}$ |
| one km ${ }^{2}$ | $=\sim 3 / 8 \mathrm{sq} \mathrm{mi}$ | $=1000000 \mathrm{~m}^{2}=100 \mathrm{ha}^{2}$ |

By permission: Structural-Drafting-Net-Expert.com

### 6.0.3.2 EN, DIN, JIS Standards

The term steel specification is very often closely related to and used interchangeably with standards, although their meaning is not really identical. Hence, German steel specifications often start with the letters DIN, Japanese with JIS international with ISO, and so on.

The most widely used standard steel specifications in the United States are those published by ASTM; these steel specifications represent a consensus drawn from producers, fabricators, and users of steel mill products. In many cases, the dimensions, tolerances, limits, and restrictions in the ASTM specifications are the same as the corresponding items of the standard practices in the AISI steel product manuals.

Many of the ASTM specifications have been adopted by the American Society of Mechanical Engineers (ASME) with slight or no modifications. ASME uses the prefix S with the ASTM specifications; for example, ASME SA 213 and ASTMA 213 are the same. SAE/AISI designations for the chemical compositions of carbon and alloy steels are sometimes included in the ASTM specifications for bars, wires, and billets for forging. Some ASTM specifications for sheet products incorporate SAE-AISI designations for chemical composition.

EN (Euronorm) is a harmonized system of European countries. Although it is accepted and effectively used in all European countries, "obsolete" national systems, such as German DIN, British BS, French AFNOR, and Italian UNI, can still often be found in many documents.

DIN standards are developed by Deutsches Institute für Normung in Germany. All German steel standards and specifications are represented by the letters DIN and followed by an alphanumeric or a numeric code. For example, DIN $40 \mathrm{NiCrMo66}$ or 1.6565 is a $\mathrm{Ni}-\mathrm{Cr}-\mathrm{Mo}$ steel that contains $0.35-0.45 \% \mathrm{C}, 0.9-1.4 \% \mathrm{Cr}$, $0.5-0.7 \% \mathrm{Mn}, 0.2-0.3 \% \mathrm{Mo}, 1.4-1.7 \% \mathrm{Ni}, 0.035 \%$ S; DIN 172001.1149 or DIN 17200 Cm 22 is a nonresulfurized carbon steel containing $0.17-0.245 \mathrm{C}, 0.3-0.6 \% \mathrm{Mn}, 0.02-0.035 \% \mathrm{~S}$ and $0.4 \% \mathrm{max} \mathrm{Si}$.

JIS standards are developed by the Japanese Industrial Standards Committee (JISC) in Tokyo. The specifications begin with the prefix JIS, followed by a letter G for carbon and low-alloy steels. Examples: JIS G3445

STKM11A is a low-carbon tube steel containing $0.12 \% \mathrm{C}, 0.35 \% \mathrm{Si}, 0.60 \% \mathrm{Mn}, 0.04 \% \mathrm{P}, 0.04 \% \mathrm{~S}$; JIS G4403 SKH2 (AISI T1Grade) is a tungsten high-speed tool steel containing $0.73-0.83 \% \mathrm{C}, 3.8-4.5 \% \mathrm{Cr}, 0.4 \% \mathrm{Mn}$, $0.4 \% \mathrm{Si}, 0.8-1.2 \% \mathrm{~V}$ and $17-19 \% \mathrm{~W}$.

The KEY to METALS database brings global metal specifications and properties together into one integrated and searchable database. Quick and easy access to the mechanical properties, chemical composition, cross-reference tables, and more provide users with an unprecedented wealth of information. Click the buttons below to learn more from the Guided Tour or to test drive the KEY to METALS database.

### 6.0.3.3 Tolerance on JIS Dimension and Shape of WF Beams

|  | Hot-rolled Wide Flange Shapes for Building Structure |  | Remarks |
| :---: | :---: | :---: | :---: |
|  | Range | Tolerance |  |
| Width (B) | $\begin{aligned} & B \leqq 400 \mathrm{~mm} \\ & 400 \mathrm{~mm}<B \end{aligned}$ | $\begin{aligned} & \pm 2.0 \mathrm{~mm} \\ & \pm 3.0 \mathrm{~mm} \end{aligned}$ |  |
| Depth (H) | $\begin{gathered} \mathrm{H}<800 \mathrm{~mm} \\ \mathrm{~B} \leqq 400 \mathrm{~mm} \\ 400 \mathrm{~mm}<\mathrm{B} \\ 800 \mathrm{~mm} \leqq \mathrm{H} \end{gathered}$ | $\begin{aligned} & \pm 2.0 \mathrm{~mm} \\ & \\ & \pm 3.0 \mathrm{~mm} \\ & \pm 3.0 \mathrm{~mm} \end{aligned}$ |  |
| Thickness $\begin{gathered}\text { Flange (tz)* } \\ \\ \\ \\ \text { Web (t1) }\end{gathered}$ | $\mathrm{t} 2<16 \mathrm{~mm}$ | $\pm 1.0 \mathrm{~mm}$ |  |
|  | $16 \mathrm{~mm} \leqq \mathrm{t} 2<25 \mathrm{~mm}$ | $\pm 1.5 \mathrm{~mm}$ |  |
|  | $25 \mathrm{~mm} \leqq \mathrm{t} 2<40 \mathrm{~mm}$ | $\pm 1.7 \mathrm{~mm}$ |  |
|  | $40 \mathrm{~mm} \leqq \mathrm{t} 2$ | $\pm 2.0 \mathrm{~mm}$ |  |
|  | $\mathrm{t} 1<16 \mathrm{~mm}$ | $\pm 0.7 \mathrm{~mm}$ |  |
|  | $16 \mathrm{~mm} \leqq \mathrm{t} 1<25 \mathrm{~mm}$ | $\pm 1.0 \mathrm{~mm}$ |  |
|  | $25 \mathrm{~mm} \leqq \mathrm{t} 1<40 \mathrm{~mm}$ | $\pm 1.5 \mathrm{~mm}$ |  |
|  | $40 \mathrm{~mm} \leqq \mathrm{t} 1$ | $\pm 2.0 \mathrm{~mm}$ |  |
| Length (L) | $\mathrm{L} \leqq 7 \mathrm{~m}$ | +40mm - 0mm |  |
|  | $7 \mathrm{~m}<\mathrm{L}$ | + tolerance increases 5 mm for the increment of every 1 m or fraction thereof. |  |
| Flange Out-ofsquareness (T) | $\mathrm{H} \leqq 300 \mathrm{~mm}$ | $\leqq B \times 0.01$ The minimum tolerance shall be 1.5 mm . |  |
|  | $300 \mathrm{~mm}<\mathrm{H}$ | $\leqq \mathrm{B} \times 0.012 \text { The }$ minimum tolerance shall be 1.5 mm . |  |
| Bend | $\mathrm{H} \leqq 300 \mathrm{~mm}$ | $\leqq \mathrm{L} \times 0.0015$ | Applies to both vertical and horizontal deviations |
|  | $300 \mathrm{~mm}<\mathrm{H}$ | $\leqq \mathrm{L} \times 0.001$ |  |


|  | Hot-rolled Wide Flange Shapes for Building Structure |  |  |
| :---: | :---: | :---: | :---: |
|  | Range | Tolerance | Remarks |
| Web off Center (S) | B $\leqq 400 \mathrm{~mm}$ | $\pm 2.0 \mathrm{~mm}$ | $S=\frac{b_{1}-b_{2}}{2}$ |
|  | 400 mm < B | $\pm 3.5 \mathrm{~mm}$ |  |
| Comber of Web ( $\delta$ | $\mathrm{H} \leqq 350 \mathrm{~mm}$ | $\leqq 2.0$ |  |
|  | $350 \mathrm{~mm} \leqq \mathrm{H}<550 \mathrm{~mm}$ | $\leqq 2.5$ | $\square$ |
|  | $550 \mathrm{~mm} \leqq \mathrm{H}$ | $\leqq 3.0$ | $\because$ |
| Ends Out-of-square (e) | - | $\leqq B$ or $\mathrm{H} \times 0.016$ The minimum tolerance shall be 3.0 mm . |  |
| Out-of-squareness (t) | $B \leqq 400 \mathrm{~mm}$ | $\leqq \mathrm{b} \times 0.015$ The maximum tolerance shall be 1.5 mm |  |

*For the hot-rolled wide-flange shapes of JIS G3136, the following table should be used.

| $6 \mathrm{~mm} \leqq \mathrm{t} 2<16 \mathrm{~mm}$ | $+1.7 \mathrm{~mm}-0.3 \mathrm{~mm}$ |
| :---: | :---: |
| $16 \mathrm{~mm} \leqq \mathrm{t} 2<40 \mathrm{~mm}$ | $+2.3 \mathrm{~mm}-0.7 \mathrm{~mm}$ |
| $40 \mathrm{~mm} \leqq \mathrm{t} 2 \leqq 100 \mathrm{~mm}$ | $+2.5 \mathrm{~mm}-1.5 \mathrm{~mm}$ |

### 6.1.0 Approximate Minimum Mechanical Properties of Some Steels

Hot Rolled—Cold Drawn—Annealed—Quenched \& Tempered at $1000^{\circ} \mathrm{F}$.
The following table represents an average of results obtained from a large number of tests and is offered only as a guide in accordance with standard procedure; the specimens used were $1^{\prime \prime}$ diameter. Under no condition do we guarantee these statistics to be accurate.

The section size, finishing temperature, and cooling rate during the rolling process influence the final mechanical properties of any steel in the As-Rolled condition. The amount of size reduction in cold drawing will affect the

| Approximate Mechanical Properties <br> (Tensile and Yield Expressed in Thousands of Pounds Per Square Inch) |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| AISI GRADE | Condition | Machinability |  | Strength |  | Ductility |  | Hardness |  |
|  |  | 1212 equals $100 \%$ | S.F.M. | T.S. | Y.P. | EI 2"\% | RA | Brnl. | Rock-well |
| 1018 | H.R. | 52 | 86 | 58 | 32 | 25 | 50 | 116 | 68B |
| 1018 | C.D. | 65 | 107 | 64 | 54 | 15 | 40 | 126 | 72B |
| M1020 | H.R. | 50 | 83 | 55 | 30 | 25 | 50 | 110 | 66B |
| 1035 | H.R. | 65 | 107 | 70 | 30 | 20 | 35 | 155 | 83B |
| 1035 | C.D. | 67 | 111 | 90 | 75 | 10 | 40 | 170 | 87B |
| 1035 | Q\&T |  |  | 95 | 70 | 19 | 55 | 91 | 92B |
| 1042 | H.R. | 61 | 101 | 80 | 50 | 15 | 35 | 175 | 88B |
| 1042 | C.D. | 63 | 104 | 90 | 75 | 12 | 30 | 185 | 91B |
| 1042 | Q\&T |  |  | 105 | 80 | 15 | 40 | 215 | 96B |
| M1044 | H.R. | 53 | 87 | 82 | 49 | 15 | 30 | 170 | 87B |
| 1045 | H.R. | 56 | 92 | 85 | 50 | 15 | 30 | 175 | 88B |
| 1045 | C.D. | 60 | 99 | 90 | 80 | 10 | 30 | 195 | 93B |
| 1095 | HRA | 45 | 74 | 90 | 55 | 15 | 40 | 190 | 90B |
| 1117 | H.R. | 85 | 140 | 60 | 35 | 20 | 45 | 115 | 68B |
| 1117 | C.D. | 90 | 149 | 75 | 60 | 15 | 40 | 143 | 79B |
| 11 L 17 | H.R. | 92 | 152 | 60 | 35 | 20 | 45 | 115 | 89B |
| 11 L 17 | C.D. | 100 | 165 | 75 | 60 | 15 | 40 | 143 | 93B |
| 1137 | H.R. | 70 | 116 | 85 | 50 | 18 | 35 | 179 | 24C |
| 1137 | C.D. | 75 | 121 | 100 | 85 | 10 | 30 | 197 | 89B |
| 1137 | Q\&T |  |  | 110 | 85 | 15 | 40 | 250 | 93B |
| 1141 | H.R. | 65 | 107 | 90 | 60 | 15 | 25 | 180 | 28C |
| 1141 | C.D. | 70 | 116 | 100 | 85 | 8 | 20 | 195 | 94B |
| 1141 | Q\&T |  |  | 120 | 100 | 10 | 35 | 270 | 96B |
| 1144 | H.R. | 75 | 124 | 95 | 60 | 15 | 30 | 200 | 30C |
| 1144 | C.D. | 85 | 110 | 100 | 90 | 7 | 20 | 210 | 87B |
| 1144 | Q\&T |  |  | 130 | 110 | 15 | 45 | 286 | 87B |
| 1212 | C.D. | 100 | 165 | 80 | 70 | 10 | 40 | 170 | 87B |
| 1213 | C.D. | 150 | 248 | 80 | 70 | 10 | 40 | 170 | 81B |
| B1113 | C.D. | 150 | 248 | 80 | 70 | 10 | 40 | 170 |  |
| 12L14 | C.D. | 170 | 281 | 60 | 55 | 12 | 40 | 150 | 81B |
| (Type A Leaded) |  |  |  |  |  |  |  |  |  |
| 12L14 | C.D. | 215 | 355 | 60 | 55 | 12 | 40 | 150 | 81B |
| Selenium Treated |  |  |  |  |  |  |  |  |  |
| 1215 | C.D. | 150 | 248 | 80 | 70 | 10 | 40 | 170 | 87B |
| Jalcase 100 | C.D. | 80 | 132 | 120 | 100 | 10 | 25 | 248 | 24C |
| Jalcase 100L | C.D. | 98 | 162 | 120 | 100 | 10 | 25 | 248 | 24 C |
| (Leaded) |  |  |  |  |  |  |  |  |  |
| 4142 | H.R.A. | 56 | 92 | 85 | 55 | 20 | 45 | 170 | 87B |
| 4142 | C.D.A. | 65 | 107 | 100 | 85 | 12 | 40 | 196 | 93B |
| 4142 | Q\&T |  |  | 150 | 130 | 15 | 45 | 300 | 32C |
| 4147-50 | H.R.A. | 52 | 86 | 90 | 65 | 20 | 50 | 185 | 92B |
| 4147-50 | Q\&T |  |  | 170 | 145 | 15 | 50 | 350 | 37 C |


| Approximate Mechanical Properties-Cont'd <br> (Tensile and Yield Expressed in Thousands of Pounds Per Square Inch) |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Machinability |  | Strength |  | Ductility |  | Hardness |  |
| AISI GRADE | Condition | 1212 equals $100 \%$ | S.F.M. | T.S. | Y.P. | El $\mathbf{2}^{\prime \prime} \%$ | RA | Brnl. | Rock-well |
| 4340 | H.R.A. | 45 | 74 | 100 | 70 | 15 | 10 | 220 | 20C |
| 4340 | Q\&T |  |  | 175 | 155 | 12 | 18 | 370 | 38C |
| 8620 | H.R. | 60 | 99 | 80 | 55 | 18 | 15 | 160 | 84B |
| 8620 | Q\&T |  |  | 130 | 92 | 25 | 55 | 218 | 24C |
| 8620 | C.D. | 63 | 104 | 90 | 70 | 15 | 10 | 185 | 90B |

mechanical properties of Cold Drawn Bars. Turned and Polished as well as Turned, Ground, and Polished Bars have approximately the same mechanical properties as the Hot-Rolled Bars from which they were produced.

### 6.2.0 Common Structural Shapes for U.S. Steel Sections



The beams are known by their profile meaning:

- The length of the beam
- The shape of the cross section
- The material used

The most commonly found steel beam is the I beam, or the wide-flanged beam, also known by the name of universal beam or stouter sections as the universal column. Such beams are commonly used in the construction of bridges and steel frame buildings.

## Types of Beams

The most commonly found types of steel beams are varied, and they are as follows:

- I beams
- Wide-flange beams
- HP shape beams
- Special shape nonstandard beams
- H beams
- Junior beams


## Typical Characteristics of Beams

Beams experience tensile, sheer, and compressive stresses internally due to the loads applied to them. Generally, in cinder gravity loads there is a slight reduction in the original length of the beam. This results in a smaller radius arc enclosure at the top of the beam, thus showing compression, while the same beam at the bottom is slightly stretched, enclosing a larger radius arc due to tension. The length of the beam midway and at the bends is the same as it is not under tension or compression and is defined as the neutral axis. The beam is completely exposed to shear stress above the support. There are some reinforced concrete beams that are completely under compression. These beams are called prestressed concrete beams and are built in such a manner as to produce a compression more than the expected tension under loading conditions.

## Steel Channels, Stainless Steel Channels,...

- J channels: This kind of channel has two legs and a web. One leg is longer. This channel resembles the letter J.
- Hat channels: This channel has legs that are folded in the outward direction resembling an old-fashioned man's hat.
- U channels: This is the most common and basic channel variety. It has a base known as a web and two equal-length legs.
- C channels: In this channel the legs are folded back in the channel and resembles the letter C. C channels are known as rests.
- Hemmed channels: In this kind of channel the top of the leg is folded, hence forming double thickness.

There are other variations of channels that are available, which are customized according to the customer's needs.

## Application

Steel channels are subjected to a wide array of applications. The application fields are:

- Construction
- Appliances
- Transportation
- Used in making signposts
- Used in wood flooring for athletic purposes
- Used in installing and making windows and doors

A major variant of the channel is the mild steel channel. Such channels are generally used in heavy industries. They are used in the heavy machinery industry and the automotive industry too. The mild steel channel is divided into major variants, namely:

- Lipped channels: The letters "LL" denote the Lipped channels. In the diagram, the number following the letters, the nominal web dimension of the channel is indicated by the first three digits. The measurements are in millimeters.

- Plain channels: Such channels are represented by the letters "LL". The numbers after the letters denote the web dimensions of the channel measured in millimeters. The thickness of the material is denoted by the last two digits and is measured in the tenth of millimeter.


The steel angle finds an application in a number of things, such as the following:

- Used in framing
- Used in trims
- For reinforcement
- In brackets
- Used in transmission towers
- Bridges
- Lifting and transporting machinery
- Reactors
- Vessels
- Warehouses
- Industrial boilers
- Structural steel angles are used in rolling shutters for fabricating guides for strength and durability.



### 6.2.1 How Steel Wide-Flange Beams Are Identified

## Steel Wide-Flange I Beams



The I Beams are identified by:
W DEPTH (inches) $\times$ WEIGHT PER UNIT LENGTH (pound force per foot)
For example: W27 $\times \mathbf{1 6 1}$ is an I beam with a depth of 27 inches and a nominal weight per foot of $161 \mathrm{lbf} / \mathrm{ft}$.

### 6.2.2 How Steel Channels Are Identified

## American Standard Steel Channels



The channels are identified by:
C DEPTH (inches) $\times$ WEIGHT PER UNIT LENGTH (pound force per foot)
For example: $\mathbf{C 1 2} \times \mathbf{3 0}$ is a channel with a depth of 12 inches and a nominal weight per foot of $30 \mathrm{lbf} / \mathrm{ft}$.

### 6.2.3 How Steel Angles Are Identified

## Steel Angles



The angles are identified by:
LLEG $_{\mathrm{a}}$ inches $\times$ LEG $_{\mathrm{b}}$ inches $\times$ THICKNESS inches
For example, $\mathbf{L 4} \times \mathbf{3} \times \frac{5}{\mathbf{5}}$ is an angle with one 4-inch leg and one 3-inch leg and having a nominal weight per foot of $161 \mathrm{lbf} / \mathrm{ft}$.

### 6.2.4 Cross Sections of Standard Structural Steel Members



STEEL W TYPE I-BEAMS




ALUMINUM I-BEAMS

## Common Cross Sections




SQUARE CHANNEL

TAPERED CHANNEL


6.3.0 Calculating the U.S. Weight and Size of Wide-Flange Beams $-4^{\prime \prime} \times 4^{\prime \prime}$ to $36^{\prime \prime} \times 16^{\prime \prime}$

| W | WIDE-FLANGE BEAMS <br> Conforms to A-36 and A-572-Gr50 <br> Standard Lengths $40^{\prime}, 50^{\prime}$ and $60^{\prime}$ |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | A | B | C | D |  |  |  |
| Nominal Size in inches | Weight | Depth | Flange | Average Flange |  | Weight LBS. |  |  |
|  | Per <br> Foot | of <br> Section | Width Inches | Thickness Inches | Thickness Inches | $40^{\prime}$ | $50^{\prime}$ | $60^{\prime}$ |
| $4 \times 4$ | 13 | 4.16 | 4.060 | . 375 | . 280 | 520 | 650 | 780 |
| $5 \times 5$ | 16 | 5.00 | 5.000 | . 360 | . 240 | 640 | 800 | 960 |
| $5 \times 5$ | 19 | 5.15 | 5.030 | . 430 | . 270 | 760 | 950 | 1140 |
| $6 \times 4$ | 9 | 5.90 | 3.940 | . 215 | . 170 | 360 | 450 | 540 |
| $6 \times 4$ | 12 | 6.00 | 4.000 | . 279 | . 230 | 480 | 600 | 720 |
| $6 \times 4$ | 16 | 6.25 | 4.030 | . 404 | . 260 | 640 | 800 | 960 |
| $6 \times 6$ | 15 | 5.99 | 5.990 | . 260 | . 230 | 600 | 750 | 900 |
| $6 \times 6$ | 20 | 6.20 | 6.018 | . 367 | . 258 | 800 | 1000 | 1200 |
| $6 \times 6$ | 25 | 6.37 | 6.080 | . 456 | . 320 | 1000 | 1250 | 1500 |
| $8 \times 4$ | 10 | 7.90 | 3.940 | . 204 | . 170 | 400 | 500 | 600 |
| $8 \times 4$ | 13 | 8.00 | 4.000 | . 254 | . 230 | 520 | 650 | 780 |
| $8 \times 4$ | 15 | 8.12 | 4.015 | . 314 | . 245 | 600 | 750 | 900 |
| $8 \times 51 / 2$ | 18 | 8.14 | 5.250 | . 330 | . 230 | 720 | 900 | 1080 |
| $8 \times 5^{1 / 2}$ | 21 | 8.28 | 5.270 | . 400 | . 250 | 840 | 1050 | 1260 |
| $8 \times 6^{1 / 2}$ | 24 | 7.93 | 6.500 | . 398 | . 245 | 960 | 1200 | 1440 |
| $8 \times 6^{1 / 2}$ | 28 | 8.06 | 6.540 | . 4.63 | . 285 | 1120 | 1400 | 1680 |
| $8 \times 8$ | 31 | 8.00 | 8.000 | . 433 | . 288 | 1240 | 1550 | 1860 |
| $8 \times 8$ | 35 | 8.12 | 8.027 | . 493 | . 315 | 1400 | 1750 | 2100 |
| H | 40 | 8.00 | 8.083 | . 521 | . 458 | 1600 | 2000 | 2400 |
| $8 \times 8$ | 40 | 8.25 | 8.077 | . 558 | . 365 | 1600 | 2000 | 2400 |
| $8 \times 8$ | 48 | 8.50 | 8.117 | . 683 | . 405 | 1920 | 2400 | 2880 |
| $8 \times 8$ | 58 | 8.75 | 8.222 | . 808 | . 510 | 2320 | 2900 | 3480 |
| $8 \times 8$ | 67 | 9.00 | 8.287 | . 933 | . 575 | 2680 | 3350 | 4020 |
| $10 \times 4$ | 12 | 9.87 | 3.960 | . 210 | . 190 | 480 | 600 | 720 |
| $10 \times 4$ | 15 | 10.00 | 4.000 | . 269 | . 230 | 600 | 750 | 900 |
| $10 \times 4$ | 17 | 10.12 | 4.010 | . 329 | . 240 | 680 | 850 | 1020 |
| $10 \times 4$ | 19 | 10.25 | 4.020 | . 394 | . 250 | 760 | 950 | 1140 |
| $10 \times 53 / 4$ | 22 | 10.17 | 5.75 | . 360 | . 240 | 880 | 1100 | 1320 |
| $10 \times 53 / 4$ | 26 | 10.33 | 5.750 | . 360 | . 240 | 1040 | 1300 | 1560 |
| $10 \times 53 / 4$ | 30 | 10.47 | 5.810 | . 510 | . 300 | 1200 | 1500 | 1800 |
| $10 \times 8$ | 33 | 9.75 | 7.964 | . 433 | . 292 | 1320 | 1650 | 1980 |
| $10 \times 8$ | 39 | 9.94 | 7.990 | . 528 | . 318 | 1560 | 1950 | 2340 |
| $10 \times 8$ | 45 | 10.12 | 8.022 | . 618 | . 350 | 1800 | 2250 | 2700 |
| $10 \times 10$ | 49 | 10.00 | 10.000 | . 558 | . 340 | 1960 | 2450 | 2940 |
| $10 \times 10$ | 54 | 10.12 | 10.028 | . 618 | . 368 | 2160 | 2700 | 3240 |
| $10 \times 10$ | 60 | 10.25 | 10.075 | . 683 | . 415 | 2400 | 3000 | 3600 |
| $10 \times 10$ | 68 | 10.40 | 10.130 | . 770 | . 470 | 2720 | 3400 | 4080 |
| $10 \times 10$ | 77 | 10.62 | 10.195 | . 868 | . 535 | 3080 | 3850 | 4620 |
| $10 \times 10$ | 88 | 10.84 | 10.265 | . 990 | . 605 | 3520 | 4400 | 5280 |


| W | WIDE-FLANGE BEAMS-Cont'd |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Standard Lengths $40^{\prime}, 50^{\prime}$ and $60^{\prime}$ |  |  |  |  |  |  |  |
|  |  | A | B | C | D |  |  |  |
| Nominal Size in inches | Weight | Depth | Flange | Average Flange | Web | Weight LBS. |  |  |
|  | Foot | Section | Width Inches | Thickness <br> Inches | Thickness Inches | $40^{\prime}$ | $50^{\prime}$ | $60^{\prime}$ |
| $10 \times 10$ | 100 | 11.12 | 10.345 | 1.118 | . 685 | 4000 | 5000 | 6000 |
| $10 \times 10$ | 112 | 11.38 | 10.415 | 1.248 | . 755 | 4480 | 5600 | 6720 |
| $12 \times 4$ | 14 | 11.91 | 3.970 | . 224 | . 200 | 560 | 700 | 840 |
| $12 \times 4$ | 16 | 11.99 | 3.99 | . 265 | . 220 | 640 | 800 | 960 |
| $12 \times 4$ | 19 | 12.16 | 4.010 | . 349 | . 240 | 760 | 950 | 1140 |
| $12 \times 4$ | 22 | 12.31 | 4.030 | . 424 | . 260 | 880 | 1100 | 1320 |
| $12 \times 61 / 2$ | 26 | 12.22 | 6.490 | . 380 | . 230 | 1040 | 1300 | 1560 |
| $12 \times 61 / 2$ | 30 | 12.34 | 6.520 | . 440 | . 260 | 1200 | 1500 | 1800 |
| $12 \times 61 / 2$ | 35 | 12.50 | 6.560 | . 520 | . 300 | 1400 | 1750 | 2100 |
| $12 \times 8$ | 40 | 11.94 | 8.000 | . 516 | . 294 | 1600 | 2000 | 2400 |
| $12 \times 8$ | 45 | 12.06 | 8.042 | . 576 | . 336 | 1800 | 2250 | 2700 |
| $12 \times 8$ | 50 | 12.19 | 8.077 | . 640 | . 371 | 2000 | 2500 | 3000 |
| $12 \times 10$ | 53 | 12.06 | 10.000 | 5.76 | . 345 | 2120 | 2650 | 3180 |
| $12 \times 10$ | 58 | 12.19 | 10.014 | . 641 | . 359 | 2320 | 2900 | 3480 |
| $12 \times 12$ | 65 | 12.12 | 12.000 | . 606 | . 390 | 2600 | 3250 | 3900 |
| $12 \times 12$ | 72 | 12.25 | 12.040 | . 671 | . 430 | 2880 | 3600 | 4320 |
| $12 \times 12$ | 79 | 12.38 | 12.080 | . 736 | . 470 | 3160 | 3950 | 4740 |
| $12 \times 12$ | 87 | 12.53 | 12.125 | . 810 | . 515 | 3480 | 4350 | 5220 |
| $12 \times 12$ | 96 | 12.71 | 12.160 | . 900 | . 550 | 3840 | 4800 | 5760 |
| $12 \times 12$ | 106 | 12.88 | 12.230 | . 986 | . 620 | 4240 | 5300 | 6360 |
| $12 \times 12$ | 120 | 13.12 | 12.320 | 1.106 | . 710 | 4800 | 6000 | 7200 |
| $12 \times 12$ | 136 | 13.41 | 12.400 | 1.250 | . 790 | 5440 | 6800 | 8160 |
| $12 \times 12$ | 152 | 13.71 | 12.480 | 1.400 | . 870 | 6080 | 7600 | 9120 |
| $12 \times 12$ | 170 | 14.03 | 12.570 | 1.560 | . 960 | 6800 | 8500 | 10200 |
| $12 \times 12$ | 190 | 14.38 | 12.670 | 1.736 | 1.060 | 7600 | 9500 | 11400 |
| $14 \times 5$ | 22 | 13.72 | 5.000 | . 335 | . 230 | 880 | 1100 | 1320 |
| $14 \times 5$ | 26 | 13.89 | 5.025 | . 418 | . 255 | 1040 | 1300 | 1560 |
| $14 \times 63 / 4$ | 30 | 13.86 | 6.733 | . 383 | . 270 | 1200 | 1500 | 1800 |
| $14 \times 63 / 4$ | 34 | 14.00 | 6.750 | . 453 | . 287 | 1360 | 1700 | 2040 |
| $14 \times 63 / 4$ | 38 | 14.12 | 6.776 | . 513 | . 313 | 1520 | 1900 | 2280 |
| $14 \times 8$ | 43 | 13.68 | 8.000 | . 528 | . 308 | 1720 | 2150 | 2580 |
| $14 \times 8$ | 48 | 13.81 | 8.031 | . 593 | . 339 | 1920 | 2400 | 2880 |
| $14 \times 8$ | 53 | 13.94 | 8.062 | . 658 | . 370 | 2120 | 2650 | 3180 |
| $14 \times 10$ | 61 | 13.91 | 10.000 | . 643 | . 378 | 2440 | 3050 | 3660 |
| $14 \times 10$ | 68 | 14.06 | 10.040 | . 718 | . 418 | 2720 | 3400 | 4080 |
| $14 \times 10$ | 74 | 14.19 | 10.072 | . 783 | . 450 | 2960 | 3700 | 4440 |
| $14 \times 10$ | 82 | 14.31 | 10.130 | . 855 | . 510 | 3280 | 4100 | 4920 |
| $14 \times 141 / 2$ | 90 | 14.02 | 14.520 | . 710 | . 440 | 3600 | 4500 | 5400 |
| $14 \times 141 / 2$ | 99 | 14.16 | 14.565 | . 780 | . 485 | 3960 | 4950 | 5940 |
| $14 \times 141 / 2$ | 109 | 14.32 | 14.6050 | . 860 | . 525 | 4360 | 5450 | 6540 |
| $14 \times 141 / 2$ | 120 | 14.48 | 14.670 | . 940 | . 590 | 4800 | 6000 | 7200 |
| $14 \times 141 / 2$ | 132 | 14.66 | 14.725 | 1.030 | . 645 | 5280 | 6600 | 7920 |
| $14 \times 16$ | 145 | 14.78 | 15.500 | 1.090 | . 680 | 5800 | 7250 | 8700 |
| $14 \times 16$ | 159 | 14.98 | 15.565 | 1.190 | . 745 | 6360 | 7950 | 9540 |
| $14 \times 16$ | 176 | 15.25 | 15.640 | 1.313 | . 820 | 7040 | 8800 | 10560 |

(Continued)

| W | WIDE-FLANGE BEAMS-Cont'd |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Conforms to A-36 and A-572-Gr50 Standard Lengths $40^{\prime}, \mathbf{5 0}^{\prime}$ and $60^{\prime}$ |  |  |  |  |  |  |  |
|  |  | A | B | C | D |  |  |  |
| Nominal Size in inches | Weight | Depth | Flange | Average Flange | Web | Weight LBS. |  |  |
|  | Foot | Section | Inches | Thickness <br> Inches | Inches | $40^{\prime}$ | $50^{\prime}$ | $60^{\prime}$ |
| $14 \times 16$ | 193 | 15.50 | 15.710 | 1.438 | . 890 | 7720 | 9650 | 11580 |
| $14 \times 16$ | 211 | 15.75 | 15.800 | 1.563 | . 980 | 8440 | 10550 | 12660 |
| $14 \times 16$ | 233 | 16.04 | 15.890 | 1.720 | 1.070 | 9320 | 11650 | 13980 |
| $14 \times 16$ | 257 | 16.38 | 15.995 | 1.890 | 1.175 | 10280 | 12850 | 15420 |
| $14 \times 16$ | 283 | 16.74 | 16.110 | 2.070 | 1.290 | 11320 | 14150 | 16980 |
| $14 \times 16$ | 311 | 17.12 | 16.230 | 2.260 | 1.410 | 12440 | 15550 | 18660 |
| $14 \times 16$ | 342 | 17.56 | 16.365 | 2.468 | 1.545 | 13680 | 17100 | 20520 |
| $14 \times 16$ | 370 | 17.94 | 16.4575 | 2.658 | 1.655 | 14800 | 18500 | 22200 |
| $14 \times 16$ | 398 | 18.31 | 16.590 | 2.843 | 1.770 | 15920 | 19900 | 23880 |
| $14 \times 16$ | 426 | 18.69 | 16.695 | 3.033 | 1.875 | 17040 | 21300 | 25560 |
| $14 \times 16$ | 455 | 19.05 | 16.828 | 3.213 | 2.008 | 18200 | 22750 | 27300 |
| $14 \times 16$ | 500 | 19.63 | 17.008 | 3.501 | 2.188 | 20000 | 25000 | 30000 |
| $14 \times 16$ | 550 | 20.26 | 17.206 | 3.818 | 2.386 | 22000 | 27500 | 33000 |
| $14 \times 16$ | 605 | 20.94 | 17.418 | 4.157 | 2.598 | 24200 | 30250 | 36300 |
| $14 \times 16$ | 665 | 21.67 | 17.646 | 4.522 | 2.826 | 26600 | 33250 | 39900 |
| $14 \times 16$ | 730 | 22.44 | 17.889 | 4.910 | 3.069 | 29200 | 36500 | 43800 |
| $16 \times 51 / 2$ | 26 | 15.65 | 5.500 | . 345 | . 250 | 1040 | 1300 | 1560 |
| $16 \times 51 / 2$ | 31 | 15.84 | 5.525 | . 442 | . 275 | 1240 | 1550 | 1860 |
| $16 \times 7$ | 36 | 15.85 | 6.992 | . 428 | . 299 | 1440 | 1800 | 2160 |
| $16 \times 7$ | 40 | 16.00 | 7.000 | . 503 | . 307 | 1600 | 2000 | 2400 |
| $16 \times 7$ | 45 | 16.12 | 7.039 | . 563 | . 346 | 1800 | 2250 | 2700 |
| $16 \times 7$ | 50 | 16.25 | 7.073 | . 628 | . 380 | 2000 | 2500 | 3000 |
| $16 \times 7$ | 57 | 16.43 | 7.120 | . 715 | . 430 | 2280 | 2850 | 3420 |
| $16 \times 101 / 4$ | 67 | 16.33 | 10.235 | . 665 | . 395 | 2680 | 3350 | 4020 |
| $16 \times 101 / 4$ | 77 | 16.52 | 10.295 | . 760 | . 455 | 3080 | 3850 | 4620 |
| $16 \times 101 / 4$ | 89 | 16.75 | 10.365 | . 875 | . 525 | 3560 | 4450 | 5340 |
| $16 \times 101 / 4$ | 100 | 16.97 | 10.425 | . 985 | . 585 | 4000 | 5000 | 6000 |
| $18 \times 6$ | 35 | 17.71 | 6.000 | . 429 | . 298 | 1400 | 1750 | 2100 |
| $18 \times 6$ | 40 | 17.90 | 6.018 | . 524 | . 316 | 1600 | 2000 | 2400 |
| $18 \times 6$ | 46 | 18.06 | 6.060 | . 605 | . 360 | 1840 | 2300 | 2760 |
| $18 \times 71 / 2$ | 50 | 18.00 | 7.500 | . 570 | . 358 | 2000 | 2500 | 3000 |
| $18 \times 71 / 2$ | 55 | 18.12 | 7.532 | . 630 | . 390 | 2200 | 2750 | 3300 |
| $18 \times 71 / 2$ | 60 | 18.25 | 7.558 | . 695 | . 416 | 2400 | 3000 | 600 |
| $18 \times 71 / 2$ | 65 | 18.35 | 7.590 | . 750 | . 450 | 2600 | 3250 | 3900 |
| $18 \times 71 / 2$ | 71 | 18.47 | 7.635 | . 810 | . 495 | 2840 | 3550 | 4260 |
| $18 \times 11$ | 76 | 18.21 | 11.035 | . 680 | . 425 | 3040 | 3800 | 4560 |
| $18 \times 11$ | 86 | 18.39 | 11.090 | . 770 | . 480 | 3440 | 4300 | 5160 |
| $18 \times 11$ | 97 | 18.59 | 11.145 | . 870 | . 535 | 3880 | 4850 | 5820 |
| $18 \times 11$ | 106 | 18.73 | 11.200 | . 940 | . 590 | 4240 | 5300 | 6360 |
| $18 \times 11$ | 119 | 18.97 | 11.265 | 1.060 | . 655 | 4760 | 5950 | 7140 |
| $21 \times 61 / 2$ | 44 | 20.66 | 6.500 | . 451 | . 348 | 1760 | 2200 | 2640 |
| $21 \times 61 / 2$ | 50 | 20.83 | 6.530 | . 535 | . 380 | 2000 | 2500 | 3000 |
| $21 \times 61 / 2$ | 57 | 21.06 | 6.555 | . 650 | . 405 | 2280 | 2850 | 3420 |
| $21 \times 81 / 4$ | 62 | 20.99 | 8.240 | . 615 | . 400 | 2480 | 3100 | 3720 |
| $21 \times 81 / 4$ | 68 | 21.13 | 8.270 | . 685 | . 430 | 2720 | 3400 | 4080 |
| $21 \times 81 / 4$ | 73 | 21.24 | 8.295 | . 740 | . 455 | 2920 | 3650 | 4380 |
| $21 \times 81 / 4$ | 83 | 21.43 | 8.355 | . 835 | . 515 | 3320 | 4150 | 4980 |


| W | WIDE-FLANGE BEAMS-Cont'd |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Standard Lengths $40^{\prime}, \mathbf{5 0}^{\prime}$ and $60^{\prime}$ |  |  |  |  |  |  |  |
|  |  | A | B | C | D |  |  |  |
| Nominal Size in inches | Weight | Depth | Flange | Average Flange |  | Weight LBS. |  |  |
|  | Foot | Section | Inches | Inches | Inches | $40^{\prime}$ | $50^{\prime}$ | $60^{\prime}$ |
| $21 \times 81 / 4$ | 93 | 21.62 | 8.420 | . 930 | . 580 | 3720 | 4650 | 5580 |
| $21 \times 121 / 4$ | 101 | 21.36 | 12.290 | . 800 | . 500 | 4040 | 5050 | 6060 |
| $14 \times 16$ | 605 | 20.94 | 17.418 | 4.157 | 2.598 | 24200 | 30250 | 36300 |
| $14 \times 16$ | 665 | 21.67 | 17.646 | 4.522 | 2.826 | 26600 | 33250 | 39900 |
| $14 \times 16$ | 730 | 22.44 | 17.889 | 4.910 | 3.069 | 29200 | 36500 | 43800 |
| $16 \times 51 / 2$ | 26 | 15.65 | 5.500 | . 345 | . 250 | 1040 | 1300 | 1560 |
| $16 \times 51 / 2$ | 31 | 15.84 | 5.525 | . 442 | . 275 | 1240 | 1550 | 1860 |
| $16 \times 7$ | 36 | 15.85 | 6.992 | . 428 | . 299 | 1440 | 1800 | 2160 |
| $16 \times 7$ | 40 | 16.00 | 7.000 | . 503 | . 307 | 1600 | 2000 | 2400 |
| $16 \times 7$ | 45 | 16.12 | 7.039 | . 563 | . 346 | 1800 | 2250 | 2700 |
| $16 \times 7$ | 50 | 16.25 | 7.073 | . 628 | . 380 | 2000 | 2500 | 3000 |
| $16 \times 7$ | 57 | 16.43 | 7.120 | . 715 | . 430 | 2280 | 2850 | 3420 |
| $16 \times 101 / 4$ | 67 | 16.33 | 10.235 | . 665 | . 395 | 2680 | 3350 | 4020 |
| $16 \times 101 / 4$ | 77 | 16.52 | 10.295 | . 760 | . 455 | 3080 | 3850 | 4620 |
| $16 \times 101 / 4$ | 89 | 16.75 | 10.365 | . 875 | . 525 | 3560 | 4450 | 5340 |
| $16 \times 101 / 4$ | 100 | 16.97 | 10.425 | . 985 | . 585 | 4000 | 5000 | 6000 |
| $18 \times 6$ | 35 | 17.71 | 6.000 | . 429 | . 298 | 1400 | 1750 | 2100 |
| $18 \times 6$ | 40 | 17.90 | 6.018 | . 524 | . 316 | 1600 | 2000 | 2400 |
| $18 \times 6$ | 46 | 18.06 | 6.060 | . 605 | . 360 | 1840 | 2300 | 2760 |
| $18 \times 71 / 2$ | 50 | 18.00 | 7.500 | . 570 | . 358 | 2000 | 2500 | 3000 |
| $18 \times 71 / 2$ | 55 | 18.12 | 7.532 | . 630 | . 390 | 2200 | 2750 | 3300 |
| $18 \times 71 / 2$ | 60 | 18.25 | 7.558 | . 695 | . 416 | 2400 | 3000 | 600 |
| $18 \times 71 / 2$ | 65 | 18.35 | 7.590 | . 750 | . 450 | 2600 | 3250 | 3900 |
| $18 \times 71 / 2$ | 71 | 18.47 | 7.635 | . 810 | . 495 | 2840 | 3550 | 4260 |
| $18 \times 11$ | 76 | 18.21 | 11.035 | . 680 | . 425 | 3040 | 3800 | 4560 |
| $18 \times 11$ | 86 | 18.39 | 11.090 | . 770 | . 480 | 3440 | 4300 | 5160 |
| $18 \times 11$ | 97 | 18.59 | 11.145 | . 870 | . 535 | 3880 | 4850 | 5820 |
| $18 \times 11$ | 106 | 18.73 | 11.200 | . 940 | . 590 | 4240 | 5300 | 6360 |
| $18 \times 11$ | 119 | 18.97 | 11.265 | 1.060 | . 655 | 4760 | 5950 | 7140 |
| $21 \times 61 / 2$ | 44 | 20.66 | 6.500 | . 451 | . 348 | 1760 | 2200 | 2640 |
| $21 \times 61 / 2$ | 50 | 20.83 | 6.530 | . 535 | . 380 | 2000 | 2500 | 3000 |
| $21 \times 61 / 2$ | 57 | 21.06 | 6.555 | . 650 | . 405 | 2280 | 2850 | 3420 |
| $21 \times 81 / 4$ | 62 | 20.99 | 8.240 | . 615 | . 400 | 2480 | 3100 | 3720 |
| $21 \times 81 / 4$ | 68 | 21.13 | 8.270 | . 685 | . 430 | 2720 | 3400 | 4080 |
| $21 \times 81 / 4$ | 73 | 21.24 | 8.295 | . 740 | . 455 | 2920 | 3650 | 4380 |
| $21 \times 81 / 4$ | 83 | 21.43 | 8.355 | . 835 | . 515 | 3320 | 4150 | 4980 |
| $21 \times 81 / 4$ | 93 | 21.62 | 8.420 | . 930 | . 580 | 3720 | 4650 | 5580 |
| $21 \times 121 / 4$ | 101 | 21.36 | 12.290 | . 800 | . 500 | 4040 | 5050 | 6060 |
| $21 \times 121 / 4$ | 111 | 21.51 | 12.340 | . 875 | . 550 | 4440 | 5550 | 6660 |
| $21 \times 121 / 4$ | 122 | 21.68 | 12.390 | . 960 | . 600 | 4880 | 6100 | 7320 |
| $21 \times 121 / 4$ | 132 | 21.83 | 12.440 | 1.035 | . 650 | 5280 | 6600 | 7920 |
| $21 \times 121 / 4$ | 147 | 22.06 | 12.510 | 1.150 | . 720 | 5880 | 7350 | 8820 |



| W | WIDE-FLANGE BEAMS-Cont'd <br> Conforms to A-36 and A-572-Gr50 <br> Standard Lengths $40^{\prime}, 50^{\prime}$ and $60^{\prime}$ |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Weight Per Foot | A <br> Depth <br> of <br> Section | B <br> Flange Width Inches | C <br> Average Flange <br> Thickness Inches | D <br> Web <br> Thickness Inches | Weight LBS. |  |  |
| Nominal Size in inches |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  | $40^{\prime}$ | $50^{\prime}$ | $60^{\prime}$ |
| $36 \times 161 / 2$ | 260 | 36.24 | 16.555 | 1.440 | . 845 | 10400 | 13000 | 15600 |
| $36 \times 161 / 2$ | 280 | 36.50 | 16.595 | 1.680 | . 885 | 11200 | 14000 | 16800 |
| $36 \times 161 / 2$ | 300 | 36.72 | 16.655 | 1.680 | . 945 | 12000 | 15000 | 18000 |

### 6.3.1 Calculating the Metric Weight and Size of Wide-Flange Beams W4s to W36s

## Wide-Flange Beams

Imperial to Metric Conversions
W/D Imperial = M/D Metric * 0.017

| Imperial |  | Metric |  | Imperial | Metric |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Size (ln.) | $\times$ Wt. (lb/ft) | Size (mm) | $\times$ WT. $(\mathrm{kg} / \mathrm{m})$ | W/D (lb/ft/in.) | M/D (kg/m/m) |
| W4 | $\times 13$ | W100 | $\times 19$ | 0.65 | 39 |
| W5 | $\times 16$ | W130 | $\times 24$ | 0.65 | 39 |
| W5 | $\times 19$ | W130 | $\times 28$ | 0.76 | 45 |
| W6 | $\times 9$ | W150 | $\times 14$ | 0.39 | 23 |
| W6 | $\times 12$ | W150 | $\times 18$ | 0.51 | 30 |
| W6 | $\times 15$ | W150 | $\times 22$ | 0.51 | 30 |
| W6 | $\times 16$ | W150 | $\times 24$ | 0.66 | 39 |
| W6 | $\times 20$ | W150 | $\times 30$ | 0.67 | 40 |
| W6 | $\times 25$ | W150 | $\times 37$ | 0.82 | 49 |
| W8 | $\times 10$ | W200 | $\times 15$ | 0.37 | 22 |
| W8 | $\times 13$ | W200 | $\times 19$ | 0.47 | 28 |
| W8 | $\times 15$ | W200 | $\times 22$ | 0.54 | 32 |
| W8 | $\times 18$ | W200 | $\times 27$ | 0.57 | 34 |
| W8 | $\times 21$ | W200 | $\times 31$ | 0.66 | 39 |
| W8 | $\times 24$ | W200 | $\times 36$ | 0.69 | 41 |
| W8 | $\times 28$ | W200 | $\times 42$ | 0.8 | 48 |
| W8 | $\times 31$ | W200 | $\times 46$ | 0.79 | 47 |
| W8 | $\times 35$ | W200 | $\times 52$ | 0.88 | 52 |
| W8 | $\times 40$ | W200 | $\times 59$ | 1 | 59 |
| W8 | $\times 48$ | W200 | $\times 71$ | 1.18 | 70 |
| W8 | $\times 58$ | W200 | $\times 86$ | 1.41 | 83 |
| W8 | $\times 67$ | W200 | $\times 100$ | 1.61 | 95 |
| W10 | $\times 12$ | W250 | $\times 18$ | 0.38 | 23 |
| W10 | $\times 15$ | W250 | $\times 2$ | 0.48 | 29 |
| W10 | $\times 17$ | W250 | $\times 25$ | 0.54 | 32 |
| W10 | $\times 19$ | W250 | $\times 28$ | 0.59 | 35 |
| W10 | $\times 22$ | W250 | $\times 33$ | 0.59 | 35 |
| W10 | $\times 26$ | W250 | $\times 39$ | 0.69 | 41 |

Wide-Flange Beams-Cont'd Imperial to Metric Conversions
W/D Imperial = M/D Metric * 0.017

| Imperial |  | Metric |  | $\frac{\text { Imperial }}{\text { W/D (lb/ft/in.) }}$ | $\frac{\text { Metric }}{M / D(\mathrm{~kg} / \mathrm{m} / \mathrm{m})}$ |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Size (ln.) | $\times \mathrm{Wt}$. ( $\mathrm{lb} / \mathrm{ft}$ ) | Size (mm) | $\times$ WT. $(\mathrm{kg} / \mathrm{m})$ |  |  |
| W10 | $\times 30$ | W250 | $\times 45$ | 0.79 | 47 |
| W10 | $\times 33$ | W250 | $\times 49$ | 0.77 | 46 |
| W10 | $\times 39$ | W250 | $\times 58$ | 0.9 | 53 |
| W10 | $\times 45$ | W250 | $\times 87$ | 1.03 | 61 |
| W10 | $\times 49$ | W250 | $\times 73$ | 0.99 | 59 |
| W10 | $\times 54$ | W250 | +80 | 1.09 | 65 |
| W10 | $\times 60$ | W250 | $\times 89$ | 1.2 | 71 |
| W10 | $\times 68$ | W250 | $\times 101$ | 1.35 | 80 |
| W10 | $\times 77$ | W250 | $\times 115$ | 1.52 | 90 |
| W10 | $\times 88$ | W250 | $\times 131$ | 1.72 | 102 |
| W10 | $\times 100$ | W250 | $\times 149$ | 1.93 | 114 |
| W10 | $\times 112$ | W250 | $\times 167$ | 2.14 | 126 |
| W12 | $\times 14$ | W310 | $\times 21$ | 0.4 | 24 |
| W12 | $\times 16$ | W310 | $\times 24$ | 0.45 | 27 |
| W12 | $\times 19$ | W310 | $\times 28$ | 0.53 | 32 |
| W12 | $\times 22$ | W310 | $\times 33$ | 0.61 | 36 |
| W12 | $\times 26$ | W310 | $\times 39$ | 0.6 | 36 |
| W12 | $\times 30$ | W310 | $\times 45$ | 0.69 | 41 |
| W12 | $\times 35$ | W310 | $\times 52$ | 0.79 | 47 |
| W12 | $\times 40$ | W310 | $\times 60$ | 0.85 | 50 |
| W12 | $\times 45$ | W310 | $\times 67$ | 0.95 | 56 |
| W12 | $\times 50$ | W310 | $\times 74$ | 1.04 | 62 |
| W12 | $\times 53$ | W310 | $\times 79$ | 0.99 | 59 |
| W12 | $\times 54$ | W310 | $\times 80$ | 1.04 | 62 |
| W12 | $\times 55$ | W310 | $\times 82$ | 1.04 | 62 |
| W12 | $\times 58$ | W310 | $\times 86$ | 1.08 | 64 |
| W12 | $\times 65$ | W310 | $\times 97$ | 1.09 | 65 |
| W12 | $\times 72$ | W310 | $\times 107$ | 1.2 | 71 |
| W12 | $\times 79$ | W310 | $\times 118$ | 1.32 | 78 |
| W12 | $\times 87$ | W310 | $\times 129$ | 1.44 | 85 |
| W12 | $\times 96$ | W310 | $\times 143$ | 1.57 | 93 |
| W12 | $\times 106$ | W310 | $\times 158$ | 1.73 | 102 |
| W12 | $\times 120$ | W310 | $\times 179$ | 1.94 | 115 |
| W12 | $\times 136$ | W310 | $\times 202$ | 2.17 | 128 |
| W12 | $\times 152$ | W310 | $\times 226$ | 2.4 | 142 |
| W12 | $\times 170$ | W310 | $\times 253$ | 2.66 | 157 |
| W12 | $\times 190$ | W310 | $\times 283$ | 2.93 | 173 |
| W12 | $\times 210$ | W310 | $\times 313$ | 3.21 | 189 |
| W12 | $\times 230$ | W310 | $\times 342$ | 3.47 | 205 |
| W12 | $\times 252$ | W310 | $\times 375$ | 3.76 | 222 |
| W12 | $\times 279$ | W310 | $\times 415$ | 4.1 | 242 |
| W12 | $\times 305$ | W310 | $\times 454$ | 4.41 | 260 |
| W12 | $\times 336$ | W310 | $\times 500$ | 4.78 | 282 |
| W14 | $\times 22$ | W360 | $\times 33$ | 0.52 | 31 |
| W14 | $\times 26$ | W360 | $\times 39$ | 0.61 | 36 |
| W14 | $\times 30$ | W360 | $\times 45$ | 0.63 | 38 |
| W14 | $\times 34$ | W360 | $\times 51$ | 0.71 | 42 |
| W14 | $\times 38$ | W360 | $\times 57$ | 0.79 | 47 |
| W14 | $\times 43$ | W360 | $\times 64$ | 0.85 | 50 |
| W14 | $\times 48$ | W360 | $\times 72$ | 0.94 | 56 |
| W14 | $\times 53$ | W360 | $\times 79$ | 1.03 | 61 |
| W14 | $\times 61$ | W360 | $\times 91$ | 1.07 | 63 |

Wide-Flange Beams-Cont'd Imperial to Metric Conversions
W/D Imperial = M/D Metric * 0.017

| Imperial |  | Metric |  | Imperial | Metric |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Size (ln.) | $\times$ Wt. (lb/ft) | Size (mm) | $\times$ WT. $(\mathrm{kg} / \mathrm{m})$ | W/D (lb/ft/in.) | M/D (kg/m/m) |
| W14 | $\times 68$ | W360 | $\times 101$ | 1.19 | 70 |
| W14 | $\times 74$ | W360 | $\times 110$ | 1.28 | 76 |
| W14 | $\times 82$ | W360 | $\times 122$ | 1.41 | 83 |
| W14 | + 90 | W360 | $\times 134$ | 1.27 | 75 |
| W14 | $\times 99$ | W360 | $\times 147$ | 1.39 | 82 |
| W14 | $\times 109$ | W360 | $\times 162$ | 1.53 | 90 |
| W14 | $\times 120$ | W360 | $\times 179$ | 1.67 | 99 |
| W14 | $\times 132$ | W360 | $\times 196$ | 1.83 | 108 |
| W14 | $\times 145$ | W360 | $\times 216$ | 1.94 | 115 |
| W14 | $\times 159$ | W360 | $\times 237$ | 2.11 | 125 |
| W14 | $\times 176$ | W360 | $\times 262$ | 2.32 | 137 |
| W14 | $\times 193$ | W360 | $\times 287$ | 2.53 | 149 |
| W14 | $\times 211$ | W360 | $\times 314$ | 2.74 | 162 |
| W14 | $\times 233$ | W360 | $\times 347$ | 3 | 177 |
| W14 | $\times 257$ | W360 | $\times 382$ | 3.27 | 193 |
| W14 | $\times 283$ | W360 | $\times 421$ | 3.57 | 210 |
| W14 | $\times 311$ | W360 | $\times 463$ | 3.88 | 229 |
| W14 | $\times 342$ | W360 | $\times 509$ | 4.21 | 248 |
| W14 | $\times 370$ | W360 | $\times 551$ | 4.51 | 266 |
| W14 | $\times 398$ | W360 | $\times 592$ | 4.8 | 283 |
| W14 | $\times 426$ | W360 | $\times 634$ | 5.09 | 300 |
| W14 | $\times 455$ | W360 | $\times 677$ | 5.38 | 317 |
| W14 | $\times 500$ | W360 | $\times 744$ | 5.82 | 343 |
| W14 | $\times 550$ | W360 | $\times 816$ | 6.3 | 371 |
| W14 | $\times 605$ | W360 | $\times 900$ | 6.8 | 400 |
| W14 | $\times 665$ | W360 | $\times 990$ | 7.34 | 432 |
| W14 | $\times 730$ | W360 | $\times 1086$ | 7.9 | 465 |
| W16 | $\times 26$ | W410 | $\times 39$ | 0.55 | 33 |
| W16 | $\times 31$ | W410 | $\times 46$ | 0.65 | 39 |
| W16 | $\times 36$ | W410 | $\times 54$ | 0.69 | 41 |
| W16 | $\times 40$ | W410 | $\times 60$ | 0.76 | 45 |
| W16 | $\times 45$ | W410 | $\times 67$ | 0.85 | 50 |
| W16 | $\times 50$ | W410 | $\times 74$ | 0.94 | 56 |
| W16 | $\times 57$ | W410 | $\times 85$ | 1.07 | 63 |
| W16 | $\times 67$ | W410 | $\times 100$ | 1.07 | 63 |
| W16 | $\times 77$ | W410 | $\times 114$ | 1.22 | 72 |
| W16 | $\times 89$ | W410 | $\times 132$ | 1.4 | 83 |
| W16 | $\times 100$ | W410 | $\times 149$ | 1.56 | 92 |
| W18 | $\times 35$ | W460 | $\times 52$ | 0.66 | 39 |
| W18 | $\times 40$ | W460 | $\times 60$ | 0.75 | 45 |
| W18 | $\times 46$ | W460 | $\times 68$ | 0.86 | 51 |
| W18 | $\times 50$ | W460 | $\times 74$ | 0.87 | 52 |
| W18 | $\times 55$ | W460 | $\times 82$ | 0.95 | 56 |
| W18 | $\times 60$ | W460 | $\times 89$ | 1.03 | 61 |
| W18 | $\times 65$ | W460 | $\times 97$ | 1.11 | 66 |
| W18 | $\times 71$ | W460 | $\times 106$ | 1.21 | 72 |
| W18 | $\times 76$ | W460 | $\times 113$ | 1.11 | 66 |
| W18 | $\times 86$ | W460 | $\times 128$ | 1.24 | 73 |
| W18 | $\times 97$ | W460 | $\times 144$ | 1.39 | 82 |
| W18 | $\times 106$ | W460 | $\times 158$ | 1.52 | 90 |
| W18 | $\times 119$ | W460 | $\times 177$ | 1.68 | 99 |

Wide-Flange Beams-Cont'd Imperial to Metric Conversions
W/D Imperial = M/D Metric * 0.017

| Imperial |  | Metric |  | Imperial | Metric |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Size (ln.) | $\times$ Wt. (lb/ft) | Size (mm) | $\times$ WT. $(\mathrm{kg} / \mathrm{m})$ | W/D (lb/ft/in.) | M/D (kg/m/m) |
| W21 | $\times 44$ | W530 | $\times 66$ | 0.73 | 43 |
| W21 | $\times 50$ | W530 | $\times 74$ | 0.83 | 49 |
| W21 | $\times 57$ | W530 | $\times 85$ | 0.93 | 55 |
| W21 | $\times 62$ | W530 | $\times 92$ | 0.94 | 56 |
| W21 | $\times 68$ | W530 | $\times 101$ | 1.03 | 61 |
| W22 | $\times 73$ | W530 | $\times 108$ | 1.1 | 65 |
| W21 | $\times 83$ | W530 | $\times 123$ | 1.24 | 73 |
| W21 | + 93 | W530 | $\times 138$ | 1.38 | 82 |
| W21 | $\times 101$ | W530 | $\times 150$ | 1.29 | 76 |
| W21 | $\times 111$ | W530 | $\times 165$ | 1.41 | 83 |
| W21 | $\times 122$ | W530 | $\times 182$ | 1.54 | 91 |
| W21 | $\times 132$ | W530 | $\times 196$ | 1.66 | 98 |
| W21 | $\times 147$ | W530 | $\times 219$ | 1.83 | 108 |
| W24 | $\times 55$ | W610 | $\times 82$ | 0.82 | 49 |
| W24 | $\times 62$ | W610 | $\times 92$ | 0.92 | 55 |
| W24 | $\times 68$ | W610 | $\times 101$ | 0.93 | 55 |
| W24 | $\times 76$ | W610 | $\times 113$ | 1.02 | 60 |
| W24 | $\times 84$ | W610 | $\times 125$ | 1.13 | 67 |
| W24 | $\times 94$ | W610 | $\times 140$ | 1.26 | 75 |
| W24 | $\times 104$ | W610 | $\times 153$ | 1.22 | 72 |
| W24 | $\times 117$ | W610 | $\times 174$ | 1.36 | 80 |
| W24 | $\times 131$ | W610 | $\times 195$ | 1.52 | 90 |
| W24 | $\times 146$ | W610 | $\times 217$ | 1.68 | 99 |
| W24 | $\times 162$ | W610 | $\times 241$ | 1.85 | 109 |
| W27 | $\times 84$ | W690 | $\times 125$ | 1.02 | 60 |
| W27 | $\times 94$ | W690 | $\times 140$ | 1.13 | 67 |
| W27 | $\times 102$ | W690 | $\times 152$ | 1.23 | 73 |
| W27 | $\times 114$ | W690 | $\times 170$ | 1.36 | 80 |
| W27 | $\times 146$ | W690 | $\times 217$ | 1.53 | 90 |
| W27 | $\times 161$ | W690 | $\times 240$ | 1.68 | 99 |
| W27 | $\times 178$ | W690 | $\times 265$ | 1.85 | 109 |
| W30 | $\times 99$ | W760 | $\times 147$ | 1.1 | 65 |
| W30 | $\times 108$ | W760 | $\times 161$ | 1.2 | 71 |
| W30 | $\times 116$ | W760 | $\times 173$ | 1.28 | 76 |
| W30 | $\times 124$ | W760 | $\times 185$ | 1.37 | 81 |
| W30 | $\times 132$ | W760 | $\times 196$ | 1.45 | 86 |
| W30 | $\times 173$ | W760 | $\times 257$ | 1.66 | 98 |
| W30 | $\times 191$ | W760 | $\times 284$ | 1.82 | 108 |
| W30 | $\times 211$ | W760 | $\times 314$ | 2 | 118 |
| W33 | $\times 118$ | W840 | $\times 176$ | 1.19 | 70 |
| W33 | $\times 130$ | W840 | $\times 193$ | 1.31 | 78 |
| W33 | $\times 141$ | W840 | $\times 210$ | 1.41 | 83 |
| W33 | $\times 152$ | W840 | $\times 226$ | 1.51 | 89 |
| W33 | $\times 201$ | W840 | $\times 299$ | 1.78 | 105 |
| W33 | $\times 221$ | W840 | $\times 329$ | 1.94 | 115 |
| W33 | $\times 241$ | W840 | $\times 359$ | 2.11 | 125 |


| Wide-Flange Beams-Cont'd Imperial to Metric Conversions W/D Imperial = M/D Metric * 0.017 |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Imperial |  | Metric |  | Imperial | Metric |
| Size (ln.) | $\times$ Wt. (lb/ft) | Size (mm) | $\times$ WT. $(\mathrm{kg} / \mathrm{m})$ | W/D (lb/ft/in.) | M/D (kg/m/m) |
| W36 | $\times 135$ | W920 | $\times 201$ | 1.28 | 76 |
| W36 | $\times 150$ | W920 | $\times 223$ | 1.41 | 83 |
| W36 | $\times 160$ | W920 | $\times 238$ | 1.5 | 89 |
| W36 | $\times 170$ | W920 | $\times 253$ | 1.59 | 94 |
| W36 | $\times 182$ | W920 | $\times 271$ | 1.69 | 100 |
| W36 | $\times 194$ | W920 | $\times 289$ | 1.8 | 106 |
| W36 | $\times 210$ | W920 | $\times 313$ | 1.94 | 115 |
| W36 | $\times 230$ | W920 | $\times 342$ | 1.92 | 113 |
| W36 | $\times 245$ | W920 | $\times 365$ | 2.04 | 120 |
| W36 | $\times 260$ | W920 | $\times 387$ | 2.16 | 128 |
| W36 | $\times 280$ | W920 | $\times 417$ | 2.31 | 136 |
| W36 | $\times 300$ | W920 | $\times 446$ | 2.47 | 146 |

### 6.3.2 Calculating the Weight and Size of I Beams and Junior Beams

## STANDARD JUNIOR

## I BEAMS AND JUNIOR BEAMS

## Structural Steel Shapes

Structural Steel Shapes are usually ordered as specification ASTM-36.
This Standard of the American Society for Testing and Materials is issued under the designation A-36.
The number immediately following the designation includes the year of original adoption, or, in the case of revision, the year of last revision.

## A-36 Specification

Tensile strength, psi 56,000-80,000
Min. yield strength, psi 36,000
Carbon Content, .26 max.
Standard Lengths $40^{\prime}, 50^{\prime}, 60^{\prime}$

## A-572 Specification

Tensile strength, psi 65,000
Min. yield strength, psi 36,000
Carbon Content, . 26 max.
Standard Lengths $\mathbf{4 0}^{\prime}, \mathbf{5 0}^{\prime}, \mathbf{6 0}^{\prime}$
Source: Cardinal Metals, St. Louis, MO

Conforms to A-36
Standard Lengths $\mathbf{2 0}^{\prime}, \mathbf{4 0}^{\prime}, \mathbf{6 0}^{\prime}$

| A Depth in Inches | Weight Lbs per Foot | B <br> Flange Width Inches | C <br> Web <br> Thickness In. | Weight Lbs |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | 20 Ft <br> Length | 30 Ft <br> Length | 40 Ft <br> Length | 60 Ft Length |
| 3 | 5.7 | 2.330 | . 170 | 114 | 171 | 228 | 342 |
|  | 7.5 | 2.509 | . 349 | 150 | 225 | 300 | 450 |
| 4 | 7.7 | 2.660 | . 190 | 154 | 231 | 308 | 462 |
|  | 9.5 | 2.796 | . 326 | 190 | 285 | 380 | 470 |
| 5 | 10.0 | 3.000 | . 210 | 200 | 300 | 400 | 600 |
|  | 14.75 | 3.284 | . 494 | 295 | 443 | 590 | 885 |
| 6 | 12.5 | 3.330 | . 230 | 250 | 375 | 500 | 750 |
|  | 17.25 | 3.565 | . 465 | 345 | 518 | 690 | 1035 |
| 7 | 15.3 | 3.660 | . 250 | 306 | 459 | 612 | 918 |
|  | 20.0 | 3.860 | . 450 | 400 | 600 | 800 | 1200 |
| 8 | 18.4 | 4.000 | . 270 | 368 | 552 | 736 | 1104 |
|  | 23.0 | 4.171 | . 441 | 460 | 690 | 920 | 1380 |
| 10 | 25.4 | 4.660 | . 310 | 508 | 762 | 1016 | 1524 |
|  | 35.0 | 4.944 | . 594 | 700 | 1050 | 1400 | 2100 |
| 12 | 31.8 | 5.000 | . 350 | 636 | 954 | 1272 | 1908 |
|  | 35.0 | 5.078 | . 428 | 700 | 1050 | 1400 | 2100 |
|  | 40.8 | 5.250 | . 460 | 816 | 1224 | 1632 | 2448 |
|  | 50.0 | 5.477 | . 687 | 1000 | 1500 | 2000 | 3000 |
| 15 | 42.9 | 5.500 | . 410 | 858 | 1287 | 1716 | 2574 |
|  | 50.0 | 5.640 | . 550 | 1000 | 1500 | 2000 | 3000 |
| 18 | 54.7 | 6.000 | . 460 | 1094 | 1641 | 2188 | 3282 |
|  | 70.0 | 6.251 | . 711 | 1400 | 2100 | 2800 | 4200 |
| 20 | 66.0 | 6.255 | . 505 | 1320 | 1980 | 2640 | 3960 |
|  | 75.0 | 6.385 | . 635 | 1500 | 2250 | 3000 | 4500 |
|  | 86.0 | 7.060 | . 660 | 1720 | 2580 | 3440 | 5160 |
|  | 96.0 | 7.200 | . 800 | 1920 | 2880 | 3840 | 5760 |
| 24 | 80.0 | 7.000 | . 500 | 1600 | 2400 | 3200 | 4800 |
|  | 90.0 | 7.125 | . 625 | 1800 | 2700 | 3600 | 5400 |
|  | 100.0 | 7.245 | . 745 | 2000 | 3000 | 4000 | 6000 |
|  | 106.0 | 7.870 | . 620 | 2120 | 3180 | 4240 | 6360 |
|  | 121.0 | 8.050 | . 800 | 2420 | 3630 | 4840 | 7260 |

JUNIOR BEAMS
Conforms to A-36
Standard Lengths $20^{\prime}$ and $40^{\prime}$

| In Inches | Per <br> Foot | Width <br> Inches | Thick Ness <br> In. | 20 FT. <br> Length | 30 FT. <br> Length | 40 FT. <br> Length | 60 FT. <br> Length |
| :--- | ---: | :--- | :--- | :---: | :---: | :---: | :---: |
| 6 | 4.4 | 1.844 | .114 | 88 | 132 | 176 |  |
| 8 | 6.5 | 2.281 | .135 | 130 | 195 | 260 |  |
| 10 | 9.0 | 2.688 | .155 | 180 | 270 | 360 |  |
| 12 | 11.8 | 3.063 | .175 | 236 | 354 | 472 |  |

6.4.0 Calculating the Weight and Size of U.S. Square High-Strength Steel Sections

(Continued)

| Nominal Size |  |  |  |  | Weight Per Foot | Wall <br> Thickness <br> t | b/t | h/t | Cross <br> Section <br> Area | $\begin{aligned} & \text { l } \\ & \text { in. } \end{aligned}$ | $\begin{aligned} & \mathrm{S} \\ & \text { in. }^{3} \end{aligned}$ | $\begin{aligned} & \text { r } \\ & \text { in. } \end{aligned}$ | $\begin{aligned} & \mathrm{Z} \\ & \text { in. }{ }^{3} \end{aligned}$ | Torsional Stiffness Constant J$\text { in. }{ }^{4}$ | Torsional Shear Constant C$\text { in. }{ }^{3}$ | Surface <br> Area <br> Per <br> Foot <br> $f t .^{2}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| in. |  | in. |  | in. | ib. | in. |  |  | in. ${ }^{2}$ |  |  |  |  |  |  |  |
| 20 | $\times$ | 20 | $\times$ | 5/8* | 157.75 | 0.625 | 29.0 | 29.0 | 46.4 | 2830 | 283 | 7.81 | 331 | 4670 | 465 | 6.34 |
|  |  |  |  | 1/2* | 129.06 | 0.500 | 37.0 | 37.0 | 37.9 | 2370 | 237 | 7.90 | 275 | 3790 | 379 | 6.45 |
|  |  |  |  | 3/8* | 98.12 | 0.375 | 50.3 | 50.3 | 28.8 | 1830 | 183 | 7.97 | 211 | 2880 | 288 | 6.51 |
| 18 | $\times$ | 18 | $\times$ | 5/8* | 140.73 | 0.625 | 25.8 | 25.8 | 41.4 | 2020 | 224 | 6.99 | 264 | 3370 | 373 | 5.68 |
|  |  |  |  | 1/2* | 115.45 | 0.500 | 33.0 | 33.0 | 33.9 | 1700 | 189 | 7.08 | 220 | 2740 | 305 | 5.79 |
|  |  |  |  | 3/8* | 87.91 | 0.375 | 45.0 | 45.0 | 25.8 | 1320 | 147 | 7.15 | 169 | 2090 | 232 | 5.84 |
| 16 | $\times$ | 16 | $\times$ | 5/8 | 127.37 | 0.581 | 24.5 | 24.5 | 35.0 | 1370 | 171 | 6.25 | 200 | 2170 | 276 | 5.17 |
|  |  |  |  | 1/2 | 103.30 | 0.465 | 31.4 | 31.4 | 28.3 | 1130 | 141 | 6.31 | 164 | 1770 | 224 | 5.20 |
|  |  |  |  | 3/8 | 78.52 | 0.349 | 42.8 | 42.8 | 21.5 | 873 | 109 | 6.37 | 126 | 1350 | 171 | 5.23 |
|  |  |  |  | 5/16 | 65.87 | 0.291 | 52.0 | 52.0 | 18.1 | 739 | 92.3 | 6.39 | 106 | 1140 | 144 | 5.25 |
| 14 | $\times$ | 14 | $\times$ | 5/8 | 110.36 | 0.581 | 21.1 | 21.1 | 30.3 | 896 | 128 | 5.44 | 151 | 1430 | 208 | 4.50 |
|  |  |  |  | 1/2 | 89.68 | 0.465 | 27.1 | 27.1 | 24.6 | 743 | 106 | 5.49 | 124 | 1170 | 170 | 4.53 |
|  |  |  |  | 3/8 | 68.31 | 0.349 | 37.1 | 37.1 | 18.7 | 577 | 82.5 | 5.55 | 95.4 | 900 | 130 | 4.57 |
|  |  |  |  | 5/16 | 57.36 | 0.291 | 45.1 | 45.1 | 15.7 | 490 | 69.9 | 5.58 | 80.5 | 759 | 109 | 4.58 |
| 12 | $\times$ | 12 | $\times$ | 5/8 | 93.34 | 0.581 | 17.7 | 17.7 | 25.7 | 548 | 91.3 | 4.62 | 109 | 885 | 151 | 3.83 |
|  |  |  |  | 1/2 | 76.07 | 0.465 | 22.8 | 22.8 | 20.9 | 457 | 76.2 | 4.68 | 89.6 | 728 | 123 | 3.87 |
|  |  |  |  | 3/8 | 58.10 | 0.349 | 31.4 | 31.4 | 16.0 | 357 | 59.5 | 4.73 | 69.2 | 561 | 94.6 | 3.90 |
|  |  |  |  | 5/16 | 48.86 | 0.291 | 38.2 | 38.2 | 13.4 | 304 | 50.7 | 4.76 | 58.6 | 474 | 79.7 | 3.92 |
|  |  |  |  | 1/4 | 39.43 | 0.233 | 48.5 | 48.5 | 10.8 | 248 | 41.4 | 4.79 | 47.6 | 384 | 64.5 | 3.93 |
| 10 | $\times$ | 10 | $\times$ | 5/8 | 76.33 | 0.581 | 14.2 | 14.2 | 21.0 | 304 | 60.8 | 3.80 | 73.2 | 498 | 102 | 3.17 |
|  |  |  |  | 1/2 | 62.46 | 0.465 | 18.5 | 18.5 | 17.2 | 256 | 51.2 | 3.86 | 60.7 | 412 | 84.2 | 3.20 |
|  |  |  |  | 3/8 | 47.90 | 0.349 | 25.7 | 25.7 | 13.2 | 202 | 40.4 | 3.92 | 47.2 | 320 | 64.8 | 3.23 |
|  |  |  |  | 5/16 | 40.35 | 0.291 | 31.4 | 31.4 | 11.1 | 172 | 34.5 | 3.94 | 40.1 | 271 | 54.8 | 3.25 |
|  |  |  |  | 1/4 | 32.63 | 0.233 | 39.9 | 39.9 | 8.96 | 141 | 28.3 | 3.97 | 32.7 | 220 | 44.4 | 3.27 |
|  |  |  |  | 3/16 | 24.73 | 0.174 | 54.5 | 54.5 | 6.76 | 108 | 21.6 | 4.00 | 24.8 | 167 | 33.6 | 3.28 |
| 9 | $\times$ | 9 | $\times$ | 1/2 | 55.66 | 0.465 | 16.4 | 16.4 | 15.3 | 182 | 40.6 | 3.45 | 48.4 | 296 | 67.4 | 2.87 |
|  |  |  |  | 3/8 | 42.79 | 0.349 | 22.8 | 22.8 | 11.8 | 145 | 32.2 | 3.51 | 37.8 | 231 | 52.1 | 2.90 |
|  |  |  |  | 5/16 | 36.10 | 0.291 | 27.9 | 27.9 | 9.92 | 124 | 27.6 | 3.54 | 32.1 | 196 | 44.0 | 2.92 |
|  |  |  |  | 1/4 | 29.23 | 0.233 | 35.6 | 35.6 | 8.03 | 102 | 22.7 | 3.56 | 26.2 | 159 | 35.8 | 2.93 |
|  |  |  |  | 3/16 | 22.18 | 0.174 | 48.7 | 48.7 | 6.06 | 78.2 | 17.4 | 3.59 | 20.0 | 121 | 27.1 | 2.95 |
| 8 | $\times$ | 8 | $\times$ | 5/8 | 59.32 | 0.581 | 10.8 | 10.8 | 16.4 | 146 | 36.5 | 2.99 | 44.7 | 244 | 63.2 | 2.50 |
|  |  |  |  | 1/2 | 48.85 | 0.465 | 14.2 | 14.2 | 13.5 | 125 | 31.2 | 3.04 | 37.5 | 204 | 52.4 | 2.53 |
|  |  |  |  | 3/8 | 37.69 | 0.349 | 19.9 | 19.9 | 10.4 | 99.6 | 24.9 | 3.10 | 29.4 | 160 | 40.7 | 2.57 |
|  |  |  |  | 5/16 | 31.84 | 0.291 | 24.5 | 24.5 | 8.76 | 85.6 | 21.4 | 3.13 | 25.1 | 136 | 34.5 | 2.58 |
|  |  |  |  | 1/4 | 25.82 | 0.233 | 31.3 | 31.3 | 7.10 | 70.7 | 17.7 | 3.15 | 20.5 | 111 | 28.1 | 2.60 |


|  |  |  |  | 3/16 | 19.63 | 0.174 | 43.0 | 43.0 | 5.37 | 54.4 | 13.6 | 3.18 | 15.7 | 84.5 | 21.3 | 2.62 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 7 | $\times$ | 7 | $\times$ | 5/8 | 5.81 | 0.581 | 9.0 | 9.0 | 14.0 | 93.3 | 26.7 | 2.58 | 33.1 | 158 | 47.1 | 2.17 |
|  |  |  |  | 1/2 | 42.05 | 0.465 | 12.1 | 12.1 | 11.6 | 80.5 | 23.0 | 2.63 | 27.9 | 133 | 39.3 | 2.20 |
|  |  |  |  | 3/8 | 32.58 | 0.349 | 17.1 | 17.1 | 8.97 | 64.9 | 18.6 | 2.69 | 22.1 | 105 | 30.7 | 2.23 |
|  |  |  |  | 5/16 | 27.59 | 0.291 | 21.1 | 21.1 | 7.59 | 56.1 | 16.0 | 2.72 | 18.9 | 89.7 | 26.1 | 2.25 |
|  |  |  |  | 1/4 | 22.42 | 0.233 | 27.0 | 27.0 | 6.17 | 46.5 | 13.3 | 2.75 | 15.5 | 73.5 | 21.3 | 2.27 |
|  |  |  |  | 3/16 | 17.08 | 0.174 | 37.2 | 37.2 | 4.67 | 36.0 | 10.3 | 2.77 | 11.9 | 56.1 | 16.2 | 2.28 |
| 6 | $\times$ | 6 | $\times$ | 5/8 | 42.30 | 0.581 | 7.3 | 7.3 | 11.7 | 55.1 | 18.4 | 2.17 | 23.2 | 94.9 | 33.4 | 1.83 |
|  |  |  |  | 1/2 | 35.24 | 0.465 | 9.9 | 9.9 | 9.74 | 48.2 | 16.1 | 2.23 | 19.8 | 81.1 | 28.1 | 1.87 |
|  |  |  |  | 3/8 | 27.48 | 0.349 | 14.2 | 14.2 | 7.58 | 39.4 | 13.1 | 2.28 | 15.8 | 64.6 | 22.1 | 1.90 |
|  |  |  |  | 5/16 | 23.34 | 0.291 | 17.6 | 17.6 | 6.43 | 34.3 | 11.4 | 2.31 | 13.6 | 55.4 | 18.9 | 1.92 |
|  |  |  |  | 1/4 | 19.02 | 0.233 | 22.8 | 22.8 | 5.24 | 28.6 | 9.54 | 2.34 | 11.2 | 45.6 | 15.4 | 1.93 |
|  |  |  |  | 3/16 | 14.53 | 0.174 | 31.5 | 31.5 | 3.98 | 22.3 | 7.42 | 2.37 | 8.63 | 35.0 | 11.8 | 1.95 |
|  |  |  |  | 1/8 | 9.86 | 0.116 | 48.7 | 48.7 | 2.70 | 15.5 | 5.15 | 2.39 | 5.92 | 23.9 | 8.03 | 1.97 |
| $51 / 2$ | $\times$ | $51 / 2$ | $\times$ | 3/8 | 24.93 | 0.349 | 12.8 | 12.8 | 6.88 | 29.7 | 10.8 | 2.08 | 13.1 | 49.0 | 18.4 | 1.73 |
|  |  |  |  | 5/16 | 21.21 | 0.291 | 15.9 | 15.9 | 5.85 | 25.9 | 9.43 | 2.11 | 11.3 | 42.2 | 15.7 | 1.75 |
|  |  |  |  | 1/4 | 17.32 | 0.233 | 20.6 | 20.6 | 4.77 | 21.7 | 7.90 | 2.13 | 9.32 | 34.8 | 12.9 | 1.77 |
|  |  |  |  | 3/16 | 13.25 | 0.174 | 28.6 | 28.6 | 3.63 | 17.0 | 6.17 | 2.16 | 7.19 | 26.7 | 9.85 | 1.78 |
|  |  |  |  | 1/8 | 9.01 | 0.116 | 44.4 | 44.4 | 2.46 | 11.8 | 4.30 | 2.19 | 4.95 | 18.3 | 6.72 | 1.80 |
| 5 | $\times$ | 5 | $\times$ | 1/2 | 28.43 | 0.465 | 7.8 | 7.8 | 7.88 | 26.0 | 10.4 | 1.82 | 13.1 | 44.6 | 18.7 | 1.53 |
|  |  |  |  | 3/8 | 22.37 | 0.349 | 11.3 | 11.3 | 6.18 | 21.7 | 8.67 | 1.87 | 10.6 | 36.1 | 14.9 | 1.57 |
|  |  |  |  | 5/16 | 19.08 | 0.291 | 14.2 | 14.2 | 5.26 | 19.0 | 7.61 | 1.90 | 9.16 | 31.2 | 12.8 | 1.58 |
|  |  |  |  | 1/4 | 15.62 | 0.233 | 18.5 | 18.5 | 4.30 | 16.0 | 6.41 | 1.93 | 7.61 | 25.8 | 10.5 | 1.60 |
|  |  |  |  | 3/16 | 11.97 | 0.174 | 25.7 | 25.7 | 3.28 | 12.6 | 5.03 | 1.96 | 5.89 | 19.9 | 8.08 | 1.62 |
|  |  |  |  | 1/8 | 8.16 | 0.116 | 40.1 | 40.1 | 2.23 | 8.80 | 3.52 | 1.99 | 4.07 | 13.7 | 5.53 | 1.63 |
| $41 / 2$ | $\times$ | $41 / 2$ | $\times$ | 1/2 | 25.03 | 0.465 | 6.7 | 6.7 | 6.95 | 18.0 | 8.02 | 1.61 | 10.2 | 31.3 | 14.8 | 1.37 |
|  |  |  |  | 3/8 | 19.82 | 0.349 | 9.9 | 9.9 | 5.48 | 15.3 | 6.78 | 1.67 | 8.36 | 25.7 | 11.9 | 1.40 |
|  |  |  |  | 5/16 | 16.96 | 0.291 | 12.5 | 12.5 | 4.68 | 13.5 | 5.99 | 1.70 | 7.27 | 22.3 | 10.2 | 1.42 |
|  |  |  |  | 1/4 | 13.91 | 0.233 | 16.3 | 16.3 | 3.84 | 11.4 | 5.08 | 1.73 | 6.06 | 18.5 | 8.44 | 1.43 |
|  |  |  |  | 3/16 | 10.70 | 0.174 | 22.9 | 22.9 | 2.93 | 9.02 | 4.01 | 1.75 | 4.71 | 14.4 | 6.49 | 1.45 |
|  |  |  |  | 1/8 | 7.31 | 0.116 | 35.8 | 35.8 | 2.00 | 6.35 | 2.82 | 1.78 | 3.27 | 9.92 | 4.45 | 1.47 |
| 4 | $\times$ | 4 | $\times$ | 1/2 | 21.63 | 0.465 | 5.6 | 5.6 | 6.02 | 11.9 | 5.95 | 1.41 | 7.70 | 21.0 | 11.2 | 1.20 |
|  |  |  |  | 3/8 | 17.27 | 0.349 | 8.5 | 8.5 | 4.78 | 10.3 | 5.13 | 1.46 | 6.39 | 17.5 | 9.14 | 1.23 |
|  |  |  |  | 5/16 | 14.83 | 0.291 | 10.7 | 10.7 | 4.10 | 9.14 | 4.57 | 1.49 | 5.59 | 15.3 | 7.91 | 1.25 |
|  |  |  |  | 1/4 | 12.21 | 0.233 | 14.2 | 14.2 | 3.37 | 7.80 | 3.90 | 1.52 | 4.69 | 12.8 | 6.56 | 1.27 |
|  |  |  |  | 3/16 | 9.42 | 0.174 | 20.0 | 20.0 | 2.58 | 6.21 | 3.10 | 1.55 | 3.67 | 9.96 | 5.07 | 1.28 |


| Nominal Size |  |  |  |  | Weight Per Foot$i b .$ | Wall <br> Thickness <br> t <br> in. | b/t | h/t | Cross <br> Section <br> Area $i n .^{2}$ | $\begin{aligned} & \text { I } \\ & \text { in. }^{4} \end{aligned}$ | $\begin{aligned} & \text { S } \\ & \text { in. }{ }^{3} \end{aligned}$ | $\begin{aligned} & \text { r } \\ & \text { in. } \end{aligned}$ | $\begin{aligned} & \mathrm{z} \\ & \text { in. }^{3} \end{aligned}$ | Torsional Stiffness Constant J$i n .{ }^{4}$ | Torsional Shear Constant C$i n .^{3}$ | Surface <br> Area <br> Per <br> Foot <br> $f t{ }^{2}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| in. |  | in. |  | in. |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  | 1/8 | 6.46 | 0.116 | 31.5 | 31.5 | 1.77 | 4.40 | 2.20 | 1.58 | 2.56 | 6.91 | 3.49 | 1.30 |
| $31 / 2$ | $\times$ | $31 / 2$ | $\times$ | 3/8 | 14.72 | 0.349 | 7.0 | 7.0 | 4.09 | 6.48 | 3.70 | 1.26 | 4.69 | 11.2 | 6.77 | 1.07 |
|  |  |  |  | 5/16 | 12.70 | 0.291 | 9.0 | 9.0 | 3.52 | 5.84 | 3.34 | 1.29 | 4.14 | 9.89 | 5.90 | 1.08 |
|  |  |  |  | 1/4 | 10.51 | 0.233 | 12.0 | 12.0 | 2.91 | 5.04 | 2.88 | 1.32 | 3.50 | 8.35 | 4.92 | 1.10 |
|  |  |  |  | 3/16 | 8.15 | 0.174 | 17.1 | 17.1 | 2.24 | 4.05 | 2.31 | 1.35 | 2.76 | 6.56 | 3.83 | 1.12 |
|  |  |  |  | 1/8 | 5.61 | 0.116 | 27.2 | 27.2 | 1.54 | 2.90 | 1.66 | 1.37 | 1.93 | 4.58 | 2.65 | 1.13 |
| 3 | $\times$ | 3 | $\times$ | 3/8 | 12.17 | 0.349 | 5.6 | 5.6 | 3.39 | 3.77 | 2.51 | 1.05 | 3.25 | 6.64 | 4.74 | 0.90 |
|  |  |  |  | 5/16 | 10.58 | 0.291 | 7.3 | 7.3 | 2.94 | 3.45 | 2.30 | 1.08 | 2.90 | 5.94 | 4.18 | 0.92 |
|  |  |  |  | 1/4 | 8.81 | 0.233 | 9.9 | 9.9 | 2.44 | 3.02 | 2.01 | 1.11 | 2.48 | 5.08 | 3.52 | 0.93 |
|  |  |  |  | 3/16 | 6.87 | 0.174 | 14.2 | 14.2 | 1.89 | 2.46 | 1.64 | 1.14 | 1.97 | 4.03 | 2.76 | 0.95 |
|  |  |  |  | 1/8 | 4.75 | 0.116 | 22.9 | 22.9 | 1.30 | 1.78 | 1.19 | 1.17 | 1.40 | 2.84 | 1.92 | 0.97 |
| $21 / 2$ | $\times$ | $21 / 2$ | $\times$ | 5/16 | 8.45 | 0.291 | 5.6 | 5.6 | 2.35 | 1.82 | 1.45 | 0.879 | 1.88 | 3.20 | 2.74 | 0.75 |
|  |  |  |  | 1/4 | 7.11 | 0.233 | 7.7 | 7.7 | 1.97 | 1.63 | 1.30 | 0.908 | 1.63 | 2.79 | 2.35 | 0.77 |
|  |  |  |  | 3/16 | 5.59 | 0.174 | 11.4 | 11.4 | 1.54 | 1.35 | 1.08 | 0.937 | 1.32 | 2.25 | 1.86 | 0.78 |
|  |  |  |  | 1/8 | 3.90 | 0.116 | 18.6 | 18.6 | 1.07 | 0.998 | 0.798 | 0.965 | 0.947 | 1.61 | 1.31 | 0.80 |
| $21 / 4$ | $\times$ | $21 / 4$ | $\times$ | 1/4 | 6.26 | 0.233 | 6.7 | 6.7 | 1.74 | 1.13 | 1.00 | 0.805 | 1.28 | 1.96 | 1.85 | 0.68 |
|  |  |  |  | 3/16 | 4.96 | 0.174 | 9.9 | 9.9 | 1.37 | 0.952 | 0.847 | 0.835 | 1.04 | 1.60 | 1.48 | 0.70 |
|  |  |  |  | 1/8 | 3.48 | 0.116 | 16.4 | 16.4 | 0.96 | 0.712 | 0.633 | 0.863 | 0.755 | 1.15 | 1.05 | 0.72 |
| 2 | $\times$ | 2 | $\times$ | 1/4 | 5.41 | 0.233 | 5.6 | 5.6 | 1.51 | 0.745 | 0.745 | 0.703 | 0.964 | 1.31 | 1.41 | 0.60 |
|  |  |  |  | 3/16 | 4.32 | 0.174 | 8.5 | 8.5 | 1.19 | 0.640 | 0.640 | 0.732 | 0.797 | 1.09 | 1.14 | 0.62 |
|  |  |  |  | 1/8 | 3.05 | 0.116 | 14.2 | 14.2 | 0.84 | 0.486 | 0.486 | 0.761 | 0.584 | 0.796 | 0.817 | 0.63 |
| $13 / 4$ | $\times$ | $13 / 4$ | $\times$ | 3/16 | 3.68 | 0.174 | 7.1 | 7.1 | 1.02 | 0.405 | 0.462 | 0.630 | 0.585 | 0.699 | 0.844 | 0.53 |
| $15 / 8$ | $\times$ | $15 / 8$ | $\times$ | 3/16 | 3.36 | 0.174 | 6.3 | 6.3 | 0.93 | 0.312 | 0.384 | 0.579 | 0.491 | 0.544 | 0.712 | 0.49 |
|  |  |  |  | 1/8 | 2.42 | 0.116 | 11.0 | 11.0 | 0.67 | 0.246 | 0.302 | 0.608 | 0.370 | 0.410 | 0.522 | 0.51 |
| $11 / 2$ | $\times$ | $11 / 2$ | $\times$ | 3/16 | 3.04 | 0.174 | 5.6 | 5.6 | 0.84 | 0.235 | 0.314 | 0.528 | 0.406 | 0.414 | 0.592 | 0.45 |
|  |  |  |  | 1/8 | 2.20 | 0.116 | 9.9 | 9.9 | 0.61 | 0.188 | 0.251 | 0.556 | 0.309 | 0.316 | 0.438 | 0.47 |
| 1 1/4 | $\times$ | $11 / 4$ | $\times$ | 3/16 | 2.40 | 0.174 | 4.2 | 4.2 | 0.67 | 0.121 | 0.194 | 0.425 | 0.259 | 0.218 | 0.383 | 0.37 |
|  |  |  |  | 1/8 | 1.78 | 0.116 | 7.8 | 7.8 | 0.49 | 0.101 | 0.162 | 0.454 | 0.204 | 0.174 | 0.292 | 0.38 |

6.4.1 Calculating the Weight and Size of Metric Square High-Strength Steel Sections


| Nominal Size |  | Mass <br> Per <br> Meter | Weight <br> Per <br> Meter | Design <br> Wall <br> Thickness | b/t | h/t | Area | $1 / 10^{6}$ | S/10 ${ }^{3}$ | r | $\mathrm{Z} / 10^{3}$ | Torsional Stiffness Constant $\mathrm{J} / 10^{3}$ | Torsional Shear Constant C/10 ${ }^{3}$ | Surface <br> Area <br> Per <br> Meter |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| US Customary | SI/Metric |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Inches | Millimeters | $k g$ | $k N$ | mm |  |  | $m m^{2}$ | $m m^{4}$ | $m m^{3}$ | mm | $m m^{3}$ | $m m^{4}$ | $m m^{3}$ | $m^{2}$ |
| $32 \times 32 \times 5 / 8^{*}$ | $812.8 \times 812.8 \times 15.9 *$ | 387.3 | 3.798 | 15.9 | 48.1 | 48.1 | 49300 | 5140 | 12700 | 323 | 14600 | 7960000 | 20100 | 3.15 |
| 1/2* | 12.7* | 313.6 | 3.076 | 12.7 | 61.0 | 61.0 | 40000 | 4220 | 10400 | 325 | 11900 | 6440000 | 16200 | 3.19 |
| 3/8* | 9.5* | 236.6 | 2.320 | 9.5 | 82.6 | 82.6 | 30100 | 3220 | 7920 | 327 | 9040 | 4890000 | 12300 | 3.20 |
| $30 \times 30 \times 5 / 8^{*}$ | $762.0 \times 762.0 \times 15.9 *$ | 361.9 | 3.549 | 15.9 | 44.9 | 44.9 | 46100 | 4210 | 11000 | 302 | 12800 | 6520000 | 17600 | 2.95 |
| 1/2* | 12.7* | 293.4 | 2.877 | 12.7 | 57.0 | 57.0 | 37400 | 3460 | 9090 | 304 | 10400 | 5280000 | 14200 | 2.98 |
| 3/8* | 9.5* | 221.4 | 2.171 | 9.5 | 77.2 | 77.2 | 28200 | 2640 | 6940 | 306 | 7920 | 4020000 | 10700 | 3.00 |
| $28 \times 28 \times 5.8^{*}$ | $711.2 \times 711.2 \times 15.9 *$ | 336.6 | 3.301 | 15.9 | 41.7 | 41.7 | 42900 | 3390 | 9550 | 281 | 11100 | 5270000 | 15300 | 2.75 |
| 1/2* | 12.7* | 273.1 | 2.678 | 12.7 | 53.0 | 53.0 | 34800 | 2800 | 7870 | 284 | 9050 | 4270000 | 12400 | 2.78 |
| 3/8* | 9.5* | 206.3 | 2.023 | 9.5 | 71.9 | 71.9 | 26300 | 2140 | 6020 | 285 | 6880 | 3250000 | 9350 | 2.80 |
| $26 \times 26 \times 5 / 8^{*}$ | $660.4 \times 660.4 \times 15.9 *$ | 311.2 | 3.052 | 15.9 | 38.5 | 38.5 | 39600 | 2690 | 8150 | 261 | 9460 | 4190000 | 13100 | 2.54 |
| 1/2* | 12.7* | 252.9 | 2.480 | 12.7 | 49.0 | 49.0 | 32200 | 2230 | 6740 | 263 | 7760 | 340000 | 10600 | 2.58 |
| 3/8* | 9.5* | 191.1 | 1.874 | 9.5 | 66.5 | 66.5 | 24300 | 1700 | 5160 | 265 | 5910 | 2590000 | 8040 | 2.59 |
| $24 \times 24 \times 5 / 8^{*}$ | $609.6 \times 609.6 \times 15.9 *$ | 285.8 | 2.803 | 15.9 | 35.3 | 35.3 | 36400 | 2090 | 6870 | 240 | 8000 | 3270000 | 11100 | 2.34 |
| 1/2* | 12.7* | 232.6 | 2.281 | 12.7 | 45.0 | 45.0 | 29600 | 1740 | 5700 | 242 | 6580 | 2660000 | 9030 | 2.37 |
| 3/8* | 9.5* | 176.0 | 1.726 | 9.5 | 61.2 | 61.2 | 22400 | 1330 | 4370 | 244 | 5010 | 2030000 | 6830 | 2.39 |
| $22 \times 22 \times 5 / 8^{*}$ | $558.8 \times 558.8 \times 15.9 *$ | 260.5 | 2.554 | 15.9 | 32.1 | 32.1 | 33200 | 1590 | 5700 | 219 | 6650 | 2490000 | 9310 | 2.14 |
| 1/2* | 12.7* | 212.3 | 2.082 | 12.7 | 41.0 | 41.0 | 27000 | 1330 | 4750 | 221 | 5490 | 2030000 | 7550 | 2.17 |
| 3/8* | 9.5* | 160.8 | 1.577 | 9.5 | 55.8 | 55.8 | 20500 | 1020 | 3650 | 223 | 4190 | 1560000 | 5720 | 2.19 |
| $20 \times 20 \times 5 / 8^{*}$ | $508.0 \times 508.0 \times 15.9 *$ | 235.1 | 2.306 | 15.9 | 28.9 | 28.9 | 30000 | 1180 | 4640 | 198 | 5430 | 1850000 | 7630 | 1.93 |
| 1/2* | 12.7* | 192.1 | 1.884 | 12.7 | 37.0 | 37.0 | 24500 | 985 | 3880 | 201 | 4500 | 1510000 | 6210 | 1.97 |
| 3/8* | 9.5* | 145.7 | 1.428 | 9.5 | 50.5 | 50.5 | 18600 | 760 | 2990 | 202 | 3440 | 1160000 | 4710 | 1.98 |
| $18 \times 18 \times 5 / 8^{*}$ | $457.2 \times 457.2 \times 15.9 *$ | 209.8 | 2.057 | 15.9 | 25.8 | 25.8 | 26700 | 842 | 3680 | 177 | 4340 | 1330000 | 6130 | 1.73 |
| 1/2* | 12.7* | 171.8 | 1.685 | 12.7 | 33.0 | 33.0 | 21900 | 708 | 3100 | 180 | 3610 | 1090000 | 5000 | 1.76 |
| 3/8* | 9.5* | 130.5 | 1.280 | 9.5 | 45.1 | 45.1 | 16600 | 548 | 2400 | 182 | 2770 | 840000 | 3800 | 1.78 |
| $16 \times 16 \times 5 / 8$ | $406.4 \times 406.4 \times 15.9$ | 189.9 | 1.862 | 14.8 | 24.5 | 24.5 | 22600 | 571 | 2810 | 159 | 3290 | 854000 | 4530 | 1.57 |
| 1/2* | 12.7 | 153.7 | 1.508 | 11.8 | 31.4 | 31.4 | 18300 | 469 | 2310 | 160 | 2680 | 703000 | 3670 | 1.59 |
| 3/8 | 9.5 | 116.6 | 1.143 | 8.9 | 42.7 | 42.7 | 13900 | 365 | 1790 | 162 | 2070 | 547000 | 2810 | 1.60 |


| Nominal Size |  | Mass <br> Per <br> Meter | Weight <br> Per <br> Meter | Design <br> Wall <br> Thickness | b/t | h/t | Area | $1 / 10^{6}$ | S/10 ${ }^{3}$ | r | $\mathrm{Z} / 10^{3}$ | Torsional Stiffness Constant $\mathrm{J} / 10^{3}$ | Torsional <br> Shear <br> Constant C/10 ${ }^{3}$ | Surface <br> Area <br> Per <br> Meter |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| US Customary | SI/Metric |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Inches | Millimeters | $k g$ | $k N$ | mm |  |  | $m m^{2}$ | $m m^{4}$ | $m m^{3}$ | $m m$ | $m m^{3}$ | $m m^{4}$ | $m m^{3}$ | $m^{2}$ |
| 5/16 | 7.9 | 97.6 | 0.957 | 7.4 | 51.9 | 51.9 | 11700 | 308 | 1510 | 162 | 1740 | 461000 | 2350 | 1.60 |
| $14 \times 14 \times 5 / 8$ | $355.6 \times 355.6 \times 15.9$ | 164.5 | 1.613 | 14.8 | 21.0 | 21.0 | 19600 | 374 | 2100 | 138 | 2480 | 559000 | 3430 | 1.37 |
| 1/2 | 12.7 | 133.5 | 1.309 | 11.8 | 27.1 | 27.1 | 15900 | 309 | 1740 | 140 | 2030 | 463000 | 2780 | 1.38 |
| 3/8 | 9.5 | 101.4 | 0.995 | 8.9 | 37.0 | 37.0 | 12100 | 241 | 1360 | 141 | 1570 | 361000 | 2140 | 1.39 |
| 5/16 | 7.9 | 85.0 | 0.833 | 7.4 | 45.1 | 45.1 | 10200 | 204 | 1150 | 142 | 1320 | 306000 | 1790 | 1.40 |
| $12 \times 12 \times 5 / 8$ | $304.8 \times 304.8 \times 15.9$ | 139.1 | 1.364 | 14.8 | 17.6 | 17.6 | 16600 | 229 | 1500 | 117 | 1780 | 341000 | 2480 | 1.17 |
| 1/2 | 12.7 | 113.2 | 1.110 | 11.8 | 22.8 | 22.8 | 13500 | 190 | 1250 | 119 | 1470 | 284000 | 2020 | 1.18 |
| 3/8 | 9.5 | 86.3 | 0.846 | 8.9 | 31.2 | 31.2 | 10300 | 149 | 979 | 120 | 1140 | 224000 | 1560 | 1.19 |
| 5/16 | 7.9 | 72.4 | 0.710 | 7.4 | 38.2 | 38.2 | 8660 | 127 | 831 | 121 | 961 | 190000 | 1310 | 1.19 |
| 1/4 | 6.4 | 59.1 | 0.580 | 5.9 | 48.7 | 48.7 | 6960 | 103 | 676 | 122 | 777 | 154000 | 1050 | 1.20 |
| $10 \times 10 \times 5 / 8$ | $254.0 \times 254.0 \times 15.9$ | 113.8 | 1.116 | 14.8 | 14.2 | 14.2 | 13600 | 127 | 1000 | 96.6 | 1200 | 189000 | 1680 | 0.97 |
| 1/2 | 12.7 | 93.0 | 0.912 | 11.8 | 18.5 | 18.5 | 11100 | 106 | 838 | 98.0 | 994 | 159000 | 1380 | 0.98 |
| 3/8 | 9.5 | 71.1 | 0.697 | 8.9 | 25.5 | 25.5 | 8520 | 84.3 | 664 | 99.4 | 777 | 126000 | 1070 | 0.99 |
| 5/16 | 7.9 | 59.8 | 0.586 | 7.4 | 31.3 | 31.3 | 7160 | 71.8 | 566 | 100 | 657 | 108000 | 898 | 0.99 |
| 1/4 | 6.4 | 48.9 | 0.480 | 5.9 | 40.1 | 40.1 | 5770 | 58.7 | 462 | 101 | 534 | 88000 | 726 | 1.00 |
| 3/16 | 4.8 | 37.1 | 0.364 | 4.4 | 54.7 | 54.7 | 4340 | 44.8 | 353 | 102 | 405 | 67200 | 548 | 1.00 |
| $9 \times 9 \times 1 / 2$ | $228.6 \times 228.6 \times 12.7$ | 82.8 | 0.812 | 11.8 | 16.4 | 16.4 | 9870 | 75.9 | 664 | 87.7 | 793 | 113000 | 1100 | 0.87 |
| 3/8 | 9.5 | 63.5 | 0.623 | 8.9 | 22.7 | 22.7 | 7620 | 60.4 | 529 | 89.1 | 622 | 90400 | 856 | 0.88 |
| 5/16 | 7.9 | 53.5 | 0.525 | 7.4 | 27.9 | 27.9 | 6410 | 51.7 | 452 | 89.8 | 527 | 77300 | 723 | 0.89 |
| 1/4 | 6.4 | 43.8 | 0.430 | 5.9 | 35.7 | 35.7 | 5170 | 42.3 | 370 | 90.5 | 429 | 63400 | 584 | 0.89 |
| 3/16 | 4.8 | 33.3 | 0.326 | 4.4 | 49.0 | 49.0 | 3900 | 32.4 | 284 | 91.2 | 326 | 48600 | 442 | 0.90 |
| $8 \times 8 \times 5 / 8$ | $203.2 \times 203.2 \times 15.9$ | 88.4 | 0.867 | 14.8 | 10.7 | 10.7 | 10600 | 60.9 | 599 | 75.8 | 734 | 90100 | 1040 | 0.76 |
| 1/2 | 12.7 | 72.7 | 0.713 | 11.8 | 14.2 | 14.2 | 8680 | 51.8 | 510 | 77.3 | 614 | 77100 | 858 | 0.77 |
| 3/8 | 9.5 | 56.0 | 0.549 | 8.9 | 19.8 | 19.8 | 6710 | 41.6 | 409 | 78.7 | 484 | 62100 | 669 | 0.78 |
| 5/16 | 7.9 | 47.2 | 0.463 | 7.4 | 24.5 | 24.5 | 5650 | 35.7 | 351 | 79.4 | 412 | 53400 | 566 | 0.79 |
| 1/4 | 6.4 | 38.7 | 0.380 | 5.9 | 31.4 | 31.4 | 4570 | 29.3 | 289 | 80.1 | 336 | 43900 | 459 | 0.79 |
| 3/16 | 4.8 | 29.4 | 0.289 | 4.4 | 43.2 | 43.2 | 3450 | 22.6 | 222 | 80.9 | 256 | 33800 | 347 | 0.80 |
| $7 \times 7 \times 5 / 8$ | $177.8 \times 177.8 \times 15.9$ | 75.7 | 0.743 | 14.8 | 9.0 | 9.0 | 9090 | 38.9 | 438 | 65.4 | 543 | 57300 | 774 | 0.66 |
| 1/2 | 12.7 | 62.6 | 0.614 | 11.8 | 12.1 | 12.1 | 7480 | 33.5 | 377 | 66.9 | 457 | 49700 | 644 | 0.67 |
| 3/8 | 9.5 | 48.4 | 0.474 | 8.9 | 17.0 | 17.0 | 5810 | 27.1 | 305 | 68.3 | 363 | 40400 | 505 | 0.68 |
| 5/16 | 7.9 | 40.9 | 0.401 | 7.4 | 21.0 | 21.0 | 4900 | 23.4 | 263 | 69.0 | 310 | 34900 | 428 | 0.69 |
| 1/4 | 6.4 | 33.6 | 0.330 | 5.9 | 27.1 | 27.1 | 3970 | 19.3 | 217 | 69.8 | 254 | 28900 | 348 | 0.69 |
| 3/16 | 4.8 | 25.6 | 0.251 | 4.4 | 37.4 | 37.4 | 3000 | 14.9 | 168 | 70.5 | 194 | 22400 | 264 | 0.70 |
| $6 \times 6 \times 5 / 8$ | $152.4 \times 152.4 \times 15.9$ | 63.0 | 0.618 | 14.8 | 7.3 | 7.3 | 7580 | 23.0 | 301 | 55.0 | 381 | 33500 | 548 | 0.56 |
| 1/2 | 12.7 | 52.4 | 0.514 | 11.8 | 9.9 | 9.9 | 6280 | 20.1 | 263 | 56.5 | 324 | 29600 | 460 | 0.57 |
| 3/8 | 9.5 | 40.8 | 0.400 | 8.9 | 14.1 | 14.1 | 4900 | 16.5 | 216 | 57.9 | 260 | 24500 | 364 | 0.58 |
| 5/16 | 7.9 | 34.6 | 0.339 | 7.4 | 17.6 | 17.6 | 4150 | 14.3 | 188 | 58.7 | 223 | 21300 | 310 | 0.58 |
| 1/4 | 6.4 | 28.5 | 0.280 | 5.9 | 22.8 | 22.8 | 3370 | 11.9 | 156 | 59.4 | 183 | 17800 | 252 | 0.59 |


| 3/16 | 4.8 | 21.8 | 0.214 | 4.4 | 31.6 | 31.6 | 2550 | 9.23 | 121 | 60.1 | 141 | 13800 | 192 | 0.59 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1/8 | 3.2 | 14.8 | 0.145 | 3.0 | 47.8 | 47.8 | 1770 | 6.54 | 85.8 | 60.8 | 98.7 | 9800 | 134 | 0.60 |
| $51 / 2 \times 51 / 2 \times 3 / 8$ | $139.7 \times 139.7 \times 9.5$ | 37.0 | 0.363 | 8.9 | 12.7 | 12.7 | 4450 | 12.4 | 177 | 52.8 | 215 | 18400 | 302 | 0.53 |
| 5/16 | 7.9 | 31.4 | 0.308 | 7.4 | 15.9 | 15.9 | 3780 | 10.8 | 155 | 53.5 | 185 | 16100 | 257 | 0.53 |
| 1/4 | 6.4 | 26.0 | 0.255 | 5.9 | 20.7 | 20.7 | 3070 | 9.02 | 129 | 54.2 | 152 | 13500 | 210 | 0.54 |
| 3/16 | 4.8 | 19.9 | 0.195 | 4.4 | 28.8 | 28.8 | 2330 | 7.04 | 101 | 54.9 | 117 | 10500 | 161 | 0.54 |
| 1/8 | 3.2 | 13.5 | 0.132 | 3.0 | 43.6 | 43.6 | 1620 | 5.00 | 71.6 | 55.6 | 82.5 | 7500 | 112 | 0.55 |
| $5 \times 5 \times 1 / 2$ | $127.0 \times 127.0 \times 12.7$ | 42.3 | 0.415 | 11.8 | 7.8 | 7.8 | 5080 | 10.8 | 170 | 46.1 | 214 | 15800 | 307 | 0.47 |
| 3/8 | 9.5 | 33.2 | 0.326 | 8.9 | 11.3 | 11.3 | 4000 | 9.05 | 143 | 47.6 | 174 | 13400 | 246 | 0.48 |
| 5/16 | 7.9 | 28.3 | 0.277 | 7.4 | 14.2 | 14.2 | 3400 | 7.93 | 125 | 48.3 | 150 | 11800 | 210 | 0.48 |
| 1/4 | 6.4 | 23.4 | 0.230 | 5.9 | 18.5 | 18.5 | 2770 | 6.65 | 105 | 49.0 | 124 | 9930 | 172 | 0.49 |
| 3/16 | 4.8 | 18.0 | 0.176 | 4.4 | 25.9 | 25.9 | 2110 | 5.22 | 82.2 | 49.7 | 96.1 | 7810 | 132 | 0.49 |
| 1/8 | 3.2 | 12.2 | 0.120 | 3.0 | 39.3 | 39.3 | 1460 | 3.72 | 58.6 | 50.4 | 67.7 | 5580 | 92.2 | 0.50 |
| $41 / 2 \times 41 / 2 \times 1 / 2$ | $114.3 \times 114.3 \times 12.7$ | 37.3 | 0.365 | 11.8 | 6.7 | 6.7 | 4480 | 7.51 | 131 | 40.9 | 167 | 10900 | 242 | 0.42 |
| 3/8 | 9.5 | 29.4 | 0.289 | 8.9 | 9.8 | 9.8 | 3550 | 6.37 | 111 | 42.4 | 137 | 9400 | 195 | 0.43 |
| 5/16 | 7.9 | 25.1 | 0.246 | 7.4 | 12.4 | 12.4 | 3020 | 5.62 | 98.3 | 43.1 | 119 | 8340 | 168 | 0.43 |
| 1/4 | 6.4 | 20.9 | 0.205 | 5.9 | 16.4 | 16.4 | 2470 | 4.74 | 83.0 | 43.8 | 99.1 | 7070 | 138 | 0.44 |
| 3/16 | 4.8 | 16.0 | 0.157 | 4.4 | 23.0 | 23.0 | 1880 | 3.74 | 65.5 | 44.6 | 76.9 | 5600 | 106 | 0.44 |
| 1/8 | 3.2 | 11.0 | 0.107 | 3.0 | 35.1 | 35.1 | 1310 | 2.69 | 47.0 | 45.2 | 54.4 | 4020 | 74.2 | 0.45 |
| $4 \times 4 \times 1 / 2$ | $101.6 \times 101.6 \times 12.7$ | 32.2 | 0.316 | 11.8 | 5.6 | 5.6 | 3880 | 4.95 | 97.5 | 35.7 | 126 | 7120 | 184 | 0.37 |
| 3/8 | 9.5 | 25.6 | 0.252 | 8.9 | 8.4 | 8.4 | 3100 | 4.28 | 84.2 | 37.2 | 105 | 6280 | 150 | 0.38 |
| 5/16 | 7.9 | 22.0 | 0.216 | 7.4 | 10.7 | 10.7 | 2650 | 3.81 | 74.9 | 37.9 | 91.7 | 5630 | 130 | 0.38 |
| 1/4 | 6.4 | 18.3 | 0.179 | 5.9 | 14.2 | 14.2 | 2170 | 3.24 | 63.8 | 38.6 | 76.7 | 4820 | 107 | 0.39 |
| 3/16 | 4.8 | 14.1 | 0.139 | 4.4 | 20.1 | 20.1 | 1660 | 2.57 | 50.7 | 39.4 | 59.9 | 3850 | 82.8 | 0.39 |
| 1/8 | 3.2 | 9.7 | 0.095 | 3.0 | 30.9 | 30.9 | 1160 | 1.86 | 36.6 | 40.0 | 42.6 | 2790 | 58.2 | 0.40 |
| $31 / 2 \times 31 / 2 \times 3 / 8$ | $88.9 \times 88.9 \times 9.5$ | 21.9 | 0.214 | 8.9 | 7.0 | 7.0 | 2640 | 2.70 | 60.8 | 32.0 | 77.1 | 3940 | 111 | 0.33 |
| 5/16 | 7.9 | 18.8 | 0.185 | 7.4 | 9.0 | 9.0 | 2270 | 2.43 | 54.7 | 32.7 | 67.9 | 3580 | 96.7 | 0.33 |
| 1/4 | 6.4 | 15.8 | 0.154 | 5.9 | 12.1 | 12.1 | 1870 | 2.09 | 47.1 | 33.5 | 57.2 | 3100 | 80.5 | 0.34 |
| 3/16 | 4.8 | 12.2 | 0.120 | 4.4 | 17.2 | 17.2 | 1440 | 1.68 | 37.8 | 34.2 | 45.0 | 2510 | 62.5 | 0.34 |
| 1/8 | 3.2 | 8.4 | 0.082 | 3.0 | 26.6 | 26.6 | 1010 | 1.22 | 27.6 | 34.9 | 32.2 | 1830 | 44.2 | 0.35 |
| $3 \times 3 \times 3 / 8$ | $76.2 \times 76.2 \times 9.5$ | 18.1 | 0.177 | 8.9 | 5.6 | 5.6 | 2190 | 1.57 | 41.2 | 26.8 | 53.4 | 2260 | 77.9 | 0.27 |
| 5/16 | 7.9 | 15.7 | 0.154 | 7.4 | 7.3 | 7.3 | 1900 | 1.44 | 37.7 | 27.5 | 47.6 | 2090 | 68.5 | 0.28 |
| 1/4 | 6.4 | 13.2 | 0.129 | 5.9 | 9.9 | 9.9 | 1570 | 1.25 | 32.9 | 28.3 | 40.5 | 1850 | 57.5 | 0.28 |
| 3/16 | 4.8 | 10.3 | 0.101 | 4.4 | 14.3 | 14.3 | 1210 | 1.02 | 26.8 | 29.0 | 32.2 | 1520 | 45.0 | 0.29 |
| 1/8 | 3.2 | 7.1 | 0.070 | 3.0 | 22.4 | 22.4 | 855 | 0.753 | 19.8 | 29.7 | 23.3 | 1130 | 32.0 | 0.29 |
| $21 / 2 \times 21 / 2 \times 5 / 16$ | $63.5 \times 63.5 \times 7.9$ | 12.5 | 0.123 | 7.4 | 5.6 | 5.6 | 1520 | 0.757 | 23.8 | 22.3 | 30.9 | 1090 | 45.0 | 0.23 |
| 1/4 | 6.4 | 10.6 | 0.104 | 5.9 | 7.8 | 7.8 | 1270 | 0.676 | 21.3 | 23.1 | 26.7 | 988 | 38.4 | 0.23 |
| 3/16 | 4.8 | 8.4 | 0.082 | 4.4 | 11.4 | 11.4 | 990 | 0.561 | 17.7 | 23.8 | 21.6 | 832 | 30.4 | 0.24 |
| 1/8 | 3.2 | 5.9 | 0.057 | 3.0 | 18.2 | 18.2 | 703 | 0.421 | 13.3 | 24.5 | 15.8 | 629 | 21.9 | 0.24 |
| $21 / 4 \times 21 / 4 \times 1 / 4$ | $57.2 \times 57.2 \times 6.4$ | 9.4 | 0.092 | 5.9 | 6.7 | 6.7 | 1120 | 0.471 | 16.5 | 20.5 | 20.9 | 683 | 30.3 | 0.21 |
| 3/16 | 4.8 | 7.4 | 0.073 | 4.4 | 10.0 | 10.0 | 879 | 0.396 | 13.9 | 21.2 | 17.1 | 585 | 24.2 | 0.21 |
| 1/8 | 3.2 | 5.2 | 0.051 | 3.0 | 16.1 | 16.1 | 627 | 0.301 | 10.5 | 21.9 | 12.6 | 449 | 17.5 | 0.22 |

6.5.0 Calculating the Weight and Size of U.S. Rectangular High-Strength Steel Sections



| 20 | $\times$ | 8 | $\times$ | 5/8 | 110.36 | 0.581 | 10.8 | 31.4 | 30.3 | 1440 | 144 | 6.89 | 185 | 338 | 84.6 | 3.34 | 96.4 | 916 | 167 | 4.50 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | 1/2 | 89.68 | 0.465 | 14.2 | 40.0 | 24.6 | 1190 | 119 | 6.96 | 152 | 283 | 70.8 | 3.39 | 79.5 | 757 | 137 | 4.53 |
|  |  |  |  | 3/8 | 68.31 | 0.349 | 19.9 | 54.3 | 18.7 | 926 | 92.6 | 7.03 | 117 | 222 | 55.6 | 3.44 | 61.5 | 586 | 105 | 4.57 |
|  |  |  |  | 5/16 | 57.36 | 0.291 | 24.5 | 65.7 | 15.7 | 786 | 78.6 | 7.07 | 98.6 | 189 | 47.4 | 3.47 | 52.0 | 496 | 88.3 | 4.58 |
| 20 | $\times$ | 4 | $\times$ | 1/2 | 76.07 | 0.465 | 5.6 | 40.0 | 20.9 | 838 | 83.8 | 6.33 | 115 | 58.7 | 29.3 | 1.68 | 34.0 | 195 | 63.8 | 3.87 |
|  |  |  |  | 3/8 | 58.10 | 0.349 | 8.5 | 54.3 | 16.0 | 657 | 65.7 | 6.42 | 89.3 | 47.6 | 23.8 | 1.73 | 26.8 | 156 | 49.9 | 3.90 |
|  |  |  |  | 5/16 | 48.86 | 0.291 | 10.7 | 65.7 | 13.4 | 560 | 56.0 | 6.46 | 75.6 | 41.2 | 20.6 | 1.75 | 22.9 | 134 | 42.4 | 3.92 |
| 18 | $\times$ | 12 | $\times$ | 5/8* | 115.21 | 0.625 | 16.2 | 25.8 | 33.9 | 1450 | 161 | 6.55 | 199 | 783 | 131 | 4.81 | 152 | 1740 | 243 | 4.68 |
|  |  |  |  | 1/2* | 95.03 | 0.500 | 21.0 | 33.0 | 27.9 | 1240 | 138 | 6.67 | 168 | 668 | 111 | 4.89 | 127 | 1430 | 200 | 4.79 |
|  |  |  |  | 3/8* | 72.59 | 0.375 | 29.0 | 45.0 | 21.3 | 971 | 108 | 6.75 | 130 | 524 | 87.3 | 4.95 | 98.6 | 1100 | 153 | 4.84 |
| 18 | $\times$ | 6 | $\times$ | 5/8 | 93.34 | 0.581 | 7.3 | 28.0 | 25.7 | 923 | 103 | 6.00 | 135 | 158 | 52.6 | 2.48 | 61.0 | 462 | 109 | 3.83 |
|  |  |  |  | 1/2 | 76.07 | 0.465 | 9.9 | 35.7 | 20.9 | 770 | 85.6 | 6.07 | 112 | 134 | 44.6 | 2.53 | 50.7 | 387 | 89.9 | 3.87 |
|  |  |  |  | 3/8 | 58.10 | 0.349 | 14.2 | 48.6 | 16.0 | 602 | 66.9 | 6.15 | 86.4 | 106 | 35.5 | 2.58 | 39.5 | 302 | 69.5 | 3.90 |
|  |  |  |  | 5/16 | 48.86 | 0.291 | 17.6 | 58.9 | 13.4 | 513 | 57.0 | 6.18 | 73.1 | 91.3 | 30.4 | 2.61 | 33.5 | 257 | 58.7 | 3.92 |
|  |  |  |  | 1/4 | 39.43 | 0.233 | 22.8 | 74.3 | 10.8 | 419 | 46.5 | 6.22 | 59.4 | 75.1 | 25.0 | 2.63 | 27.3 | 210 | 47.7 | 3.93 |
| 16 | $\times$ | 12 | $\times$ | 5/8* | 106.71 | 0.625 | 16.2 | 22.6 | 31.4 | 1090 | 136 | 5.89 | 167 | 702 | 117 | 4.73 | 137 | 1470 | 215 | 4.34 |
|  |  |  |  | 1/2 | 89.68 | 0.465 | 22.8 | 31.4 | 24.6 | 904 | 113 | 6.06 | 135 | 581 | 96.8 | 4.86 | 111 | 1120 | 166 | 4.53 |
|  |  |  |  | 3/8 | 68.31 | 0.349 | 31.4 | 42.8 | 18.7 | 702 | 87.7 | 6.12 | 104 | 452 | 75.3 | 4.91 | 85.5 | 862 | 127 | 4.57 |
|  |  |  |  | 5/16 | 57.36 | 0.291 | 38.2 | 52.0 | 15.7 | 595 | 74.4 | 6.15 | 87.7 | 384 | 64.0 | 4.94 | 72.2 | 727 | 107 | 4.58 |
| 16 | $\times$ | 8 | $\times$ | 5/8 | 93.34 | 0.581 | 10.8 | 24.5 | 25.7 | 815 | 102 | 5.63 | 129 | 274 | 68.5 | 3.27 | 79.2 | 681 | 132 | 3.83 |
|  |  |  |  | 1/2 | 76.07 | 0.465 | 14.2 | 31.4 | 20.9 | 679 | 84.9 | 5.70 | 106 | 230 | 57.6 | 3.32 | 65.5 | 563 | 108 | 3.87 |
|  |  |  |  | 3/8 | 58.10 | 0.349 | 19.9 | 42.8 | 16.0 | 531 | 66.3 | 5.77 | 82.1 | 181 | 45.3 | 3.37 | 50.8 | 436 | 83.4 | 3.90 |
|  |  |  |  | 5/16 | 48.86 | 0.291 | 24.5 | 52.0 | 13.4 | 451 | 56.4 | 5.80 | 69.4 | 155 | 38.7 | 3.40 | 43.0 | 369 | 70.4 | 3.92 |
| 16 | $\times$ | 4 | $\times$ | 1/2 | 62.46 | 0.465 | 5.6 | 31.4 | 17.2 | 455 | 56.9 | 5.15 | 77.3 | 47.0 | 23.5 | 1.65 | 27.4 | 150 | 50.7 | 3.20 |
|  |  |  |  | 3/8 | 47.90 | 0.349 | 8.5 | 42.8 | 13.2 | 360 | 45.0 | 5.23 | 60.2 | 38.3 | 19.1 | 1.71 | 21.7 | 120 | 39.7 | 3.23 |
|  |  |  |  | 5/16 | 40.35 | 0.291 | 10.7 | 52.0 | 11.1 | 308 | 38.5 | 5.27 | 51.1 | 33.2 | 16.6 | 1.73 | 18.5 | 103 | 33.8 | 3.25 |
| 14 | $\times$ | 12 | $\times$ | 1/2* | 81.42 | 0.500 | 21.0 | 25.0 | 23.9 | 678 | 96.9 | 5.32 | 116 | 536 | 89.3 | 4.73 | 104 | 990 | 154 | 4.12 |
|  |  |  |  | 3/8* | 62.39 | 0.375 | 29.0 | 34.3 | 18.3 | 534 | 76.3 | 5.40 | 90.0 | 422 | 70.4 | 4.80 | 81.2 | 762 | 118 | 4.17 |
| 14 | $\times$ | 10 | $\times$ | 5/8 | 93.34 | 0.581 | 14.2 | 21.1 | 25.7 | 687 | 98.2 | 5.17 | 120 | 407 | 81.5 | 3.98 | 95.1 | 832 | 146 | 3.83 |
|  |  |  |  | 1/2 | 76.07 | 0.465 | 18.5 | 27.1 | 20.9 | 573 | 81.8 | 5.23 | 98.8 | 341 | 68.1 | 4.04 | 78.5 | 685 | 120 | 3.87 |
|  |  |  |  | 3/8 | 58.10 | 0.349 | 25.7 | 37.1 | 16.0 | 447 | 63.9 | 5.29 | 76.3 | 267 | 53.4 | 4.09 | 60.7 | 528 | 91.8 | 3.90 |
|  |  |  |  | 5/16 | 48.86 | 0.291 | 31.4 | 45.1 | 13.4 | 380 | 54.3 | 5.32 | 64.6 | 227 | 45.5 | 4.12 | 51.4 | 446 | 77.4 | 3.92 |
|  |  |  |  | 1/4 | 39.43 | 0.233 | 39.9 | 57.1 | 10.8 | 310 | 44.3 | 5.35 | 52.4 | 186 | 37.2 | 4.14 | 41.8 | 362 | 62.6 | 3.93 |
| 14 | $\times$ | 6 | $\times$ | 5/8 | 76.33 | 0.581 | 7.3 | 21.1 | 21.0 | 478 | 68.2 | 4.77 | 88.7 | 124 | 41.2 | 2.43 | 48.4 | 334 | 83.7 | 3.17 |
|  |  |  |  | 1/2 | 62.46 | 0.465 | 9.9 | 27.1 | 17.2 | 402 | 57.4 | 4.84 | 73.6 | 105 | 35.1 | 2.48 | 40.4 | 279 | 69.3 | 3.20 |
|  |  |  |  | 3/8 | 47.90 | 0.349 | 14.2 | 37.1 | 13.2 | 317 | 45.3 | 4.91 | 57.3 | 84.1 | 28.0 | 2.53 | 31.6 | 219 | 53.7 | 3.23 |
|  |  |  |  | 5/16 | 40.35 | 0.291 | 17.6 | 45.1 | 11.1 | 271 | 38.7 | 4.94 | 48.6 | 72.3 | 24.1 | 2.55 | 26.9 | 186 | 45.5 | 3.25 |
|  |  |  |  | 1/4 | 32.63 | 0.233 | 22.8 | 57.1 | 8.96 | 222 | 31.7 | 4.98 | 39.6 | 59.6 | 19.9 | 2.58 | 22.0 | 152 | 36.9 | 3.27 |
|  |  |  |  | 3/16 | 24.73 | 0.174 | 31.5 | 77.5 | 6.76 | 170 | 24.3 | 5.01 | 30.1 | 45.9 | 15.3 | 2.61 | 16.7 | 116 | 28.0 | 3.28 |
| 14 | $\times$ | 4 | $\times$ | 5/8 | 67.82 | 0.581 | 3.9 | 21.1 | 18.7 | 373 | 53.3 | 4.47 | 73.1 | 47.1 | 23.6 | 1.59 | 28.5 | 148 | 52.6 | 2.83 |
|  |  |  |  | 1/2 | 55.66 | 0.465 | 5.6 | 27.1 | 15.3 | 317 | 45.3 | 4.55 | 61.0 | 41.1 | 20.6 | 1.64 | 24.1 | 127 | 44.1 | 2.87 |

(Continued)


|  |  |  |  | 3/16 | 19.63 | 0.174 | 31.5 | 54.5 | 5.37 | 74.6 | 14.9 | 3.73 | 18.0 | 34.1 | 11.4 | 2.52 | 12.7 | 73.8 | 19.9 | 2.62 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 10 | $\times$ | 5 | $\times$ | 3/8 | 35.13 | 0.349 | 11.3 | 25.7 | 9.67 | 120 | 24.1 | 3.53 | 30.4 | 40.6 | 16.2 | 2.05 | 18.7 | 100 | 31.2 | 2.40 |
|  |  |  |  | 5/16 | 29.72 | 0.291 | 14.2 | 31.4 | 8.17 | 104 | 20.8 | 3.56 | 26.0 | 35.2 | 14.1 | 2.07 | 16.0 | 86.0 | 26.5 | 2.42 |
|  |  |  |  | 1/4 | 24.12 | 0.233 | 18.5 | 39.9 | 6.63 | 85.8 | 17.2 | 3.60 | 21.3 | 29.3 | 11.7 | 2.10 | 13.2 | 70.7 | 21.6 | 2.43 |
|  |  |  |  | 3/16 | 18.35 | 0.174 | 25.7 | 54.5 | 5.02 | 66.2 | 13.2 | 3.63 | 16.3 | 22.7 | 9.09 | 2.13 | 10.1 | 54.1 | 16.5 | 2.45 |
| 10 | $\times$ | 4 | $\times$ | 5/8 | 50.81 | 0.581 | 3.9 | 14.2 | 14.0 | 149 | 29.9 | 3.26 | 40.3 | 33.4 | 16.7 | 1.54 | 20.6 | 95.7 | 36.7 | 2.17 |
|  |  |  |  | 1/2 | 42.05 | 0.465 | 5.6 | 18.5 | 11.6 | 129 | 25.8 | 3.34 | 34.1 | 29.4 | 14.7 | 1.59 | 17.6 | 82.6 | 31.0 | 2.20 |
|  |  |  |  | 3/8 | 32.58 | 0.349 | 8.5 | 25.7 | 8.97 | 104 | 20.8 | 3.41 | 27.0 | 24.3 | 12.1 | 1.64 | 14.0 | 66.5 | 24.4 | 2.23 |
|  |  |  |  | 5/16 | 27.59 | 0.291 | 10.7 | 31.4 | 7.59 | 90.1 | 18.0 | 3.44 | 23.1 | 21.2 | 10.6 | 1.67 | 12.1 | 57.3 | 20.9 | 2.25 |
|  |  |  |  | 1/4 | 22.42 | 0.233 | 14.2 | 39.9 | 6.17 | 74.7 | 14.9 | 3.48 | 19.0 | 17.7 | 8.87 | 1.70 | 9.96 | 47.4 | 17.1 | 2.27 |
|  |  |  |  | 3/16 | 17.08 | 0.174 | 20.0 | 54.5 | 4.67 | 57.8 | 11.6 | 3.52 | 14.6 | 13.9 | 6.93 | 1.72 | 7.66 | 36.5 | 13.1 | 2.28 |
| 10 | $\times 31 / 2$ |  | $\times$ | 3/16 | 16.44 | 0.174 | 17.1 | 54.5 | 4.50 | 53.6 | 10.7 | 3.45 | 13.7 | 10.3 | 5.89 | 1.51 | 6.52 | 28.6 | 11.4 | 2.20 |
| 10 | $\times$ | 3 | $\times$ | 3/8 | 30.03 | 0.349 | 5.6 | 25.7 | 8.27 | 88.0 | 17.6 | 3.26 | 23.7 | 12.4 | 8.27 | 1.22 | 9.73 | 37.8 | 17.7 | 2.07 |
|  |  |  |  | 5/16 | 25.46 | 0.291 | 7.3 | 31.4 | 7.01 | 76.3 | 15.3 | 3.30 | 20.3 | 11.0 | 7.30 | 1.25 | 8.42 | 33.0 | 15.2 | 2.08 |
|  |  |  |  | 1/4 | 20.72 | 0.233 | 9.9 | 39.9 | 5.70 | 63.6 | 12.7 | 3.34 | 16.7 | 9.28 | 6.18 | 1.28 | 6.99 | 27.6 | 12.5 | 2.10 |
|  |  |  |  | 3/16 | 15.80 | 0.174 | 14.2 | 54.5 | 4.32 | 49.4 | 9.87 | 3.38 | 12.8 | 7.33 | 4.89 | 1.30 | 5.41 | 21.5 | 9.64 | 2.12 |
|  |  |  |  | 1/8 | 10.71 | 0.116 | 22.9 | 83.2 | 2.93 | 34.2 | 6.83 | 3.42 | 8.80 | 5.16 | 3.44 | 1.33 | 3.74 | 14.9 | 6.61 | 2.13 |
| 10 | $\times$ | 2 | $\times$ | 3/8 | 27.48 | 0.349 | 2.7 | 25.7 | 7.58 | 71.7 | 14.3 | 3.08 | 20.3 | 4.69 | 4.69 | 0.786 | 5.76 | 15.9 | 11.0 | 1.90 |
|  |  |  |  | 5/16 | 23.34 | 0.291 | 3.9 | 31.4 | 6.43 | 62.6 | 12.5 | 3.12 | 17.5 | 4.24 | 4.24 | 0.812 | 5.06 | 14.2 | 9.56 | 1.92 |
|  |  |  |  | 1/4 | 19.02 | 0.233 | 5.6 | 39.9 | 5.24 | 52.5 | 10.5 | 3.17 | 14.4 | 3.67 | 3.67 | 0.837 | 4.26 | 12.2 | 7.99 | 1.93 |
|  |  |  |  | 3/16 | 14.53 | 0.174 | 8.5 | 54.5 | 3.98 | 41.0 | 8.19 | 3.21 | 11.1 | 2.97 | 2.97 | 0.864 | 3.34 | 9.74 | 6.22 | 1.95 |
| 9 | $\times$ | 7 | $\times$ | 5/8 | 59.32 | 0.581 | 9.0 | 12.5 | 16.4 | 174 | 38.7 | 3.26 | 48.3 | 117 | 33.5 | 2.68 | 40.5 | 235 | 62.0 | 2.50 |
|  |  |  |  | 1/2 | 48.85 | 0.465 | 12.1 | 16.4 | 13.5 | 149 | 33.0 | 3.32 | 40.5 | 100 | 28.7 | 2.73 | 34.0 | 197 | 51.5 | 2.53 |
|  |  |  |  | 3/8 | 37.69 | 0.349 | 17.1 | 22.8 | 10.4 | 119 | 26.4 | 3.38 | 31.8 | 80.4 | 23.0 | 2.78 | 26.7 | 154 | 40.0 | 2.57 |
|  |  |  |  | 5/16 | 31.84 | 0.291 | 21.1 | 27.9 | 8.76 | 102 | 22.6 | 3.41 | 27.1 | 69.2 | 19.8 | 2.81 | 22.8 | 131 | 33.9 | 2.58 |
|  |  |  |  | 1/4 | 25.32 | 0.233 | 27.0 | 35.6 | 7.10 | 84.1 | 18.7 | 3.44 | 22.2 | 57.2 | 16.3 | 2.84 | 18.7 | 107 | 27.6 | 2.60 |
|  |  |  |  | 3/16 | 19.63 | 0.174 | 37.2 | 48.7 | 5.37 | 64.7 | 14.4 | 3.47 | 16.9 | 44.1 | 12.6 | 2.87 | 14.3 | 81.7 | 20.9 | 2.62 |
| 9 | $\times$ | 5 | $\times$ | 5/8 | 50.81 | 0.581 | 5.6 | 12.5 | 14.0 | 133 | 29.6 | 3.08 | 38.5 | 51.9 | 20.8 | 1.92 | 25.3 | 128 | 42.5 | 2.17 |
|  |  |  |  | 1/2 | 42.05 | 0.465 | 7.8 | 16.4 | 11.6 | 115 | 25.5 | 3.14 | 32.5 | 45.2 | 18.1 | 1.97 | 21.5 | 109 | 35.6 | 2.20 |
|  |  |  |  | 3/8 | 32.58 | 0.349 | 11.3 | 22.8 | 8.97 | 92.5 | 20.5 | 3.21 | 25.7 | 36.8 | 14.7 | 2.03 | 17.1 | 86.9 | 27.9 | 2.23 |
|  |  |  |  | 5/16 | 27.59 | 0.291 | 14.2 | 27.9 | 7.59 | 79.8 | 17.7 | 3.24 | 22.0 | 32.0 | 12.8 | 2.05 | 14.6 | 74.4 | 23.8 | 2.25 |
|  |  |  |  | 1/4 | 22.42 | 0.233 | 18.5 | 35.6 | 6.17 | 66.1 | 14.7 | 3.27 | 18.1 | 26.6 | 10.6 | 2.08 | 12.0 | 61.2 | 19.4 | 2.27 |
|  |  |  |  | 3/16 | 17.08 | 0.174 | 25.7 | 48.7 | 4.67 | 51.1 | 11.4 | 3.31 | 13.8 | 20.7 | 8.28 | 2.10 | 9.25 | 46.9 | 14.8 | 2.28 |
| 9 | $\times$ | 3 | $\times$ | 1/12 | 35.24 | 0.465 | 3.5 | 16.4 | 9.74 | 80.8 | 17.9 | 2.88 | 24.6 | 13.2 | 8.79 | 1.16 | 10.8 | 40.0 | 19.7 | 1.87 |
|  |  |  |  | 3/8 | 27.48 | 0.349 | 5.6 | 22.8 | 7.58 | 66.3 | 14.7 | 2.96 | 19.7 | 11.2 | 7.45 | 1.21 | 8.80 | 33.1 | 15.8 | 1.90 |
|  |  |  |  | 5/16 | 23.34 | 0.291 | 7.3 | 27.9 | 6.43 | 57.7 | 12.8 | 3.00 | 16.9 | 9.88 | 6.59 | 1.24 | 7.63 | 28.9 | 13.6 | 1.92 |
|  |  |  |  | 1/4 | 19.02 | 0.233 | 9.9 | 35.6 | 5.24 | 48.2 | 10.7 | 3.04 | 14.0 | 8.38 | 5.59 | 1.27 | 6.35 | 24.2 | 11.3 | 1.93 |
|  |  |  |  | 3/16 | 14.53 | 0.174 | 14.2 | 48.7 | 3.98 | 37.6 | 8.35 | 3.07 | 10.8 | 6.63 | 4.42 | 1.29 | 4.92 | 18.9 | 8.66 | 1.95 |

6.5.1 Calculating the Weight and Size of Metric Rectangular High-Strength Steel Sections



| $20 \times 16 \times$ | $508.0 \times 406.4 \times 15.9^{*}$ | 209.8 | 2.057 | 15.9 | 22.6 | 28.9 | 26700 | 982 | 3870 | 192 | 4640 | 700 | 3440 | 162 | 4000 | 1290000 | 6040 | 1.73 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 5/8* |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1/2* | 12.7* | 171.8 | 1.685 | 12.7 | 29.0 | 37.0 | 21900 | 827 | 3250 | 194 | 3860 | 589 | 2900 | 164 | 3320 | 1060000 | 4930 | 1.76 |
| 3/8* | 9.5* | 130.5 | 1.280 | 9.5 | 39.8 | 50.5 | 16600 | 640 | 2520 | 196 | 2960 | 456 | 2250 | 166 | 2550 | 81800 | 3750 | 1.78 |
| $20 \times 12 \times$ | $508.0 \times 304.8 \times 15.9 *$ | 184.4 | 1.808 | 15.9 | 16.2 | 28.9 | 23500 | 786 | 3090 | 183 | 3850 | 360 | 2360 | 124 | 2720 | 791000 | 4450 | 1.53 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1/2 | 12.7 | 153.7 | 1.508 | 11.8 | 22.8 | 40.1 | 18300 | 644 | 2530 | 188 | 3080 | 293 | 1920 | 127 | 2170 | 613000 | 3420 | 1.59 |
| 3/8 | 9.5 | 116.6 | 1.143 | 8.9 | 31.2 | 54.1 | 13900 | 500 | 1970 | 189 | 2370 | 228 | 1500 | 128 | 1670 | 477000 | 2630 | 1.60 |
| 5/16 | 7.9 | 97.6 | 0.957 | 7.4 | 38.2 | 65.6 | 11700 | 422 | 1660 | 190 | 1990 | 193 | 1270 | 129 | 1410 | 404000 | 2200 | 1.60 |
| $20 \times 8 \times$ | $508.0 \times 203.2 \times 15.9$ | 164.5 | 1.613 | 14.8 | 10.7 | 31.3 | 19600 | 600 | 2360 | 175 | 3040 | 141 | 1390 | 84.8 | 1580 | 357000 | 2740 | 1.37 |
| 5/8 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1/2 | 12.7 | 133.5 | 1.309 | 11.8 | 14.2 | 40.1 | 15900 | 496 | 1950 | 177 | 2480 | 118 | 1160 | 86.2 | 1300 | 298000 | 2240 | 1.38 |
| 3/8 | 9.5 | 101.4 | 0.995 | 8.9 | 19.8 | 54.1 | 12100 | 387 | 1520 | 179 | 1920 | 92.8 | 914 | 87.4 | 1010 | 235000 | 1720 | 1.39 |
| 5/16 | 7.9 | 85.0 | 0.833 | 7.4 | 24.5 | 65.6 | 10200 | 327 | 1290 | 179 | 1620 | 78.9 | 777 | 88.1 | 853 | 200000 | 1450 | 1.40 |
| $20 \times 4 \times 1 / 2$ | $\begin{aligned} & 508 \times \\ & 101.6 \times 12.7 \end{aligned}$ | 113.2 | 1.110 | 11.8 | 5.6 | 40.1 | 13500 | 348 | 1370 | 161 | 1890 | 24.4 | 480 | 42.6 | 557 | 76200 | 1050 | 1.18 |
| 3/8 | 9.5 | 86.3 | 0.846 | 8.9 | 8.4 | 54.1 | 10300 | 274 | 1080 | 163 | 1470 | 19.9 | 391 | 43.9 | 440 | 62200 | 821 | 1.19 |
| 5/16 | 7.9 | 72.4 | 0.710 | 7.4 | 10.7 | 65.6 | 8660 | 233 | 918 | 164 | 1240 | 17.2 | 338 | 44.5 | 375 | 53800 | 696 | 1.19 |
| $18 \times 12 \times 5 /$ | $457.2 \times 304.8 \times 15.9^{*}$ | 171.7 | 1.684 | 15.9 | 16.2 | 25.8 | 21900 | 606 | 2650 | 166 | 3270 | 326 | 2140 | 122 | 2490 | 678000 | 3990 | 1.43 |
| 8* |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1/2* | 12.7* | 141.4 | 1.387 | 12.7 | 21.0 | 33.0 | 18000 | 517 | 2260 | 169 | 2750 | 278 | 1830 | 124 | 2090 | 564000 | 3280 | 1.46 |
| 3/8* | 9.5* | 107.8 | 1.057 | 9.5 | 29.1 | 45.1 | 13700 | 403 | 1760 | 171 | 2120 | 217 | 1430 | 126 | 1610 | 438000 | 2500 | 1.48 |
| $18 \times 6 \times 5 / 8$ | $457.2 \times 152.4 \times 15.9$ | 139.1 | 1.364 | 14.8 | 7.3 | 27.9 | 16600 | 385 | 1680 | 152 | 2220 | 65.8 | 864 | 63.0 | 1000 | 178000 | 1790 | 1.17 |
| 1/2 | 12.7 | 113.2 | 1.110 | 11.8 | 9.9 | 35.7 | 13500 | 320 | 1400 | 154 | 1830 | 55.7 | 731 | 64.3 | 830 | 151000 | 1470 | 1.18 |
| 3/8 | 9.5 | 86.3 | 0.486 | 8.9 | 14.1 | 48.4 | 10300 | 252 | 1100 | 156 | 1420 | 44.4 | 583 | 65.6 | 649 | 15100 | 1470 | 1.18 |
| 5/16 | 7.9 | 72.4 | 0.710 | 7.4 | 17.6 | 58.8 | 8660 | 214 | 934 | 157 | 1200 | 38.0 | 499 | 66.3 | 550 | 103000 | 964 | 1.19 |
| 1/4 | 6.4 | 59.1 | 0.580 | 5.9 | 22.8 | 74.5 | 6960 | 174 | 760 | 158 | 971 | 31.2 | 409 | 66.9 | 447 | 84600 | 779 | 1.20 |
| $16 \times 12 \times 5 /$ | $406.4 \times 304.8 \times 15.9^{*}$ | 159.0 | 1.560 | 15.9 | 16.2 | 22.6 | 20300 | 453 | 2230 | 150 | 2730 | 293 | 1920 | 120 | 2250 | 568000 | 3520 | 1.32 |
| 8* |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1/2 | 12.7 | 133.5 | 1.309 | 11.8 | 22.8 | 31.4 | 15900 | 376 | 1850 | 154 | 2210 | 242 | 1590 | 123 | 1820 | 443000 | 2720 | 1.38 |
| 3/8 | 9.5 | 101.4 | 0.995 | 8.9 | 31.2 | 42.7 | 12100 | 293 | 1440 | 155 | 1710 | 189 | 1240 | 125 | 1410 | 346000 | 2090 | 1.39 |
| 5/16 | 7.9 | 85.0 | 0.833 | 7.4 | 38.2 | 51.9 | 10200 | 248 | 1220 | 156 | 1440 | 160 | 1050 | 125 | 1180 | 293000 | 1750 | 1.40 |
| $16 \times 8 \times 5 / 8$ | $406.4 \times 203.2 \times 15.9$ | 139.1 | 1.364 | 14.8 | 10.7 | 24.5 | 16600 | 340 | 1670 | 143 | 2120 | 114 | 1130 | 83.0 | 1300 | 262000 | 2170 | 1.17 |
| 1/2 | 12.7 | 113.2 | 1.110 | 11.8 | 14.2 | 31.4 | 13500 | 283 | 1390 | 145 | 1740 | 95.8 | 943 | 84.3 | 1070 | 220000 | 1780 | 1.18 |
| 3/8 | 9.5 | 86.3 | 0.846 | 8.9 | 19.8 | 42.7 | 10300 | 222 | 1090 | 146 | 1350 | 75.7 | 745 | 85.6 | 835 | 174000 | 1370 | 1.19 |
| 5/16 | 7.9 | 72.4 | 0.710 | 7.4 | 24.5 | 51.9 | 8660 | 188 | 925 | 147 | 1140 | 64.5 | 635 | 86.3 | 706 | 148000 | 1150 | 1.19 |
| $16 \times 4 \times 1 / 2$ | $406.4 \times 101.6 \times 12.7$ | 93.0 | 0.912 | 11.8 | 5.6 | 31.4 | 11100 | 189 | 931 | 131 | 1270 | 19.5 | 385 | 42.0 | 449 | 57600 | 830 | 0.98 |
| 3/8 | 9.5 | 71.1 | 0.697 | 8.9 | 8.4 | 42.7 | 8520 | 150 | 739 | 133 | 990 | 16.0 | 314 | 43.3 | 357 | 47300 | 653 | 0.99 |
| 5/16 | 7.9 | 59.8 | 0.586 | 7.4 | 10.7 | 51.9 | 7160 | 128 | 631 | 134 | 839 | 13.8 | 272 | 44.0 | 304 | 41000 | 555 | 0.99 |
| $\begin{aligned} & 14 \times 12 \times \\ & 1 / 2^{*} \end{aligned}$ | $355.6 \times 304.8 \times 12.7^{*}$ | 21.2 | 1.188 | 12.7 | 21.0 | 25.0 | 15400 | 282 | 1590 | 135 | 1900 | 223 | 1460 | 120 | 1710 | 387000 | 2520 | 1.26 |
| 3/8* | 9.5* | 92.6 | 0.908 | 9.5 | 29.1 | 34.4 | 11800 | 222 | 1250 | 137 | 1470 | 175 | 1150 | 122 | 1330 | 302000 | 1930 | 1.27 |
| $14 \times 10 \times$ | $355.6 \times 254.0 \times 15.9$ | 139.1 | 1.364 | 14.8 | 14.2 | 21.0 | 16600 | 287 | 1610 | . 131 | 1970 | 170 | *1340 | 101 | 1560 | 320000 | 2400 | 1.17 |
| 5/8 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1/2 | 12.7 | 113.2 | 1.110 | 11.8 | 18.5 | 27.1 | 13500 | 238 | 1340 | 133 | 1620 | 142 | 1120 | 103 | 1280 | 267000 | 1960 | 1.18 |
| 3/8 | 9.5 | 86.3 | 0.846 | 8.9 | 25.5 | 37.0 | 10300 | 187 | 1050 | 134 | 1260 | 111 | 878 | 104 | 998 | 211000 | 1510 | 1.19 |
| 5/16 | 7.9 | 72.4 | 0.710 | 7.4 | 31.3 | 45.1 | 8660 | 158 | 891 | 135 | 1060 | 94.7 | 746 | 105 | 843 | 179000 | 1270 | 1.19 |
| 1/4 | 6.4 | 59.1 | 0.580 | 5.9 | 40.1 | 57.3 | 6960 | 129 | 724 | 136 | 857 | 77.1 | 607 | 105 | 682 | 146000 | 1020 | 1.20 |



| $12 \times 2 \times 1 / 4$ | $304.8 \times 50.8 \times 6.4$ | 33.6 | 0.330 | 5.9 | 5.6 | 48.7 | 3970 | 36.1 | 237 | 95.4 | 329 | 1.83 | 72.0 | 21.5 | 83.1 | 5930 | 158 | 0.69 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 3/16 | 4.8 | 25.6 | 0.251 | 4.4 | 8.5 | 66.3 | 3000 | 28.0 | 183 | 96.5 | 252 | 1.47 | 58.0 | 22.1 | 64.9 | 4790 | 122 | 0.70 |
| $10 \times 8 \times 1 / 2$ | $254.0 \times 203.2 \times 12.7$ | 82.8 | 0.812 | 11.8 | 14.2 | 18.5 | 9870 | 88.9 | 700 | 94.9 | 849 | 62.8 | 618 | 79.8 | 728 | 110000 | 1090 | 0.87 |
| 3/8 | 9.5 | 63.5 | 0.623 | 8.9 | 19.8 | 25.5 | 7620 | 70.7 | 557 | 96.3 | 666 | 50.1 | 493 | 81.1 | 572 | 88000 | 845 | 0.88 |
| 5/16 | 7.9 | 53.5 | 0.525 | 7.4 | 24.5 | 31.3 | 6410 | 60.4 | 476 | 97.1 | 565 | 42.9 | 422 | 81.8 | 485 | 75300 | 713 | 0.89 |
| 1/4 | 6.4 | 43.8 | 0.430 | 5.9 | 31.4 | 40.1 | 5170 | 49.5 | 389 | 97.8 | 459 | 35.2 | 346 | 82.5 | 395 | 61800 | 577 | 0.89 |
| 3/16 | 4.8 | 33.3 | 0.326 | 4.4 | 43.2 | 54.7 | 3900 | 37.9 | 298 | 98.6 | 349 | 27.0 | 265 | 83.2 | 300 | 47400 | 436 | 0.90 |
| $10 \times 6 \times 5 / 8$ | $254.0 \times 152.4 \times 15.9$ | 88.4 | 0.867 | 14.8 | 7.3 | 14.2 | 10600 | 83.8 | 660 | 89.0 | 842 | 37.3 | 489 | 59.3 | 587 | 77300 | 962 | 0.76 |
| 1/2 | 12.7 | 72.7 | 0.713 | 11.8 | 9.9 | 18.5 | 8680 | 71.3 | 561 | 90.6 | 704 | 31.9 | 419 | 60.7 | 493 | 66600 | 797 | 0.77 |
| 3/8 | 9.5 | 56.0 | 0.549 | 8.9 | 14.1 | 25.5 | 6710 | 57.1 | 450 | 92.2 | 555 | 25.8 | 338 | 62.0 | 390 | 53900 | 623 | 0.78 |
| 5/16 | 7.9 | 47.2 | 0.463 | 7.4 | 17.6 | 31.3 | 5650 | 49.0 | 385 | 93.0 | 472 | 22.2 | 291 | 62.7 | 332 | 46400 | 528 | 0.79 |
| 1/4 | 6.4 | 38.7 | 0.380 | 5.9 | 22.8 | 40.1 | 4570 | 40.2 | 317 | 93.9 | 385 | 18.3 | 240 | 63.3 | 271 | 38300 | 428 | 0.79 |
| 3/16 | 4.8 | 29.4 | 0.289 | 4.4 | 31.6 | 54.7 | 3450 | 30.9 | 243 | 94.7 | 293 | 14.1 | 185 | 64.0 | 207 | 29500 | 325 | 0.80 |
| $10 \times 5 \times 3 / 8$ | $254.0 \times 127.0 \times 9.5$ | 52.2 | 0.512 | 8.9 | 11.3 | 25.5 | 6260 | 50.3 | 396 | 89.6 | 500 | 16.9 | 267 | 52.0 | 307 | 38900 | 513 | 0.73 |
| 5/16 | 7.9 | 44.0 | 0.432 | 7.4 | 14.2 | 31.3 | 5280 | 43.2 | 340 | 90.5 | 426 | 14.7 | 231 | 52.7 | 263 | 33600 | 435 | 0.74 |
| 1/4 | 6.4 | 36.2 | 0.355 | 5.9 | 18.5 | 40.1 | 4270 | 35.6 | 280 | 91.4 | 348 | 12.2 | 191 | 53.4 | 215 | 27900 | 354 | 0.74 |
| 3/16 | 4.8 | 27.5 | 0.270 | 4.4 | 25.9 | 54.7 | 3230 | 27.4 | 216 | 92.2 | 265 | 9.42 | 148 | 54.0 | 165 | 21600 | 269 | 0.75 |
| $10 \times 4 \times 5 / 8$ | $254.0 \times 101.6 \times 15.9$ | 75.7 | 0.743 | 14.8 | 3.9 | 14.2 | 9090 | 62.3 | 490 | 82.8 | 662 | 13.9 | 274 | 39.2 | 338 | 34600 | 602 | 0.66 |
| 1/2 | 12.7 | 62.6 | 0.614 | 11.8 | 5.6 | 18.5 | 7480 | 53.7 | 423 | 84.7 | 559 | 12.2 | 241 | 40.5 | 288 | 30800 | 507 | 0.67 |
| 3/8 | 9.5 | 48.4 | 0.474 | 8.9 | 8.4 | 25.5 | 5810 | 43.5 | 343 | 86.6 | 444 | 10.1 | 199 | 41.7 | 231 | 25600 | 402 | 0.68 |
| 5/16 | 7.9 | 40.9 | 0.401 | 7.4 | 10.7 | 31.3 | 4900 | 37.5 | 295 | 87.5 | 379 | 8.82 | 174 | 42.4 | 198 | 22300 | 342 | 0.69 |
| 1/4 | 6.4 | 33.6 | 0.330 | 5.9 | 14.2 | 40.1 | 3970 | 31.0 | 244 | 88.4 | 310 | 7.36 | 145 | 43.1 | 163 | 18600 | 279 | 0.69 |
| 3/16 | 4.8 | 25.6 | 0.251 | 4.4 | 20.1 | 54.7 | 3000 | 23.9 | 189 | 89.3 | 238 | 5.74 | 113 | 43.7 | 125 | 14500 | 213 | 0.70 |
| $\begin{aligned} & 10 \times 31 / 2 \times \\ & 3 / 16 \end{aligned}$ | $254.0 \times 88.9 \times 4.8$ | 24.7 | 0.242 | 4.4 | 17.2 | 54.7 | 2890 | 22.2 | 175 | 87.6 | 224 | 4.28 | 96.2 | 38.5 | 106 | 11400 | 185 | 0.67 |
| $10 \times 3 \times 3 / 8$ | $254.0 \times 76.2 \times 9.5$ | 44.6 | 0.437 | 8.9 | 5.6 | 25.5 | 5360 | 36.7 | 289 | 82.8 | 389 | 5.18 | 136 | 31.1 | 160 | 14500 | 291 | 0.63 |
| 5/16 | 7.9 | 37.7 | 0.370 | 7.4 | 7.3 | 31.3 | 4530 | 31.8 | 250 | 83.8 | 333 | 4.56 | 120 | 31.7 | 138 | 12800 | 250 | 0.63 |
| $1 / 4$ | 6.4 | 31.1 | 0.305 | 5.9 | 9.9 | 40.1 | 3670 | 26.4 | 208 | 84.8 | 273 | 3.85 | 101 | 32.4 | 114 | 10800 | 205 | 0.64 |
| 3/16 | 4.8 | 23.7 | 0.232 | 4.4 | 14.3 | 54.7 | 2780 | 20.5 | 161 | 85.8 | 210 | 3.04 | 79.8 | 33.1 | 88.4 | 8540 | 157 | 0.65 |
| 1/8 | 3.2 | 16.1 | 0.158 | 3.0 | 22.4 | 81.7 | 1920 | 14.5 | 114 | 86.7 | 147 | 2.18 | 57.3 | 33.7 | 62.3 | 6130 | 110 | 0.65 |
| $10 \times 2 \times 3 / 8$ | $254.0 \times 50.8 \times 9.5$ | 40.8 | 0.400 | 8.9 | 2.7 | 25.5 | 4900 | 29.9 | 236 | 78.1 | 334 | 1.95 | 76.9 | 20.0 | 94.7 | 6000 | 180 | 0.58 |
| 5/16 | 7.9 | 34.6 | 0.339 | 7.4 | 3.9 | 31.3 | 4150 | 26.1 | 205 | 79.3 | 287 | 1.76 | 69.5 | 20.6 | 83.0 | 5470 | 157 | 0.58 |
| 1/4 | 6.4 | 28.5 | 0.280 | 5.9 | 5.6 | 40.1 | 3370 | 21.8 | 171 | 80.4 | 236 | 1.53 | 60.0 | 21.3 | 69.6 | 4760 | 131 | 0.59 |
| 3/16 | 4.8 | 21.8 | 0.214 | 4.4 | 8.5 | 54.7 | 2550 | 17.0 | 134 | 81.5 | 182 | 1.23 | 48.5 | 22.0 | 54.5 | 3860 | 102 | 0.59 |
| $9 \times 7 \times 5 / 8$ | $228.6 \times \times 177.8 \times 15.9$ | 88.4 | 0.867 | 14.8 | 9.0 | 12.4 | 10600 | 72.7 | 636 | 82.8 | 793 | 489 | 550 | 68.0 | 665 | 86800 | 1020 | 0.76 |
| 1/2 | 12.7 | 72.7 | 0.713 | 11.8 | 12.1 | 16.4 | 8680 | 61.8 | 541 | 84.4 | 663 | 11.7 | 470 | 69.4 | 557 | 74400 | 843 | 0.77 |
| 3/8 | 9.5 | 56.0 | 0.549 | 8.9 | 17.0 | 22.7 | 6710 | 49.5 | 433 | 85.9 | 522 | 33.6 | 378 | 70.7 | 440 | 60000 | 658 | 0.78 |
| 5/16 | 7.9 | 47.2 | 0.463 | 7.4 | 21.0 | 27.9 | 5650 | 42.5 | 371 | 86.7 | 444 | 28.8 | 324 | 71.4 | 374 | 51600 | 556 | 0.79 |
| 1/4 | 6.4 | 38.7 | 0.380 | 5.9 | 27.1 | 35.7 | 4570 | 34.9 | 305 | 87.4 | 362 | 23.7 | 267 | 72.1 | 305 | 42500 | 451 | 0.79 |
| 3/16 | 4.8 | 29.4 | 0.289 | 4.4 | 37.4 | 49.0 | 3450 | 26.8 | 235 | 88.2 | 276 | 18.3 | 206 | 72.8 | 233 | 32700 | 342 | 0.80 |
| $9 \times 5 \times 5 / 8$ | $228.6 \times 127.0 \times 15.9$ | 75.7 | 0.743 | 14.8 | 5.6 | 12.4 | 9090 | 55.5 | 485 | 78.1 | 632 | 21.6 | 341 | 48.8 | 416 | 46500 | 698 | 0.66 |
| 1/2 | 12.7 | 62.6 | 0.614 | 11.8 | 7.8 | 16.4 | 7480 | 47.7 | 417 | 79.9 | 533 | 18.8 | 296 | 50.1 | 352 | 40700 | 583 | 0.67 |
| 3/8 | 9.5 | 48.4 | 0.474 | 8.9 | 11.3 | 22.7 | 5810 | 38.6 | 338 | 81.5 | 423 | 15.4 | 242 | 51.4 | 281 | 33400 | 459 | 0.68 |
| 5/16 | 7.9 | 40.9 | 0.401 | 7.4 | 142 | 27.9 | 4900 | 33.3 | 291 | 82.4 | 361 | 13.3 | 210 | 52.1 | 240 | 29000 | 390 | 0.69 |
| 1/4 | 6.4 | 33.6 | 0.330 | 5.9 | 18.5 | 35.7 | 3970 | 27.5 | 240 | 83.2 | 295 | 11.1 | 174 | 52.8 | 197 | 24100 | 317 | 0.69 |
| 3/16 | 4.8 | 25.6 | 0.251 | 4.4 | 25.9 | 49.0 | 3000 | 21.2 | 185 | 84.0 | 226 | 8.58 | 135 | 53.4 | 151 | 18700 | 242 | 0.70 |


| US | Nominal Size <br> SI/Metric | Mass per <br> Meter | Weight per <br> Meter | Design <br> Wall <br> Thickness <br> t | b/t | Area |  | X-X Axis |  |  |  | Y-Y Axis |  |  |  | Torsional Stiffness Constant$J / 10^{3}$ | Torsional Shear Constanl$C / 10^{3}$ |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  | h/t |  | $\begin{aligned} & I_{x} / \\ & 10^{6} \end{aligned}$ | $S_{x} / 10^{3}$ | $r_{x}$ | $Z_{x} / 10^{3}$ | $I_{y} / 10^{6}$ | $S_{y} / 10^{3}$ | $r_{y}$ | $Z_{y} / 10^{3}$ |  |  |  |
| Inches | Millimeters | kg | $k N$ | mm |  |  | $\mathrm{mm}^{2}$ | $\mathrm{mm}^{4}$ | $m m^{3}$ | mm | $m m^{3}$ | $m m^{4}$ | $m m^{3}$ | mm | $\mathrm{mm}^{3}$ | $m m^{4}$ | $m m^{3}$ | $m^{2}$ |
| $9 \times 3 \times 1 / 2$ | $228.6 \times 76.2 \times 12.7$ | 52.4 | 0.514 | 11.8 | 3.5 | 16.4 | 6280 | 33.6 | 294 | 73.2 | 403 | 5.49 | 144 | 29.6 | 177 | 14600 | 323 | 0.57 |
| 3/8 | 9.5 | 40.8 | 0.400 | 8.9 | 5.6 | 22.7 | 4900 | 27.7 | 242 | 75.1 | 324 | 4.66 | 122 | 30.8 | 145 | 12600 | 260 | 0.58 |
| 5/16 | 7.9 | 34.6 | 0.339 | 7.4 | 7.3 | 27.9 | 4150 | 24.1 | 210 | 76.1 | 278 | 4.12 | 108 | 31.5 | 125 | 11100 | 224 | 0.58 |
| 1/4 | 6.4 | 28.5 | 0.280 | 5.9 | 9.9 | 35.7 | 3370 | 20.0 | 175 | 77.1 | 229 | 3.48 | 91.4 | 32.1 | 104 | 9430 | 184 | 0.59 |
| 3/16 | 4.8 | 21.8 | 0.214 | 4.4 | 14.3 | 49.0 | 2550 | 15.6 | 136 | 78.1 | 176 | 2.75 | 72.2 | 32.8 | 80.4 | 7460 | 141 | 0.59 |
| $8 \times 6 \times 5 / 8$ | $203.2 \times 152.4 \times 15.9$ | 75.7 | 0.743 | 14.8 | 7.3 | 10.7 | 9090 | 47.5 | 468 | 72.3 | 592 | 30.1 | 395 | 57.6 | 484 | 54500 | 755 | 0.66 |
| 1/2 | 12.7 | 62.6 | 0.614 | 11.8 | 9.9 | 14.2 | 7480 | 40.8 | 102 | 73.9 | 499 | 26.0 | 341 | 59.0 | 408 | 47300 | 629 | 0.67 |
| 3/8 | 9.5 | 48.4 | 0.474 | 8.9 | 14.1 | 19.8 | 5810 | 33.0 | 325 | 75.4 | 396 | 21.1 | 277 | 60.3 | 325 | 38600 | 494 | 0.68 |
| 5/16 | 7.9 | 40.9 | 0.401 | 7.4 | 17.6 | 24.5 | 4900 | 28.5 | 280 | 76.2 | 338 | 18.2 | 239 | 61.0 | 278 | 33400 | 419 | 0.69 |
| 1/4 | 6.4 | 33.6 | 0.330 | 5.9 | 22.8 | 31.4 | 3970 | 23.5 | 231 | 77.0 | 276 | 15.1 | 198 | 61.7 | 227 | 27700 | 340 | 0.69 |
| 3/16 | 4.8 | 25.6 | 0.251 | 4.4 | 31.6 | 43.2 | 3000 | 18.1 | 178 | 77.7 | 211 | 11.7 | 153 | 62.4 | 174 | 21400 | 259 | 0.70 |
| $8 \times 4 \times 5 / 8$ | $203.2 \times 101.6 \times 15.9$ | 63.0 | 0.618 | 14.8 | 3.9 | 10.7 | 7580 | 34.1 | 336 | 67.1 | 451 | 11.1 | 218 | 38.2 | 272 | 24800 | 472 | 0.56 |
| 1/2 | 12.7 | 52.4 | 0.514 | 11.8 | 5.6 | 14.2 | 6280 | 29.8 | 294 | 68.9 | 384 | 9.81 | 193 | 39.5 | 234 | 22300 | 399 | 0.57 |
| 3/8 | 9.5 | 40.8 | 0.400 | 8.9 | 8.4 | 19.8 | 4900 | 24.5 | 241 | 70.7 | 308 | 8.18 | 161 | 40.8 | 189 | 18700 | 318 | 0.58 |
| 5/16 | 7.9 | 34.6 | 0.339 | 7.4 | 10.7 | 24.5 | 4150 | 21.3 | 209 | 71.6 | 264 | 7.15 | 141 | 41.5 | 163 | 16400 | 271 | 0.58 |
| 1/4 | 6.4 | 28.5 | 0.280 | 5.9 | 14.2 | 31.4 | 3370 | 17.7 | 174 | 72.4 | 217 | 5.99 | 118 | 42.2 | 134 | 13700 | 222 | 0.59 |
| 3/16 | 4.8 | 21.8 | 0.214 | 4.4 | 20.1 | 43.2 | 2550 | 13.7 | 135 | 73.3 | 167 | 4.39 | 92.3 | 42.8 | 103 | 10800 | 170 | 0.59 |
| 1/8 | 3.2 | 14.8 | 0.145 | 3.0 | 30.9 | 64.7 | 1770 | 9.71 | 95.5 | 74.1 | 117 | 3.34 | 65.8 | 43.5 | 72.7 | 7670 | 118 | 0.60 |
| $8 \times 3 \times 1 / 2$ | $203.2 \times 76.2 \times 12.7$ | 47.4 | 0.465 | 11.8 | 3.5 | 14.2 | 5680 | 24.3 | 240 | 65.5 | 327 | 4.86 | 127 | 29.2 | 158 | 12400 | 285 | 0.52 |
| 3/8 | 9.5 | 37.0 | 0.363 | 8.9 | 5.6 | 19.8 | 4450 | 20.2 | 199 | 67.4 | 264 | 4.15 | 109 | 30.5 | 129 | 10700 | 230 | 0.53 |
| 5/16 | 7.9 | 31.4 | 0.308 | 7.4 | 7.3 | 24.5 | 3780 | 17.6 | 174 | 68.4 | 228 | 3.67 | 96.3 | 31.2 | 112 | 9500 | 198 | 0.53 |
| 1/4 | 6.4 | 26.0 | 0.255 | 5.9 | 9.9 | 31.4 | 3070 | 14.7 | 145 | 69.3 | 188 | 3.11 | 81.6 | 31.8 | 93.2 | 8070 | 163 | 0.54 |
| 3/16 | 4.8 | 19.9 | 0.195 | 4.4 | 14.3 | 43.2 | 2330 | 11.5 | 113 | 70.3 | 145 | 2.46 | 84.6 | 32.5 | 72.3 | 6400 | 125 | 0.54 |
| 1/8 | 3.2 | 13.5 | 0.132 | 3.0 | 22.4 | 64.7 | 1620 | 8.18 | 80.5 | 71.1 | 102 | 1.77 | 46.6 | 33.1 | 51.1 | 4610 | 87.8 | 0.55 |
| $8 \times 2 \times 3 / 8$ | $203.2 \times 50.8 \times 9.5$ | 33.2 | 0.326 | 8.9 | 2.7 | 19.8 | 4000 | 16.0 | 157 | 63.2 | 220 | 1.55 | 61.1 | 19.7 | 75.8 | 4490 | 142 | 0.48 |
| 5/16 | 7.9 | 28.3 | 0.277 | 7.4 | 3.9 | 24.5 | 3400 | 14.0 | 138 | 64.3 | 191 | 1.41 | 55.4 | 20.3 | 66.7 | 4120 | 124 | 0.48 |
| 1/4 | 6.4 | 23.4 | 0.230 | 5.9 | 5.6 | 31.4 | 2770 | 11.8 | 116 | 65.4 | 158 | 1.22 | 48.1 | 21.0 | 56.1 | 3600 | 104 | 0.49 |
| 3/16 | 4.8 | 18.0 | 0.176 | 4.4 | 8.5 | 43.2 | 2110 | 9.30 | 91.5 | 66.4 | 123 | 0.990 | 39.0 | 21.7 | 44.1 | 2930 | 80.8 | 0.49 |
| 1/8 | 3.2 | 12.2 | 0.120 | 3.0 | 13.9 | 64.7 | 1460 | 6.55 | 65.5 | 67.4 | 86.5 | 0.728 | 28.7 | 22.3 | 31.6 | 2160 | 57.3 | 0.50 |

6.6.0 Calculating the Weight and Size of U.S. Round High-Strength Steel Sections


| Nominal Size |  | Weight per Foot | Wall Thickness t | D/t | Cross- <br> Sectional <br> Area | I | S | r | Z | Torsional Stiffness Constant | Torsional Shear Constant | Surface Area per Foot |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Outside <br> Diameter | Wall |  |  |  |  |  |  |  |  |  |  |  |
| in. | in. | $l b$. | in. |  | in. ${ }^{2}$ | in. ${ }^{4}$ | in. ${ }^{3}$ | in. | in. ${ }^{3}$ | in. ${ }^{4}$ | in. ${ }^{3}$ | $f \mathrm{t}^{2}$ |
| $20.000 \times$ | 0.500 | 104.23 | 0.465 | 43.0 | 28.5 | 1360 | 136 | 6.91 | 177 | 2720 | 272 | 5.24 |
|  | 0.375 | 78.67 | 0.349 | 57.3 | 21.5 | 1040 | 104 | 6.95 | 135 | 2080 | 208 | 5.24 |
| $18.000 \times$ | 0.500 | 93.54 | 0.465 | 38.7 | 25.6 | 985 | 109 | 6.20 | 143 | 1970 | 219 | 4.71 |
|  | 0.375 | 70.65 | 0.349 | 51.6 | 19.4 | 754 | 83.8 | 6.24 | 109 | 1510 | 168 | 4.71 |
| $16.000 \times$ | 0.500 | 82.85 | 0.465 | 34.4 | 22.7 | 685 | 85.7 | 5.49 | 112 | 1370 | 171 | 4.19 |
|  | 0.438 | 72.86 | 0.407 | 39.3 | 19.9 | 606 | 75.8 | 5.51 | 99.0 | 1210 | 152 | 4.19 |
|  | 0.375 | 62.64 | 0.349 | 45.8 | 17.2 | 526 | 65.7 | 5.53 | 85.5 | 1050 | 131 | 4.19 |
|  | 0.312 | 52.32 | 0.291 | 55.0 | 14.4 | 443 | 55.4 | 5.55 | 71.8 | 886 | 111 | 4.19 |
| $14.000 \times$ | 0.500 | 72.16 | 0.465 | 30.1 | 19.8 | 453 | 64.8 | 4.79 | 85.2 | 907 | 130 | 3.67 |
|  | 0.375 | 54.62 | 0.349 | 40.1 | 15.0 | 349 | 49.8 | 4.83 | 65.1 | 698 | 99.7 | 3.67 |
|  | 0.312 | 45.65 | 0.291 | 48.1 | 12.5 | 295 | 42.1 | 4.85 | 54.7 | 589 | 84.2 | 3.67 |
| $12.750 \times$ | 0.500 | 65.48 | 0.465 | 27.4 | 17.9 | 339 | 53.2 | 4.35 | 70.2 | 678 | 106 | 3.34 |
|  | 0.375 | 49.61 | 0.349 | 36.5 | 13.6 | 262 | 41.0 | 4.39 | 53.7 | 523 | 82.1 | 3.34 |
|  | 0.250 | 33.41 | 0.233 | 54.7 | 9.16 | 180 | 28.2 | 4.43 | 36.5 | 359 | 56.6 | 3.34 |
| $12.500 \times$ | 0.625 | 79.34 | 0.581 | 21.5 | 21.8 | 387 | 62.0 | 4.22 | 82.6 | 774 | 124 | 3.27 |
|  | 0.500 | 64.14 | 0.465 | 26.9 | 17.6 | 319 | 51.0 | 4.26 | 67.4 | 638 | 102 | 3.27 |
|  | 0.375 | 48.61 | 0.349 | 35.8 | 13.3 | 246 | 39.4 | 4.30 | 51.5 | 492 | 78.7 | 3.27 |
|  | 0.312 | 40.65 | 0.291 | 43.0 | 11.2 | 208 | 33.3 | 4.32 | 43.4 | 416 | 66.6 | 3.27 |
|  | 0.250 | 32.74 | 0.233 | 53.6 | 8.98 | 169 | 27.0 | 4.34 | 35.1 | 338 | 54.1 | 3.27 |
|  | 0.188 | 24.74 | 0.174 | 71.8 | 6.74 | 128 | 20.5 | 4.36 | 26.4 | 256 | 41.0 | 3.27 |
| $12.313 \times$ | 0.625 | 78.09 | 0.581 | 21.2 | 21.4 | 369 | 60.0 | 4.15 | 80.0 | 739 | 120 | 3.22 |
|  | 0.500 | 63.14 | 0.465 | 26.5 | 17.3 | 304 | 49.4 | 4.19 | 65.3 | 608 | 98.8 | 3.22 |
|  | 0.375 | 47.86 | 0.349 | 35.3 | 13.1 | 235 | 38.2 | 4.23 | 50.0 | 470 | 76.3 | 3.22 |
|  | 0.312 | 40.03 | 0.291 | 42.3 | 11.0 | 199 | 32.3 | 4.25 | 42.1 | 397 | 64.5 | 3.22 |
|  | 0.250 | 32.24 | 0.233 | 52.8 | 8.84 | 161 | 26.2 | 4.27 | 34.0 | 323 | 52.4 | 3.22 |
|  | 0.188 | 24.37 | 0.174 | 70.8 | 6.64 | 122 | 19.9 | 4.29 | 25.6 | 244 | 39.7 | 3.22 |
| $12.250 \times$ | 0.625 | 77.67 | 0.581 | 21.1 | 21.3 | 363 | 59.3 | 4.13 | 79.2 | 727 | 119 | 3.21 |

## Nominal Size

| Outside <br> Diameter | Wall | Weight per Foot | Wall <br> Thickness t | D/t | Cross- <br> Sectional <br> Area | I | S | r | Z | Torsional <br> Stiffness <br> Constant | Torsional Shear Constant | Surface Area per Foot |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| in. | in. | lb. | in. |  | in. ${ }^{2}$ | in. ${ }^{4}$ | in. ${ }^{3}$ | in. | in. ${ }^{3}$ | in. ${ }^{4}$ | in. ${ }^{3}$ | ft. ${ }^{2}$ |
| $11.250 \times$ | 0.500 | 62.80 | 0.465 | 26.3 | 17.2 | 299 | 48.9 | 4.17 | 64.6 | 599 | 97.7 | 3.21 |
|  | 0.375 | 47.60 | 0.349 | 35.1 | 13.0 | 231 | 37.7 | 4.21 | 49.4 | 462 | 75.5 | 3.21 |
|  | 0.312 | 39.82 | 0.291 | 42.1 | 10.9 | 196 | 31.9 | 4.23 | 41.6 | 391 | 63.9 | 3.21 |
|  | 0.250 | 32.07 | 0.233 | 52.6 | 8.80 | 159 | 25.9 | 4.25 | 33.7 | 318 | 51.9 | 3.21 |
|  | 0.188 | 24.24 | 0.174 | 70.4 | 6.60 | 120 | 19.6 | 4.27 | 25.4 | 241 | 39.3 | 3.21 |
|  | 0.625 | 70.99 | 0.581 | 19.4 | 19.5 | 278 | 49.4 | 3.78 | 66.2 | 556 | 98.8 | 2.95 |
|  | 0.500 | 57.46 | 0.465 | 24.2 | 15.8 | 229 | 40.8 | 3.82 | 54.1 | 459 | 81.6 | 2.95 |
|  | 0.375 | 43.60 | 0.349 | 32.2 | 12.0 | 178 | 31.6 | 3.86 | 41.5 | 355 | 63.2 | 2.95 |
|  | 0.312 | 36.48 | 0.291 | 38.7 | 10.0 | 151 | 26.8 | 3.88 | 35.0 | 301 | 53.5 | 2.95 |
|  | 0.250 | 29.40 | 0.233 | 48.3 | 8.06 | 122 | 21.8 | 3.90 | 28.3 | 245 | 43.5 | 2.95 |
|  | 0.188 | 22.23 | 0.174 | 64.7 | 6.05 | 92.9 | 16.5 | 3.92 | 21.3 | 186 | 33.0 | 2.95 |
| $10.750 \times$ | 0.500 | 54.79 | 0.465 | 23.1 | 15.0 | 199 | 37.0 | 3.64 | 49.2 | 398 | 74.1 | 2.81 |
|  | 0.365 | 40.52 | 0.340 | 31.6 | 11.1 | 151 | 28.1 | 3.68 | 36.9 | 36.9 | 56.1 | 2.81 |
|  | 0.250 | 28.06 | 0.233 | 46.1 | 7.70 | 106 | 19.8 | 3.72 | 25.8 | 25.8 | 39.6 | 2.81 |
| $10.000 \times$ | 0.625 | 62.64 | 0.581 | 17.2 | 17.2 | 191 | 38.3 | 3.34 | 51.6 | 383 | 76.6 | 2.62 |
|  | 0.500 | 50.78 | 0.465 | 21.5 | 13.9 | 159 | 37.1 | 3.38 | 42.3 | 317 | 63.5 | 2.62 |
|  | 0.375 | 38.58 | 0.349 | 28.7 | 10.6 | 123 | 24.7 | 3.41 | 32.5 | 247 | 49.3 | 2.62 |
|  | 0.312 | 32.31 | 0.291 | 34.4 | 8.88 | 105 | 20.9 | 3.43 | 27.4 | 209 | 41.9 | 2.62 |
|  | 0.250 | 26.06 | 0.233 | 42.9 | 7.15 | 85.3 | 17.1 | 3.45 | 22.2 | 171 | 34.1 | 2.62 |
|  | 0.188 | 19.72 | 0.174 | 57.5 | 5.37 | 64.8 | 13.0 | 3.47 | 16.8 | 130 | 25.9 | 2.62 |
| $9.625 \times$ | 0.500 | 48.77 | 0.465 | 20.7 | 13.4 | 141 | 29.2 | 3.24 | 39.0 | 281 | 58.5 | 2.52 |
|  | 0.375 | 37.08 | 0.349 | 27.6 | 10.2 | 110 | 22.8 | 3.28 | 30.0 | 219 | 45.5 | 2.52 |
|  | 0.312 | 31.06 | 0.291 | 33.1 | 8.53 | 93.0 | 19.3 | 3.30 | 25.4 | 186 | 38.7 | 2.52 |
|  | 0.250 | 25.05 | 0.233 | 41.3 | 6.87 | 75.9 | 15.8 | 3.32 | 20.6 | 152 | 31.5 | 2.52 |
|  | 0.188 | 18.97 | 0.174 | 55.3 | 5.17 | 57.7 | 12.0 | 3.34 | 15.5 | 115 | 24.0 | 2.52 |
| $8.750 \times$ | 0.500 | 44.10 | 0.465 | 18.8 | 12.1 | 104 | 23.8 | 2.93 | 32.0 | 208 | 47.6 | 2.29 |
|  | 0.375 | 33.57 | 0.349 | 25.1 | 9.21 | 81.4 | 18.6 | 2.97 | 24.6 | 163 | 37.2 | 2.29 |
|  | 0.312 | 28.14 | 0.291 | 30.1 | 7.73 | 69.3 | 15.8 | 2.99 | 20.8 | 139 | 31.7 | 2.29 |
|  | 0.250 | 22.72 | 0.233 | 37.6 | 6.23 | 56.6 | 12.9 | 3.01 | 16.9 | 113 | 25.9 | 2.29 |
|  | 0.188 | 17.21 | 0.174 | 50.3 | 4.69 | 43.1 | 9.86 | 3.03 | 12.8 | 86.2 | 19.7 | 2.29 |
| $8.625 \times$ | 0.500 | 43.43 | 0.465 | 18.5 | 11.9 | 99.5 | 23.1 | 2.89 | 31.0 | 199 | 46.2 | 2.26 |
|  | 0.375 | 33.07 | 0.349 | 24.7 | 9.07 | 77.8 | 18.0 | 2.93 | 23.9 | 156 | 36.1 | 2.26 |
|  | 0.322 | 28.58 | 0.300 | 28.7 | 7.85 | 68.1 | 15.8 | 2.95 | 20.8 | 136 | 31.6 | 2.26 |
|  | 0.250 | 22.38 | 0.233 | 37.0 | 6.14 | 54.1 | 12.5 | 2.97 | 16.4 | 108 | 25.1 | 2.26 |
|  | 0.188 | 16.96 | 0.174 | 49.6 | 4.62 | 41.3 | 9.57 | 2.99 | 12.4 | 82.5 | 19.1 | 2.26 |
| $7.625 \times$ | 0.375 | 29.06 | 0.349 | 21.8 | 7.98 | 52.9 | 13.9 | 2.58 | 18.5 | 106 | 27.8 | 2.00 |
|  | 0.328 | 25.59 | 0.305 | 25.0 | 7.01 | 47.1 | 12.3 | 2.59 | 16.4 | 94.1 | 24.7 | 2.00 |
|  | 0.125 | 10.02 | 0.116 | 65.7 | 2.74 | 19.3 | 5.06 | 2.66 | 6.54 | 38.6 | 10.1 | 2.00 |


| 7.500 | $\times$ | 0.500 | 37.42 | 0.465 | 16.1 | 10.3 | 63.9 | 17.0 | 2.49 | 23.0 | 128 | 34.1 | 1.96 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 0.375 | 28.56 | 0.349 | 21.5 | 7.84 | 50.2 | 13.4 | 2.53 | 17.9 | 100 | 26.8 | 1.96 |
|  |  | 0.312 | 23.97 | 0.291 | 25.8 | 6.59 | 42.9 | 11.4 | 2.55 | 15.1 | 85.8 | 22.9 | 1.96 |
|  |  | 0.250 | 19.38 | 0.233 | 32.2 | 5.32 | 35.2 | 9.37 | 2.57 | 12.3 | 70.3 | 18.7 | 1.96 |
|  |  | 0.188 | 14.70 | 0.174 | 43.1 | 4.00 | 26.9 | 7.17 | 2.59 | 9.34 | 53.8 | 14.3 | 1.96 |
| 7.000 | $\times$ | 0.500 | 34.74 | 0.465 | 15.1 | 9.55 | 51.2 | 14.6 | 2.32 | 19.9 | 102 | 29.3 | 1.83 |
|  |  | 0.375 | 26.56 | 0.349 | 20.1 | 7.29 | 40.4 | 11.6 | 2.35 | 15.5 | 80.9 | 23.1 | 1.83 |
|  |  | 0.312 | 22.31 | 0.291 | 24.1 | 6.13 | 34.6 | 9.88 | 2.37 | 13.1 | 69.1 | 19.8 | 1.83 |
|  |  | 0.250 | 18.04 | 0.233 | 30.0 | 4.95 | 28.4 | 8.11 | 2.39 | 10.7 | 56.8 | 16.2 | 1.83 |
|  |  | 0.188 | 13.69 | 0.174 | 40.2 | 3.73 | 21.7 | 6.21 | 2.41 | 8.11 | 43.5 | 12.4 | 1.83 |
|  |  | 0.125 | 9.19 | 0.116 | 60.3 | 2.51 | 14.9 | 4.25 | 2.43 | 5.50 | 29.7 | 8.49 | 1.83 |
| 6.875 | $\times$ | 0.500 | 34.07 | 0.465 | 14.8 | 9.36 | 48.3 | 14.1 | 2.27 | 19.1 | 96.7 | 28.1 | 1.80 |
|  |  | 0.375 | 26.06 | 0.349 | 19.7 | 7.16 | 38.2 | 11.1 | 2.31 | 14.9 | 76.4 | 22.2 | 1.80 |
|  |  | 0.312 | 21.89 | 0.291 | 23.6 | 6.02 | 32.7 | 9.51 | 2.33 | 12.6 | 65.4 | 19.0 | 1.80 |
|  |  | 0.250 | 17.71 | 0.233 | 29.5 | 4.86 | 26.8 | 7.81 | 2.35 | 10.3 | 53.7 | 15.6 | 1.80 |
|  |  | 0.188 | 13.44 | 0.174 | 39.5 | 3.66 | 20.6 | 5.99 | 2.37 | 7.81 | 41.1 | 12.0 | 1.80 |
| 6.625 | $\times$ | 0.500 | 32.74 | 0.465 | 14.2 | 9.00 | 42.9 | 13.0 | 2.18 | 17.7 | 85.9 | 25.9 | 1.73 |
|  |  | 0.432 | 28.60 | 0.403 | 16.4 | 7.88 | 38.3 | 11.6 | 2.20 | 15.6 | 76.6 | 23.1 | 1.73 |
|  |  | 0.375 | 25.05 | 0.349 | 19.0 | 6.88 | 34.0 | 10.3 | 2.22 | 13.8 | 68.0 | 20.5 | 1.73 |
|  |  | 0.312 | 21.06 | 0.291 | 22.8 | 5.79 | 29.1 | 8.79 | 2.24 | 11.7 | 58.2 | 17.6 | 1.73 |
|  |  | 0.280 | 18.99 | 0.261 | 25.4 | 5.22 | 26.5 | 7.99 | 2.25 | 10.6 | 52.9 | 16.0 | 1.73 |
|  |  | 0.250 | 17.04 | 0.233 | 28.4 | 4.68 | 23.9 | 7.22 | 2.26 | 9.52 | 47.9 | 14.4 | 1.73 |
|  |  | 0.188 | 12.94 | 0.174 | 38.1 | 3.53 | 18.4 | 5.54 | 2.28 | 7.24 | 36.7 | 11.1 | 1.73 |
|  |  | 0.125 | 8.69 | 0.116 | 57.1 | 2.37 | 12.6 | 3.79 | 2.30 | 4.92 | 25.1 | 7.59 | 1.73 |
| 6.125 | $\times$ | 0.500 | 30.07 | 0.465 | 13.2 | 8.27 | 33.3 | 10.9 | 2.01 | 14.9 | 66.7 | 21.8 | 1.60 |
|  |  | 0.375 | 23.05 | 0.349 | 17.6 | 6.33 | 26.5 | 8.66 | 02.05 | 11.7 | 53.0 | 17.3 | 1.60 |
|  |  | 0.312 | 19.39 | 0.291 | 21.0 | 5.33 | 22.7 | 7.43 | 2.07 | 9.91 | 45.5 | 14.9 | 1.60 |
|  |  | 0.250 | 15.70 | 0.233 | 26.3 | 4.31 | 18.7 | 6.12 | 2.08 | 8.09 | 37.5 | 12.2 | 1.60 |
|  |  | 0.188 | 11.93 | 0.174 | 35.2 | 3.25 | 14.4 | 4.71 | 2.10 | 6.16 | 28.8 | 9.41 | 1.60 |
| 6.000 | $\times$ | 0.500 | 29.40 | 0.465 | 12.9 | 8.09 | 31.2 | 10.4 | 1.96 | 14.3 | 62.4 | 20.8 | 1.57 |
|  |  | 0.375 | 22.55 | 0.349 | 17.2 | 6.20 | 24.8 | 8.28 | 2.00 | 11.2 | 49.7 | 16.6 | 1.57 |
|  |  | 0.312 | 18.97 | 0.291 | 20.6 | 5.22 | 21.3 | 7.11 | 2.02 | 9.49 | 42.6 | 14.2 | 1.57 |
|  |  | 0.280 | 17.12 | 0.261 | 23.0 | 4.71 | 19.4 | 6.47 | 2.03 | 8.60 | 38.8 | 12.9 | 1.57 |
|  |  | 0.250 | 15.37 | 0.233 | 25.8 | 4.22 | 17.6 | 5.86 | 2.04 | 7.75 | 35.2 | 11.7 | 1.57 |
|  |  | 0.188 | 11.68 | 0.174 | 34.5 | 3.18 | 13.5 | 4.51 | 2.06 | 5.91 | 27.0 | 9.02 | 1.57 |
|  |  | 0.125 | 7.85 | 0.116 | 51.7 | 2.14 | 9.28 | 3.09 | 2.08 | 4.02 | 18.6 | 6.19 | 1.57 |


| Nominal Size |  | Weight per Foot | Wall <br> Thickness t | D/t | Cross- <br> Sectional Area | I | S | r | Z | Torsional Stiffness Constant | Torsional Shear Constant | Surface <br> Area per <br> Foot |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Outside <br> Diameter | Wall |  |  |  |  |  |  |  |  |  |  |  |
| in. | in. | ib | in. |  | in. ${ }^{2}$ | in. ${ }^{4}$ | in. ${ }^{3}$ | in. | in. ${ }^{3}$ | in. ${ }^{4}$ | in. ${ }^{3}$ | $f \mathrm{t}^{2}$ |
| 5.563 | 0.375 | 20.80 | 0.349 | 15.9 | 5.72 | 19.5 | 7.02 | 1.85 | 9.50 | 39.0 | 14.0 | 1.46 |
|  | 0.258 | 14.63 | 0.241 | 23.1 | 4.03 | 14.3 | 5.14 | 1.88 | 6.83 | 28.6 | 10.3 | 1.46 |
|  | 0.188 | 10.80 | 0.174 | 32.0 | 2.95 | 10.7 | 3.85 | 1.91 | 5.05 | 21.4 | 7.70 | 1.46 |
|  | 0.134 | 7.78 | 0.125 | 44.5 | 2.14 | 7.90 | 2.84 | 1.92 | 3.70 | 15.8 | 5.68 | 1.46 |
| $5.500 \times$ | 0.500 | 26.73 | 0.465 | 11.8 | 7.36 | 23.5 | 8.55 | 1.79 | 11.8 | 47.0 | 17.1 | 1.44 |
|  | 0.375 | 20.54 | 0.349 | 15.8 | 5.65 | 18.8 | 6.84 | 1.83 | 9.27 | 37.6 | 13.7 | 1.44 |
|  | 0.258 | 14.46 | 0.241 | 22.8 | 3.98 | 13.8 | 5.02 | 1.86 | 6.67 | 27.6 | 10.0 | 1.44 |
| 5.000 x | 0.500 | 24.05 | 0.465 | 10.8 | 6.62 | 17.2 | 6.88 | 1.61 | 9.60 | 34.4 | 13.8 | 1.31 |
|  | 0.375 | 18.54 | 0.349 | 14.3 | 5.10 | 13.9 | 5.55 | 1.65 | 7.56 | 27.7 | 11.1 | 1.31 |
|  | 0.312 | 15.64 | 0.291 | 17.2 | 4.30 | 12.0 | 4.79 | 1.67 | 6.46 | 24.0 | 9.58 | 1.31 |
|  | 0.258 | 13.08 | 0.241 | 20.7 | 3.60 | 10.2 | 4.09 | 1.68 | 5.46 | 20.5 | 8.18 | 1.31 |
|  | 0.250 | 12.69 | 0.233 | 21.5 | 3.49 | 9.94 | 3.97 | 1.69 | 5.30 | 19.9 | 7.95 | 1.31 |
|  | 0.188 | 9.67 | 0.174 | 28.7 | 2.64 | 7.69 | 3.08 | 1.71 | 4.05 | 15.4 | 6.15 | 1.31 |
|  | 0.125 | 6.51 | 0.116 | 43.1 | 1.78 | 5.31 | 2.12 | 1.73 | 2.77 | 10.6 | 4.25 | 1.31 |
| $4.500 \times$ | 0.337 | 15.00 | 0.315 | 14.3 | 4.14 | 9.12 | 4.05 | 1.48 | 5.53 | 18.2 | 8.11 | 1.18 |
|  | 0.237 | 10.80 | 0.221 | 20.4 | 2.97 | 6.82 | 3.03 | 1.51 | 4.05 | 13.6 | 6.06 | 1.18 |
|  | 0.188 | 8.67 | 0.174 | 25.9 | 2.36 | 5.54 | 2.46 | 1.53 | 3.26 | 11.1 | 4.93 | 1.18 |
|  | 0.125 | 5.85 | 0.116 | 38.8 | 1.60 | 3.84 | 1.71 | 1.55 | 2.23 | 7.68 | 3.41 | 1.18 |
| 4.000 x | 0.337 | 13.20 | 0.315 | 12.7 | 3.65 | 6.24 | 3.12 | 1.31 | 4.29 | 12.5 | 6.24 | 1.05 |
|  | 0.313 | 12.34 | 0.291 | 13.7 | 3.39 | 5.87 | 2.93 | 1.32 | 4.01 | 11.7 | 5.87 | 1.05 |
|  | 0.250 | 10.02 | 0.233 | 17.2 | 2.76 | 4.91 | 2.45 | 1.33 | 3.31 | 9.82 | 4.91 | 1.05 |
|  | 0.237 | 9.53 | 0.221 | 18.1 | 2.62 | 4.70 | 2.35 | 1.34 | 3.16 | 9.40 | 4.70 | 1.05 |
|  | 0.226 | 9.12 | 0.211 | 19.0 | 2.51 | 4.52 | 2.26 | 1.34 | 3.03 | 9.04 | 4.52 | 1.05 |
|  | 0.220 | 8.89 | 0.205 | 19.5 | 2.44 | 4.41 | 2.21 | 1.34 | 2.96 | 8.83 | 4.41 | 1.05 |
|  | 0.188 | 7.66 | 0.174 | 23.0 | 2.09 | 3.83 | 1.92 | 1.35 | 2.55 | 7.67 | 3.83 | 1.05 |
|  | 0.125 | 5.18 | 0.116 | 34.5 | 1.42 | 2.67 | 1.34 | 1.37 | 1.75 | 5.34 | 2.67 | 1.05 |

6.6.1 Calculating the Weight and Size of Metric Round High-Strength Steel Sections


| Normal Size |  | Mass <br> per <br> Meter | Design <br> Weight per <br> Meter | Wall <br> Thickness t | D/t | Area | 1/106 | S/103 | $r$ | Z/103 | Torsional Stiffness Constant J/103 | Torsional Shear Constant C/103 | Surface <br> Area per <br> Meter |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| U.S. <br> Customary <br> Outside <br> Diameter Wall | SI /Metric <br> Outside <br> Diameter Wall |  |  |  |  |  |  |  |  |  |  |  |  |
| Inches | Millimeters | $k g$ | $k N$ | mm |  | $m m^{2}$ | $m m^{*}$ | $m m^{3}$ | mm | $m m^{3}$ | $m m^{4}$ | $m m^{3}$ | $m^{2}$ |
| 20:000 $\times 0.500$ | $508.0 \times 12.7$ | 155.1 | 1.521 | 11.8 | 43.1 | 18400 | 566 | 2230 | 175 | 2910 | 1130000 | 4460000 | 1.60 |
| 0.375 | 9.5 | 116.8 | 1.145 | 8.9 | 57.1 | 14000 | 435 | 1710 | 176 | 2220 | 869000 | 3420000 | 1.60 |
| $18.000 \times 0.500$ | $457.2 \times 12.7$ | 139.2 | 1.365 | 11.8 | 38.7 | 16500 | 410 | 1790 | 158 | 2340 | 819000 | 3580000 | 1.44 |
| 0.375 | 9.5 | 104.9 | 1.029 | 8.9 | 51.4 | 12500 | 315 | 1380 | 159 | 1790 | 630000 | 2760000 | 1.44 |
| $16.000 \times 0.500$ | $406.4 \times 12.7$ | 123.3 | 1.209 | 11.8 | 34.4 | 14600 | 285 | 1400 | 140 | 1840 | 570000 | 2800000 | 1.28 |
| 0.438 | 11.1 | 108.2 | 1.061 | 10.3 | 39.5 | 12800 | 252 | 1240 | 140 | 1620 | 503000 | 2480000 | 1.28 |
| 0.375 | 9.5 | 93.0 | 0.912 | 8.9 | 45.7 | 11100 | 220 | 1080 | 141 | 1410 | 439000 | 2160000 | 1.28 |
| 0.312 | 7.9 | 77.6 | 0.761 | 7.4 | 54.9 | 9280 | 185 | 909 | 141 | 1180 | 369000 | 1820000 | 1.28 |
| $14.000 \times 0.500$ | $355.6 \times 12.7$ | 107.4 | 1.053 | 11.8 | 30.1 | 12700 | 189 | 1060 | 122 | 1400 | 377000 | 2120000 | 1.12 |
| 0.375 | 9.5 | 81.1 | 0.795 | 8.9 | 40.0 | 9690 | 146 | 820 | 123 | 1070 | 291000 | 1640000 | 1.12 |
| 0.312 | 7.9 | 67.7 | 0.664 | 7.4 | 48.1 | 8090 | 123 | 690 | 123 | 897 | 245000 | 1380000 | 1.12 |
| $12.750 \times 0.500$ | $323.9 \times 12.7$ | 97.5 | 0.956 | 11.8 | 27.4 | 11600 | 141 | 871 | 110 | 1150 | 282000 | 1740000 | 1.02 |
| 0.375 | 9.5 | 73.7 | 0.722 | 8.9 | 36.4 | 8810 | 109 | 675 | 111 | 883 | 219000 | 1350000 | 1.02 |
| 0.250 | 6.4 | 50.1 | 0.491 | 5.9 | 54.9 | 5890 | 74.5 | 460 | 112 | 597 | 149000 | 920000 | 1.02 |
| $12.500 \times 0.625$ | $317.5 \times 15.9$ | 118.3 | 1.160 | 14.8 | 21.5 | 14100 | 162 | 1020 | 107 | 1360 | 323000 | 2040000 | 1.00 |
| 0.500 | 12.7 | 95.5 | 0.936 | 11.8 | 26.9 | 11300 | 133 | 835 | 108 | 1100 | 265000 | 1670000 | 1.00 |
| 0.375 | 9.5 | 72.2 | 0.708 | 8.9 | 35.7 | 8630 | 103 | 648 | 109 | 848 | 206000 | 1300000 | 1.00 |
| 0.312 | 7.9 | 60.3 | 0.592 | 7.4 | 42.9 | 7210 | 86.7 | 546 | 110 | 712 | 173000 | 1090000 | 1.00 |
| 0.250 | 6.4 | 49.1 | 0.482 | 5.9 | 53.8 | 5780 | 70.1 | 442 | 110 | 573 | 140000 | 883000 | 1.00 |
| 0.188 | 4.8 | 37.0 | 0.363 | 4.4 | 72.2 | 4330 | 53.0 | 334 | 111 | 431 | 106000 | 668000 | 1.00 |
| $12.313 \times 0.625$ | $312.8 \times 15.9$ | 116.4 | 1.141 | 14.8 | 21.1 | 13900 | 154 | 985 | 105 | 1310 | 308000 | 1970000 | 0.98 |
| 0.500 | 12.7 | 94.0 | 0.922 | 11.8 | 26.5 | 11200 | 127 | 809 | 106 | 1070 | 253000 | 1620000 | 0.98 |
| 0.375 | 9.5 | 71.0 | 0.697 | 8.9 | 35.1 | 8500 | 98.1 | 628 | 107 | 822 | 196000 | 1260000 | 0.98 |
| 0.312 | 7.9 | 59.4 | 0.582 | 7.4 | 42.3 | 7100 | 82.8 | 529 | 108 | 690 | 166000 | 1060000 | 0.98 |
| 0.250 | 6.4 | 48.4 | 0.474 | 5.9 | 53.0 | 5690 | 67.0 | 428 | 109 | 556 | 134000 | 856000 | 0.98 |
| 0.188 | 4.8 | 36.5 | 0.357 | 4.4 | 71.1 | 4260 | 50.7 | 324 | 109 | 418 | 101000 | 648000 | 0.98 |



## Metric Dimenstions and Section Properties of Round HSS



| Normal Size |  | Mass <br> per <br> Meter | Design <br> Weight <br> per <br> Meter | Wall <br> Thickness <br> t | D/t | Area | I/106 | S/103 | r | Z/103 | Torsional <br> Stiffness <br> Constant <br> J/103 | Torsional <br> Shear <br> Constant <br> C/103 | Surface <br> Area <br> per <br> Meter |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| U.S. <br> Customary <br> Outside <br> Diameter Wall | SI /Metric <br> Outside <br> Diameter Wall |  |  |  |  |  |  |  |  |  |  |  |  |
| Inches | Millimeters | kg | kN | mm |  | $\mathrm{mm}^{2}$ | mm* | $\mathrm{mm}^{3}$ | mm | $\mathrm{mm}^{3}$ | $\mathrm{mm}^{4}$ | $\mathrm{mm}^{3}$ | $\mathrm{m}^{2}$ |
| $7.500 \times 0.500$ | $190.5 \times 12.7$ | 55.7 | 0.546 | 11.8 | 16.1 | 6620 | 26.6 | 279 | 63.3 | 377 | 53100 | 558000 | 0.60 |
| 0.375 | 9.5 | 42.4 | 0.416 | 8.9 | 21.4 | 5080 | 21.0 | 220 | 64.3 | 294 | 42000 | 441000 | 0.60 |
| 0.312 | 7.9 | 35.6 | 0.349 | 7.4 | 25.7 | 4260 | 17.9 | 188 | 64.8 | 248 | 35700 | 375000 | 0.60 |
| 0.250 | 6.4 | 29.1 | 0.285 | 5.9 | 32.3 | 3420 | 14.6 | 153 | 65.3 | 201 | 29200 | 306000 | 0.60 |
| 0.188 | 4.8 | 22.0 | 0.216 | 4.4 | 43.3 | 2570 | 11.1 | 117 | 65.8 | 152 | 22300 | 234000 | 0.60 |
| $7.000 \times 0.500$ | $177.8 \times 12.7$ | 51.7 | 0.507 | 11.8 | 15.1 | 6150 | 21.3 | 240 | 58.8 | 326 | 42600 | 479000 | 0.56 |
| 0.375 | 9.5 | 39.4 | 0.387 | 8.9 | 20.0 | 4720 | 16.9 | 190 | 59.8 | 254 | 33800 | 380000 | 0.56 |
| 0.312 | 7.9 | 33.1 | 0.325 | 7.4 | 24.0 | 3960 | 14.4 | 162 | 60.3 | 215 | 28800 | 324000 | 0.56 |
| 0.250 | 6.4 | 27.1 | 0.265 | 5.9 | 30.1 | 3190 | 11.8 | 133 | 60.8 | 174 | 23600 | 265000 | 0.56 |
| 0.188 | 4.8 | 20.5 | 0.201 | 4.4 | 40.4 | 2400 | 9.01 | 101 | 61.3 | 132 | 18000 | 203000 | 0.56 |
| 0.125 | 3.2 | 13.8 | 0.135 | 2.9 | 61.3 | 1590 | 6.09 | 68.6 | 61.8 | 88.7 | 12200 | 137000 | 0.56 |
| $6.875 \times 0.500$ | $174.6 \times 12.7$ | 50.7 | 0.497 | 11.8 | 14.8 | 6040 | 20.1 | 230 | 57.7 | 313 | 40200 | 460000 | 0.55 |
| 0.375 | 9.5 | 38.7 | 0.379 | 8.9 | 19.6 | 4630 | 15.9 | 183 | 58.7 | 245 | 31900 | 365000 | 0.55 |
| 0.312 | 7.9 | 32.5 | 0.318 | 7.4 | 23.6 | 3890 | 13.6 | 156 | 59.2 | 207 | 27200 | 312000 | 0.55 |
| 0.250 | 6.4 | 26.5 | 0.260 | 5.9 | 29.6 | 3130 | 11.1 | 128 | 59.7 | 168 | 22300 | 255000 | 0.55 |
| 0.188 | 4.8 | 20.1 | 0.197 | 4.4 | 39.7 | 2350 | 8.52 | 97.6 | 60.2 | 127 | 17000 | 195000 | 0.55 |
| $6.625 \times 0.500$ | $168.3 \times 12.7$ | 48.7 | 0.478 | 11.8 | 14.3 | 5800 | 17.9 | 212 | 55.5 | 290 | 35700 | 425000 | 0.53 |
| 0.432 | 11.0 | 42.7 | 0.418 | 10.2 | 16.5 | 5070 | 15.9 | 189 | 56.0 | 255 | 31800 | 378000 | 0.53 |


| Normal Size |  | Mass <br> per <br> Meter | Design <br> Weight per <br> Meter | Wall <br> Thickness <br> t | D/t | Area | I/106 | S/103 | r | Z/103 | Torsional <br> Stiffness <br> Constant <br> J/103 | Torsional <br> Shear <br> Constant <br> C/103 | Surface <br> Area <br> per <br> Meter |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| U.S. | SI /Metric |  |  |  |  |  |  |  |  |  |  |  |  |
| Outside | Outside |  |  |  |  |  |  |  |  |  |  |  |  |
| Diameter Wall | Diameter Wall |  |  |  |  |  |  |  |  |  |  |  |  |
| Inches | Millimeters | kg | $k N$ | mm |  | $\mathrm{mm}^{2}$ | mm* | $\mathrm{mm}^{3}$ | mm | $\mathrm{mm}^{3}$ | $\mathrm{mm}^{4}$ | $\mathrm{mm}^{3}$ | $m^{2}$ |
| 0.375 | 9.5 | 37.2 | 0.365 | 8.9 | 18.9 | 4460 | 14.2 | 169 | 56.4 | 226 | 28400 | 337000 | 0.53 |
| 0.312 | 7.9 | 31.3 | 0.306 | 7.4 | 22.7 | 3740 | 12.1 | 144 | 56.9 | 192 | 24300 | 288000 | 0.53 |
| 0.280 | 7.1 | 28.2 | 0.277 | 6.6 | 25.5 | 3350 | 11.0 | 130 | 57.2 | 173 | 22000 | 261000 | 0.53 |
| 0.250 | 6.4 | 25.6 | 0.251 | 5.9 | 28.5 | 3010 | 9.94 | 118 | 57.5 | 156 | 19900 | 236000 | 0.53 |
| 0.188 | 4.8 | 19.4 | 0.190 | 4.4 | 38.3 | 2270 | 7.61 | 90.5 | 58.0 | 118 | 15200 | 181000 | 0.53 |
| 0.125 | 3.2 | 13.0 | 0.128 | 2.9 | 58.0 | 1510 | 5.15 | 61.3 | 58.5 | 79.3 | 10300 | 123000 | 0.53 |
| $6.125 \times 0.500$ | $155.6 \times 12.7$ | 44.8 | 0.439 | 11.8 | 13.2 | 5330 | 13.9 | 178 | 51.0 | 245 | 27700 | 357000 | 0.49 |
| 0.375 | 9.5 | 34.2 | 0.336 | 8.9 | 17.5 | 4100 | 11.1 | 142 | 52.0 | 192 | 22100 | 285000 | 0.49 |
| 0.312 | 7.9 | 28.8 | 0.282 | 7.4 | 21.0 | 3450 | 9.48 | 122 | 52.5 | 163 | 19000 | 244000 | 0.49 |
| 0.250 | 6.4 | 23.5 | 0.231 | 5.9 | 26.4 | 2770 | 7.78 | 100 | 53.0 | 132 | 15600 | 200000 | 0.49 |
| 0.188 | 4.8 | 17.9 | 0.175 | 4.4 | 35.4 | 2090 | 5.98 | 76.8 | 53.5 | 101 | 12000 | 154000 | 0.49 |
| $6.000 \times 0.500$ | $152.4 \times 12.7$ | 43.8 | 0.429 | 11.8 | 12.9 | 5210 | 13.0 | 170 | 49.9 | 234 | 25900 | 340000 | 0.48 |
| 0.375 | 9.5 | 33.5 | 0.328 | 8.9 | 17.1 | 4010 | 10.4 | 136 | 50.8 | 184 | 20700 | 272000 | 0.48 |
| 0.312 | 7.9 | 28.2 | 0.276 | 7.4 | 20.6 | 3370 | 8.88 | 117 | 51.3 | 156 | 17800 | 233000 | 0.48 |
| 0.280 | 7.1 | 25.4 | 0.249 | 6.6 | 23.1 | 3020 | 8.05 | 106 | 51.6 | 140 | 16100 | 211000 | 0.48 |
| 0.250 | 6.4 | 23.0 | 0.226 | 5.9 | 25.8 | 2720 | 7.30 | 95.8 | 51.8 | 127 | 14600 | 192000 | 0.48 |
| 0.188 | 4.8 | 17.5 | 0.171 | 4.4 | 34.6 | 2050 | 5.61 | 73.6 | 52.3 | 96.4 | 11200 | 147000 | 0.48 |
| 0.125 | 3.2 | 11.8 | 0.115 | 2.9 | 52.6 | 1360 | 3.81 | 50.0 | 52.9 | 64.8 | 7610 | 99900 | 0.48 |

### 6.6.2 Calculating the Weight of Standard, Extra Strong, and Double Strong Steel Pipes

| Steel Pipe |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Standard Weight Pipe |  | Extra Strong Pipe |  | Double Extra Strong Pipe |  |
| $1 / 2^{\prime \prime}$ | $0.85 \mathrm{lbs} / \mathrm{lnft}$ | $1 / 2{ }^{\prime \prime}$ | $1.09 \mathrm{lbs} / \mathrm{lnft}$ | $2^{\prime \prime}$ | 9.03 |
| $3 / 4$ " | 1.13 | $3 / 4{ }^{\prime \prime}$ | 1.47 | $21 / 2^{\prime \prime}$ | 13.69 |
| $1^{\prime \prime}$ | 1.68 | $1^{\prime \prime}$ | 2.17 | $3^{\prime \prime}$ | 18.58 |
| $11 / 4^{\prime \prime}$ | 2.27 | $11 / 4^{\prime \prime}$ | 3.00 | $4^{\prime \prime}$ | 27.54 |
| $11 / 2^{\prime \prime}$ | 2.72 | $11 / 2^{\prime \prime}$ | 3.63 | 5" | 38.55 |
| $2^{\prime \prime}$ | 3.65 | $2^{\prime \prime}$ | 5.02 | $6^{\prime \prime}$ | 53.16 |
| $21 / 2^{\prime \prime}$ | 5.79 | $21 / 2^{\prime \prime}$ | 7.66 | $8^{\prime \prime}$ | 74.42 |
| 3 " | 7.58 | $3 \prime \prime$ | 10.25 |  |  |
| $31 / 2^{\prime \prime}$ | 9.11 | $31 / 2^{\prime \prime}$ | 12.50 |  |  |
| $4{ }^{\prime \prime}$ | 10.79 | $4^{\prime \prime}$ | 14.98 |  |  |
| 5" | 14.62 | $5^{\prime \prime}$ | 20.78 |  |  |
| $6^{\prime \prime}$ | 18.97 | $6{ }^{\prime \prime}$ | 28.57 |  |  |
| $8^{\prime \prime}$ | 28.55 | $8^{\prime \prime}$ | 43.39 |  |  |
| $10^{\prime \prime}$ | 40.48 | $10^{\prime \prime}$ | 54.74 |  |  |
| $12^{\prime \prime}$ | 49.56 | $12^{\prime \prime}$ | 65.42 |  |  |

### 6.7.0 Calculating the Weight and Size of U.S. Steel C Channels




Channels are available in Carbon, Stainless Steel, and Aluminum. Sizes shown are for Carbon only.

| MC-Shapes | A Depth in Inches | B Width in Inches | C Web in Inches | Weight Lbs |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Per Ft. | 20-Ft. | 30-Ft. | 40-Ft. |
| MC $8 \times 18.7$ | 8 | 2.978 | . 353 | 18.7 | 374 | 561 | 748 |
| MC $8 \times 20$ | 8 | 3.025 | . 400 | 20 | 400 | 600 | 800 |
| MC $8 \times 21.4$ | 8 | 3.450 | . 375 | 21.4 | 428 | 642 | 856 |
| MC $8 \times 22.8$ | 8 | 3.502 | . 353 | 22.8 | 456 | 684 | 912 |
| MC $9 \times 23.9$ | 9 | 3.450 | . 400 | 23.9 | 478 | 717 | 956 |
| MC $9 \times 25.4$ | 9 | 3.500 | . 450 | 25.4 | 508 | 762 | 1016 |
| MC $10 \times 6.5$ | 10 | 1.125 | . 150 | 6.5 | 130 | 195 | 260 |
| MC $10 \times 8.4$ | 10 | 1.500 | . 170 | 8.4 | 168 | 252 | 336 |
| MC $10 \times 22$ | 10 | 3.315 | . 290 | 22 | 440 | 660 | 880 |
| MC $10 \times 25$ | 10 | 3.405 | . 380 | 25 | 500 | 750 | 1000 |
| MC $10 \times 28.5$ | 10 | 3.950 | . 425 | 28.5 | 570 | 855 | 1140 |
| MC $10 \times 33.6$ | 10 | 4.100 | . 575 | 33.6 | 672 | 1008 | 1344 |
| MC $12 \times 10.6$ | 12 | 1.500 | 1.90 | 10.6 | 212 | 318 | 424 |
| MC $12 \times 31$ | 12 | 3.670 | . 370 | 31.0 | 620 | 930 | 1240 |
| MC $12 \times 35$ | 12 | 3.767 | . 467 | 35.0 | 700 | 1050 | 1400 |
| MC $12 \times 37$ | 12 | 3.600 | . 600 | 37.0 | 740 | 1110 | 1480 |
| MC $12 \times 40$ | 12 | 3.890 | . 590 | 40.0 | 800 | 1200 | 1600 |
| MC $12 \times 45$ | 12 | 4.012 | . 712 | 45.0 | 900 | 1350 | 1800 |
| MC $12 \times 50$ | 12 | 4.135 | . 835 | 50.0 | 1000 | 1500 | 2000 |
| MC $13 \times 31.8$ | 13 | 4.000 | . 375 | 31.8 | 636 | 954 | 1272 |
| MC $13 \times 40$ | 13 | 4.185 | . 560 | 40.0 | 800 | 1200 | 1600 |
| MC $13 \times 50$ | 13 | 4.412 | . 787 | 50.0 | 1000 | 1500 | 2000 |
| MC $18 \times 42.7$ | 18 | 3.950 | . 450 | 42.7 | 854 | 1281 | 1708 |
| MC $18 \times 45.8$ | 18 | 4.000 | . 500 | 45.8 | 916 | 1374 | 1832 |
| MC $18 \times 51.9$ | 18 | 4.100 | . 600 | 51.9 | 1038 | 1557 | 2076 |
| MC $18 \times 58$ | 18 | 4.200 | . 700 | 58.0 | 1160 | 1740 | 2320 |

### 6.7.0.1 Calculating the Weight and Size of U.S. A-36 and A-36 Modified C Channels

Conforms to A-36 and A-36 Modified Standard Lengths 2,' 40,' and 60'

| A <br> Depth in Inches | Weight Lbs per Foot | B <br> Flange Width Inches | C <br> Web <br> Thickness In. | Weight Lbs |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | 20 Ft <br> Length | 30 Ft <br> Length | 40 Ft <br> Length | 60 Ft <br> Length |
| 3 | 4.1 | 1.410 | . 170 | 82 | 123 | 164 | 246 |
| 3 | 5.0 | 1.498 | . 258 | 100 | 150 | 200 | 300 |
| 3 | 6.0 | 1.596 | . 356 | 120 | 180 | 240 | 360 |
| 4 | 5.4 | 1.580 | . 180 | 108 | 162 | 216 | 324 |
| 4 | 6.25 | 1.647 | . 247 | 125 | 188 | 250 | 375 |
| 4 | 7.25 | 1.720 | . 320 | 145 | 218 | 290 | 435 |
| 5 | 6.7 | 1.750 | . 190 | 134 | 201 | 268 | 402 |
| 5 | 9 | 1.885 | . 325 | 180 | 270 | 360 | 540 |
| 6 | 8.2 | 1.920 | . 200 | 164 | 246 | 328 | 492 |
| 6 | 10.5 | 2.034 | . 314 | 210 | 315 | 420 | 630 |
| 6 | 13.0 | 2.157 | . 437 | 260 | 390 | 520 | 780 |
| 7 | 9.8 | 2.090 | . 210 | 196 | 294 | 392 | 588 |
| 7 | 12.25 | 2.194 | . 314 | 245 | 368 | 490 | 735 |
| 7 | 14.75 | 2.299 | . 419 | 295 | 443 | 590 | 885 |
| 8 | 8.55 | 1.875 | . 180 | 170 | 255 | 340 | 510 |
| 8 | 11.5 | 2.260 | . 220 | 230 | 345 | 460 | 690 |
| 8 | 13.75 | 2.343 | . 303 | 275 | 413 | 550 | 825 |
| 9 | 18.75 | 2.527 | . 487 | 375 | 563 | 750 | 1125 |
| 9 | 1.3.4 | 2.430 | . 230 | 268 | 402 | 536 | 801 |
| 9 | 15.0 | 2.485 | . 285 | 300 | 450 | 600 | 900 |
| 10 | 20.0 | 2.648 | . 448 | 400 | 600 | 800 | 1200 |
| 10 | 15.3 | 2.600 | . 240 | 306 | 459 | 612 | 918 |
| 10 | 20.0 | 2.739 | . 379 | 400 | 600 | 800 | 1200 |
| 10 | 25.0 | 2.886 | . 526 | 500 | 750 | 1000 | 1500 |
| 12 | 30.0 | 3.033 | . 673 | 600 | 900 | 1200 | 1800 |
| 12 | 20.7 | 2.940 | . 280 | 414 | 621 | 828 | 1242 |
| 12 | 25.0 | 3.047 | . 387 | 500 | 750 | 1000 | 1500 |
| 12 | 30.0 | 3.170 | . 510 | 600 | 900 | 1200 | 1800 |
| 15 | 33.9 | 3.400 | . 400 | 678 | 1017 | 1356 | 2034 |
| 15 | 40.0 | 3.520 | . 520 | 800 | 1200 | 1600 | 2400 |
| 15 | 50.0 | 3.716 | . 716 | 1000 | 1500 | 2000 | 3000 |

### 6.7.0.2 Calculating the Weight and Size of U.S. Channels-Ship and Car



### 6.7.1 Calculating the Weight and Size of Metric Steel C Channels

Disclaimer: The information on this page has not been checked by an independent person. Use this information at your own risk.


## Dimensions

$\mathrm{M}=$ Mass per $\mathrm{m}, \mathrm{D}=$ Depth of Section, $\mathrm{B}=$ Width of Section, $\mathrm{T} 1=$ Web Thickness, T2 $=$ Flange Thickness, R1 $=$ Root Radius, R2 $=$ Toe Rad, Area $=$ Area of Section

| Serial Size <br> mm | $\begin{aligned} & \mathrm{M} \\ & \mathrm{~kg} \end{aligned}$ | D <br> mm | B <br> mm | $\begin{aligned} & \mathrm{T} 1 \\ & \mathrm{~mm} \end{aligned}$ | $\begin{aligned} & \mathrm{T} 2 \\ & \mathrm{~mm} \end{aligned}$ | $\begin{aligned} & \mathrm{R} 1 \\ & \mathrm{~mm} \end{aligned}$ | $\begin{aligned} & \mathrm{R} 2 \\ & \mathrm{~mm} \end{aligned}$ | $\begin{aligned} & \text { DT } \\ & \mathrm{mm} \end{aligned}$ | D/T | Area $\mathrm{cm}^{2}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $432 \times 102$ | 65.54 | 431.8 | 101.6 | 12.2 | 16.8 | 15.2 | 4.8 | 362.5 | 25.7 | 83.49 |
| $381 \times 102$ | 55.1 | 381.0 | 101.6 | 10.4 | 16.3 | 15.2 | 4.8 | 312.4 | 23.4 | 70.19 |
| $305 \times 102$ | 46.18 | 304.8 | 101.6 | 10.2 | 14.8 | 15.2 | 4.8 | 239.3 | 20.6 | 58.83 |
| $305 \times 89$ | 41.69 | 304.8 | 88.9 | 10.2 | 13.7 | 13.7 | 3.2 | 245.4 | 22.2 | 53.11 |
| $254 \times 89$ | 35.74 | 254.0 | 88.9 | 9.1 | 13.6 | 13.7 | 3.2 | 194.8 | 18.7 | 45.52 |
| $254 \times 76$ | 28.29 | 254.0 | 76.2 | 8.1 | 10.9 | 12.2 | 3.2 | 203.7 | 23.3 | 36.03 |
| $229 \times 89$ | 32.76 | 228.6 | 88.9 | 8.6 | 13.3 | 13.7 | 3.2 | 169.9 | 17.2 | 41.73 |
| $\underline{229 \times 76}$ | 26.06 | 228.6 | 76.2 | 7.6 | 11.2 | 12.2 | 3.2 | 178.1 | 20.4 | 33.20 |
| $203 \times 89$ | 29.78 | 203.2 | 88.9 | 8.1 | 12.9 | 13.7 | 3.2 | 145.3 | 15.8 | 37.94 |
| $203 \times 76$ | 23.82 | 203.2 | 76.2 | 7.1 | 11.2 | 12.2 | 3.2 | 152.4 | 18.1 | 30.34 |
| $178 \times 89$ | 26.81 | 177.8 | 88.9 | 7.6 | 12.3 | 13.7 | 3.2 | 120.9 | 14.5 | 34.15 |
| $178 \times 76$ | 20.84 | 177.8 | 76.2 | 6.6 | 10.3 | 12.2 | 3.2 | 128.8 | 17.3 | 26.54 |
| $152 \times 89$ | 23.84 | 152.4 | 88.9 | 7.1 | 11.6 | 13.7 | 3.2 | 97.0 | 13.1 | 30.36 |
| $152 \times 76$ | 17.88 | 152.4 | 76.2 | 6.4 | 9.0 | 12.2 | 2.4 | 105.9 | 16.9 | 22.77 |
| $127 \times 64$ | 14.90 | 127.0 | 63.5 | 6.4 | 9.2 | 10.7 | 2.4 | 84.1 | 13.8 | 18.98 |
| $102 \times 51$ | 10.42 | 101.6 | 50.8 | 6.1 | 7.6 | 9.1 | 2.4 | 65.8 | 13.4 | 13.28 |

### 6.7.1.1 Calculating the Weight and Size of Metric Channel, Box, Rectangular, and Square Tubing

| Web O.D | Web I.D | Leg O.D | Leg I.D | Returns O.D. | Gap/Opening I.D. | Metal Thickness |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| .425" | . $377 \prime$ | .679" | .631" | .191" | .011" | 0.024 |
| . 50011 | . 342 " | .500" | . $342^{\prime \prime}$ | .125" | . 250 " | 0.079 |
| . $500{ }^{\prime \prime}$ | . 380 " | . $500{ }^{\prime \prime}$ | . $380{ }^{\prime \prime}$ | .125" | . 250 " | 0.060 |
| . $500{ }^{\prime \prime}$ | . $428{ }^{\prime \prime}$ | . $500{ }^{\prime \prime}$ | .428" | .125" | . 250 " | 0.036 |
| . $600^{\prime \prime}$ | .528" | .148" | .076" | .135" | . 330 " | 0.036 |
| . 609 " | .535" | .487" | .413" | .145" | . $319^{\prime \prime}$ | 0.037 |
| . 745 " | .625" | . 620 " | . $500^{\prime \prime}$ | .185" | . $375^{\prime \prime}$ | 0.060 |
| . 750 " | .702" | . 281 " | . 233 " | . 250 " | . 250 " ${ }^{\prime \prime}$ | 0.024 |
| . 750 " | . $600{ }^{\prime \prime}$ | . 750 " | . $600^{\prime \prime}$ | . 250 " | . 250 " | 0.075 |
| . 750 " | . $630^{\prime \prime}$ | . 750 " | . $630^{\prime \prime}$ | . $250^{\prime \prime}$ | . 250 " | 0.060 |
| . 780 " | .742" | . $575{ }^{\prime \prime}$ | .537" | . 269 " | . 242 " | 0.019 |
| .813" | .733" | . $406{ }^{\prime \prime}$ | .272" | . $188{ }^{\prime \prime}$ | . $437{ }^{\prime \prime}$ | 0.040 |
| .860" | . $800{ }^{\prime \prime}$ | . $160{ }^{\prime \prime}$ | . $100^{\prime \prime}$ | . $115^{\prime \prime}$ | . $630^{\prime \prime}$ | 0.030 |
| .904" | . 784 " | 1.004 ${ }^{\prime \prime}$ | .793" | . $226^{\prime \prime}$ | . $452{ }^{\prime \prime}$ | 0.060 |
| .922" | . 850 " | .178" | .106" | . $156{ }^{\prime \prime}$ | . $610^{\prime \prime}$ | 0.036 |
| .961" | . $913^{\prime \prime}$ | .704" | . $656^{\prime \prime}$ | . 303 " | . $355^{\prime \prime}$ | 0.024 |
| .970" | .898" | .187" | . $115^{\prime \prime}$ | . $145^{\prime \prime}$ | . $680^{\prime \prime}$ | 0.036 |
| .975" | .903" | .178" | .106" | .219" | . $537{ }^{\prime \prime}$ | 0.036 |
| 1.000 ${ }^{\prime \prime}$ | . 964 " | . $200{ }^{\prime \prime}$ | .164" | .095" | . $810^{\prime \prime}$ | 0.018 |
| 1.000 ${ }^{\prime \prime}$ | . $952^{\prime \prime}$ | . $500{ }^{\prime \prime}$ | .452" | . 343 " | . $315^{\prime \prime}$ | 0.024 |
| $1.000^{\prime \prime}$ | . 904 " | $1.000^{\prime \prime}$ | .904" | . $375^{\prime \prime}$ | . 250 " | 0.048 |
| $1.000^{\prime \prime}$ | .880" | $1.000^{\prime \prime}$ | . $880{ }^{\prime \prime}$ | . $375^{\prime \prime}$ | . 250 " | 0.060 |
| 1.000 ${ }^{\prime \prime}$ | .896" | $1.500^{\prime \prime}$ | $1.396^{\prime \prime}$ | .171" | .658" | 0.052 |
| 1.038 ${ }^{\prime \prime}$ | . 982 " | . 923 " | .867" | .288" | . $462^{\prime \prime}$ | 0.028 |
| 1.042" | . 982 " | . $471^{\prime \prime}$ | .411" | . $117^{\prime \prime}$ \& $.283^{\prime \prime}$ | . $642^{\prime \prime}$ | 0.030 |
| 1.050" | . 984 " | . $780^{\prime \prime}$ | . $714^{\prime \prime}$ | . $240^{\prime \prime}$ | . $570^{\prime \prime}$ | 0.033 |
| 1.062" | . $990{ }^{\prime \prime}$ | . $156{ }^{\prime \prime}$ | .084" | . $156^{\prime \prime}$ | . 750 " | 0.036 |
| 1.215" | $1.143^{\prime \prime}$ | .188" | . $116^{\prime \prime}$ | . $145^{\prime \prime}$ | .925" | 0.036 |
| 1.236 ${ }^{\prime \prime}$ | $1.176^{\prime \prime}$ | .866" | .806" | . $278{ }^{\prime \prime}$ | . $680^{\prime \prime}$ | 0.030 |
| 1.249 ${ }^{\prime \prime}$ | 1.199 ${ }^{\prime \prime}$ | . $219^{\prime \prime}$ | .169" | . $156{ }^{\prime \prime}$ | . $937{ }^{\prime \prime}$ | 0.025 |
| 1.250 ${ }^{\prime \prime}$ | $1.156^{\prime \prime}$ | . $437^{\prime \prime}$ | . $312^{\prime \prime}$ | . $156{ }^{\prime \prime}$ | . $938{ }^{\prime \prime}$ | 0.047 |
| 1.250 ${ }^{\prime \prime}$ | $1.124^{\prime \prime}$ | $1.250^{\prime \prime}$ | 1.124" | . 250 " | . 750 " | 0.063 |
| 1.250 ${ }^{\prime \prime}$ | $1.130^{\prime \prime}$ | $2.000^{\prime \prime}$ | $1.880^{\prime \prime}$ | . 595 " | . 060 " ${ }^{\prime \prime}$ | 0.060 |


| Web O.D | Web I.D | Leg O.D | Leg I.D | Returns O.D. | Gap/Opening I.D. | Metal Thickness |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1.253" | $1.133^{\prime \prime}$ | .209" | .089" | . 470 " | . $313^{\prime \prime}$ | 0.060 |
| 1.259 ${ }^{\prime \prime}$ | 1.199" | .197" | .137" | . 100 " | $1.060^{\prime \prime}$ | 0.030 |
| 1.375 ${ }^{\prime \prime}$ | $1.225^{\prime \prime}$ | . 480 " | . 330 " | . 250 " | .875" | 0.075 |
| 1.403" | 1.291" | . $468{ }^{\prime \prime}$ | . 256 " | . 468 " | . 467 " | 0.056 |
| 1.491" | 1.431" | . $221^{\prime \prime}$ | .161" | . 221 " | 1.049" | 0.030 |
| 1:491" | $1.423^{\prime \prime}$ | . 246 " | .178" | . 280 " | .931" | 0.034 |
| 1.494" | $1.444^{\prime \prime}$ | . $219^{\prime \prime}$ | .169 ${ }^{\prime \prime}$ | .161" | 1.172" | 0.025 |
| 1.500 ${ }^{\prime \prime}$ | $1.404^{\prime \prime}$ | .500" | . $404{ }^{\prime \prime}$ | . $500^{\prime \prime}$ | .500" | 0.048 |
| 1.500 ${ }^{\prime \prime}$ | $1.380^{\prime \prime}$ | . $500^{\prime \prime}$ | . $380{ }^{\prime \prime}$ | . $500{ }^{\prime \prime}$ | . $500^{\prime \prime}$ | 0.060 |
| 1.500 ${ }^{\prime \prime}$ | $1.380^{\prime \prime}$ | .625" | .505" | . $312^{\prime \prime}$ | . $876{ }^{\prime \prime}$ | 0.060 |
| 1.500 ${ }^{\prime \prime}$ | $1.350^{\prime \prime}$ | $1.000^{\prime \prime}$ | . $850{ }^{\prime \prime}$ | . $312^{\prime \prime}$ | . $876{ }^{\prime \prime}$ | 0.075 |
| 1.500 ${ }^{\prime \prime}$ | $1.380^{\prime \prime}$ | $1.500^{\prime \prime}$ | $1.380^{\prime \prime}$ | . $500^{\prime \prime}$ | . $500{ }^{\prime \prime}$ | 0.060 |
| 1.725 ${ }^{\prime \prime}$ | 1.653" | .188" | .116 ${ }^{\prime \prime}$ | .145" | $1.435^{\prime \prime}$ | 0.036 |
| 1.998 ${ }^{\prime \prime}$ | $1.902^{\prime \prime}$ | $1.498{ }^{\prime \prime}$ | $1.402^{\prime \prime}$ | .655" | .688" | 0.048 |
| 2.000 ${ }^{\prime \prime}$ | . 760 " ${ }^{\prime \prime}$ | $2.000^{\prime \prime}$ | $1.760^{\prime \prime}$ | .735" | . $531{ }^{\prime \prime}$ | 0.120 |
| $2.000^{\prime \prime}$ | . 880 " | $2.000^{\prime \prime}$ | .880 ${ }^{\prime \prime}$ | .735" | . 531 " | 0.060 |
| 2.004 ${ }^{\prime \prime}$ | . 954 " | . $219^{\prime \prime}$ | .169 ${ }^{\prime \prime}$ | .161" | 1.682" | 0.025 |
| 2.125" | 2.029" | . $313^{\prime \prime}$ | .217" | . $313^{\prime \prime}$ | 1.499" | 0.048 |
| $2.250^{\prime \prime}$ | $2.122^{\prime \prime}$ | $2.000^{\prime \prime}$ | 1.872" | . $638^{\prime \prime}$ | .975" | 0.064 |
| $2.500^{\prime \prime}$ | $2.380^{\prime \prime}$ | $1.750^{\prime \prime}$ | $1.630^{\prime \prime}$ | . 563 " | $1.375^{\prime \prime}$ | 0.060 |
| $2.500^{\prime \prime}$ | $2.452^{\prime \prime}$ | . 563 " | . $469{ }^{\prime \prime}$ | . $312^{\prime \prime}$ | 1.875" | 0.024 |
| 3.438 ${ }^{\prime \prime}$ | $3.318^{\prime \prime}$ | $1.750^{\prime \prime}$ | . $630{ }^{\prime \prime}$ | . 532 " | $2.375^{\prime \prime}$ | 0.060 |
| $4.000^{\prime \prime}$ | $3.950^{\prime \prime}$ | . $100^{\prime \prime}$ | .050" | . $100^{\prime \prime}$ | $3.800^{\prime \prime}$ | 0.025 |
| $5.000^{\prime \prime}$ | $4.952^{\prime \prime}$ | . 563 " | . $469{ }^{\prime \prime}$ | .281" | $4.438^{\prime \prime}$ | 0.024 |

- Gap/Openings can be larger than shown; narrower requires tooling modification.
- Materials used: Steel, Stainless, Aluminum, Brass, Copper, Bronze, Tempered, Textured, Embossed, Perforated, Alloys, Clad.
- Prefinishes Used: Plain, Dull, Prepolished, Prepainted, Preanodized, Strippable-PVC Coated, Prelaminated Vinyl, Preplated, Prelaquered, Galvanized (Hot Dip: G30, G60, G90, G210, Bonderized, Electro, Chromate) Galvannealled, Aluminized, Galvanized.
- Many other sizes not shown here are possible.
- Thickness can vary slightly using same tooling.
- Inline Press Fabricating: Holes, Notching, End Fabricating, Perforating, Embossing, Cut-to-Length, and More.
- I.D's will vary with different thicknesses used.


## Source: Johnsonrollforming.com

### 6.8.0 Calculating the Weight and Size of Structural Steel Angles

|  |  | L6 $\times 6 \times 9 / 16$ | 21.9 | $\mathrm{L} 4 \times 31 / 2 \times 1 / 2$ | 11.9 |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | L6 $\times 6 \times 1 / 2$ | 19.6 | $\mathrm{L} 4 \times 311 / 2 \times 7 / 16$ | 10.6 |
| Angles |  | L6 $\times 6 \times 7 / 16$ | 17.2 | $\mathrm{L} 4 \times 3$ 12 $2 \times 3 / 8$ | 9.1 |
|  |  | L6 $\times 6 \times 3 / 8$ | 14.9 | $\mathrm{L} 4 \times 311 / 2 \times 5 / 16$ | 7.7 |
| Weight per Lnft |  | L6 $\times 6 \times 5 / 16$ | 12.4 | $\mathrm{L} 4 \times 31 / 2 \times 1 / 4$ | 6.2 |
|  |  | L6 $\times 4 \times 7 / 8$ | 27.2 | $\mathrm{L} 4 \times 3 \times 1 / 2$ | 11.1 |
| L9 $\times 4 \times 5 / 8$ | $26.3 \mathrm{lbs} / \mathrm{lnft}$ | L6 $\times 4 \times 3 / 4$ | 23.6 | L4 $\times 3 \times 7 / 16$ | 9.8 |
| L9 $\times 4 \times 9 / 16$ | 23.8 | L6 $\times 4 \times 5 / 8$ | 20.0 | L4 $\times 33 / 8$ | 8.5 |
| L9 $\times 4 \times 1 / 2$ | 21.3 | L6 $\times 4 \times 9 / 16$ | 18.1 | L4 $\times 3 \times 5 / 16$ | 7.2 |
|  |  | L6 $\times 4 \times 1 / 2$ | 16.2 | $\mathrm{L} 4 \times 3 \times 1 / 4$ | 5.8 |
| L8 $\times 8 \times 1-1 / 8$ | 56.9 | L6 $\times 4 \times 7 / 16$ | 14.3 |  |  |
| L8 $\times 8 \times 1$ | 51.0 | L6 $\times 4 \times 3 / 8$ | 12.3 | L3 ${ }^{1} 2 \times 31 / 2 \times 1 / 2$ | 11.1 |
| L8 $\times 8 \times 7 / 8$ | 45.0 | L6 $\times 4 \times 5 / 16$ | 10.3 | L3 $312 \times 31 / 2 \times 7 / 16$ | 9.8 |
| L8 $\times 8 \times 3 / 4$ | $\begin{array}{ll}38.9 & \\ 32.7 & \mathrm{~L} 6 \times 31 / 2 \times 1 / 2\end{array}$ |  |  | L3 $1 / 2 \times 31 / 2 \times 3 / 8$ |  |
| L8 $\times 8 \times 5 / 8$ |  |  |  | 15.3 | L3 $1 / 2 \times 31 / 2 \times 5 / 16$ | 8.8 7.2 |
| L8 $\times 8 \times 9 / 16$ | 29.6 | L6 $\times 31 / 2 \times 3 / 8$ | 11.7 | L3 $31 / 2 \times 31 / 2 \times 1 / 4$ | 5.8 |
| L8 $\times 8 \times 1 / 2$ | 26.4 | $\mathrm{L} 6 \times 3112 \times 5 / 16$ | 9.8 |  |  |
|  |  |  |  | $31 / 2 \times 3 \times 1 / 2$ | 10.2 |
| L8 $\times 6 \times 1$ | 44.2 | L5 $\times 5 \times 7 / 8$ | 27.2 | L3 $312 \times 3 \times 7 / 16$ | 9.1 |
| L8 $\times 6 \times 7 / 8$ | 39.1 | L5 $\times 5 \times 3 / 4$ | 23.6 | L3 $1 / 2 \times 3 \times 3 / 8$ | 7.9 |
| L8 $\times 6 \times 3 / 4$ | 33.8 | L5 $\times 3 \times 5 / 8$ | 20.0 | L3 $112 \times 3 \times 5 / 16$ | 6.6 |
| L8 $\times 6 \times 5 / 8$ | 28.5 | L5 $\times 3 \times 1 / 2$ | 16.2 | L3 ${ }^{1} / 2 \times 3 \times 1 / 4$ | 5.4 |
| L8 $\times 6 \times 9 / 16$ | 25.7 | L5 $\times 3 \times 7 / 16$ | 14.3 |  |  |
| L8 $\times 6 \times 1 / 2$ | 23.0 | L5 $\times 5 \times 3 / 8$ | 12.3 | L3 ${ }^{1} 2 \times 21 / 2 \times 1 / 2$ | 9.4 |
| L8 $\times 6 \times 7 / 16$ | 20.2 | L5 $\times 5 \times 5 / 16$ | 10.3 | L3 $1 / 2 \times 21 / 2 \times 7 / 16$ | 8.3 |
|  |  |  |  | L3 $31 / 2 \times 21 / 2 \times 3 / 8$ | 7.2 |
| L8 $\times 4 \times 1$ | 37.4 | L5 $\times 31 / 2 \times 3 / 4$ | 19.8 | L3 ${ }^{1} 2 \times 21 / 2 \times 5 / 16$ | 6.1 |
| $\mathrm{L} 8 \times 4 \times 3 / 4$ | 28.7 | L5 $\times 31 / 2 \times 5 / 8$ | 16.8 | L $31 / 2 \times 21 / 2 \times 1 / 4$ | 4.9 |
| $\mathrm{L} 8 \times 4 \times 9 / 16$ | 21.9 | L5 $\times 31 / 2 \times 1 / 2$ | 13.6 |  |  |
| L8 $\times 4 \times 1 / 2$ | 19.6 | L5 $\times 31 / 2 \times 7 / 16$ | 12.0 | L3 $\times 3 \times 1 / 2$ | 9.4 |
|  |  | L5 $\times 31 / 2 \times 3 / 8$ | 10.4 | L3 $\times 3 \times 7 / 16$ | 8.3 |
| L7 $\times 4 \times 3 / 4$ | 26.2 | L5 $\times 31 / 2 \times 5 / 16$ | 8.7 | L3 $\times 3 \times 3 / 8$ | 7.2 |
| L7 $\times 4 \times 5 / 8$ | 22.1 | L5 $\times 31 / 2 \times 1 / 4$ | 7.0 | $\mathrm{L} 3 \times 3 \times 5 / 16$ | 6.1 |
| L7 $\times 4 \times 1 / 2$ | 17.9 |  |  | L3 $\times 3 \times 1 / 4$ | 4.9 |
| L7 $\times 4 \times 3 / 8$ | 13.6 | L5 $\times 3 \times 5 / 8$ | 15.7 | L3 $\times 3 \times 3 / 16$ | 3.71 |
|  |  | L5 $\times 3 \times 1 / 2$ | 12.8 - |  |  |
| $\mathrm{L} 6 \times 6 \times 1$ | 37.4 | L5 $\times 3 \times 7 / 16$ | 11.3 | L3 $\times 2^{1 / 2 \times 1 / 2}$ | 8.5 |
| L6 $\times 6 \times 7 / 8$ | 33.1 | L5 $\times 3 \times 3 / 8$ | 9.8 | L3 $\times 2 \underline{1 / 2 \times 7 / 16 ~}$ | 7.6 |
| L6 $\times 6 \times 3 / 4$ | 28.7 | L5 $\times 3 \times 5 / 16$ | 8.2 | L3 $\times 2 \underline{1 / 2} \times 3 / 8$ | 6.6 |
| L6 $\times 6 \times 5 / 8$ | 24.2 | L5 $\times 3 \times 1 / 4$ | 6.6 | L3 $\times 2 \underline{1 / 2 \times 5 / 16 ~}$ | 5.6 |
|  |  |  |  | L3 $\times 21 / 2 \times 1 / 4$ | 4.5 |
|  |  | $\mathrm{L} 4 \times 4 \times 3 / 4$ | 18.5 | L3 $\times 2 \underline{1} 2 \times 3 / 16$ | 3.39 |
|  |  | L4 $\times 4 \times 5 / 8$ | 15.7 |  |  |
|  |  | $\mathrm{L} 4 \times 4 \times 1 / 2$ | 12.8 | L3 $\times 2 \times 1 / 2$ | 7.7 |
|  |  | L4 $\times 4 \times 7 / 16$ | 11.3 | L3 $\times 2 \times 7 / 16$ | 6.8 |
|  |  | L4 $\times 4 \times 3 / 8$ | 9.8 | L3 $\times 2 \times 3 / 8$ | 5.9 |
|  |  | L4 $\times 4 \times 5 / 16$ | 8.2 | L3 $\times 2 \times 5 / 16$ | 5.0 |
|  |  | L4 $\times 4 \times 1 / 4$ | 6.6 | L3 $\times 2 \times 1 / 4$ | 4.1 |
|  |  |  |  | L3 $\times 2 \times 3 / 16$ | 3.0 |
|  |  |  |  | L2 ${ }^{1} / 2 \times 21 / 2 \times 1 / 2$ | 7.7 |
|  |  |  |  | L2 $1^{1} 2 \times 21 / 2 \times 3 / 8$ | 5.9 |
|  |  |  |  | L2 ${ }^{1} 2 \times 21 / 2 \times 5 / 16$ | 5.0 |
|  |  |  |  | L2 $1^{1 / 2} \times 21 / 2 \times 1 / 4$ | 4.1 |



Source: Mc2-ice.com-Wt of Steel Angles, p. 2

### 6.9.0 Calculating the Weight and Size of Universal Mill Plates




Source: Illinois Steel Service

### 6.10.0 Bar Size Tees-Calculating Their Weight and Size



### 6.11.0 Cold and Hot Rolled Rounds-Calculating Their Weight and Size



### 6.12.0 Aluminum Structural Angles-Calculate Their Weight and Size

## Structural Angles



6061-T6 Aluminum Structural Angles
25 Ft Lengths-Equal Leg-ASTM-B221-or- QQ-A-200/16

| Size |  |  |  |  | Approx. Wt. per Lin. Ft | Approx. Wt. per Length | Group/Size/Grade |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A |  | A |  | C |  |  |  |
| 3/4 | $\times$ | 3/4 | $\times$ | 1/8 | . 201 | 5.03 | 6061A\.75-.125\T6 |
| 1 | $\times$ | 1 | $\times$ | 1/8 | . 275 | 6.8 | 6061A\1-.125\T6 |
|  |  |  |  | 3/16 | . 400 | 10.00 | $6061 \mathrm{~A} \backslash 1$-. $1875 \backslash$ T6 |
|  |  |  |  | 1/4 | . 514 | 12.85 | 6061A\1-.25\T6 |
| 1-1/4 | $\times$ | 1-1/4 | $\times$ | 1/8 | . 343 | 8.58 | 6061A\1.25-.125\T6 |
|  |  |  |  | 3/16 | . 510 | 12.75 | 6061A\1.25-.1875\T6 |
|  |  |  |  | 1/4 | . 656 | 16.40 | 6061A\1.25-.25\T6 |
| 1-1/2 | $\times$ | 1-1/2 | $\times$ | 1/8 | . 423 | 10.58 | 6061A\1.5-. $125 \backslash$ T6 |
|  |  |  |  | 3/16 | . 619 | 15.48 | 6061A\1.5-.1875\T6 |
|  |  |  |  | 1/4 | . 809 | 20.23 | $6061 \mathrm{~A} \backslash 1.5-.25 \backslash \mathrm{T6}$ |
| 1-3/4 | $\times$ | 1-3/4 | $\times$ | 1/4 | . 980 | 24.50 | $6061 \mathrm{~A} \backslash 1.75-.25 \backslash \mathrm{~T} 6$ |
| 2 | $\times$ | 2 | $\times$ | 1/8 | . 577 | 14.43 | 6061A\2-.125\T6 |
|  |  |  |  | 3/16 | . 850 | 21.25 | 6061A\2-.1875\T6 |
|  |  |  |  | 1/4 | 1.110 | 27.75 | $6061 \mathrm{~A} \backslash 2-.25 \backslash$ T6 |
|  |  |  |  | 3/8 | 1.606 | 40.15 | $6061 \mathrm{~A} \backslash 2-.375 \backslash T 6$ |
| 2-1/2 | $\times$ | 2-1/2 | $\times$ | 3/16 | 1.070 | 26.75 | $6061 \mathrm{~A} \backslash 2.5-1875 \backslash \mathrm{T6}$ |
|  |  |  |  | 1/4 | 1.404 | 35.10 | $6061 \mathrm{~A} \backslash 2.5-.25 \backslash \mathrm{~T} 6$ |
|  |  |  |  | 3/8 | 2.047 | 51.18 | $6061 \mathrm{~A} \backslash 2.5-.375 \backslash \mathrm{~T} 6$ |
| 3 | $\times$ | 3 | $\times$ | 3/16 | 1.275 | 31.88 | 6061A\3-.1875\T6 |
|  |  |  |  | 1/4 | 1.684 | 42.10 | 6061A\3-.25\T6 |
|  |  |  |  | 3/8 | 2.474 | 61.85 | 6061A\3-.375\T6 |
|  |  |  |  | 1/2 | 3.227 | 80.68 | $6061 \mathrm{~A} \backslash 3-.5 \backslash \mathrm{~T} 6$ |
| 3-1/2 | $\times$ | 3-1/2 | $\times$ | 1/4 | 1.989 | 49.73 | $6061 \mathrm{~A} \backslash 3.5-.25 \backslash T 6$ |
| 4 | $\times$ | 4 | $\times$ | 1/4 | 2.283 | 57.08 | $6061 \mathrm{~A} \backslash 4-.25 \backslash$ T6 |
|  |  |  |  | 3/8 | 3.366 | 84.15 | $6061 \mathrm{~A} \backslash 4$-. $375 \backslash \mathrm{~T} 6$ |
|  |  |  |  | 1/2 | 4.414 | 110.35 | $6061 \mathrm{~A} \backslash 4$-. $5 \backslash \mathrm{T6}$ |
| 5 | $\times$ | 5 | $\times$ | 3/8 | 4.237 | 105.93 | $6061 \mathrm{~A} \backslash 5-.375 \backslash T 6$ |
| 6 | $\times$ | 6 | $\times$ | 1/2 | 6.754 | 168.85 | $6061 \mathrm{~A} \backslash 6$-. $5 \backslash \mathrm{~T} 6$ |
| 8 | $\times$ | 8 | $\times$ | 1/2 | 9.141 | 228.53 | $6061 \mathrm{~A} \backslash 8$-. $5 \backslash \mathrm{~T} 6$ |

[^13]
## 6061-T6 Aluminum Structural Angles

25 Ft. Lengths - Unequal Leg-ASTM-B221-or- QQ-A-200/16

| A | Size |  |  | C | Approx. Wt. Per Lin. Ft. | Approx. Wt. Per Length | Group/Size/Grade |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | B |  |  |  |  |  |
| 1-1/4 | $\times$ | 1 | $\times$ | 1/8 | . 31 | 7.75 | $6061 \mathrm{~A} \backslash 1.25-1.125 \backslash$ T6 |
| 1-1/2 | $\times$ | 1-1/4 | $\times$ | 3/16 | . 57 | 14.25 | 6061A\1.5-1.25.-18\T6 |
| 2 | $\times$ | 1-1/2 | $\times$ | 1/8 | . 496 | 12.40 | 6061A\2-1.5-.125\T6 |
|  |  |  |  | 3/16 | . 731 | 18.28 | 6061A\2-1.5-.1875\T6 |
|  |  |  |  | 1/4 | . 956 | 23.90 | $6061 \mathrm{~A} \backslash 2-1.5-.25 \backslash T 6$ |
| 2-1/2 | $\times$ | 2 | $\times$ | 3/16 | . 961 | 24.03 | $6061 \mathrm{~A} \backslash 2.5-2-.1875 \backslash T 6$ |
|  |  |  |  | 1/4 | 1.257 | 31.43 | $6061 \mathrm{~A} \backslash 2.5-2-.25 \backslash T 6$ |
| 3 | $\times$ | 1-1/2 | $\times$ | 3/16 | . 971 | 24.28 | 6061A\3-1.5-1875\T6 |
|  |  | 2 | $\times$ | 3/16 | 1.071 | 26.78 | 6061A\3-2-.1875\T6 |
|  |  |  |  | 1/4 | 1.403 | 35.08 | $6061 \mathrm{~A} \backslash 3-2-.25 \backslash T 6$ |
|  |  |  |  | 3/8 | 2.046 | 51.15 | 6061A\3-2-.375\T6 |
|  |  | 2-1/2 | $\times$ | 1/4 | 1.537 | 38.43 | 6061A\3-2.5-25\T6 |
| 3-1/2 | $\times$ | 2-1/2 | $\times$ | 1/4 | 1.684 | 42.10 | 6061A\3.5-2.5-.25\T6 |
| 4 | $\times$ | 3 | $\times$ | 1/4 | 1.988 | 49.70 | 6061A\4-3-.25\T6 |
|  |  |  |  | 3/8 | 2.926 | 73.15 | 6061A\4-3-.375\T6 |
| 5 | $\times$ | 3 | $\times$ | 3/8 | 3.450 | 86.30 | 6061A\5-3-.375\T6 |
| 6 | $\times$ |  | $\times$ | 3/8 | 4.237 | 105.93 | 6061A\6-4-.375\T6 |

### 6.12.1 Aluminum Channels-Calculate Their Weight and Size



## 6061-T6 Aluminum Structural Channels-Cont'd

| Size (Inches) <br> a | Web Thick <br> C | Flange Width <br> B | Approx. Wt. <br> per Lin. Ft | Approx. Wt. <br> per Length | Group/Size/Grade |
| :--- | :--- | :--- | :---: | :---: | :---: |
| 9 | .230 | 2.430 | 4.604 | 115.10 | $6061 N \backslash 9-.23 \backslash T 6-2$ |
| 10 | .240 | 2.600 | 5.278 | 131.95 | $6061 N \backslash 10-.24 \backslash T 6-2$ |
|  | .526 | 2.886 | 8.641 | 216.30 | $6061 N \backslash 10-.526 \backslash T 6-2$ |
| 12 | .300 | 2.960 | 7.415 | 185.38 | $6061 N \backslash 12-.3 \backslash T 6-2$ |
| 12 | .510 | 3.170 | 10.374 | 259.35 | $6061 N \backslash 12-.51 \backslash T 6-2$ |

6061-T6 Aluminum Structural Channels
Aluminum Association (AA)-25 Ft Lengths-ASTM-B221-or-QQ-A-200/16

| Size <br> (Inches) <br> A | Web <br> Thick <br> C | Flange Width B | Flange <br> Thick <br> C-1 | Radius R | Approx. Wt. per Lin. Ft | Approx. Wt. per Length | Group/Size/Grade |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 2 | . 13 | 1.00 | . 13 | . 10 | . 577 | 14.43 | 6061N\2-.13\T6-1 |
|  | . 17 | 1.25 | . 26 | . 15 | 1.071 | 26.78 | 6061N\2-.17\T6-1 |
| 3 | . 13 | 1.50 | . 20 | . 25 | 1.135 | 28.93 | 6061N $\backslash 3$-.13\T6-1 |
|  | . 17 | 1.75 | . 26 | . 25 | 1.597 | 39.93 | 6061N\3-.17\T6-1 |
| 4 | . 15 | 2.00 | . 23 | . 25 | 1.738 | 43.45 | $6061 \mathrm{~N} \backslash 4-.15 \backslash \mathrm{T6-1}$ |
|  | . 19 | 2.25 | . 29 | . 25 | 2.331 | 58.28 | $6061 \mathrm{~N} \backslash 4.19$ \T6-1 |
| 5 | . 15 | 2.25 | . 26 | . 30 | 2.212 | 55.30 | $6061 N \backslash 5-.15 \backslash T 6-1$ |
|  | . 19 | 2.75 | . 32 | . 30 | 3.089 | 77.23 | 6061N\5-.19\T6-1 |
| 6 | . 17 | 2.50 | . 29 | . 30 | 2.834 | 70.85 | 6061N\6-.17\T6-1 |
|  | . 21 | 3.25 | . 35 | . 30 | 4.060 | 100.75 | $6061 \mathrm{~N} \backslash 6$-.21 ${ }^{\text {T }}$ 6-1 |
| 7 | . 21 | 3.50 | . 38 | . 30 | 4.715 | 103.68 | $6061 \mathrm{~N} \backslash 7-.21 \backslash \mathrm{~T} 6-1$ |
| 8 | . 19 | 3.00 | . 35 | . 30 | 4.147 | 103.68 | $6061 N \backslash 8-.19$ \T6-1 |
|  | . 25 | 3.75 | . 41 | . 35 | 5.789 | 144.73 | $6061 N \backslash 8-.25 \backslash T 6-1$ |
| 10 | . 25 | 3.50 | . 41 | . 35 | 6.139 | 153.40 | $6061 \mathrm{~N} \backslash 10-.25 \backslash$ T6-1 |
| 12 | . 29 | 4.00 | . 41 | . 35 | 8.274 | 206.90 | $6061 N \backslash 12-.29 \backslash T 69-1$ |

6061-T6 Special Aluminum Channel
Sharp Corners-QQ-A-200/8—Die \# 40517-22 Ft Length—ASTM-B221

| Size (Inches) <br> A | Web Thick <br> C | Flange Width <br> B | Approx. Wt. <br> per Lin. Ft | Approx. Wt. <br> per Length | Group/Size/Grade |
| :--- | :--- | :--- | :---: | :---: | :---: |
| 2.50 | .125 | 1.50 | .787 | 17.31 | $6061 \mathrm{~N} \backslash 2.5-1.5-.125 \backslash$ T6-3 |

### 6.12.2 Aluminum Structural Beams-Calculate Their Weight and Size

## Beams

6061-T6 Aluminum Structural I Beam
American Standard-25 Ft Length—ASTM-B221-or-QQ-A-200/16

| Size (Inches) <br> A | Web Thick <br> C | Flange Width <br> B | Approx. Wt. <br> per Lin. Ft | Approx. Wt. <br> per Length | Group/Size/Grade |
| :--- | :---: | :---: | :---: | :---: | :--- |
| 3 | .170 | 2.330 | 1.963 | 49.80 | $6061 \backslash 13-.17 \backslash T 6-2$ |
| 4 | .349 | 2.509 | 2.591 | 64.78 | $6061 \backslash \backslash 3-.349 \backslash T 6-2$ |
|  | .190 | 2.660 | 2.664 | 66.60 | $6061 \backslash \backslash 4-.19 \backslash T 6-2$ |
| 5 | .326 | 2.796 | 3.283 | 82.08 | $6061 \backslash 4-.326 \backslash T 6-2$ |
| 6 | .210 | 3.000 | 3.430 | 85.75 | $6061 \backslash \backslash 5-.21 \backslash T 6-2$ |
|  | .494 | 3.284 | 5.000 | 127.50 | $6061 \backslash \backslash 5-.494 \backslash T 6-2$ |

6061-T6 Aluminum Structural I Beam
Aluminum Association (AA) - 25 Ft-Length-ASTM-B221-or- QQ-A-200/16
$\left.\begin{array}{llllllll}\hline \begin{array}{l}\text { Size } \\ \text { (Inches) }\end{array} & \begin{array}{l}\text { Web } \\ \text { Thick }\end{array} & \begin{array}{l}\text { Flange } \\ \text { Width } \\ \text { B }\end{array} & \begin{array}{l}\text { Flange } \\ \text { Thick } \\ \text { C-1 }\end{array} & \begin{array}{l}\text { Radius } \\ \mathbf{R}\end{array} & \begin{array}{l}\text { Approx. Wt. } \\ \text { per } \\ \text { Lin. Ft }\end{array} & \begin{array}{l}\text { Approx. Wt. } \\ \text { per } \\ \text { Length }\end{array} & \text { Group/Size/Grade }\end{array}\right]$

6061-T6 Aluminum Structural H Beams
25 PL Length - ASTM-B221 - or-QQ-A-200/16

| Size (in.) | Web <br> Thick | Flange <br> Width | Approx. <br> Wt. per <br> Lin. Ft | Approx. <br> Wt. per <br> Length | Group/Size/Grade |
| :--- | :--- | :--- | :--- | :--- | :--- |

6061-T6 Aluminum Structural Wide Flange Beams
Aluminum Association (AA) - 25 Ft Length-ASTM-B221-or- QQ-A-200/16
$\left.\begin{array}{llllll}\hline \begin{array}{l}\text { Size (in.) } \\ \text { A }\end{array} & \begin{array}{l}\text { Web Thick } \\ \text { C }\end{array} & \begin{array}{l}\text { Flange Width } \\ \text { B }\end{array} & \begin{array}{l}\text { Approx. Wt. per } \\ \text { Lin. Ft }\end{array} & \begin{array}{l}\text { Approx. Wt. per } \\ \text { Length }\end{array} & \text { Group/Size/Grade }\end{array}\right]$

### 6.13.0 Plate Steel-3/16" to $6^{\prime \prime}$ Thickness-Calculate TheirWeight and Size

Plate Steel
Hot Rolled C-33 MAX, ASTM-A36

| Size in Inches | Wt. Per Ft, Lb | Size in Inches | Wt. per Ft, Lb |
| :---: | :---: | :---: | :---: |
| *3/16 $\times$ (7.65 Lb per Sq Ft) |  | $11 / 8 \times(45.9 \mathrm{Lb}$ per Sq Ft) |  |
| 84 | 53.55 | 72 | 275.40 |
| 96 | 61.20 | 96 | 367.20 |
| 1/4 $\times(10.20$ Lb Per Sq Ft) |  | $11 / 4 \times(51.00 \mathrm{Lb}$. per Sq Ft) |  |
| 30 | 25.50 | 36 | 153.00 |
| 36 | 30.60 | 48 | 204.00 |
| 42 | 35.70 | 60 | 255.00 |
| 48 | 40.80 | 72 | 306.00 |
| 54 | 45.90 | 84 | 357.00 |
| 60 | 51.00 | 96 | 408.00 |
| 72 | 61.20 | $13 / 8 \times(56.10 \mathrm{Lb}$ per Sq Ft) |  |
| 84 | 71.40 | 60 | 280.50 |
| 96 | 81.60 | 72 | 336.60 |
| 120 | 102.00 | 84 | 392.70 |
| $5 / 16 \times(12.75 \mathrm{Lb}$ per Sq Ft) |  | 96 | 448.80 |
| 30 | 31.88 | $11 / 2 \times(61.20$ per Sq Ft $)$ |  |
| 36 | 38.25 | 48 | 244.80 |
| 48 | 51.00 | 60 | 306.00 |
| 60 | 63.75 | 72 | 367.20 |
| 72 | 76.50 | 84 | 428.40 |
| 84 | 89.25 | $15 / 8 \times(66.3 \mathrm{Lb}$ per Sq Ft) |  |
| 96 | 102.00 | 72 | 397.80 |
| 120 | 127.50 | $13 / 4 \times(71.40 \mathrm{Lb}$ per Sq Ft) |  |
| $3 / 8 \times(15.30 \mathrm{Lb}$ per Sq Ft) |  | 48 | 285.60 |
| 30 | 38.25 | 60 | 357.00 |
| 36 | 45.90 | 72 | 428.40 |
| 42 | 53.55 | 96 | 571.20 |
| 48 | 61.20 | $2 \times$ (81.60 Lb per Sq Ft) |  |
| 60 | 76.50 | 48 | 326.40 |
| 72 | 91.80 | 60 | 408.00 |
| 84 | 107.10 | 72 | 489.60 |
| 96 | 122.40 | 84 | 571.20 |
| 120 | 153.00 | 96 | 652.80 |

Plate Steel-Cont'd

| Size in Inches | Wt. Per Ft, Lb | Size in Inches | Wt. per Ft, Lb |
| :---: | :---: | :---: | :---: |
| 7/16 $\times$ ( 17.85 Lb Per Sq Ft) |  | $21 / 4 \times(91.80 \mathrm{Lb}$ per Sq Ft $)$ |  |
| 30 | 44.63 | 72 | 550.80 |
| 36 | 53.55 | $21 / 2 \times$ (102.0 Lb per Sq Ft.) |  |
| 42 | 62.48 | 48 | 408.00 |
| 48 | 71.40 | 60 | 510.00 |
| 60 | 89.25 | 72 | 612.00 |
| 72 | 107.10 | $23 / 4 \times(112.20 \mathrm{Lb}$ per Sq Ft) |  |
| 84 | 124.95 | 48 | 448.80 |
| 96 | 142.80 | 60 | 561.00 |
| 120 | 178.50 | 72 | 673.20 |
| 1/2 $\times(20.40 \mathrm{Lb}$ per Sq Ft) |  | $3 \times(122.40 \mathrm{Lb}$ per Sq Ft) |  |
| 30 | 51.00 | 48 | 489.60 |
| 36 | 61.20 | 60 | 612.00 |
| 42 | 71.40 | 72 | 734.40 |
| 48 | 81.60 | $31 / 4 \times(132.60 \mathrm{Lb}$ per Sq Ft) |  |
| 60 | 102.00 | 48 | 530.40 |
| 72 | 122.40 | 60 | 663.00 |
| 84 | 142.80 | 72 | 795.60 |
| 96 | 163.20 | $31 / 2 \times(142.80$ Lb. per Sq Ft) |  |
| 120 | 204.00 | 48 | 571.00 |
| 9/16 $\times(22.95 \mathrm{Lb}$ per Sq Ft) |  | 60 | 714.00 |
| 72 | 137.70 | 72 | 856.80 |
| $5 / 8 \times(25.75 \mathrm{Lb}$ per Sq Ft) |  | $33 / 4 \times(153.00 \mathrm{Lb}$ per Sq Ft) |  |
| 30 | 64.38 | 48 | 612.00 |
| 36 | 77.25 | 60 | 765.00 |
| 42 | 90.13 | 72 | 918.00 |
| 48 | 103.00 | $4 \times(163.20 \mathrm{Lb}$ per Sq Ft) |  |
| 60 | 128.75 | 48 | 652.80 |
| 72 | 154.50 | 60 | 816.00 |
| 84 | 180.25 | 72 | 979.20 |
| 96 | 206.00 | $41 / 2 \times(183.60 \mathrm{Lb}$ per Sq Ft) |  |
| 120 | 257.50 | 48 | 734.40 |
| 11/16 $\times$ ( 28.05 Lb per Sq Ft) |  | 72 | 1101.60 |
| 72 | 168.30 | $5 \times(204.0$ Lb per Sq Ft) |  |
| 96 | 224.40 | 48 | 816.00 |
| $3 / 4 \times(30.60$ Lb per Sq Ft) |  | 60 | 1020.00 |
| 30 | 76.50 | 72 | 1224.00 |
| 36 | 91.80 | 84 | 1428.00 |
| 42 | 107.10 | $51 / 2 \times(224.40 \mathrm{Lb}$ per Sq Ft) |  |
| 48 | 122.40 | 48 | 897.00 |
| 60 | 153.00 | 60 | 1122.00 |
| 72 | 183.60 | 72 | 1346.40 |
| 84 | 214.20 | $6 \times(244.80 \mathrm{Lb}$ per Sq Ft $)$ |  |
| 96 | 244.80 | 48 | 979.20 |

### 6.14.0 Sheet and Coil Steel—Types and Uses

HOT COLD ROLLED STEEL SHEETS STANDARD SHEET GAUGE \& WEIGHTS

AISI THICKNESS TOLERANCE RANGE GALVANIZED


## Sheet and Coil Selection Guide

## Low carbon

A low-carbon sheet is a low-cost steel sheet, soft enough to bend flat on itself in any direction, without cracking, ductile enough for shallow drawing. Carbon is .10 max. (instead of .15 max.) for improved welding and forming. Surface has normal mill oxide. It conforms to ASTM A569 and is used for tanks, barrels, farm implements, and other applications where finish is secondary.

## Pickled and oiled

Acid pickling provides a smoother, more uniform surface. Paint and enamel adhere well. Properties and characteristics are the same as low-carbon sheets. It conforms to ASTM A569. Typical applications include auto parts, appliances, and toys.

## Abrasions resisting

Medium carbon content plus higher manganese greatly improve resistance to abrasion. 210 min . Brinell. For scrapers, liners, chutes, conveyors-outlasting low-carbon steel by two to ten times. Moderate formability.

## Cold rolled

## Low carbon

This continuous mill product is made with a high degree of gauge accuracy and uniformity of physical characteristics. The smooth deoxidized matte finish gives an excellent base for paint, lacquer, and enamel. Box annealing and absence of scale permit stamping and moderate drawing operations. Sheets bend flat on themselves without cracking. Oiling protects surface against rust. It conforms to ASTM A366-with carbon content held to .10 max . (instead of .15 max . as the spec permits) for improved welding and forming.

## Galvanized

The term galvanized has long been used to describe steel sheets coated with zinc. It is usually associated with the hot-dipped process, but the zinc coating can also be applied by electroplating. Our hot-dipped galvanized sheets are produced to conform with ASTM A525. Electrogalvanized conforms to ASTM A591.

In recent years, the producing mills have accomplished many technological advancements in the manufacture of both hot-dipped and electrogalvanized sheets. The end use should determine the type of coated product and surface
condition required regardless of the coating weight, spangle size, method of manufacture, or surface preparation required. Following are descriptions of the grades and conditions of galvanized sheets used most often.

| Coating |  |
| :--- | :--- |
| ASTM Designation | Minimum Check Limit by Triple <br> Spot Test (oz/sq ft) |
| G-60 Light Commercial | .060 |
| G-90 Commercial up to | 0.90 |
| G-210 | 2.10 |

### 6.14.1 Calculating the Weight of Various Types of Carbon, Stainless, and Galvanized Sheet Steel

| Standard Sheet Gauge and Weights <br> For Accuracy <br> Specify Thickness by Decimal Part of an Inch |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Carbon Sheets to U.S.S. or Mfrs. Gauge |  |  | Galvanized Sheets to Galvanized Sheet Gauge |  | Stainless Sheets to Stainless Sheet Gauge |  |  |
| Gauge <br> No. | Thickness in Inches | Pounds per$S q F t$ | Thickness in Inches | Pounds per Sq. Ft | Thickness in Inches | Pounds per Sq Ft |  |
|  |  |  |  |  |  | $\begin{aligned} & 200 \& 300 \\ & \text { Series } \end{aligned}$ | 400 <br> Series |
| 3 | . 2391 | 10.00 |  |  |  |  |  |
| 4 | . 2242 | 9.375 |  |  |  |  |  |
| 5 | . 2092 | 8.750 |  |  |  |  |  |
| 6 | . 1943 | 8.125 |  |  |  |  |  |
| 7 | . 1793 | 7.500 |  |  |  |  |  |
| 8 | . 1644 | 6.875 | . 1681 | 7.031 | . 171875 | 7.2187 | 7.0813 |
| 9 | . 1495 | 6.250 | . 1532 | 6.406 | . 156250 | 6.5625 | 6.4375 |
| 10 | . 1345 | 5.625 | . 1382 | 5.781 | . 140625 | 5.9062 | 5.7937 |
| 11 | . 1196 | 5.000 | . 1233 | 5.516 | . 125000 | 5.2500 | 5.1500 |
| 12 | . 1046 | 4.375 | . 1084 | 4.531 | . 109375 | 4.5937 | 4.5063 |
| 13 | . 0897 | 3.750 | . 0934 | 3.906 | . 09375 | 3.9375 | 3.8625 |
| 14 | . 0747 | 3.125 | . 0785 | 3.281 | . 078125 | 3.2812 | 3.2187 |
| 15 | . 0673 | 2.813 | . 0710 | 2.969 | . 070313 | 2.9531 | 2.8968 |
| 16 | . 0598 | 2.500 | . 0635 | 2.656 | . 062500 | 2.6250 | 2.5750 |
| 17 | . 0538 | 2.250 | . 0575 | 2.406 | . 056250 | 2.3625 | 2.3175 |
| 18 | . 0478 | 2.000 | . 0516 | 2.156 | . 050000 | 2.1000 | 2.0600 |
| 19 | . 0418 | 1.750 | . 0456 | 1.906 | . 043750 | 1.8375 | 1.8025 |


| Carbon Sheets to U.S.S. or Mfrs. Gauge |  |  | Galvanized Sheets to Galvanized Sheet Gauge |  | Stainless Sheets to Stainless Sheet Gauge |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  | Pounds | Sq Ft |
| Gauge <br> No. | Thickness in Inches | Pounds per Sq Ft | Thickness in Inches | Pounds per Sq. Ft | Thickness in Inches | $200 \& 300$ <br> Series | 400 <br> Series |
| 20 | . 0359 | 1.500 | . 0396 | 1.656 | . 037500 | 1.5750 | 1.5450 |
| 21 | . 0329 | 1.375 | . 0366 | 1.531 | . 034375 | 1.4437 | 1.4160 |
| 22 | . 0299 | 1.250 | . 0336 | 1.406 | . 031250 | 1.3125 | 1.2875 |
| 23 | . 0269 | 1.125 | . 0306 | 1.281 | . 028125 | 1.1813 | 1.1587 |
| 24 | . 0239 | 1.000 | . 0276 | 1.156 | . 025000 | 1.0500 | 1.0300 |
| 25 | . 0209 | . 8750 | . 0247 | 1.031 | . 021875 | . 9187 | . 9013 |
| 26 | . 0179 | . 7500 | . 0217 | . 9063 | . 018750 | . 7875 | . 7725 |
| 27 | . 0164 | . 6875 | . 0202 | . 8438 | . 017188 | . 7218 | . 7081 |
| 28 | . 0149 | . 6250 | . 0187 | . 7813 | . 015625 | . 6562 | . 6438 |
| 29 | . 0135 | . 5625 | . 0172 | . 7188 | . 014063 | . 5906 | . 5794 |
| 30 | . 0120 | . 5000 | . 0157 | . 6563 | . 012500 | . 5250 | . 5150 |

1/4" Thick \& Heavier classed as plates.

### 6.14.2 Calculating the Weight of Low-Carbon, Hot-Dipped, Galvanized Roof Deck




## GALVANIZED

CORRUGATED ROOFING


| Hot-Dipped G90 Coating | Wt. per Sheet | Sheets per Square | Wt per Square |  |  |
| :--- | :--- | ---: | :---: | :---: | :---: |
| $24 \mathrm{Ga} . \times 27$ | $1 / 2$ | $\times$ | 96 | 23.1 | 5.4 |
| $24 \mathrm{Ga} \times 27$ | $1 / 2$ | $\times$ | 120 | 28.9 | 4.3 |
| $24 \mathrm{Ga} \times 27$ | $\times 1 / 2$ | $\times$ | 144 | 34.7 | 3.6 |
| $26 \mathrm{Ga} . \times 27$ | $1 / 2$ | $\times$ | 96 | 22.6 | 4.3 |
| $26 \mathrm{Ga} . \times 27$ | $1 / 2$ | $\times$ | 120 | 28.2 | 3.4 |
| $26 \mathrm{Ga} . \times 27$ | $1 / 2$ | $\times$ | 144 | 33.8 | 2.8 |
| $28 \mathrm{Ga} \times 27$ | $1 / 2$ | $\times$ | 96 | 15.6 | 5.4 |
| $28 \mathrm{Ga} . \times 27$ | $1 / 2$ | $\times$ | 120 | 19.5 | 4.3 |
| $28 \mathrm{Ga} . \times 27$ | $1 / 2$ | $\times$ | 144 | 23.4 | 3.6 |

Electrogalvanized sheets are offered subject to inquiry

| ABRASION RESISTING <br> STEEL SHEETS |  |  |  |
| :--- | :--- | :--- | :--- |
| Thickness Inches | Size Inches |  | Est Weight per <br> Sheet Lbs |
| No. 14 ( $3.125 \mathrm{lbs} / \mathrm{sq} \mathrm{ft})$ |  | $\times$ |  |
| .0747 | 36 | $\times$ | 120 |
| No. $12(4.375 \mathrm{lbs} / \mathrm{sq} \mathrm{ft)}$. |  |  | 93.80 |
| .1046 | 36 |  | 120 |
| $1 / 8^{\prime \prime}(5.10 \mathrm{lbs} / \mathrm{sq} \mathrm{ft})$ |  |  | 163.20 |
| .125 | 48 |  |  |
| For heavier gauges see plate section page |  |  |  |

### 6.14.3 Converting Gauge Inches to Decimals for Sheet Steel, Aluminum, Stainless Steel

| Galvanized Steel |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Specified Width, Inches |  | Thickness tolerance, inch over only. No tolerance under specified minimum thickness |  |  |  |  |  |
|  | thru 32 | over . 101 <br> thru 177 <br> . 016 | over . 075 <br> thru . 101 <br> . 014 | over . 061 <br> thru . 075 <br> . 012 | $\begin{aligned} & \text { over } .043 \\ & \text { thru } .061 \\ & .010 \end{aligned}$ | $\begin{aligned} & \text { over } .023 \\ & \text { thru } .043 \\ & .008 \end{aligned}$ | .023 and thinner . 006 |
| over 32 | thru 40 | . 016 | . 016 | . 012 | . 010 | . 008 | . 006 |
| over 40 | thru 60 | . 018 | . 016 | . 012 | . 010 | . 008 | . 006 |
| over 60 | thru 72 | . 018 | . 018 | . 012 | . 010 | . 008 | - |


| Hot-Rolled Steel |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Specified Width, Inches |  | Thickness tolerance, inch over only. No tolerance under specified minimum thickness |  |  |  |
| - | thru 20 | over . 179 <br> thru . 230 <br> . 016 | $\begin{aligned} & \text { over } .097 \\ & \text { thru } .179 \\ & .016 \end{aligned}$ | $\begin{aligned} & \text { over } .082 \\ & \text { thru } .097 \\ & .014 \end{aligned}$ | $\begin{aligned} & \text { over } .071 \\ & \text { thru } .082 \\ & .014 \end{aligned}$ |
| over 20 | thru 32 | . 018 | . 016 | . 014 | . 014 |
| over 32 | thru 40 | . 018 | . 018 | . 016 | . 014 |
| over 40 | thru 48 | . 020 | . 020 | . 016 | . 014 |
| over 48 | thru 60 | - | . 020 | . 016 | . 014 |
| over 60 | thru 72 | - | . 022 | . 018 | . 016 |
| over 72 | - | - | . 024 | . 018 | . 016 |


| Specified Width, Inches | Thickness tolerance, inch over only. No tolerance under specified minimum thickness |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | over . 098 | over . 071 | over . 057 | over . 039 | over . 019 | .over . 014 |
|  | thru . 142 | thru . 098 | thru . 071 | thru . 057 | thru . 039 | thru . 019 |
| 24 thru 72 | . 012 | . 010 | . 010 | . 008 | . 006 | . 004 |
| over 72 | . 014 | . 012 | . 010 | . 008 | . 006 | - |

By permission: Corrugated Metals, Inc., Belvidere, ILL

### 6.15.0 Carbon Steel Expanded Metal Grating—ASTM A1011



Expanded Metal Grating - ASTM A1011
$\underline{\text { larger image }}$

Results 1-8 of 8

| Item \# | Item Name |  | Opening Width | - | Opening <br> Length | - | Open <br> Area | - | Overall <br> Thickness | - |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 2 lb - | Expanded Metal Grating-Carbon Steel |  | 1.000 in . | - | 3.600 in . | - | 77\% | - | 0.460 in . | - |
| 3 lb - | Expanded Metal Grating-Carbon Steel |  | 0.938 in. | - | 3.438 in . | - | 73\% | - | 0.500 in . | - |
| 3. 14 lb - | Expanded Metal Grating-Carbon Steel | - | 1.625 in . | - | 4.875 in . | - | 74\% | - | 0.562 in . | - |
| 4 lb - | Expanded Metal Grating-Carbon Steel | - | 0.938 in. | - | 3.438 in . | - | 65\% | - | 0.625 in. | - |
| 4.27 lb - | Expanded Metal Grating-Carbon Steel | - | 1.000 in . | - | 2.875 in . | - | 58\% | - | 0.625 in. | - |
| 5 lb - | Expanded Metal Grating-Carbon Steel | - | 0.813 in . | - | 3.375 in . | - | 52\% | - | 0.625 in. | - |
| 6.25 lb - | Expanded Metal Grating-Carbon Steel | - | 0.813 in . | - | 3.375 in . | - | 55\% | - | 0.750 in . | - |
| 7 lb - | Expanded Metal Grating-Carbon Steel | - | 0.813 in . | - | 3.375 in. | - | 60\% | - | 0.750 in . | - |

Source: Direct Metals, Inc., Kennesaw, GA

### 6.15.1 Carbon Steel Catwalk Expanded Metal Grating—ASTMA569/569M

Carbon Steel Catwalk Expanded Metal Grating
Check up to five results to perform an action.


Results 1-7 of 7

| Item \# | Item Name | Opening Width | - | Opening <br> Length | - | Open <br> Area | - | Overall <br> Thickness |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 2 lb - | Catwalk Expanded Metal Grating- | 1.000 in . | - | 3.600 in . | - | 77\% | - | 0.460 in . |  |
| 3 lb - | Catwalk Expanded Metal Grating- | 0.938 in. | - | 3.438 in. | - | 73\% | - | 0.500 in . | - |
| 3. 14 lb - | Catwalk Expanded Metal Grating- | 1.625 in . | - | 4.875 in . | - | 74\% | - | 0.562 in . | - |
| 4 lb - | Catwalk Expanded Metal Grating- | 0.938 in . | - | 3.438 in . | - | 65\% | - | 0.625 in . | - |
| 4.27 lb - | Catwalk Expanded Metal Grating- | 1.000 in | - | 2.875 in. | - | 58\% | - | 0.625 in. | - |
| 5 lb - | Catwalk Expanded Metal Grating- | 0.813 in . | - | 3.375 in. | - | 52\% | - | 0.625 in . | - |
| 6.25 lb - | Catwalk Expanded Metal Grating- | 0.813 in . | - | 3.375 in. | - | 55\% | - | 0.750 in . | - |

### 6.15.2 Aluminum Expanded Metal Grating

Item \# 2 lb , Aluminum Expanded Metal Grating

large image

## Aluminum Expanded Metal Grating

Aluminum Expanded Metal Grating-Alloy 5052 H32

|  |  |
| :--- | :--- |
| Specifications | 2 lb |
| Style | 200 lb |
| Weight Plain per CSF | 4 ft |
| Standard Width | 8 ft |
| Standard Length | 1.000 in. |
| Opening Width | 3.600 in. |
| Opening Length | 1.333 in. |
| Center to Center of Bond Width | 5.330 in. |
| Center to Center to Bond Length | 0.235 in. |
| Stands Width | $77 \%$ |
| Open Area | 9 |
| No. Diamonds per ft SWD | 0.460 in. |
| Overall Thickness |  |

Source: Direct Metals, Inc., Kennesaw, GA

| Bar Grating <br> Aluminum Rectangular Bar (SG Series) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | Clear Span |  |  |  |  |  |  |  |  |  |  |  |
| GRATING TYPES AVAILABLE IN THE RECTANGULAR BAR SG SERIES | Bar Size | Wt. *Lbs Sq Ft | Sect. Prop. Ft of Width |  | $2^{\prime}-0^{\prime \prime}$ | $2^{\prime}-6^{\prime \prime}$ | $3^{\prime}-0^{\prime \prime}$ | $3^{\prime}-6^{\prime}$ | $4^{\prime}-0^{\prime \prime}$ | $4^{\prime}-6^{\prime \prime}$ | $5^{\prime}-0^{\prime \prime}$ |  |  | $6^{\prime}-6^{\prime \prime}$ | $7{ }^{\prime}-0$ | $8^{\prime}-0^{\prime \prime}$ |
|  | $\times 1 / 8$ | 1.85 | $\begin{aligned} S x & =.228 \\ 1 x & =.114 \end{aligned}$ | $\begin{aligned} & \mathrm{U} \\ & \mathrm{D} \\ & \mathrm{C} \\ & \mathrm{D} \end{aligned}$ | $\begin{aligned} & 458 \\ & .144 \\ & 458 \\ & .115 \end{aligned}$ | $\begin{aligned} & 293 \\ & .225 \\ & 366 \\ & .180 \end{aligned}$ | $\begin{aligned} & 203 \\ & .324 \\ & 305 \\ & .259 \end{aligned}$ | $\begin{aligned} & 149 \\ & 441 \\ & 261 \\ & 352 \end{aligned}$ | $\begin{aligned} & 114 \\ & 576 \\ & 229 \\ & 461 \end{aligned}$ |  | U- Safe uniform load in pounds/sq ft C-Safe concentrated load in pounds/ft grating width D-Deflection in inches |  |  |  |  |  |
|  | $\times 1 / 4 \times 1 / 8$ | 2.25 | $\begin{aligned} & S x=.358 \\ & 1 x=.223 \end{aligned}$ | $\begin{aligned} & \mathrm{U} \\ & \mathrm{D} \\ & \mathrm{C} \\ & \mathrm{D} \end{aligned}$ | $\begin{aligned} & 686 \\ & .144 \\ & 686 \\ & .115 \end{aligned}$ | $\begin{aligned} & 439 \\ & .225 \\ & 549 \\ & .190 \end{aligned}$ | $\begin{aligned} & 305 \\ & .324 \\ & 458 \\ & .259 \end{aligned}$ | $\begin{aligned} & 224 \\ & .441 \\ & 392 \\ & .352 \end{aligned}$ | $\begin{aligned} & 172 \\ & 576 \\ & 343 \\ & .461 \end{aligned}$ | $\begin{aligned} & 136 \\ & 728 \\ & 305 \\ & 583 \end{aligned}$ |  | Loads and deflections given in this table are theoretical and are based on a unit stress of 12,000 psi. |  |  |  |  |
|  | $\times 1 / 4 \times 1 / 8$ | 2.25 | $\begin{array}{r}\text { Sx }\end{array}=.358$ | U D C D | $\begin{aligned} & 715 \\ & .115 \\ & 715 \\ & .092 \end{aligned}$ | $\begin{aligned} & 458 \\ & .180 \\ & 572 \\ & .144 \end{aligned}$ | $\begin{aligned} & 318 \\ & .259 \\ & 477 \\ & .207 \end{aligned}$ | $\begin{aligned} & 233 \\ & .351 \\ & 408 \\ & .282 \end{aligned}$ | $\begin{aligned} & 179 \\ & 480 \\ & 358 \\ & 369 \end{aligned}$ | $\begin{aligned} & 141 \\ & 581 \\ & 318 \\ & 466 \end{aligned}$ | $\begin{aligned} & 114 \\ & 717 \\ & 286 \\ & 575 \end{aligned}$ |  |  |  |  |  |
|  | $1 / 4 \times 3 / 16$ | 3.25 | $\begin{aligned} & S x=.536 \\ & 1 x=.335 \end{aligned}$ | U D C D | $\begin{aligned} & 1074 \\ & .115 \\ & 1074 \\ & .092 \end{aligned}$ | $\begin{aligned} & 687 \\ & .180 \\ & 859 \\ & .144 \end{aligned}$ | $\begin{aligned} & 477 \\ & .259 \\ & 716 \\ & .207 \end{aligned}$ | $\begin{aligned} & 350 \\ & .351 \\ & 614 \\ & .282 \end{aligned}$ | $\begin{aligned} & 268 \\ & .460 \\ & 537 \\ & 369 \end{aligned}$ | $\begin{aligned} & 212 \\ & 581 \\ & 477 \\ & 486 \end{aligned}$ | $\begin{aligned} & 172 \\ & 717 \\ & 429 \\ & 575 \end{aligned}$ |  |  |  |  |  |
| $\text { 1-3/16" } \mathrm{I} \underset{ }{\mathrm{r}} \mathrm{P}$ | $1 / 2 \times 1 / 8$ | 2.65 | $S x=.515$ $1 x=.387$ | U D C D | $\begin{aligned} & 1030 \\ & .096 \\ & 1030 \\ & .077 \end{aligned}$ | $\begin{aligned} & 659 \\ & .150 \\ & 824 \\ & .120 \end{aligned}$ | $\begin{aligned} & 458 \\ & .216 \\ & 686 \\ & .173 \end{aligned}$ | $\begin{aligned} & 336 \\ & .294 \\ & 588 \\ & .235 \end{aligned}$ | $\begin{aligned} & 257 \\ & .383 \\ & 515 \\ & .307 \end{aligned}$ | $\begin{aligned} & 203 \\ & .485 \\ & 458 \\ & .389 \end{aligned}$ | $\begin{aligned} & 165 \\ & 599 \\ & 412 \\ & 480 \end{aligned}$ | $\begin{aligned} & 136 \\ & 724 \\ & 374 \\ & 579 \end{aligned}$ |  |  |  |  |
|  | $1 / 2 \times 3 / 16$ | 3.86 | $\begin{aligned} & S x=773 \\ & 1 x=.579 \end{aligned}$ | $\begin{aligned} & \mathrm{U} \\ & \mathrm{D} \\ & \mathrm{C} \\ & \mathrm{D} \end{aligned}$ | $\begin{aligned} & 1547 \\ & .096 \\ & 1547 \\ & .077 \end{aligned}$ | $\begin{aligned} & 990 \\ & .150 \\ & 1237 \\ & .120 \end{aligned}$ | $\begin{aligned} & 687 \\ & .216 \\ & 1031 \\ & .173 \end{aligned}$ | $\begin{aligned} & 505 \\ & .294 \\ & 884 \\ & .235 \end{aligned}$ | $\begin{aligned} & 387 \\ & .383 \\ & 773 \\ & .307 \end{aligned}$ | $\begin{aligned} & 306 \\ & .485 \\ & 687 \\ & 389 \end{aligned}$ | $\begin{aligned} & 247 \\ & 599 \\ & 619 \\ & 480 \end{aligned}$ | $\begin{aligned} & 204 \\ & 724 \\ & 562 \\ & 579 \end{aligned}$ | $\begin{aligned} & 172 \\ & 861 \\ & 516 \\ & 690 \end{aligned}$ |  |  |  |
|  | $3 / 4 \times 3 / 16$ | 4.48 | $\begin{aligned} & S x=1.052 \\ & 1 x=.902 \end{aligned}$ | U D C D | $\begin{aligned} & 2105 \\ & .082 \\ & 2105 \\ & .066 \end{aligned}$ | $\begin{aligned} & 1347 \\ & .128 \\ & 1684 \\ & .103 \end{aligned}$ | $\begin{aligned} & 936 \\ & .185 \\ & 1404 \\ & .148 \end{aligned}$ | $\begin{aligned} & 687 \\ & .252 \\ & 1203 \\ & .202 \end{aligned}$ | $\begin{aligned} & 526 \\ & 629 \\ & 1053 \\ & 264 \end{aligned}$ | $\begin{aligned} & 416 \\ & 417 \\ & 936 \\ & 334 \end{aligned}$ | $\begin{aligned} & 337 \\ & 515 \\ & 842 \\ & 412 \end{aligned}$ | $\begin{aligned} & 278 \\ & 622 \\ & 766 \\ & 498 \end{aligned}$ | $\begin{aligned} & 234 \\ & 741 \\ & 702 \\ & 593 \end{aligned}$ | $\begin{aligned} & 199 \\ & 868 \\ & 648 \\ & 696 \end{aligned}$ |  |  |
| SG 2 | $\times 3 / 16$ | 5.08 | $\begin{aligned} S x & =1.375 \\ 1 x & =1.375 \end{aligned}$ | U D C D | $\begin{aligned} & 2750 \\ & .072 \\ & 2750 \\ & .058 \end{aligned}$ | $\begin{aligned} & 1760 \\ & .112 \\ & 2200 \\ & .090 \end{aligned}$ | $\begin{aligned} & 1222 \\ & .162 \\ & 1833 \\ & .130 \end{aligned}$ | $\begin{aligned} & 898 \\ & 220 \\ & 1571 \\ & .176 \end{aligned}$ | $\begin{aligned} & 688 \\ & .288 \\ & 1375 \\ & .230 \end{aligned}$ | $\begin{aligned} & 543 \\ & .364 \\ & 1222 \\ & .292 \end{aligned}$ | $\begin{aligned} & 440 \\ & .450 \\ & 1100 \\ & .360 \end{aligned}$ | $\begin{aligned} & 364 \\ & .545 \\ & 1000 \\ & .436 \end{aligned}$ | $\begin{aligned} & 306 \\ & .649 \\ & 917 \\ & .518 \end{aligned}$ | $\begin{aligned} & 260 \\ & 759 \\ & 846 \\ & 608 \end{aligned}$ | $\begin{aligned} & 224 \\ & 880 \\ & 786 \\ & 706 \end{aligned}$ |  |

## Bar Grating-Cont'd

|  |  |  |  | Clear Span |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Bar Size | Wt. Lbs Sq Ft | Sect. Prop. <br> Ft of Width |  | $2^{\prime}-0^{\prime \prime}$ | $2^{\prime}-6^{\prime \prime}$ | $3^{\prime}-0^{\prime \prime}$ | $3^{\prime}-6^{\prime}$ | $4^{\prime}-0^{\prime \prime}$ | $4^{\prime}-6^{\prime \prime}$ | $5^{\prime}-0^{\prime \prime}$ | $5^{\prime}-6^{\prime \prime}$ | $6^{\prime}-0^{\prime \prime}$ | $6^{\prime}-6^{\prime \prime}$ | $7^{\prime}-0^{\prime \prime}$ | $8^{\prime}-0^{\prime \prime}$ |
| $2-1 / 4 \times 3 / 16$ | 5.69 | Sx $=1.740$ | U | 3480 | 2227 | 1547 | 1136 | 870 | 687 | 557 | 460 | 387 | 330 | 284 | 218 |
|  |  |  | D | . 064 | . 100 | . 144 | . 196 | . 256 | . 324 | 400 | . 484 | . 577 | . 677 | 784 | 1.027 |
|  |  | $1 \mathrm{x}=1.958$ | C | 3480 | 2784 | 2320 | 1989 | 1740 | 1547 | 1392 | 1266 | 1160 | 1071 | 994 | 870 |
|  |  |  | D | . 051 | . 080 | . 115 | . 157 | . 205 | . 259 | . 320 | . 387 | . 461 | . 541 | 627 | . 819 |
| $2-1 / 2 \times 3 / 16$ | 6.29 | $S x=1.148$ | U | 4297 | 2750 | 1910 | 1403 | 1074 | 849 | 687 | 568 | 477 | 407 | 351 | 268 |
|  |  |  | D | . 058 | . 090 | . 130 | . 176 | . 230 | . 292 | . 360 | . 436 | . 518 | . 609 | . 706 | . 920 |
|  |  | $1 \mathrm{x}=2.685$ | C | 4297 | 3437 | 2864 | 2455 | 2148 | 1910 | 1719 | 1562 | 1432 | 1322 | 1228 | 1074 |
|  |  |  | D | . 046 | . 072 | . 104 | . 141 | . 184 | . 233 | . 288 | . 348 | . 415 | . 487 | . 565 | . 737 |

*Based on 11 bars/ft. of grating width. Bearing bars 1-3/16* c.c. Add. $4 \mathrm{lbs} / \mathrm{sq}$ ft for 19.SG.2.
NOTE: Grating for spans to the left of the heavy line have a deflection less than $1 / 4^{\prime \prime}$ for uniform loads of $100 \mathrm{lbs} / \mathrm{sq} / \mathrm{ft}$. This is the maximum deflection to afford pedestrian comfort and can be exceeded for other types of load at the discretion of the engineer. When serrated grating is specified, the depth of grating required for a specified load will be $1 / 4^{\prime \prime}$ greater than that shown in these tables. $1^{\prime \prime}$ serrated grating not available.

## 19-SG-4/19-SG-2 Panel Width Chart (in.)

## Dimensions Are Out-to-Out of Bearing Bars**

| No. of Bars | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | 15 | 16 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & 3 / 16^{\prime \prime} \\ & \text { Bar } \end{aligned}$ | 1-3/8 | 2-9/16 | 3-3/4 | 4-15/16 | 6-1/8 | 7-5/16 | 8-1/2 | 9-11/16 | 10-7/8 | 12-1/16 | 13-1/4 | 14-7/16 | 15-5/8 | 16-13/16 | 18 |
| No. of Bars | 17 | 18 | 19 | 20 | 21 | 22 | 23 | 24 | 25 | 26 | 27 | 28 | 29 | 30 | 31 |
| $\begin{aligned} & 3 / 16^{\prime \prime} \\ & \text { Bars } \end{aligned}$ | 19-3/16 | 20-3/8 | 21-9/16 | 22-3/4 | 23-15/16 | 25-1/8 | 26-5/16 | 27-1/2 | 28-11/16 | 29-7/8 | 31-1/16 | 32-1/4 | 33-7/16 | 34-5/8 | 35-15/16 |

[^14]
### 6.16.1 Aluminium I Bar and Rec Bar Grating

|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | ClearSpan |  |  |  |  |  |  |  |  |  |  |  |
| Bar <br> Size, In. | Ped <br> Span, In. | Wt. *Lbs SqFt | Sec. <br> Prop Sx* in $^{3}$ |  | $2^{\prime}-0^{\prime \prime}$ | $2^{\prime}-6^{\prime \prime}$ | $3^{\prime}-0^{\prime \prime}$ | $3^{\prime}-6^{\prime \prime}$ | $4^{\prime}-0^{\prime \prime}$ | $4^{\prime}-6^{\prime \prime}$ | $5^{\prime}-0^{\prime \prime}$ | $5^{\prime}-6{ }^{\prime \prime}$ | $6^{\prime}-0^{\prime \prime}$ | $6^{\prime}-6^{\prime \prime}$ | $7^{\prime}-0^{\prime \prime}$ | $8^{\prime}-0^{\prime \prime}$ |
| $\begin{aligned} & 1 \times 3 / 16 \\ & 1^{\prime \prime} 1 \text {-Bar } \end{aligned}$ | 56 | 6.30 4.79 | 0.857 0.429 | $\begin{aligned} & \mathrm{U} \\ & \mathrm{D} \\ & \mathrm{C} \\ & \mathrm{D} \end{aligned}$ | $\begin{aligned} & 1714 \\ & 0.144 \\ & 1714 \\ & 0.115 \end{aligned}$ | $\begin{aligned} & 1097 \\ & 0.225 \\ & 1371 \\ & 0.180 \end{aligned}$ | $\begin{aligned} & 762 \\ & 0.324 \\ & 1143 \\ & 0.259 \end{aligned}$ | $\begin{aligned} & 560 \\ & 0.441 \\ & 980 \\ & 0.353 \end{aligned}$ | $\begin{aligned} & 429 \\ & 0.577 \\ & 857 \\ & 0.461 \end{aligned}$ | $\begin{aligned} & 339 \\ & 0.730 \\ & 762 \\ & 0.583 \end{aligned}$ | $\begin{aligned} & 274 \\ & 0.899 \\ & 686 \\ & 0.720 \end{aligned}$ | U-Safe uniform load in pounds/sq ft C-Safe concentrated load in pounds/ft fataing width D- Deflection in inches |  |  |  |  |
| $1-1 / 4 \times 3 / 16$ $1-1 / 4^{\prime \prime} 1$-Bar | 66 | 7.78 5.75 | 1.339 0.837 | $\begin{aligned} & \text { U } \\ & \text { D } \\ & \text { C } \\ & \text { D } \end{aligned}$ | $\begin{aligned} & 2679 \\ & 0.115 \\ & 2679 \\ & 0.092 \end{aligned}$ | $\begin{aligned} & 1714 \\ & 0.180 \\ & 2143 \\ & 0.144 \end{aligned}$ | $\begin{aligned} & 1190 \\ & 0.259 \\ & 1786 \\ & 0.207 \end{aligned}$ | $\begin{aligned} & 875 \\ & 0.353 \\ & 1531 \\ & 0.282 \end{aligned}$ | $\begin{aligned} & 670 \\ & 0.461 \\ & 1339 \\ & 0.369 \end{aligned}$ | $\begin{aligned} & 529 \\ & 0.583 \\ & 1190 \\ & 0.466 \end{aligned}$ | $\begin{aligned} & 429 \\ & 0.721 \\ & 1071 \\ & 0.576 \end{aligned}$ | $\begin{aligned} & 354 \\ & 0.871 \\ & 974 \\ & 0.697 \end{aligned}$ | $\begin{aligned} & 298 \\ & 1.038 \\ & 893 \\ & 0.830 \end{aligned}$ | Loads and deflections given in this table are theoretical and are based on a unit stress of $12,000 \mathrm{psi}$. |  |  |
| $1-1 / 2 \times 3 / 16$ $1-1 / 2^{\prime \prime} 1$-Bar | 76 | 9.28 6.74 | 1.929 1.446 | $\begin{aligned} & \hline \mathrm{U} \\ & \mathrm{D} \\ & \mathrm{C} \\ & \mathrm{D} \end{aligned}$ | $\begin{aligned} & 3857 \\ & 0.096 \\ & 3857 \\ & 0.077 \end{aligned}$ | $\begin{aligned} & 2469 \\ & 0.150 \\ & 3086 \\ & 0.120 \end{aligned}$ | $\begin{aligned} & 1714 \\ & 0.216 \\ & 2571 \\ & 0.173 \end{aligned}$ | $\begin{aligned} & 1259 \\ & 0.294 \\ & 2204 \\ & 0.235 \end{aligned}$ | $\begin{aligned} & 964 \\ & 0.384 \\ & 1929 \\ & 0.307 \end{aligned}$ | $\begin{aligned} & 762 \\ & 0.486 \\ & 1714 \\ & 0.389 \end{aligned}$ | $\begin{aligned} & 617 \\ & 0.600 \\ & 1543 \\ & 0.480 \end{aligned}$ | $\begin{aligned} & 510 \\ & 0.726 \\ & 1403 \\ & 0.581 \end{aligned}$ | $\begin{aligned} & 429 \\ & 0.865 \\ & 1286 \\ & 0.691 \end{aligned}$ | $\begin{aligned} & 365 \\ & 1.014 \\ & 1187 \\ & 0.811 \end{aligned}$ |  |  |
| $1-3 / 4 \times 3 / 16$ $1-3 / 4^{\prime \prime} 1$-Bar | 85 | 10.80 7.70 | 2.625 2.297 | U D C D | $\begin{aligned} & 5250 \\ & 0.082 \\ & 5250 \\ & 0.066 \end{aligned}$ | $\begin{aligned} & 3360 \\ & 0.129 \\ & 4200 \\ & 0.103 \end{aligned}$ | $\begin{aligned} & 2333 \\ & 0.185 \\ & 3500 \\ & 0.148 \end{aligned}$ | $\begin{aligned} & 1714 \\ & 0.252 \\ & 3000 \\ & 0.202 \end{aligned}$ | $\begin{aligned} & 1313 \\ & 0.329 \\ & 2625 \\ & 0.263 \end{aligned}$ | $\begin{aligned} & 1037 \\ & 0.417 \\ & 2333 \\ & 0.333 \end{aligned}$ | $\begin{aligned} & 840 \\ & 0.514 \\ & 2100 \\ & 0.411 \end{aligned}$ | $\begin{aligned} & 694 \\ & 0.622 \\ & 1909 \\ & 0.498 \end{aligned}$ | $\begin{aligned} & 583 \\ & 0.740 \\ & 1750 \\ & 0.592 \end{aligned}$ | $\begin{aligned} & 497 \\ & 0.869 \\ & 1615 \\ & 0.695 \end{aligned}$ | $\begin{aligned} & 429 \\ & 1.009 \\ & 1500 \\ & 0.806 \end{aligned}$ | $\begin{aligned} & 328 \\ & 1.316 \\ & 1313 \\ & 1.054 \end{aligned}$ |
| $\begin{aligned} & 2 \times 3 / 16 \\ & 2^{\prime \prime} 1 \text {-Bar } \end{aligned}$ | 94 | 12.32 8.71 | 3.429 3.429 | $\begin{aligned} & \mathrm{U} \\ & \mathrm{D} \\ & \mathrm{C} \\ & \mathrm{D} \end{aligned}$ | $\begin{aligned} & 6857 \\ & 0.072 \\ & 6857 \\ & 0.058 \end{aligned}$ | $\begin{aligned} & 4389 \\ & 0.113 \\ & 5486 \\ & 0.090 \end{aligned}$ | $\begin{aligned} & 3048 \\ & 0.162 \\ & 4572 \\ & 0.130 \end{aligned}$ | $\begin{aligned} & 2239 \\ & 0.220 \\ & 3918 \\ & 0.176 \end{aligned}$ | $\begin{aligned} & 1714 \\ & 0.288 \\ & 3429 \\ & 0.230 \end{aligned}$ | $\begin{aligned} & 1355 \\ & 0.365 \\ & 3048 \\ & 0.292 \end{aligned}$ | $\begin{aligned} & 1097 \\ & 0.450 \\ & 2743 \\ & 0.360 \end{aligned}$ | $\begin{aligned} & 907 \\ & 0.545 \\ & 2494 \\ & 0.436 \end{aligned}$ | $\begin{aligned} & 762 \\ & 0.648 \\ & 2286 \\ & 0.518 \end{aligned}$ | $\begin{aligned} & 649 \\ & 0.760 \\ & 2110 \\ & 0.608 \end{aligned}$ | $\begin{aligned} & 560 \\ & 0.882 \\ & 1959 \\ & 0.706 \end{aligned}$ | $\begin{aligned} & 429 \\ & 1.153 \\ & 1714 \\ & 0.921 \end{aligned}$ |
| $2-1 / 4 \times 3 / 16$ $2-1 / 4^{\prime \prime} 1$-Bar | 103 | 13.83 9.59 | 4.339 4.882 | U | $\begin{aligned} & 8679 \\ & 0.064 \\ & 8679 \\ & 0.051 \end{aligned}$ | $\begin{aligned} & 5554 \\ & 0.100 \\ & 6943 \\ & 0.080 \end{aligned}$ | $\begin{aligned} & 3857 \\ & 0.144 \\ & 5786 \\ & 0.115 \end{aligned}$ | $\begin{aligned} & 2834 \\ & 0.196 \\ & 4959 \\ & 0.157 \end{aligned}$ | $\begin{aligned} & 2170 \\ & 0.256 \\ & 4339 \\ & 0.205 \end{aligned}$ | $\begin{aligned} & 1714 \\ & 0.324 \\ & 3857 \\ & 0.259 \end{aligned}$ | $\begin{aligned} & 1389 \\ & 0.400 \\ & 3471 \\ & 0.320 \end{aligned}$ | $\begin{aligned} & 1148 \\ & 0.484 \\ & 3156 \\ & 0.387 \end{aligned}$ | $\begin{aligned} & 864 \\ & 0.576 \\ & 2893 \\ & 0.461 \end{aligned}$ | $\begin{aligned} & 822 \\ & 0.676 \\ & 2670 \\ & 0.541 \end{aligned}$ | $\begin{aligned} & 708 \\ & 0.783 \\ & 2480 \\ & 0.627 \end{aligned}$ | $\begin{aligned} & 542 \\ & 1.023 \\ & 2170 \\ & 0.819 \end{aligned}$ |
| $2-1 / 2 \times 3 / 16$ $2-1 / 2^{\prime \prime} 11-B a r$ | 111 | 15.33 10.66 | 5.357 6.697 | U D C D | $\begin{aligned} & 10714 \\ & 0.058 \\ & 10714 \\ & 0.046 \end{aligned}$ | $\begin{aligned} & 6857 \\ & 0.090 \\ & 8572 \\ & 0.072 \end{aligned}$ | $\begin{aligned} & 4762 \\ & 0.130 \\ & 7143 \\ & 0.104 \end{aligned}$ | $\begin{aligned} & 3499 \\ & 0.176 \\ & 6123 \\ & 0.141 \end{aligned}$ | $\begin{aligned} & 2679 \\ & 0.230 \\ & 5357 \\ & 0.184 \end{aligned}$ | $\begin{aligned} & 2116 \\ & 0.292 \\ & 4762 \\ & 0.233 \end{aligned}$ | $\begin{aligned} & 1714 \\ & 0.360 \\ & 4286 \\ & 0.288 \end{aligned}$ | $\begin{aligned} & 1417 \\ & 0.436 \\ & 3896 \\ & 0.348 \end{aligned}$ | $\begin{aligned} & 1190 \\ & 0.518 \\ & 3571 \\ & 0.415 \end{aligned}$ | $\begin{aligned} & 1014 \\ & 0.608 \\ & 3297 \\ & 0.487 \end{aligned}$ | $\begin{aligned} & 875 \\ & 0.706 \\ & 3061 \\ & 0.564 \end{aligned}$ | $\begin{aligned} & 670 \\ & 0.922 \\ & 2679 \\ & 0.737 \end{aligned}$ |

[^15] deflection less than $1 / 4^{\prime \prime}$ for uniform loads of $100 \mathrm{lbs} / \mathrm{sq} \mathrm{ft}$. This is the maximum deflection to afford pedestrian comfort and can be exceeded for other types of load at the discretion of the engineer.

## 7-SGI-4 7-SGI-2 Panel Width Chart (in.) Dimensions Are Out-to-Out of Bearing Bars**

| No. of Bars | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | 15 | 16 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1/4 ${ }^{\prime \prime}$ Flange | 11/16 | 1-1/8 | 1-9/16 | 2 | 2-7/16 | 2-7/8 | 3-5/16 | 3-3/4 | 4-3/16 | 4-5/8 | 5-1/16 | 5-15/2 | 6-3/16 | 6-3/8 | 6-13/16 |
| No. of Bars | 17 | 18 | 19 | 20 | 21 | 22 | 23 | 24 | 25 | 26 | 27 | 28 | 29 | 30 | 31 |
| 1/4' Flange | 7-1/4 | 7-11/16 | 8-1/8 | 8-9/16 | 9 | 9-7/16 | 9-7/8 | 10-5/16 | 10-3/4 | 11-3/16 | 11-5/8 | 12-1/16 | 12-1/2 | 12-15/16 | 13-3/8 |
| No. of Bars | 32 | 33 | 34 | 35 | 36 | 37 | 38 | 39 | 40 | 41 | 42 | 43 | 44 | 45 | 46 |
| 1/4" Bars | 13-13/16 | 14-1/4 | 14-11/16 | 15-1/8 | 15-9/16 | 16 | 16-7/16 | 16-7/8 | 17-5/16 | 17-3/4 | 18-3/16 | 18-8/8 | 19-1/16 | 19-1/2 | 19-15/16 |
| No. of Bars | 47 | 48 | 49 | 50 | 51 | 52 | 53 | 54 | 55 | 56 | 57 | 58 | 59 | 60 | 61 |
| 1/4" Bars | 20-3/8 | 20-13/16 | 21-1/4 | 21-11/16 | 22-1/8 | 22-9/16 | 23 | 23-7/16 | 23-7/8 | 24-5/16 | 24-3/4 | 25-3/16 | 25-5/8 | 26-1/16 | 26-1/2 |
| No. of Bars | 62 | 63 | 64 | 65 | 66 | 67 | 68 | 69 | 70 | 71 | 72 | 73 | 74 | 75 | 76 |
| 1/4" Bars <br> No. of Bars | $\begin{aligned} & 26-15 / 16 \\ & 77 \end{aligned}$ | $\begin{aligned} & 27-3 / 8 \\ & 78 \end{aligned}$ | $\begin{aligned} & 27-13 / 16 \\ & 79 \end{aligned}$ | $\begin{aligned} & 28-1 / 4 \\ & 80 \end{aligned}$ | $\begin{aligned} & 28-11 / 16 \\ & 81 \end{aligned}$ | $\begin{aligned} & 29-1 / 8 \\ & 82 \end{aligned}$ | $\begin{aligned} & 29-9 / 16 \\ & 83 \end{aligned}$ | 30 | 30-7/16 | 30-7/8 | 31-5/16 | 31-3/4 | 32-3/16 | 32-5/8 | 33-1/16 |
| 1/4" Flange | $33-1 / 2$ | $33-15 / 16$ | 34-3/8 | 34-13/16 | 35-1/4 | 35-11/16 | 36-1/8 |  |  |  |  |  |  |  |  |

${ }^{* *}$ Bars thickness is $1 / 4^{\prime \prime}$ at top and bottom. Add $1 / 4^{\prime \prime}$ for extended cross bars.

### 6.16.2 Aluminum Plank Sections and Pattern Availability

## Bar grating

## Aluminum Plank Section Availability

Aluminum plank is structurally sound and cosmetically attractive. Plank grating is nonsparking, nonmagnetic, nonskid, and relatively maintenance free. It is durable and corrosion resistant and possesses a high strength-to-weight ratio. The surface can be provided unpunched or with a variety of punch/patterns for the passage of air, light, heat, or moisture. The interconnecting webs offer a flush top walking surface. Aluminum plank grating has found application in sewage and wastewater treatment plants, as well as in the marine refrigerator (reefer), freezer, and cargo-hold flooring market. Aluminum plank grating is available in five cross-sectional designs: Heavy Duty (plain sides), Heavy Duty (interlocking sides), Light Series (plain sides), Reefer (plain sides), and Reefer (interlocking sides).

The Heavy Duty sections are used primarily in water and waste treatment and the marine markets, while the Light Series and Reefer sections are exclusively in the marine refrigerated stores application. Interlocking Heavy Duty, Reefer sections, and edge sections are available in $1^{\prime \prime}$ deep grating only.


## Plank Punch/Pattern Availability

Aluminum plank grating is available unpunched or with a variety of punch/patterns as shown below. Rectangular or square punched holes are most commonly used for water and waste treatment plants and in marine applications, while the round holes find application primarily in the marine market. The surface of plank grating can be specified as plain or with one of two styles of upsets (OGI or WACO) designed to promote a slip-resistant walkway, especially in the presence of moisture, oil, or other spilled substances.


# Lumber-Calculations to Select Framing and Trim Materials 

| 7.0.1 | How Lumber Is Cut from a Log Affects |
| :--- | :--- |
| Its Grain, Drying Process, and Waste |  |
|  | Factor |

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### 7.0.1 How Lumber Is Cut from a Log Affects Its Grain, Drying Process, and Waste Factor

## Quarter Sawn



Rift sawing at a 30-degree or greater angle to the growth rings produces narrow boards with accentuated vertical grain pattern. Rift-sawn boards are often favored for fine furniture and other applications where matching grain is important. This type of lumber is available in limited quantities and species and, as can be seen, produces more wasted material per log.

## Rift Sawn



To summarize: if the growth rings are at an angle of $0^{\circ}$ to $45^{\circ}$ relative to the face of the board, the board is flat sawn, and if the growth rings are $45^{\circ}$ to $90^{\circ}$ the board is quarter sawn. Quarter-sawn boards are less likely to distort or crack during the drying process and are more stable in service.

Wood shrinks and expands when it rains a lot with high moisture content. Some wooden doors may stick a bit because the wood has expanded. This can become an issue when using wood.

## Plain Sawn



Quarter sawing means cutting a log radially (at a 90 -degree angle) to the growth rings to produce a vertical grain pattern. This pattern produces fewer and narrower boards per log than plain sawing. However, the quality of each board is much better. Quarter-sawn boards will expand and contract less than boards sawn by other methods. Quarter-sawn logs require a lot of processing and produce lots of quarter-sawn boards of various grades. There is also little waste in the process.

### 7.0.2 The Physical Properties of Wood-Illustrated

## Physical Properties Illustrated

## Extreme Fiber Stress in Bending - $\mathrm{F}_{\mathrm{b}}$ (Fig. 1)

When loads are applied, structural members bend, producing tension in the fibers along the faces farthest from the applied load and compression in the fibers along the face nearest to the applied load. These induced stresses in the fibers are designated as "extreme fiber stress in bending" $\left(F_{b}\right)$.


FIGURE 1

## Single Member $F_{b}$

Design values are used in design where the strength of an individual piece, such as a beam, may be solely responsible for carrying a specific design load.

## Repetitive Member $F_{b}$

Design values are used in design when three or more load-sharing members, such as joists, rafters, or studs, are spaced no more than 24 " apart and are joined by flooring, sheathing, or other load-distributing elements. Repetitive members are also used where pieces are adjacent, such as decking.

Fiber Stress in Tension - F (Fig. 2)
Tensile stresses are similar to compression parallel to grain in that they act across the full cross section and tend to stretch the piece. Length does not affect tensile stresses.


FIGURE 2
Horizontal Shear - F (Fig. 3)
Horizontal shear stresses tend to slide fibers over each other horizontally. Most predominate in short, heavily loaded deep beams. Increasing beam cross section decreases shear stresses.


FIGURE 3

## Compression Perpendicular to Grain - $\mathrm{F}_{\mathrm{c} \perp}$ (Fig. 4)

Where a joist, beam, or similar piece of lumber bears on supports, the load tends to compress the fibers. It is necessary that the bearing area be sufficient to prevent side-grain crushing.


FIGURE 4

## Compression Parallel to Grain - $\mathrm{F}_{\mathrm{c}}$ (Fig. 5)

In many parts of a structure, stress grades are used where the loads are supported on the ends of the pieces. Such uses are as studs, posts, columns and struts. The internal stress induced by this kind of loading is the same across the whole cross section, and the fibers are uniformly stressed parallel to and along the full length of the piece.


FIGURE 5
Modulus of Elasticity - E (Fig. 6)
The modulus of elasticity is a ratio of the amount a material will deflect in proportion to an applied load.


FIGURE 6

## COURTEST: WESTERN WOOD PRODUCTS ASSOCIATION

### 7.0.3 Lumber Industry Abbreviations

The following abbreviations are commonly used for softwood lumber, although not all of them are necessarily applicable to all species. Additional abbreviations that are applicable to a particular region or species shall not be used unless included in certified grading rules.

Abbreviations are commonly used in the forms indicated, but variations such as the use of upper- and lowercase type, and the use or omission of periods and other forms of punctuation, are not required.

| AD | Air-dried |
| :---: | :---: |
| ADF | After deducting freight sides |
| ALS | American Softwood Lumber Standard |
| AV or AVG | Average |
| Bd | Board |
| Bd ft | Board foot or feet |
| BdI | Bundle |
| Bev | Beveled |
| B/L | Bill of lading |
| BM | Board Measure |
| Btr | Better |
| B\&B or B\&Btr | $B$ and better |
| $B \& S$ | Beams and stringers |
| CB1S | Center bead one side |
| CB2S | Center bead two sides |
| CF | Cost and freight |
| CG2E | Center groove two edges |
| CIF | Cost, insurance, and freight |
| CIFE | Cost, insurance, freight, and exchange |
| Clg | Ceiling |
| Cl | Clear |
| CM | Center matched |
| Com | Common |
| CS | Caulking seam |
| Csg | Casing |
| CuFt | Cubic foot or feet |
| CV1S | Center Vee one side |
| CV2S | Center Vee two sides |
| D\&H | Dressed and headed |
| D\&M | Dressed and matched |
| DB Clg | Double-beaded ceiling (E\&CB1S) |
| DB Part | Double-beaded partition (E\&CB2S) |
| DET | Double end trimmed |
| Dim | Dimension |
| Dkg | Decking |
| D/S or D/Sdg | Drop siding |
| EB1S | Edge bead one side |
| EB2S | Edge bead two sides |
| E\&CB1S | Edge and center bead one side |
| E\&CB2S | Edge and center bead two sides |
| E\&CV1S | Edge and center Vee one side |
| E\&CV2S | Edge and center Vee two sides |
| EE | Eased edges |
| EG | Edge (vertical) grain |
| EM | End matched |
| EV1S | Edge Vee one side |
| EV2S | Edge Vee two sides |
| Fac | Factory |
| FAS | Free alongside (named vessel) |


| FBM | Foot or board measure |
| :---: | :---: |
| FG | Flat (slash) grain |
| Fig | Flooring |
| FOB | Free on board (named point) |
| FOHC | Free of heart center or centers |
| FOK | Free of knots |
| Frt | Freight |
| Ft | Foot or feet |
| GM | Grade marked |
| $\mathrm{G} / \mathrm{R}$ or $\mathrm{G} / \mathrm{Rfg}$ | Grooved roofing |
| HB | Hollow back |
| H\&M | Hit-and-miss |
| H or M | Hit-or-miss |
| Hrt | Heart |
| Hrt CC | Heart cubical content |
| Hrt FA | Heart facial area |
| Hrt G | Heart girth |
| IN | Inch or inches |
| J\&P | Joists and planks |
| KD | Kiln-dried |
| Lbr | Lumber |
| LCL | Less than carload |
| LFT or Lin Ft | Linear foot or feet |
| Lgr | Longer |
| Lgth | Length |
| Lin | Linear |
| Lng | Lining |
| M | Thousand |
| MBM | Thousand (feet) board measure |
| MC | Moisture content |
| Merch | Merchantable |
| Mldg | Molding |
| Mm | Millimeter |
| No | Number |
| N1E | Nosed one edge |
| N2E | Nosed two edges |
| Og | Ogee |
| Ord | Order |
| Par | Paragraph |
| Part | Partition |
| Pat | Pattern |
| Pc | Piece |
| Pcs | Pieces |
| PE | Plain end |
| PO | Purchase order |
| P\&T | Post and timbers |
| Reg | Regular |
| Res | Resawed or resawn |
| Rfg | Roofing |
| Rgh | Rough |
| TUL | Random lengths |
| R/W | Random widths |
| R/W\&L | Random widths and lengths |
| Sdg | Siding |
| Sel | Select |
| S\&E | Side and Edge (surfaced on) |
| SE Sdg | Square edge siding |
| SE \& S | Square edge and sound |
| S/L or S/LAP | Shiplap |
| SL\&C | Shipper's load and count |


| SM or Std M | Standard matched |
| :--- | :--- |
| Specs | Specifications |
| Std | Standard |
| Stpg | Stepping |
| Str or Struc | Structural |
| S1E | Surfaced one edge |
| S1S | Surfaced one side |
| S1S1E | Surfaced one side and one edge |
| S1S2E | Surfaced one side and two edges |
| S2E | Surfaced two edges |
| S2E | Surfaced two sides |
| S2S | Surfaced two sides and one edge |
| S2S1E | Surfaced two sides and center matched |
| S2S\&CM | Surfaced two sides and standard matched |
| S2S\&SM | Surfaced four sides |
| S4S | Surfaced four sides and caulking seam |
| S4S\&CS | Tongued and grooved |
| T\&G | Vertical grain |
| VG | Wider |
| Wdr | Weight |
| Wt | Pattern |

### 7.1.0 Softwood Lumber—Commercial Names of Principal Softwood Species

The commercial names listed below are intended to provide a correlation between commercial names for lumber and the botanical names of the species from which the lumber is to be manufactured. In some instances, more than one species is associated with a single commercial name. For stress-graded lumber, the species to be associated with a commercial name will be determined in accordance with 6.3.2.1. These commercial names are to be used in grading rule descriptions and in specifications [see 2.15]. The provisions of this Standard apply to lumber manufactured from hardwood species or lumber manufactured from foreign species when the species is included in rules certified by the Board of Review. The information contained herein is a partial list of commercial names of the principal softwood species and species groups. Additional species and species groups are provided in ASTM Standard D 1165-03 and the rules certified by the Board of Review.

| Commercial Species or Species <br> Group Names |  |  |
| :--- | :--- | :--- |
| Cedar: | Official Common <br> Tree Names |  |
| Alaska Cedar |  | Botanical Names |
| Incense Cedar | Alaska-cedar | Chamaecyparis nootkatensis |
| Port Orford Cedar | incense-cedar | Libocedrus decurrens |
| Eastern Red Cedar | Port-Orford-cedar | Chamaecyparis lawsoniana |
|  | eastern redcedar | Juniperus virginiana |
| Western Red Cedar | southern redcedar | J. silicicola |
| Northern White Cedar | wetern redcedar | Thuja plicata |
| Southern White Cedar | northern white-cedar | T. occidentalis |
| Atlantic white-cedar | Chamaecyparis thyoides |  |
| Cypress ${ }^{3}$ |  |  |
| Baldcypress |  |  |
| Pond cypress | Baldcypress | Pond cypress |


| Commercial Species or Species Group Names ${ }^{1}$ | Official Common Tree Names ${ }^{2}$ | Botanical Names |
| :---: | :---: | :---: |
| Bigcone Douglas fir Noble Fir <br> Alpine Fir California Red Fir Grand Fir Pacific Grand Fir White Fir | Bigcone Douglas fir noble fir subalpine fir (alpine fir) California red fir grand fir Pacific silver fir white fir | P. macrocarpa Abies procera <br> A. lasiocarpa <br> A. magnifica <br> A. grandis <br> A. amabilis <br> A. concolor |
| Hemlock: |  |  |
| Carolina Hemlock <br> Eastern Hemlock <br> Mountain Hemlock <br> Western Hemlock | Carolina hemlock eastern hemlock mountain hemlock western hemlock | Tsuga caroliniana <br> T. Canadensis <br> T. mertensiana <br> T. heterophylla |
| Juniper: |  |  |
| Western Juniper | alligator juniper Rocky Mountain juniper Utah juniper western juniper | Juniperus deppeana <br> J. scopulorum <br> J. osteosperma <br> J. occidentalis |
| Larch: |  |  |
| Western Larch | western larch | Larix occidentalis |
| Pine: |  |  |
| Bishop Pine <br> Digger Pine <br> Knobcone Pine <br> Coulter Pine <br> Jeffrey Pine <br> Jack Pine <br> Limber Pine <br> Lodgepole Pine <br> Norway Pine <br> Pitch Pine <br> Ponderosa Pine <br> Radiata/Monterey Pine <br> Sugar Pine <br> Whitebark Pine <br> Idaho White Pine <br> Northern White Pine <br> Longleaf Pine ${ }^{6}$ <br> Southern Pine (Major) <br> Southern Pine (Minor) | Bishop pine <br> Digger pine knobcone pine <br> Coulter pine <br> Jeffrey pine <br> jack pine <br> limber pine <br> lodgepole pine <br> red pine <br> pitch pine <br> ponderosa pine <br> Monterey pine <br> sugar pine <br> whitebark pine <br> western white pine <br> eastern white pine <br> longleaf pine <br> slash pine <br> loblolly pine <br> longleaf pine <br> shortleaf pine <br> slash pine <br> pond pine <br> Virginia pine <br> sand pine <br> spruce pine | Pinus muricata <br> P. sabiniana <br> P. attenuata <br> P. coulteri <br> P. jeffreyi <br> P. banksiana <br> P. flexilis <br> P. contorta <br> P. resinosa <br> P. rigida <br> P. ponderosa <br> P. radiate <br> P. lambertiana <br> P. albicaulis <br> P. monticola <br> P. strobes <br> P. palustris <br> P. elliottii <br> P. taeda <br> P. palustris <br> P. echinata <br> P. elliottii <br> P. serotina <br> P. virginiana <br> P. clausa <br> P. glabra |
| Redwood: |  |  |
| Redwood | Redwood | Sequoia sempervirens |
| Spruce: |  |  |
| Black spruce | black spruce | Picea mariana |



### 7.1.1 Standard Sizes for Framing Lumber

Nominal and Dressed (based on Western Lumber Grading Rules)

| Product | Description |  |  | Dressed Dimensions |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Nominal Size |  | Thicknesses and Widths |  |  |  | Length (feet) |
|  |  | Thickness (inch) | Width (inch) | Surfaced Dry |  | Surfaced Unseasoned |  |  |
|  |  |  |  | inch | mm | inch | mm |  |
| DIMENSION | S4S | 2 | 2 | $11 / 2$ | 38 | $19 / 16$ | 40 | $6^{\prime}(183 \mathrm{~cm})$ and longer, generally shipped in multiples of $2^{\prime}$ ( 61 cm ) |
|  |  | 3 | 3 | $21 / 2$ | 61 | $29 / 16$ | 65 |  |
|  |  | 4 | 4 | $31 / 2$ | 89 | 3 9/16 | 90 |  |
|  |  |  | 5 | $41 / 2$ | 114 | 4 5/8 | 117 |  |
|  |  |  | 6 | $51 / 2$ | 140 | 5 5/8 | 143 |  |
|  |  |  | 8 | $71 / 4$ | 184 | $71 / 2$ | 191 |  |
|  |  |  | 10 | $9^{1 / 4}$ | 235 | $91 / 2$ | 241 |  |
|  |  |  | 12 | $11^{1 / 4}$ | 289 | $11^{1 / 2}$ | 292 |  |
|  |  |  | over 12 | $3 / 4$ off nominal | 19 off nominal | $1 / 2$ off nominal | 13 off nominal |  |
|  |  |  |  | Thickness unseasoned |  | Width unseasoned |  | $6^{\prime}$ (183 cm) and longer, generally shipped in multiples of $2^{\prime}$ ( 61 cm ) |
| TIMBERS | Rough or S4S (shipped unseasoned) | 5 and larger | $\begin{aligned} & 5 \text { and } \\ & \text { larger } \end{aligned}$ | $1 / 2^{\prime \prime}(13 \mathrm{~mm})$ off nominal (S4S). See 3.20 of WWPA Grading Rules for Rough. |  |  |  |  |

Nominal and Dressed (based on Western Lumber Grading Rules) - Cont'd

| Product | Description | Thickness | Width | Thickness Dry |  | Width Dry |  | Length feet |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | inch | mm | inch | mm |  |
| DECKING | 2" (Single <br> T\&G) | 2 | 5 | $11 / 2$ | 38 | 4 | 102 | $6^{\prime}(183 \mathrm{~cm})$ and longer, generally shipped in multiples of $2^{\prime}$ ( 61 cm ) |
|  |  |  | 6 |  |  | 5 | 127 |  |
|  |  |  | 8 |  |  | $63 / 4$ | 172 |  |
|  |  |  | 10 |  |  | $8^{3 / 4}$ | 222 |  |
|  |  |  | 12 |  |  | $103 / 4$ | 273 |  |
|  | 3" and 4" | 3 | 6 | $21 / 2$ | 64 | $51 / 4$ | 133 |  |
|  | (Double T\&G) | 4 |  | $31 / 2$ | 89 |  |  |  |

Abbreviations: T\&F—Tongued and grooved Rough—Unsurfaced S4S—Surfaced four sides
Note on Metrics: Metric equivalents are provided for surfaces (actual) sizes.

### 7.1.2 Nominal and Minimum Dressed Sizes of Finish Lumber

The thicknesses apply to all widths, and all widths apply to all thicknesses except as modified. Sizes are given in millimeters and inches. Metric units are based on dressed size.

Nominal and minimum-dressed dry sizes of finish, flooring, ceiling, partition, and stepping at $19 \%$ maximum-moisture content

| Item | Thicknesses |  |  | Nominal (inch) | Widths |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Nominal Inch | Minimum Dressed |  |  | Minimum Dressed |  |
|  |  | mm | inch |  | mm | inch |
| Finish | 3/8 | 8 | 5/16 | 2 | 38 | 1-1/2 |
|  | 1/2 | 11 | 7/16 | 3 | 64 | 2-1/2 |
|  | 5/8 | 14 | 9/16 | 4 | 89 | 3-1/2 |
|  | 3/4 | 16 | 5/8 | 5 | 114 | 4-1/2 |
|  | 1 | 19 | 3/4 | 6 | 140 | 5-1/2 |
|  | 1-1/4 | 25 | 1 | 7 | 165 | 6-1/2 |
|  | 1-1/2 | 32 | 1-1/4 | 8 | 184 | 7-1/4 |
|  | 1-3/4 | 35 | 1-3/8 | 9 | 210 | 8-1/4 |
|  | 2 | 38 | 1-1/2 | 10 | 235 | 9-1/4 |
|  | 2-1/2 | 51 | 2 | 11 | 260 | 10-1/4 |
|  | 3 | 64 | 2-1/2 | 12 | 286 | 11-1/4 |
|  | 3-1/2 | 76 | 3 | 14 | 337 | 13-1/4 |
|  | 4 | 89 | 3-1/2 | 16 | 387 | 15-1/4 |
| Flooring ${ }^{\text {a }}$ | 3/8 | 8 | 5/16 | 2 | 29 | 1-1/8 |
|  | 1/2 | 11 | 7/16 | 3 | 54 | 2-1/8 |
|  | 5/8 | 14 | 9/16 | 4 | 79 | 3-1/8 |
|  | 1 | 19 | 3/4 | 5 | 105 | 4-1/8 |
|  | 1-1/4 | 25 | 1 | 6 | 130 | 5-1/8 |
|  | 1-1/2 | 32 | 1-1/4 |  |  |  |
| Ceiling ${ }^{\text {a }}$ | 3/8 | 8 | 5/16 | 3 | 54 | 2-1/8 |
|  | 1/2 | 11 | 7/16 | 4 | 79 | 3-1/8 |
|  | 5/8 | 14 | 9/16 | 5 | 105 | 4-1/8 |

Nominal and minimum-dressed dry sizes of finish, flooring, ceiling, partition, and stepping at $19 \%$ maximum-moisture content-Cont'd

| Item | Thicknesses |  |  | Nominal (inch) | Widths |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Nominal Inch | Minimum Dressed |  |  | Minimum Dressed |  |
|  |  | mm | inch |  | mm | inch |
|  | 3/4 | 17 | 11/16 | 6 | 130 | 5-1/8 |
| Partition ${ }^{\text {a }}$ |  |  |  | 3 | 54 | 2-1/8 |
|  |  |  |  | 4 | 79 | 3-1/8 |
|  | 1 | 18 | 23/32 | 5 | 105 | 4-1/8 |
|  |  |  |  | 6 | 130 | 5-1/8 |
| Stepping | 1 | 19 | 3/4 | 8 | 184 | 7-1/4 |
|  | 1-1/4 | 25 | 1 | 10 | 235 | 9-1/4 |
|  | 1-1/2 | 32 | 1/4 | 12 | 286 | 11-1/4 |
|  | 2 | 38 | 1-1/2 |  |  |  |

${ }^{a}$ In tongued-and-grooved flooring and in tongued-and-grooved and shiplapped ceiling of 8 mm (5/16 inch), 11 mm (7/16 inch), and 14 mm (9/16 inch) dressed thicknesses, the tongue or lap shall be 5 mm (3/16 inch) wide, with the overall widths 5 mm (3/16 inch) wider than the face widths shown in the above table. In all other worked lumber shown in this table of dressed thicknesses of 16 mm ( $5 / 8$ inch) to 32 mm (1-1/4 inches), the tongue shall be 6 mm ( $1 / 4 \mathrm{inch}$ ) wide or wider in tongued-and-grooved lumber, and the lap shall be 10 mm (3/8 inch) wide or wider in shiplapped lumber, and the overall widths shall be not less than the dressed face widths shown in the above table plus the width of the tongue or lap.

The thicknesses apply to all widths, and all widths apply to all thicknesses. Sizes are given in millimeters and inches. Metric units are based on dressed size. See B1, Appendix B, for rounding rule for metric units.

Nominal and minimum-dressed sizes of worked lumber

| Item | Nominal Inch | Thicknesses |  |  |  | Nominal Inch | Widths |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Minimum Dressed |  |  |  |  | Minimum Dressed |  |  |  |
|  |  | Dry ${ }^{\text {a }}$ |  | Green ${ }^{\text {a }}$ |  |  | Dry ${ }^{\text {a }}$ |  | Green ${ }^{\text {a }}$ |  |
|  |  | mm | inch | mm | inch |  | mm | inch | mm | inch |
| Shiplap, 10 mm (3/8 inch) lap | 1 | $19^{\text {c }}$ | $3 / 4^{\text {c }}$ | 20 | 25/32 | 4 | 79 | 3-1/8 | 81 | 3-3/16 |
|  |  |  |  |  |  | 6 | 130 | 5-1/8 | 133 | 5-1/4 |
|  |  |  |  |  |  | 8 | 175 | 6-7/8 | 181 | 7-1/8 |
|  |  |  |  |  |  | 10 | 225 | 8-7/8 | 232 | 9-1/8 |
|  |  |  |  |  |  | 12 | 276 | 10-7/8 | 283 | 11-1/8 |
|  |  |  |  |  |  | 14 | 327 | 12-7/8 | 333 | 13-1/8 |
|  |  |  |  |  |  | 16 | 378 | 14-7/8 | 384 | 15-1/8 |
| Shiplap, 13 mm (1/2 inch) lap ${ }^{b}$ | 1 | $19^{\text {c }}$ | $3 / 4^{\text {c }}$ | 20 | 25/32 | 4 | 76 | 3 | 78 | 3-1/16 |
|  | 2 | 38 | 1-1/2 | 40 | 1-9/16 | 6 | 127 | 5 | 130 | 5-1/8 |
|  | 2-1/2 | 51 | 2 | 52 | 2-1/16 | 8 | 171 | 6-3/4 | 178 | 7 |
|  | 3 | 64 | 2-1/2 | 65 | 2-9/16 | 10 | 222 | 8-3/4 | 229 | 9 |
|  | 3-1/2 | 76 | 3 | 78 | 3-1/16 | 12 | 273 | 10-3/4 | 279 | 11 |
|  | 4 | 89 | 3-1/2 | 90 | 3-9/16 | 14 | 324 | 12-3/4 | 330 | 13 |
|  | 4-1/2 | 102 | 4 | 103 | 4-1/16 | 16 | 375 | 14-3/4 | 381 | 15 |

Nominal and minimum-dressed sizes of worked lumber-Cont'd

| Item | Nominal Inch | Thicknesses |  |  |  | Nominal Inch | Widths |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Minimum Dressed |  |  |  |  | Minimum Dressed |  |  |  |
|  |  | Dry ${ }^{\text {a }}$ |  | Green ${ }^{\text {a }}$ |  |  | Dry ${ }^{\text {a }}$ |  | Green ${ }^{\text {a }}$ |  |
|  |  | $m m$ | inch | $m m$ | inch |  | mm | inch | $m m$ | inch |
| Centermatch <br>  <br> Groove), <br> 6 mm (1/4 <br> inch) tongue |  |  |  |  |  | 4 | 79 | 3-1/8 | 81 | 3-3/16 |
|  | 1 | $19^{\text {c }}$ | $3 / 4^{\text {c }}$ | 20 | 25/32 | 5 | 105 | 4-1/8 | 108 | 4-1/4 |
|  | 1-1/4 | 25 | 1 | 26 | 1-1/32 | 6 | 130 | 5-1/8 | 133 | 5-1/4 |
|  | 1-1/2 | 32 | 1-1/4 | 33 | 1-9/32 | 8 | 175 | 6-7/8 | 181 | 7-1/8 |
|  |  |  |  |  |  | 10 | 225 | 8-7/8 | 232 | 9-1/8 |
|  |  |  |  |  |  | 12 | 276 | 10-7/8 | 283 | 11-1/8 |
| Centermatch <br> (Tongue \& Groove), 10 mm (3/8 inch) tongue ${ }^{\text {b }}$ | 2 | 38 | 1-1/2 | 40 | 1-9/16 | 4 | 76 | 3 | 78 | 3-1/16 |
|  | 2-1/2 | 51 | 2 | 52 | 2-1/16 | 6 | 127 | 5 | 130 | 5-1/8 |
|  | 3 | 64 | 2-1/2 | 65 | 2-9/16 | 8 | 171 | 6-3/4 | 178 | 7 |
|  | 3-1/2 | 76 | 3 | 78 | 3-1/16 | 10 | 222 | 8-3/4 | 229 | 9 |
|  | 4 | 89 | 3-1/2 | 90 | 3-9/16 | 12 | 273 | 10-3/4 | 279 | 11 |
|  | 4-1/2 | 102 | 4 | 103 | 4-1/16 |  |  |  |  |  |
| Grooved-for-Splines | 2-1/2 | 51 | 2 | 52 | 2-1/16 | 4 | 89 | 3-1/2 | 90 | 3-9/16 |
|  | 3 | 64 | 2-1/2 | 65 | 2-9/16 | 6 | 140 | 5-1/2 | 143 | 5-5/8 |
|  | 3-1/2 | 76 | 3 | 78 | 3-1/16 | 8 | 184 | 7-1/4 | 190 | 7-1/2 |
|  | 4 | 89 | 3-1/2 | 90 | 3-9/16 | 10 | 235 | 9-1/4 | 241 | 9-1/2 |
|  | 4-1/2 | 102 | 4 | 103 | 4-1/16 | 12 | 286 | 11-1/4 | 292 | 11-1/2 |

### 7.1.3 Appearance Lumber--Grades, Nominal, and Dressed Sizes

|  | Product | Grades ${ }^{1}$ | Equivalent Grades in Idaho White Pine | WWPA Grading Rules Section Number |
| :---: | :---: | :---: | :---: | :---: |
|  | Selects (all species) | B \& BTR SELECT | SUPREME | 10.11 |
|  |  | C SELECT | CHOICE | 10.12 |
|  |  | D SELECT | QUALITY | 10.13 |
|  | Finish (usually available only in | SUPERIOR |  | 10.51 |
|  | Doug Fir and Hem-Fir) | PRIME |  | 10.52 |
|  |  | E |  | 10.53 |
|  | Special Western | CLEAR HEART |  | 20.11 |
|  | Red Cedar | A GRADE |  | 20.12 |
|  | Pattern ${ }^{2}$ Grades | B GRADE |  | 20.13 |

(Continued)

|  | Product | Grades ${ }^{1}$ | Equivalent Grades in Idaho White Pine | WWPA Grading Rules Section Number |
| :---: | :---: | :---: | :---: | :---: |
|  | Common Boards (WWPA Rules) (primarily in pines, spruces, and cedars) | 1 COMMON | COLONIAL | 30.11 |
|  |  | 2 COMMON | STERLING | 30.12 |
|  |  | 3 COMMON | STANDARD | 30.13 |
|  |  | 4 COMMON | UTILITY | 30.14 |
|  |  | 5 COMMON | INDUSTRIAL | 30.15 |
|  | Alternate Boards (WCLIB Rules) (primarily in Dough Fir and Hem-Fir) |  |  | WCLIB ${ }^{3,4}$ |
|  |  | SELECT |  | 118-a |
|  |  | MERCHANTABLE |  | 118-b |
|  |  | CONSTRUCTION |  | 118-c |
|  |  | STANDARD |  | 118-d |
|  |  | UTILITY |  | 118-e |
|  |  | ECONOMY |  |  |
|  | Special Western |  |  | WCLIB ${ }^{3}$ |
|  | Red Cedar | SELECT KNOTTY |  | 111-e |
|  | Pattern ${ }^{2}$ Grades | QUALITY |  | 111-f |
|  |  | KNOTTY |  |  |

${ }^{1}$ Refer to WWPA's Vol. 2, Western Wood Species book for full-color photography and to WWPA's Natural Wood Siding for complete information on siding grades, specification, and installation.
${ }^{2}$ "PATTERN" includes finish, paneling, ceiling and siding grades.
${ }^{3}$ West Coast Lumber Inspection Bureau's West Coast Lumber Standard Grading Rules.
${ }^{4}$ Also found in WWPA's Western Lumber Grading Rules.

## Standard Signs-Appearance Lumber

Nominal \& Dressed (Based on Western Lumber Grading Rules)

| Product | Description | Nominal Size |  | Dry Dressed Dimensions |  |  |  | Lengths (feet) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Thickness | Width | Thickness |  | Width |  |  |
|  |  | inches | inches | inches | mm | inches | mm |  |
| SELECTS | S1S, S2S, | 4/4 | 2 | 3/4 | 19 | $11 / 2$ | 38 | $6^{\prime}(183 \mathrm{~cm})$ and longer |
| AND | S4S, S1S1E, | 5/4 | 3 | $15 / 32$ | 29 | 21/2 | 64 | in multiples of $1^{\prime}$ |
| COMMONS | S1S2E | 6/4 | 4 | $1^{13 / 32}$ | 36 | $31 / 2$ | 89 | (31 cm), except |
|  |  | 7/4 | 5 | $1^{19 / 32}$ | 40 | 41/2 | 114 | Douglas Fir and Larch |
|  |  | 8/4 | 6 | 13136 | 46 | $51 / 2$ | 140 | Selects shall be $4^{\prime}$ |
|  |  | 9/4 | 7 | $2^{3 / 3}$ | 53 | 61/2 | 165 | $(122 \mathrm{~cm})$ and longer |
|  |  | 10/4 | 8 \& wider | $2^{3 / 8}$ | 60 | 3/4 off nominal | 19 off nominal | with $3 \%$ of $4^{\prime}(122 \mathrm{~cm})$ and $5^{\prime}$ ( 152 cm ) |
|  |  | 11/4 |  | 2\%16 | 65 |  |  |  |
|  |  | 12/4 |  | $2^{3 / 4}$ | 70 |  |  |  |
|  |  | 16/4 |  | $33 / 4$ | 95 |  |  |  |

Standard Signs-Appearance Lumber-Cont'd

| Product | Description | Nominal Size |  | Dry Dressed Dimensions |  |  |  | Lengths (feet) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Thickness <br> inches | Width <br> inches | Thickness |  | Width |  |  |
|  |  |  |  | inches | mm | inches | mm |  |
| FINISH AND | S1S, S2S, | 3/8 | 2 | 5/16 | 2 | $11 / 2$ | 38 | $3^{\prime}(91 \mathrm{~cm})$ and longer. |
| ALTERNATE | S4S, S1S1E, | 1/2 | 3 | 7/16 | 11 | $21 / 2$ | 64 | In SUPERIOR grade, |
| BOARD | S1S2E | 5/8 | 4 | \%/16 | 14 | $31 / 2$ | 89 | $3 \%$ of $3^{\prime}(91 \mathrm{~cm})$ and $4^{\prime}$ |
| GRADES |  | $3 / 4{ }^{1}$ | 5 | 5/8 | 16 | $4^{1 / 2}$ | 114 | $(122 \mathrm{~cm})$ and $7 \%$ of $5^{\prime}$ |
|  |  | $1{ }^{1}$ | 6 | 3/4 | 19 | $51 / 2$ | 140 | (152 cm) and $6^{\prime}$ |
|  |  | $11 / 4{ }^{1}$ | 7 | 1 | 25 | $61 / 2$ | 165 | $(183 \mathrm{~cm})$ are |
|  |  | $1^{1 / 2}{ }^{1}$ | 8 \& wider | $11 / 4$ | 32 | 3/4 off nominal | 16 off nominal | permitted. In PRIME grade $20 \%$ of $3^{\prime}$. |
|  |  | $13 / 4$ |  | $13 / 8$ | 35 |  |  | ( 91 cm ) to $6^{\prime}(183 \mathrm{~cm})$ |
|  |  | 2 |  | $11 / 2$ | 38 |  |  | is permitted. |
|  |  | $21 / 2$ |  | 2 | 51 |  |  |  |
|  |  | 3 |  | $21 / 2$ | 64 |  |  |  |
|  |  | $31 / 2$ |  | 3 | 76 |  |  |  |
|  |  | 4 |  | $31 / 2$ | 89 |  |  |  |

${ }^{1}$ These sizes apply only to WCLIB Alternate Board grades.
Abbreviations: S1S—Surfaced one side S1S1E—Surfaced one side, one edge S2S—Surfaced two side S1S2E—Surfaced one side, two edges S4S—Surfaced four sides
Note on Metrics: Metric equivalents are provided for surfaced (actual) sizes.

### 7.1.4 Section Properties of Joists, Beams, and Timbers



| Nominal | Surfaced | Area (A) | Section | Moment of | Board |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Size in | Size for | $A=b h$ | Modulus (S) | Inertia (I) | Feet per |
| Inches | Design in | $\left(i n^{2}\right)$ | $b h^{2}$ | $b h^{3}$ | Lineal |
| $b \times h$ | Inches |  |  | 12 | Foot of |
|  | $b \times h$ |  | $\left(i n^{3}\right)$ | (in*) | Piece |
| $2 \times 2$ | $1.5 \times 1.5$ | 2.25 | 0.562 | 0.422 | 0.33 |
| $2 \times 3$ | $1.5 \times 2.5$ | 3.75 | 1.56 | 1.95 | 0.50 |
| $2 \times 4$ | $1.5 \times 3.5$ | 5.25 | 3.06 | 5.36 | 0.67 |
| $2 \times 6$ | $1.5 \times 5.5$ | 8.25 | 7.56 | 20.80 | 1.00 |

(Continued)

| Nominal Size in Inches$b \times h$ | Surfaced | Area (A) | Section | Moment of | Board <br> Feet per <br> Lineal <br> Foot of <br> Piece |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | Size for | $A=b h$ | Modulus (S) | Inertia (I) |  |
|  | Design in | $\left(\mathrm{in}^{2}\right)$ | $c_{-}-b h^{2}$ | $l=b h^{3}$ |  |
|  | Inches |  | $6$ | $\overline{12}$ |  |
|  | $b \times h$ |  | (in ${ }^{3}$ ) | (in*) |  |
| $2 \times 8$ | $1.5 \times 7.25$ | 10.88 | 13.14 | 47.63 | 1.33 |
| $2 \times 10$ | $1.5 \times 9.25$ | 13.88 | 21.39 | 98.93 | 1.67 |
| $2 \times 12$ | $1.5 \times 11.25$ | 16.88 | 31.64 | 177.98 | 2.00 |
| $2 \times 14$ | $1.5 \times 13.25$ | 19.88 | 43.89 | 290.78 | 2.33 |
| $3 \times 3$ | $2.5 \times 2.5$ | 6.25 | 2.60 | 3.26 | 0.75 |
| $3 \times 4$ | $2.5 \times 3.5$ | 8.75 | 5.10 | 8.93 | 1.00 |
| $3 \times 6$ | $2.5 \times 5.5$ | 13.75 | 12.60 | 34.66 | 1.50 |
| $3 \times 8$ | $2.5 \times 7.25$ | 18.12 | 21.90 | 79.39 | 2.00 |
| $3 \times 10$ | $2.5 \times 9.25$ | 23.12 | 35.65 | 164.89 | 2.50 |
| $3 \times 12$ | $2.5 \times 11.25$ | 28.12 | 52.73 | 296.63 | 3.00 |
| $3 \times 14$ | $2.5 \times 13.25$ | 33.12 | 73.15 | 484.63 | 3.50 |
| $3 \times 16$ | $2.5 \times 15.25$ | 38.12 | 96.90 | 738.87 | 4.00 |
| $4 \times 4$ | $3.5 \times 3.5$ | 12.25 | 7.15 | 12.51 | 1.33 |
| $4 \times 6$ | $3.5 \times 5.5$ | 19.25 | 17.65 | 48.53 | 2.00 |
| $4 \times 8$ | $3.5 \times 7.25$ | 25.38 | 30.66 | 111.15 | 2.67 |
| $4 \times 10$ | $3.5 \times 9.25$ | 32.38 | 49.91 | 230.84 | 3.33 |
| $4 \times 12$ | $3.5 \times 11.25$ | 39.38 | 73.83 | 415.28 | 4.00 |
| $4 \times 14$ | $3.5 \times 13.25$ | 46.38 | 102.41 | 678.48 | 4.67 |
| $4 \times 16$ | $3.5 \times 15.25$ | 53.38 | 135.66 | 1034.42 | 5.33 |
| $6 \times 6$ | $5.5 \times 5.5$ | 30.25 | 27.73 | 76.26 | 3.00 |
| $6 \times 8$ | $5.5 \times 7.5$ | 41.25 | 51.56 | 193.36 | 4.00 |
| $6 \times 10$ | $5.5 \times 9.5$ | 52.25 | 82.73 | 392.96 | 5.00 |
| $6 \times 12$ | $5.5 \times 11.5$ | 63.25 | 121.23 | 697.07 | 6.00 |
| $6 \times 14$ | $5.5 \times 13.5$ | 74.25 | 167.06 | 1127.67 | 7.00 |
| $6 \times 16$ | $5.5 \times 15.5$ | 85.25 | 220.23 | 1706.78 | 8.00 |
| $6 \times 18$ | $5.5 \times 17.5$ | 96.25 | 280.73 | 2456.38 | 9.00 |
| $6 \times 20$ | $5.5 \times 19.5$ | 107.25 | 348.56 | 3398.48 | 10.00 |
| $8 \times 8$ | $7.5 \times 7.5$ | 56.25 | 70.31 | 263.67 | 5.33 |
| $8 \times 10$ | $7.5 \times 9.5$ | 71.25 | 112.81 | 535.86 | 6.67 |
| $8 \times 12$ | $7.5 \times 11.5$ | 86.25 | 165.31 | 950.55 | 8.00 |
| $8 \times 14$ | $7.5 \times 13.5$ | 101.25 | 227.81 | 1537.73 | 9.33 |
| $8 \times 16$ | $7.5 \times 15.5$ | 116.25 | 300.31 | 2327.42 | 10.67 |
| $8 \times 18$ | $7.5 \times 17.5$ | 131.25 | 382.81 | 3349.61 | 12.00 |
| $8 \times 20$ | $7.5 \times 19.5$ | 146.25 | 475.31 | 4634.30 | 13.33 |
| $8 \times 22$ | $7.5 \times 21.5$ | 161.25 | 577.81 | 6211.48 | 14.67 |
| $8 \times 24$ | $7.5 \times 23.5$ | 176.25 | 690.31 | 8111.17 | 16.00 |
| $10 \times 10$ | $9.5 \times 9.5$ | 90.25 | 142.90 | 678.76 | 8.33 |
| $10 \times 12$ | $9.5 \times 11.5$ | 109.25 | 209.40 | 1204.03 | 10.00 |
| $10 \times 14$ | $9.5 \times 13.5$ | 128.25 | 288.56 | 1947.80 | 11.67 |
| $10 \times 16$ | $9.5 \times 15.5$ | 147.25 | 380.40 | 2948.07 | 13.33 |
| $10 \times 18$ | $9.5 \times 17.5$ | 166.25 | 484.90 | 4242.84 | 15.00 |
| $10 \times 20$ | $9.5 \times 19.5$ | 185.25 | 602.06 | 5870.11 | 16.67 |
| $10 \times 22$ | $9.5 \times 21.5$ | 204.25 | 731.90 | 7867.88 | 18.33 |
| $12 \times 12$ | $11.5 \times 11.5$ | 132.25 | 253.48 | 1457.51 | 12.00 |
| $12 \times 14$ | $11.5 \times 13.5$ | 155.25 | 349.31 | 2357.86 | 14.00 |
| $12 \times 16$ | $11.5 \times 15.5$ | 178.25 | 460.48 | 3568.71 | 16.00 |
| $12 \times 18$ | $11.5 \times 17.5$ | 201.25 | 586.98 | 5136.07 | 18.00 |


| Nominal Size in Inches $\mathrm{b} \times \mathrm{h}$ | Surfaced | $\begin{aligned} & \text { Area }(A) \\ & A=b h \\ & \left(\mathrm{in}^{2}\right) \end{aligned}$ | Section <br> Modulus (S) $\begin{aligned} & S=\frac{b h^{2}}{6} \\ & \left(\mathrm{in}^{3}\right) \end{aligned}$ | Moment of Inertia (I)$\begin{aligned} & I=\frac{b h^{3}}{12} \\ & \left(\text { in* }^{*}\right) \end{aligned}$ | Board <br> Feet per <br> Lineal <br> Foot of <br> Piece |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | Size for |  |  |  |  |
|  | Design in |  |  |  |  |
|  | Inches |  |  |  |  |
|  | $b \times h$ |  |  |  |  |
| $12 \times 20$ | $11.5 \times 19.5$ | 224.25 | 728.81 | 7105.92 | 20.00 |
| $12 \times 22$ | $11.5 \times 21.5$ | 247.25 | 885.98 | 9524.28 | 22.00 |
| $12 \times 24$ | $11.5 \times 23.5$ | 270.25 | 1058.48 | 12437.13 | 24.00 |

Used with Courtesy of Western Wood Products Association.

## FRAMING LUMBER

| Beams and Stringers Design Values |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Extreme Fiber |  |  | Com | ression |  |
| Species of Group | Grade | $\begin{aligned} & \text { Bending } \\ & \hline \text { Single Member } \\ & F_{b} \\ & \hline \end{aligned}$ | Parallel to Grain $F_{t}$ | Horizontal Shear ${ }^{3}$ $\mathrm{F}_{\mathrm{V}}$ | Perpendicular $\mathrm{F}_{\mathrm{c} \perp}$ | Parallel to Grain $F_{c}$ | Modulus o Elasticity E |
| Douglas Fir-Larch | Dense Selected Structural | 1900 | 1100 | 170 | 730 | 1300 | 1,700,000 |
|  | Dense No. 1 | 1550 | 775 | 170 | 730 | 1100 | 1,700,000 |
|  | Dense No. 2 | 1000 | 500 | 170 | 730 | 700 | 1,400,000 |
|  | Select Structural | 1600 | 950 | 170 | 625 | 1100 | 1,600,000 |
|  | No. 1 | 1350 | 675 | 170 | 625 | 925 | 1,600,000 |
|  | No. 2 | 875 | 425 | 170 | 625 | 600 | 1,300,000 |
| Douglas Fir-South | Select Structural | 1550 | 900 | 165 | 520 | 1000 | 1,200,000 |
|  | No. 1 | 1300 | 625 | 165 | 520 | 850 | 1,200,000 |
|  | No. 2 | 825 | 425 | 165 | 520 | 550 | 1,000,000 |
| Hem-Fir | Select Structural | 1300 | 750 | 140 | 405 | 925 | 1,300,000 |
|  | No. 1 | 1050 | 525 | 140 | 405 | 750 | 1,300,000 |
|  | No. 2 | 675 | 350 | 140 | 405 | 500 | 1,100,000 |
| Mountain Hemlock | Select Structural | 1350 | 775 | 170 | 570 | 875 | 1,100,000 |
|  | No. 1 | 1100 | 550 | 170 | 570 | 725 | 1,100,000 |
|  | No. 2 | 725 | 375 | 170 | 570 | 475 | 900,000 |
| Sitka Spruce | Select Structural | 1200 | 675 | 140 | 435 | 825 | 1,300,000 |
|  | No. 1 | 1000 | 500 | 140 | 435 | 675 | 1,300,000 |
|  | No. 2 | 650 | 325 | 140 | 435 | 450 | 1,100,000 |
| Spruce-Pine-Fir (South) | Select Structural | 1050 | 625 | 125 | 335 | 675 | 1,200,000 |
|  | No. 1 | 900 | 450 | 125 | 335 | 550 | 1,200,000 |
|  | No. 2 | 575 | 300 | 125 | 335 | 375 | 1,000,000 |
| Western Cedars | Select Structural | 1400 | 825 | 170 | 410 | 1000 | 1,400,000 |
|  | No. 1 | 1150 | 575 | 170 | 410 | 850 | 1,400,000 |
|  | No. 2 | 750 | 375 | 170 | 410 | 550 | 1,100,000 |

Beams and Stringers Design Values-Cont'd

| Species of Group | Grade | Extreme Fiber | Tension Parallel to Grain $F_{t}$ | Horizontal <br> Shear ${ }^{3}$ <br> $F_{V}$ | Compression |  | Modulus of Elasticity E |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Bending <br> $\overline{\text { Single Member }}$ <br> $F_{b}$ |  |  | Perpendicular $F_{c \perp}$ | Parallel to <br> Grain <br> $F_{c}$ |  |
| Western Woods | Select Structural | 1050 | 625 | 125 | 345 | 750 | 1,100,000 |
| (and White | No. 1 | 900 | 450 | 125 | 345 | 625 | 1,100,000 |
|  | No. 2 | 575 | 300 | 125 | 345 | 425 | 900,000 |

## Posts and Timbers Design Values ${ }^{1}$

$5^{\prime \prime} \times 5^{\prime \prime}$ and larger, width not more than $2^{\prime \prime}$ greater than thickness ${ }^{2}$
Grades described in Section 53.00 and 80.00 of Western Lumber Grading Rules

| Species of Group | Grade | Extreme Fiber <br> Stress in <br> Bending <br> Single Member <br> $\mathrm{F}_{\mathrm{b}}$ | Tension Parallel to Grain $F_{t}$ | Horizontal <br> Shear ${ }^{3}$ <br> $\mathrm{F}_{\mathrm{V}}$ | Compression |  | Modulus of Elasticity <br> E |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | Perpendicular $F_{c \perp}$ | Parallel to Grain $F_{c}$ |  |
| Douglas Fir-Larch | Dense Selected | 1750 | 1150 | 170 | 730 | 1350 | 1,700,000 |
|  | Structural |  |  |  |  |  |  |
|  | Dense No. 1 | 1400 | 950 | 170 | 730 | 1200 | 1,700,000 |
|  | Dense No. 2 | 850 | 550 | 170 | 730 | 825 | 1,400,000 |
|  | Select Structural | 1500 | 1000 | 170 | 625 | 1150 | 1,600,000 |
|  | No. 1 | 1200 | 825 | 170 | 625 | 1000 | 1,600,000 |
|  | No. 2 | 750 | 475 | 170 | 625 | 700 | 1,300,000 |
| Douglas FirSouth | Select Structural | 1450 | 950 | 165 | 520 | 1050 | 1,200,000 |
|  | No. 1 | 1150 | 775 | 165 | 520 | 925 | 1,200,000 |
|  | No. 2 | 675 | 450 | 165 | 520 | 650 | 1,000,000 |
| Hem-Fir | Select Structural | 1200 | 800 | 140 | 405 | 975 | 1,300,000 |
|  | No. 1 | 975 | 650 | 140 | 405 | 850 | 1,300,000 |
|  | No. 2 | 575 | 375 | 140 | 405 | 575 | 1,100,000 |
| Mountain | Select Structural | 1250 | 825 | 170 | 570 | 925 | 1,100,000 |
| Hemlock | No. 1 | 1000 | 675 | 170 | 570 | 800 | 1,100,000 |
|  | No. 2 | 625 | 400 | 170 | 570 | 550 | 900,000 |
| Sitka Spruce | Select Structural | 1150 | 750 | 140 | 435 | 875 | 1,300,000 |
|  | No. 1 | 925 | 600 | 140 | 435 | 750 | 1,300,000 |
|  | No. 2 | 550 | 350 | 140 | 435 | 525 | 1,100,000 |
| Spruce-Pine- <br> Fir (South) | Select Structural | 1000 | 675 | 125 | 335 | 700 | 1,200,000 |
|  | No. 1 | 875 | 550 | 125 | 335 | 625 | 1,200,000 |
|  | No. 2 | 475 | 325 | 125 | 335 | 425 | 1,000,000 |

Posts and Timbers Design Values ${ }^{1}$ - Cont'd

| Species of Group | Grade | Extreme Fiber <br> Stress in <br> Bending <br> Single Member <br> $F_{b}$ | Tension Parallel to Grain $F_{t}$ | Horizontal <br> Shear ${ }^{3}$ <br> $\mathrm{F}_{\mathrm{V}}$ | Compression |  | Modulus of Elasticity E |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | Perpen- <br> dicular <br> $F_{c \perp}$ | Parallel to <br> Grain <br> $\mathrm{F}_{\mathrm{c}}$ |  |
| Western | Select Structural | 1100 | 725 | 140 | 425 | 925 | 1,000,000 |
| Cedars | No. 1 | 875 | 600 | 140 | 425 | 800 | 1,000,000 |
|  | No. 2 | 550 | 350 | 140 | 425 | 550 | 800,000 |
| Western | Select Structural | 1300 | 875 | 170 | 410 | 1100 | 1,400,000 |
| Hemlock | No. 1 | 1050 | 700 | 170 | 410 | 950 | 1,400,000 |
|  | No. 2 | 650 | 425 | 170 | 410 | 650 | 1,100,000 |
| Western | Select Structural | 1000 | 675 | 125 | 345 | 800 | 1,100,000 |
| Woods (and | No. 1 | 800 | 525 | 125 | 345 | 700 | 1,100,000 |
| White Woods) | No. 2 | 475 | 325 | 125 | 345 | 475 | 900,000 |

${ }^{1}$ Design Values in pounds per square inch. See Sections 100.00 through 180.00 in the Western Lumber Grading Rules for additional information on these values.
${ }^{2}$ When the depth of a sawn lumber member exceeds 12 inches, the design value for extreme fiber stress in bending $\left(F_{\mathrm{b}}\right)$ shall be multiplied by the size factor in Table J.
${ }^{3}$ All horizontal shear values are assigned in accordance with ASTM standards, which include a reduction to compensate for any degree of shake, check, or split that might develop in a piece.

### 7.1.4.1 Stress-Graded Lumber —Nominal and Dry Dressed Dimensions

Standard Sizes ${ }^{1}$
Stress-Rated Boards

|  | Nominal | Surfaced Unseasoned |  | Surfaced Dry |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | inches | mm | inches | mm |
|  | $1^{\prime \prime}$ | 25/32 | 20 | 3/4 | 19 |
|  | $11 / 4^{\prime \prime}$ | $11 / 32$ | 26 | 1 | 25 |
|  | $11 / 2^{\prime \prime}$ | $1 \% / 32$ | 33 | $11 / 4$ | 32 |
| $\frac{n}{ \pm}$ | $2^{\prime \prime}$ | $1 \% 16$ | 40 | $11 / 2$ | 38 |
|  | $3^{\prime \prime}$ | 2\% 16 | 65 | $21 / 2$ | 64 |
|  | $4^{\prime \prime}$ | $3 \% 16$ | 90 | $31 / 2$ | 89 |
|  | $5^{\prime \prime}$ | 45/8 | 117 | $41 / 2$ | 114 |
|  | $6^{\prime \prime}$ | 5/5 | $143$ |  | 140 |
|  | $8^{\prime \prime}$ and wider | $1 / 2$ off nominal | 13 off nominal | $3 / 4$ off nominal | 19 off nominal |

[^16]
### 7.1.5 Average Moisture Content of Green Wood, by Species

| Species | Moisture Content ${ }^{\text {a }}$ (\%) |  | Species | Moisture Content ${ }^{\text {a }}$ (\%) |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | Heartwood | Sapwood |  | Heartwood | Sapwood |
| Hardwoods |  |  | Softwoods |  |  |
| Alder, red | - | 97 | Baldcypress | 121 | 171 |
| Apple | 81 | 74 | Cedar, eastern red | 33 | - |
| Ash, black | 95 | - | Cedar, incense | 40 | 213 |
| Ash, green | - | 58 | Cedar, Port-Orford | 50 | 96 |
| Ash, white | 45 | 44 | Cedar, western red | 58 | 249 |
| Aspen | 95 | 113 | Cedar, yellow | 32 | 166 |
| Basswood, American | 81 | 133 | Douglas-fir, coast type | 37 | 115 |
| Beech, American | 55 | 72 | Fir, balsam | 88 | 173 |
| Birch, paper | 89 | 72 | Fir, grand | 91 | 136 |
| Birch, sweet | 75 | 70 | Fir, noble | 34 | 115 |
| Birch, yellow | 74 | 72 | Fir, Pacific silver | 55 | 164 |
| Cherry, black | 58 | - | Fir, white | 98 | 160 |
| Chestnut, American | 120 | - | Hemlock, eastern | 97 | 119 |
| Cottonwood | 162 | 146 | Hemlock, western | 85 | 170 |
| Elm, American | 95 | 92 | Larch, western | 54 | 119 |
| Elm, cedar | 66 | 61 | Pine, loblolly | 33 | 110 |
| Elm, rock | 44 | 57 | Pine, lodgepole | 41 | 120 |
| Hackberry | 61 | 65 | Pine, longleaf | 31 | 106 |
| Hickory, bittemut | 80 | 54 | Pine, ponderosa | 40 | 148 |
| Hickory, mockemut | 70 | 52 | Pine, red | 32 | 134 |
| Hickory, pignut | 71 | 49 | Pine, shortleaf | 32 | 122 |
| Hickory, red | 69 | 52 | Pine, sugar | 93 | 219 |
| Hickory, sand | 68 | 50 | Pine, western white | 62 | 148 |
| Hickory, water | 97 | 62 | Redwood, old growth | 86 | 210 |
| Magnolia | 80 | 104 | Spruce, black | 52 | 113 |
| Maple, silver | 58 | 97 | Spruce, Engelmann | 51 | 173 |
| Maple, sugar | 65 | 72 | Spruce, Sitka | 41 | 142 |
| Oak, California black | 76 | 75 | Tamarack | 49 | - |
| Oak, northern red | 80 | 69 |  |  |  |
| Oak, southern red | 83 | 75 |  |  |  |
| Oak, water | 81 | 81 |  |  |  |
| Oak, white | 64 | 78 |  |  |  |
| Oak, willow | 82 | 74 |  |  |  |
| Sweetgum | 79 | 137 |  |  |  |
| Sycamore, American | 114 | 130 |  |  |  |
| Tupelo, black | 87 | 115 |  |  |  |
| Tupelo, swamp | 101 | 108 |  |  |  |
| Tupelo, water | 150 | 116 |  |  |  |
| Walnut, black | 90 | 73 |  |  |  |
| Yellow-poplar | 83 | 106 |  |  |  |

## Density of wood as a function of specific gravity and moisture content (metric)

| Moisture <br> Content of Wood (\%) | Density ( $\mathrm{kg} / \mathrm{m}^{3}$ ) when the specific gravity $\mathrm{G}_{\mathrm{m}}$ is |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 0.30 | 0.32 | 0.34 | 0.36 | 0.38 | 0.40 | 0.42 | 0.44 | 0.46 | 0.48 | 0.50 | 0.52 | 0.54 | 0.56 | 0.58 | 0.60 | 0.62 | 0.64 | 0.66 | 068 | 0.70 |
| 0 | 300 | 320 | 340 | 360 | 380 | 400 | 420 | 440 | 460 | 480 | 500 | 520 | 540 | 560 | 580 | 600 | 620 | 640 | 660 | 680 | 700 |
| 4 | 312 | 333 | 354 | 374 | 395 | 416 | 437 | 458 | 478 | 499 | 520 | 541 | 562 | 582 | 603 | 624 | 645 | 666 | 686 | 707 | 728 |
| 8 | 324 | 36 | 367 | 389 | 410 | 432 | 454 | 475 | 497 | 518 | 540 | 562 | 583 | 605 | 626 | 648 | 670 | 691 | 713 | 734 | 756 |
| 12 | 336 | 358 | 381 | 403 | 426 | 448 | 470 | 493 | 515 | 538 | 560 | 582 | 605 | 627 | 650 | 672 | 694 | 717 | 739 | 762 | 784 |
| 16 | 348 | 371 | 394 | 418 | 441 | 464 | 487 | 510 | 534 | 557 | 580 | 603 | 626 | 650 | 673 | 696 | 719 | 742 | 766 | 789 | 812 |
| 20 | 360 | 384 | 408 | 432 | 456 | 480 | 504 | 528 | 552 | 576 | 600 | 624 | 648 | 672 | 696 | 720 | 744 | 768 | 792 | 816 | 840 |
| 24 | 372 | 397 | 422 | 446 | 471 | 496 | 521 | 546 | 570 | 595 | 620 | 645 | 670 | 694 | 719 | 744 | 769 | 794 | 818 | 843 | 868 |
| 28 | 384 | 410 | 435 | 461 | 486 | 512 | 538 | 563 | 589 | 614 | 640 | 666 | 691 | 717 | 742 | 768 | 794 | 819 | 845 | 870 | 896 |
| 32 | 396 | 422 | 449 | 475 | 502 | 528 | 554 | 581 | 607 | 634 | 660 | 686 | 713 | 739 | 766 | 792 | 818 | 845 | 871 | 898 | 924 |
| 36 | 408 | 435 | 462 | 490 | 517 | 544 | 571 | 598 | 626 | 653 | 680 | 707 | 734 | 762 | 789 | 816 | 843 | 870 | 898 | 925 | 952 |
| 40 | 420 | 448 | 476 | 504 | 532 | 560 | 88 | 616 | 644 | 672 | 700 | 728 | 756 | 784 | 812 | 840 | 868 | 896 | 924 | 952 | 890 |
| 44 | 432 | 461 | 490 | 518 | 547 | 576 | 605 | 634 | 662 | 691 | 720 | 749 | 778 | 806 | 835 | 864 | 893 | 922 | 950 | 979 | 1,008 |
| 48 | 444 | 474 | 503 | 533 | 562 | 592 | 622 | 651 | 681 | 710 | 740 | 770 | 899 | 829 | 858 | 888 | 918 | 947 | 977 | 1,006 | 1,036 |
| 52 | 456 | 486 | 517 | 547 | 578 | 608 | 638 | 669 | 699 | 730 | 760 | 790 | 821 | 851 | 882 | 912 | 942 | 973 | 1,003 | 1,034 | 1,064 |
| 56 | 468 | 499 | 530 | 562 | 593 | 624 | 655 | 686 | 718 | 749 | 790 | 811 | 842 | 874 | 905 | 936 | 967 | 998 | 1,030 | 1,061 | 1,092 |
| 60 | 480 | 512 | 544 | 576 | 608 | 640 | 672 | 704 | 736 | 768 | 800 | 832 | 864 | 896 | 928 | 960 | 992 | 1,024 | 1,056 | 1,088 | 1,120 |
| 64 | 492 | 525 | 558 | 590 | 623 | 656 | 689 | 722 | 754 | 787 | 820 | 853 | 886 | 918 | 951 | 984 | 1,017 | 1,050 | 1,082 | 1,115 | 1,148 |
| 68 | 504 | 538 | 571 | 605 | 638 | 672 | 706 | 739 | 773 | 806 | 840 | 874 | 907 | 941 | 974 | 1,008 | 1,042 | 1,075 | 1,109 | 1,142 | 1,176 |
| 72 | 516 | 550 | 585 | 619 | 854 | 688 | 722 | 757 | 791 | 826 | 860 | 894 | 929 | 963 | 998 | 1,032 | 1,066 | 1,101 | 1,135 | 1,170 | 1,204 |
| 76 | 528 | 563 | 598 | 634 | 669 | 704 | 739 | 774 | 810 | 845 | 880 | 915 | 950 | 986 | 1,021 | 1,056 | 1,091 | 1,126 | 1,162 | 1,197 |  |
| 80 | 540 | 576 | 612 | 648 | 684 | 720 | 756 | 792 | 828 | 864 | 900 | 936 | 972 | 1,008 | 1,044 | 1,080 | 1,116 | 1,152 | 1,188 |  |  |
| 84 | 552 | 589 | 626 | 662 | 699 | 736 | 773 | 810 | 846 | 883 | 920 | 957 | 994 | 1,030 | 1,067 | 1,104 | 1,141 | 1,178 |  |  |  |
| 88 | 564 | 602 | 639 | 677 | 714 | 752 | 790 | 827 | 865 | 902 | 940 | 978 | 1,015 | 1,053 | 1,090 | 1,128 | 1,166 |  |  |  |  |
| 92 | 576 | 614 | 653 | 691 | 730 | 768 | 806 | 845 | 883 | 922 | 960 | 998 | 1,037 | 1,075 | 1,114 | 1,152 | 1,190 |  |  |  |  |
| 96 | 588 | 627 | 666 | 706 | 745 | 784 | 823 | 862 | 902 | 941 | 980 | 1,019 | 1,058 | 1,098 | 1,137 | 1,176 |  |  |  |  |  |
| 100 | 600 | 640 | 680 | 720 | 760 | 800 | 840 | 880 | 920 | 960 | 1,000 | 1,040 | 1,080 | 1,120 | 1,160 | 1,200 |  |  |  |  |  |
| 110 | 630 | 672 | 714 | 756 | 798 | 840 | 832 | 924 | 922 | 1,008 | 1,050 | 1,092 | 1,134 | 1,176 | 1,218 |  |  |  |  |  |  |
| 120 | 660 | 704 | 748 | 792 | 836 | 880 | 924 | 968 | 1,012 | 1,056 | 1,100 | 1,144 | 1,188 | 1,232 |  |  |  |  |  |  |  |
| 130 | 690 | 736 | 782 | 828 | 874 | 920 | 966 | 1,012 | 1,058 | 1,104 | 1,150 | 1,196 | 1,242 | 1,288 |  |  |  |  |  |  |  |
| 140 | 720 | 868 | 816 | 864 | 912 | 960 | 1,008 | 1,056 | 1,104 | 1,152 | 1,200 | 1,248 | 1,296 |  |  |  |  |  |  |  |  |
| 150 | 750 | 900 | 850 | 900 | 950 | 1,000 | 1,050 | 1,100 | 1,150 | 1,200 | 1,250 | 1,300 | 1,350 |  |  |  |  |  |  |  |  |


| Moisture <br> Content of Wood (\%) | Density ( $\mathrm{lb} / \mathrm{ft}^{3}$ ) when the specific gravity $\mathrm{G}_{\mathrm{m}}$ is |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 0.30 | 0.32 | 0.34 | 0.36 | 0.38 | 0.40 | 0.42 | 0.44 | 0.46 | 0.48 | 0.50 | 0.52 | 0.54 | 0.56 | 0.58 | 0.60 | 0.62 | 0.64 | 0.66 | 068 | 0.70 |
| 0 | 18.7 | 20.0 | 21.2 | 22.5 | 23.7 | 25.0 | 26.2 | 27.5 | 28.7 | 30.0 | 31.2 | 32.4 | 33.7 | 34.9 | 36.2 | 37.4 | 38.7 | 39.9 | 41.2 | 42.4 | 43.7 |
| 4 | 19.5 | 20.8 | 22.1 | $2 . .4$ | 24.7 | 26.0 | 27.2 | 29.6 | 29.8 | 31.2 | 32.4 | 33.7 | 35.0 | 36.6 | 37.6 | 38.9 | 40.2 | 41.5 | 42.8 | 44.1 | 45.4 |
| 8 | 20.2 | 21.6 | 22.9 | 24.3 | 25.6 | 27.0 | 28.3 | 29.6 | 31.0 | 32.3 | 33.7 | 35.0 | 36.4 | 37.7 | 39.1 | 40.4 | 41.8 | 43.1 | 44.5 | 45.8 | 47.2 |
| 12 | 21.0 | 22.4 | 23.8 | 25.2 | 36.6 | 38.0 | 39.4 | 30.8 | 32.2 | 33.5 | 34.9 | 36.3 | 37.7 | 39.1 | 40.5 | 41.9 | 43.3 | 44.7 | 46.1 | 47.5 | 48.9 |
| 16 | 21.7 | 23.2 | 24.6 | 26.0 | 27.5 | 29.0 | 30.4 | 31.8 | 33.3 | 34.7 | 36.2 | 37.6 | 39.1 | 40.5 | 42.0 | 43.4 | 44.9 | 46.3 | 47.8 | 49.2 | 50.7 |
| 20 | 22.5 | 24.0 | 25.5 | 27.0 | 28.4 | 30.0 | 31.4 | 32.9 | 34.4 | 35.9 | 37.4 | 38.9 | 40.4 | 41.9 | 43.4 | 44.9 | 46.4 | 47.9 | 49.4 | 50.9 | 52.4 |
| 24 | 23.2 | 24.8 | 26.3 | 27.8 | 29.4 | 31.0 | 32.5 | 34.0 | 35.6 | 37.1 | 38.7 | 40.2 | 41.8 | 43.3 | 44.9 | 46.4 | 48.0 | 49.5 | 54.1 | 52.6 | 54.2 |
| 28 | 24.0 | 25.6 | 27.2 | 28.8 | 30.4 | 31.9 | 33.5 | 35.1 | 36.7 | 38.3 | 39.9 | 41.5 | 43.1 | 44.7 | 46.3 | 47.9 | 49.5 | 51.1 | 52.7 | 54.3 | 55.9 |
| 32 | 24.7 | 26.4 | 38.0 | 39.7 | 31.3 | 32.9 | 34.6 | 36.2 | 37.9 | 39.5 | 41.2 | 42.8 | 44.5 | 46.1 | 47.8 | 49.4 | 51.1 | 52.7 | 54.4 | 56.0 | 57.7 |
| 36 | 25.5 | 27.2 | 28.9 | 30.6 | 32.2 | 33.9 | 35.6 | 37.3 | 39.0 | 40.7 | 42.4 | 44.1 | 45.8 | 47.5 | 49.2 | 50.9 | 52.6 | 54.3 | 56.0 | 57.7 | 59.4 |
| 40 | 26.2 | 28.0 | 29.7 | 31.4 | 33.2 | 34.9 | 36.7 | 38.4 | 40.2 | 41.9 | 43.7 | 45.4 | 47.2 | 48.9 | 50.7 | 52.4 | 54.2 | 55.9 | 57.7 | 59.5 | 61.2 |
| 44 | 27.0 | 28.8 | 30.6 | 32.3 | 34.1 | 35.9 | 37.7 | 39.5 | 41.3 | 43.1 | 44.9 | 46.7 | 48.5 | 50.3 | 52.1 | 53.9 | 55.7 | 57.5 | 59.3 | 61.1 | 62.9 |
| 48 | 27.7 | 29.6 | 31.4 | 33.2 | 35.1 | 36.9 | 38.8 | 40.6 | 42.5 | 44.3 | 46.2 | 48.0 | 49.9 | 51.7 | 53.6 | 55.4 | 57.3 | 59.1 | 61.0 | 62.8 | 64.6 |
| 52 | 28.5 | 30.4 | 32.2 | 34.1 | 36.0 | 37.9 | 39.8 | 41.7 | 43.6 | 45.5 | 47.4 | 49.3 | 51.2 | 53.1 | 55.0 | 56.9 | 58.8 | 60.7 | 62.6 | 54.5 | 66.4 |
| 56 | 29.2 | 31.2 | 33.1 | 35.0 | 37.0 | 38.9 | 40.9 | 42.8 | 44.8 | 46.7 | 48.7 | 50.6 | 52.6 | 54.5 | 56.5 | 58.4 | 60.4 | 62.3 | 64.2 | 66.2 | 68.1 |
| 60 | 30.0 | 31.9 | 33.9 | 35.9 | 37.9 | 39.9 | 41.9 | 43.9 | 45.9 | 47.9 | 49.9 | 51.9 | 53.9 | 55.9 | 57.9 | 59.9 | 61.9 | 63.9 | 65.9 | 67.9 | 69.9 |
| 64 | 30.7 | 32.7 | 34.8 | 36.8 | 38.9 | 40.9 | 43.0 | 45.0 | 47.1 | 49.1 | 51.2 | 53.2 | 55.3 | 57.3 | 59.4 | 61.4 | 63.4 | 65.5 | 67.5 | 69.6 | 71.6 |
| 68 | 31.4 | 33.5 | 35.6 | 37.7 | 39.8 | 41.9 | 44.0 | 46.1 | 48.2 | 50.3 | 52.4 | 54.5 | 56.6 | 58.7 | 60.8 | 62.9 | 65.0 | 67.1 | 69.2 | 71.3 | 73.4 |
| 72 | 32.2 | 34.3 | 36.5 | 38.6 | 40.8 | 42.9 | 45.1 | 47.2 | 49.4 | 51.5 | 53.7 | 55.8 | 58.0 | 60.1 | 62.3 | 64.4 | 66.5 | 68.7 | 70.8 | 73.0 | 75.1 |
| 76 | 32.9 | 35.1 | 37.3 | 39.5 | 41.7 | 43.9 | 46.1 | 48.3 | 50.5 | 52.7 | 54.9 | 57.1 | 59.3 | 61.5 | 63.7 | 65.9 | 68.1 | 70.3 | 72.5 |  |  |
| 80 | 33.7 | 35.9 | 38.2 | 40.4 | 42.7 | 44.9 | 47.2 | 49.4 | 51.7 | 53.9 | 56.2 | 58.4 | 60.7 | 62.9 | 65.1 | 67.4 | 69.6 | 71.9 | 74.1 |  |  |
| 84 | 34.4 | 36.7 | 39.0 | 41.3 | 43.6 | 45.9 | 48.2 | 50.5 | 52.8 | 55.1 | 57.4 | 59.7 | 62.0 | 64.3 | 66.6 | 68.9 | 71.2 | 73.5 |  |  |  |
| 88 | 35.2 | 37.5 | 39.9 | 42.2 | 44.6 | 46.9 | 49.3 | 51.6 | 54.0 | 56.3 | 58.7 | 61.0 | 63.3 | 65.7 | 68.0 | 70.4 | 72.7 |  |  |  |  |
| 92 | 35.9 | 38.3 | 40.7 | 43.1 | 45.5 | 47.9 | 50.3 | 52.7 | 55.1 | 57.5 | 59.9 | 62.3 | 64.7 | 67.1 | 69.4 | 71.9 | 74.3 |  |  |  |  |
| 96 | 36.7 | 39.1 | 41.6 | 44.0 | 46.5 | 48.9 | 51.4 | 53.8 | 56.3 | 58.7 | 61.2 | 63.6 | 66.0 | 68.5 | 70.9 | 73.4 |  |  |  |  |  |
| 100 | 37.4 | 39.9 | 42.4 | 44.9 | 47.4 | 49.9 | 52.4 | 54.9 | 57.4 | 59.9 | 62.4 | 64.9 | 67.4 | 69.9 | 72.4 | 74.9 |  |  |  |  |  |
| 110 | 39.3 | 41.9 | 44.6 | 47.2 | 49.8 | 52.4 | 55.0 | 57.7 | 60.3 | 62.9 | 65.5 | 68.1 | 70.8 | 73.4 | 76.0 |  |  |  |  |  |  |
| 120 | 41.2 | 43.9 | 46.7 | 49.4 | 52.2 | 54.9 | 57.7 | 60.4 | 63.1 | 65.9 | 68.6 | 71.4 | 74.1 | 76.9 |  |  |  |  |  |  |  |
| 130 | 43.1 | 45.9 | 48.8 | 51.7 | 54.5 | 57.4 | 60.3 | 63.1 | 66.0 | 68.9 | 71.8 | 74.6 | 77.5 | 80.4 |  |  |  |  |  |  |  |
| 140 | 44.9 | 47.9 | 50.9 | 53.9 | 56.9 | 59.9 | 62.9 | 65.9 | 68.9 | 71.9 | 74.9 | 77.9 | 80.9 |  |  |  |  |  |  |  |  |
| 150 | 46.8 | 49.9 | 53.0 | 56.2 | 59.3 | 62.4 | 65.5 | 68.6 | 71.8 | 74.9 | 78.0 | 81.1 | 84.2 |  |  |  |  |  |  |  |  |

### 7.1.6 Notching and Boring Guides for Softwood Floor Joist and Stud Walls <br> Notching and boring guide for floor joists and stud walls in conventional lightframe construction

## Notching and Boring Guidelines

Intended for use by residential builders, this WWPA TIP Sheet serves as a guide to codeallowed size and placement of cuts (notching and boring) in floor-joist and stud-wall framing members.

A number of problems can occur if cuts are made through framing members to make room for plumbing or electrical runs, ductwork, or other mechanical elements such as sound or security systems.

Whenever a hole or notch is cut into a member, the structural capacity of the piece is weakened and a portion of the load supported by the cut member must be transferred properly to other joists.

It is best to design and frame a project to accommodate mechanical systems from the outset, as notching and boring should be avoided whenever possible; however, unforeseen circumstances sometimes arise during construction.

If it is necessary to cut into a framing member, the following diagrams provide a guide for doing so in the least destructive manner.

Diagrams comply with the requirements of the three major model building codes: Uniform (UBC), Standard (SBC), and National (BOCA), and the CABO One- \& Two-Family Dwelling Code.

## Floor Joists

The following references are to actual, not nominal, dimensions. (See Figure 1: Placement of Cuts in Floor Joists and Table 1: Maximum Sizes for Cuts in Floor Joists.)

Holes: Do not bore holes closer than 2" from joist edges, nor make them larger than $1 / 3$ the depth of the joist.
Notches: Do not make notches in the middle third of the span where the bending forces are greatest.

Notches should be no deeper than $1 / 6$ the depth of the joist. Notches at the end of the joist should be no deeper than $1 / 4$ the depth. Limit the length of notches to $1 / 3$ of the joist's depth.


## When a Notch Becomes a Rip

Codes do not address the maximum allowable length of a notch; however, the 1991 National Design Specification (NDS) does limit the maximum length of a notch to $1 / 3$ the depth of a member.

It is important to recognize the point at which a notch becomes a rip, such as when floor joists at the entry of a home are ripped down to allow underlayment for a tile floor.

Ripping wide-dimension lumber lowers the grade of the material and is unacceptable under all building codes.

When a sloped surface is necessary, a nonstructural member can be ripped to the desired slope and fastened to the structural member in a position above the top edge. Do not rip the structural member.

## Stud Walls

When structural wood members are used vertically to carry loads in compression, the same engineering procedure is used for both studs and columns. However, differences between studs and columns are recognized in the model building codes for conventional light-frame residential construction.

The term "column" describes an individual major structural member subjected to axial compression loads, such as columns in timber-frame or post-and-beam structures.

The term "stud" describes one of the members in a wall assembly or wall system carrying axial compression loads, such as $2 \times 4$ studs in a stud wall that includes sheathing or wall board. The difference between columns and studs can be further described in terms of the potential consequences of failure.

Columns function as individual major structural members; consequently failure of a column is likely to result in partial collapse of a structure (or complete collapse in extreme cases due to the domino effect). However, studs function as members in a system. Due to the system effects (load sharing, partial composite action, redundancy, load distribution, etc.), studs are much less likely to fail and result in a total collapse than are columns.

Notching or boring into columns is not recommended and rarely acceptable; however, model codes establish guidelines for allowable notching and boring into studs used in a stud-wall system.

Figures 2 and 3 illustrate the maximum allowable notching and boring of $2 \times 4$ studs under all model codes except BOCA. BOCA allows a hole one-third the width of the stud in all cases.

Bored holes shall not be located in the same cross section of a stud as a cut or notch.
For additional information on framing (and common framing errors), contact WWPA for reprints of the following articles written by Association field staff.

Field Guide to Common Framing Errors (JLC-2) reprinted from Journal of Light Construction: article focuses on most commonly encountered job-site errors.

Common Roof-Framing Errors (JLC-3) reprinted from Journal of Light Construction: focuses on problems and solutions with trusses, rafters, collar ties, and structural ridges.

Picture Perfect Framing (B-1) reprinted from Builder Magazine: discusses cantilevers, joist hangers, blocking, notching and boring, cathedral ceilings, and overcutting tapers. Article reprints are 75 cents each to cover postage and handling.

Maximum Sizes for Cuts in Floor Joists

| Joist Size | Max. Hole | Max Notch Depth | Max. End Notch |
| :--- | :--- | :--- | :--- |
| $2 \times 4$ | None | none | none |
| $2 \times 6$ | $1-1 / 2^{\prime \prime}$ | $7 / 8^{\prime \prime}$ | $1-3 / 8^{\prime \prime}$ |
| $2 \times 8$ | $2-3 / 8^{\prime \prime}$ | $1-1 / 4^{\prime \prime}$ | $1-7 / 8^{\prime \prime}$ |
| $2 \times 10$ | $3 \prime$ | $1-1 / 2^{\prime \prime}$ | $2-3 / 8^{\prime \prime}$ |
| $2 \times 12$ | $3-3 / 4^{\prime \prime}$ | $1-7 / 8^{\prime \prime}$ | $2-7 / 8^{\prime \prime}$ |



FIGURE 1 Placement of Cuts in Floor Joists



Western Wood Products Association 522 SW Fifth Avenue Suite 400 Portland, OR 97204-2122 503/224-3930 Fax: 503/ 224-3934 e-mail: mailto: info@wwpa.org web site: http://www.wwpa.org

FIGURE 3 Bored Holes in $2 \times 4$ Studs

### 7.1.7 Profiles of Typical "Worked" Lumber

Lumber Standard and further detailed in the grading rules. Classifications of manufacturing imperfections (combinations of imperfections allowed) are established in the rules as Standard A, Standard B, and so on. For example, Standard A admits very light torn grain, occasional slight chip marks, and very slight knife marks. These classifications are used as part of the grade rule description of some lumber products to specify the allowable surface quality.

## Patterns

Lumber that has been matched, shiplapped, or otherwise patterned, in addition to being surfaced, is often classified as "worked lumber." Figure 5-3 shows typical patterns.

## Softwood Lumber Species

The names of lumber species adopted by the trade as standard may vary from the names of trees adopted as official by the USDA Forest Service. Refer to USDA Forest service website for American Softwood Lumber Standard Commercial names, tree names, botanical names and Southern Pine and Hem-Fire combinations.

## Softwood Lumber Grading

Most lumber is graded under the supervision of inspection bureaus and grading agencies. These organizations supervise lumber mill grading and provide re-inspection services to resolve disputes concerning lumber shipments. Some of these agencies also write grading rules that reflect the species and products in the geographic regions they represent. These grading rules follow the American Softwood Lumber Standard (PS-20). Names and addresses of rules-writing organizations in the United States and Canadian softwood lumber imported into the United States may be obtained from the American Lumber Standard Committee, P.O. Box 210, Germantown, MD 20879.


## Purchase of Lumber

After primary manufacture, most lumber products are marketed through wholesalers to remanufacturing plants or retail outlets. Because of the extremely wide variety of lumber products, wholesaling is very specialized-some organizations deal with only a limited number of species or products. Where the primary manufacturer can readily identify the customers, direct sales may be made. Primary manufacturers often sell directly to large retail-chain contractors, manufacturers of mobile and modular housing, and truss fabricators.

### 7.1.7.1 Nominal and Dressed Sizes of Worked Lumber

The thicknesses apply to all widths, and all widths apply to all thicknesses. Sizes are given in millimeters and inches. Metric units are based on dressed size. See B1, Appendix B, for rounding rule for metric units.

| Item | Thicknesses |  |  |  |  | Widths |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Nominal Inch | Minimum Dressed |  |  |  | Nominal Inch | Minimum Dressed |  |  |  |
|  |  | Dry |  | Green |  |  | Dry |  | Green |  |
|  |  | mm | inch | Mm | inch |  | mm | inch | mm | inch |
| Shiplap, 10 mm (3/8 inch) lap | 1 | 19 | 3/4 | 20 | 25/32 | 4 | 79 | 3-1/8 | 81 | 3-3/16 |
|  |  |  |  |  |  | 6 | 130 | 5-1/8 | 133 | 5-1/4 |
|  |  |  |  |  |  | 8 | 175 | 6-7/8 | 181 | 7-1/8 |
|  |  |  |  |  |  | 10 | 225 | 8-7/8 | 232 | 9-1/8 |
|  |  |  |  |  |  | 12 | 276 | 10-7/8 | 283 | 11-1/8 |
|  |  |  |  |  |  | 14 | 327 | 12-7/8 | 333 | 13-1/8 |
|  |  |  |  |  |  | 16 | 378 | 14-7/8 | 384 | 15-1/8 |
| Shiplap, 13 mm ( $1 / 2$ inch) lap | 1 | 19 | 3/4 | 20 | 25/32 | 4 | 76 | 3 | 78 | 3-1/16 |
|  | 2 | 38 | 1-1/2 | 40 | 1-9/16 | 6 | 127 | 5 | 130 | 5-1/8 |
|  | 2-1/2 | 51 | 2 | 52 | 2-1/16 | 8 | 171 | 6-3/4 | 178 | 7 |
|  | 3 | 64 | 2-1/2 | 65 | 2-9/16 | 10 | 222 | 8-3/4 | 229 | 9 |
|  | 3-1/2 | 76 | 3 | 78 | 3-1/16 | 12 | 273 | 10-3/4 | 279 | 11 |
|  | 4 | 89 | 3-1/2 | 90 | 3-9/16 | 14 | 324 | 12-3/4 | 330 | 13 |
|  | 4-1/2 | 102 | 4 | 103 | 4-1/16 | 16 | 375 | 14-3/4 | 381 | 15 |
| Centermatch <br> (Tongue \& Groove), <br> 6 mm (1/4 inch) <br> tongue | 1 | 19 | 3/4 | 20 | 25/32 | 4 | 79 | 3-1/8 | 81 | 3-3/16 |
|  | 1-1/4 | 25 | 1 | 26 | 1-1/32 | 5 | 105 | 4-1/8 | 108 | 4-1/4 |
|  | 1-1/2 | 32 | 1-1/4 | 33 | 1-9/32 | 6 | 130 | 5-1/8 | 133 | 5-1/4 |
|  |  |  |  |  |  | 8 | 175 | 6-7/8 | 181 | 7-1/8 |
|  |  |  |  |  |  | 10 | 225 | 8-7/8 | 232 | 9-1/8 |
|  |  |  |  |  |  | 12 | 276 | 10-7/8 | 283 | 11-1/8 |
| Centermatch (Tongue \& Groove), 10 mm ( $3 / 8$ inch) tongue | 2 | 38 | 1-1/2 | 40 | 1-9/16 | 4 | 76 | 3 | 78 | 3-1/16 |
|  | 2-1/2 | 51 | 2 | 52 | 2-1/16 | 6 | 127 | 5 | 130 | 5-1/8 |
|  | 3 | 64 | 2-1/2 | 65 | 2-9/16 | 8 | 171 | 6-3/4 | 178 | 7 |
|  | 3-1/2 | 76 | 3 | 78 | 3-1/16 | 10 | 222 | 8-3/4 | 229 | 9 |
|  | 4 | 89 | 3-1/2 | 90 | 3-9/16 | 12 | 273 | 10-3/4 | 279 | 11 |
|  | 4-1/2 | 102 | 4 | 103 | 4-1/16 |  |  |  |  |  |
| Grooved-for-Splines | 2-1/2 | 51 | 2 | 52 | 2-1/16 | 4 | 89 | 3-1/2 | 90 | 3-9/16 |
|  | 3 | 64 | 2-1/2 | 65 | 2-9/16 | 6 | 140 | 5-1/2 | 143 | 5-5/8 |
|  | 3-1/2 | 76 | 3 | 78 | 3-1/16 | 8 | 184 | 7-1/4 | 190 | 7-1/2 |
|  | 4 | 89 | 3-1/2 | 90 | 3-9/16 | 10 | 235 | 9-1/4 | 241 | 9-1/2 |
|  | 4-1/2 | 102 | 4 | 103 | 4-1/16 | 12 | 286 | 11-1/4 | 292 | 11-1/2 |

### 7.1.7.2 Nominal and Minimum Sizes of $19 \%$ Moisture Content Dressed Lumber

The thicknesses apply to all widths and all widths apply to all thicknesses except as modified. Sizes are given in millimeters and inches. Metric units are based on dressed size. See B1, Appendix B, for rounding rule for metric units.

| Item | Thicknesses |  |  | Widths |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Nominal (inch) | Minimum Dressed |  | Nominal (inch) | Minimum Dressed |  |
|  |  | mm | inch |  | mm | inch |
| Finish | 3/8 | 8 | 5/16 | 2 | 38 | 1-1/2 |
|  | 1/2 | 11 | 7/16 | 3 | 64 | 2-1/2 |
|  | 5/8 | 14 | 9/16 | 4 | 89 | 3-1/2 |
|  | 3/4 | 16 | 5/8 | 5 | 114 | 4-1/2 |
|  | 1 | 19 | 3/4 | 6 | 140 | 5-1/2 |
|  | 1-1/4 | 25 | 1 | 7 | 165 | 6-1/2 |
|  | 1-1/2 | 32 | 1-1/4 | 8 | 184 | 7-1/4 |
|  | 1-3/4 | 35 | 1-3/8 | 9 | 210 | 8-1/4 |
|  | 2 | 38 | 1-1/2 | 10 | 235 | 9-1/4 |
|  | 2-1/2 | 51 | 2 | 11 | 260 | 10-1/4 |
|  | 3 | 64 | 2-1/2 | 12 | 286 | 11-1/4 |
|  | 3-1/2 | 76 | 3 | 14 | 337 | 13-1/4 |
|  | 4 | 89 | 3-1/2 | 16 | 387 | 15-1/4 |
| Flooring ${ }^{\text {a }}$ | 3/8 | 8 | 5/16 | 2 | 29 | 1-1/8 |
|  | 1/2 | 11 | 7/16 | 3 | 54 | 2-1/8 |
|  | 5/8 | 14 | 9/16 | 4 | 79 | 3-1/8 |
|  | 1 | 19 | 3/4 | 5 | 105 | 4-1/8 |
|  | 1-1/4 | 25 | 1 | 6 | 130 | 5-1/8 |
|  | 1-1/2 | 32 | 1-1/4 |  |  |  |
| Ceiling ${ }^{\text {a }}$ | 3/8 | 8 | 5/16 | 3 | 54 | 2-1/8 |
|  | 1/2 | 11 | 7/16 | 4 | 79 | 3-1/8 |
|  | 5/8 | 14 | 9/16 | 5 | 105 | 4-1/8 |
|  | 3/4 | 17 | 11/16 | 6 | 130 | 5-1/8 |
| Partition ${ }^{\text {a }}$ | 1 | 18 | 23/32 | 3 | 54 | 2-1/8 |
|  |  |  |  | 4 | 79 | 3-1/8 |
|  |  |  |  | 5 | 105 | 4-1/8 |
|  |  |  |  | 6 | 130 | 5-1/8 |
| Stepping | 1 | 19 | 3/4 | 8 | 184 | 7-1/4 |
|  | 1-1/4 | 25 | 1 | 10 | 235 | 9-1/4 |
|  | 1-1/2 | 32 | 1-1/4 | 12 | 286 | 11-1/4 |
|  | 2 | 38 | 1-1/2 |  |  |  |

${ }^{a}$ In tongued-and-grooved flooring and in tongued-and-grooved and shiplapped ceiling of 8 mm (5/16 inch), 11 mm (7/16 inch), and 14 mm ( $9 / 16$ inch) dressed thicknesses, the tongue or lap shall be 5 mm (3/16 inch) wide, with the overall widths 5 mm (3/16 inch) wider than the face widths shown in the above table. In all other worked lumber shown in this table of dressed thicknesses of 16 mm (5/8 inch) to 32 mm (1-1/4 inches), the tongue shall be 6 mm (1/4 inch) wide or wider in tongued-and-grooved lumber, and the lap shall be 10 mm (3/8 inch) wide or wider in shiplapped lumber, and the overall widths shall be not less than the dressed face widths shown in the above table plus the width of the tongue or lap.

Nominal and Minimum-Dressed Dry Sizes of Siding at 19\% Maximum-Moisture Content

| Item | Thicknesses |  |  | Widths |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Nominal (inch) | Minimum Dressed |  | Nominal (inch) | Minimum Dressed |  |
|  |  | mm | inch |  | mm | inch |
| Plain Bevel | 1/2 | 11 butt, 5 tip | 7/16 butt, 3/16 tip | 4 | 89 | 3-1/2 |
|  | 9/16 | 12 butt, 5 tip | 15/32 butt, 3/16 tip | 5 | 114 | 4-1/2 |
|  | 5/8 | 14 butt, 5 tip | 9/16 butt, 3/16 tip | 6 | 140 | 5-1/2 |
|  | $3 / 4$ | 17 butt, 5 tip | 11/16 butt, 3/16 tip | 8 | 184 | 7-1/4 |
|  | 1 | 19 butt, 5 tip | 3/4 butt, 3/16 tip | 10 | 235 | 9-1/4 |
|  |  |  |  | 12 | 286 | 11-1/4 |
| Rabbeted <br> Bevel | 1/2 | 11 butt, 5 tip | 7/16 butt, 3/16 tip | 4 | 89 | 3-1/2 |
|  | 3/4 | 17 butt, 7 tip | 11/16 butt, 9/32 tip | 6 | 140 | 5-1/2 |
|  |  |  |  | 8 | 184 | 7-1/4 |
|  |  |  |  | 10 | 235 | 9-1/4 |
|  |  |  |  | 12 | 286 | 11-1/4 |
| Bungalow | 3/4 | 17 butt, 5 tip | 11/16 butt, 3/16 tip | 6 | 140 | 5-1/2 |
|  |  |  |  | 8 | 184 | 7-1/4 |
|  |  |  |  | 10 | 235 | 9-1/4 |
|  |  |  |  | 12 | 286 | 11-1/4 |
| Shiplap <br> (10 mm [3/ <br> 8 in.] lap) | 5/8 | 14 | 9/16 | 4 | 76 | 3 |
|  | 1 | $18^{\text {a }}$ | $23 / 32^{\text {a }}$ | 5 | 102 | 4 |
|  |  |  |  | 6 | 127 | 5 |
|  |  |  |  | 8 | 171 | 6-3/4 |
|  |  |  |  | 10 | 222 | 8-3/4 |
|  |  |  |  | 12 | 273 | 10-3/4 |
| Shiplap$\begin{aligned} & (13 \mathrm{~mm}[1 / 2 \\ & \text { in.] lap) } \end{aligned}$ | 5/8 | 14 | 9/16 | 4 | 73 | 2-7/8 |
|  | 1 | $18^{\text {a }}$ | 23/32 ${ }^{\text {a }}$ | 5 | 98 | 3-7/8 |
|  | 2 | 38 | 1-1/2 | 6 | 124 | 4-7/8 |
|  |  |  |  | 8 | 168 | 6-5/8 |
|  |  |  |  | 10 | 219 | 8-5/8 |
|  |  |  |  | 12 | 270 | 10-5/8 |
| Dressed and Matched (Tongue and Grooved, 6 mm [1/4 in.] tongue) | 5/8 |  | 9/16 | 4 | 79 | 3-1/8 |
|  | 1 | $18^{\text {a }}$ | $23 / 32^{\text {a }}$ | 5 | 105 | 4-1/8 |
|  |  |  |  | 6 | 130 | 5-1/8 |
|  |  |  |  | 8 | 175 | 6-7/8 |
|  |  |  |  | $10$ | $225$ | $8-7 / 8$ |
|  |  |  |  | 12 | 276 | 10-7/8 |
| Dressed and Matched (Tongue and Grooved, 10 mm [3/ 8 in.) tongue) | 1 | $18^{\text {a }}$ | 23/32 ${ }^{\text {a }}$ | 4 | 76 | 3 |
|  |  |  |  | 6 | 127 | 5 |
|  |  |  |  | 8 | 171 | 6-3/4 |
|  |  |  |  | 10 | 222 | $8-3 / 4$ |
|  |  |  |  | 12 | 273 | 10-3/4 |
|  |  |  |  |  |  |  |

${ }^{a}$ Minimum dressed thickness for 1-inch nominal redwood and western red cedar shiplap and tongue and groove siding patterns is 17 mm (11/16 inch).

### 7.2.0 No. I and No. 2 Common Board Designations

## No. 1 Common and No. 2A Common

## Number 1 Common (No. 1C)

The Number 1 Common grade is often referred to as the Cabinet grade in the United States because of its adaptability to the standard sizes of kitchen cabinet doors used throughout the United States. Number 1 Common is widely used in the manufacture of furniture parts as well for this same reason. The Number 1 Common grades include boards that are a minimum of $3^{\prime \prime}$ wide and $4^{\prime}$ long and will yield clear face cuttings from $662 / 3 \%$ ( $8 / 12$ ths) up to, but not including, the minimum requirement for FAS $(831 / 3 \%)$. The smallest clear cuttings allowed are $3^{\prime \prime}$ by $3^{\prime}$ and $4^{\prime \prime}$ by $2^{\prime}$. The number of these clear cuttings is determined by the size of the board. Both faces of the board must meet the minimum requirement for Number 1 Common.


## Number 2A Common (No. 2AC)



There are lower NHLA (National Hardwood Lumber Association) grades than Number 2A Common, but they are usually converted into dimension parts, flooring parts, or used domestically in the United States.

The Number 2A Common grade is often referred to as the Economy grade because of its price and suitability for a wide range of furniture parts. It is also the grade of choice for the U.S. hardwood flooring industry. The Number 2A Common grade includes boards that are a minimum of $3^{\prime \prime}$ wide and $4^{\prime}$ long that yield from $50 \%$ ( $6 / 12$ ths) up to, but not including, the minimum requirement for Number 1 Common ( $662 / 3 \%$ ). The smallest clear cutting allowed is $3^{\prime \prime}$ by $2^{\prime}$, and the number of these cuttings depends on the size of the board. If the poorest face meets the minimum requirements for Number 2A Common, it does not matter what the grade of the better face is.

These Standard Grades form the framework by which all American hardwoods are traded. It is important to note that between buyer and seller any exception to these rules is permissible and even encouraged. For a complete description of the NHLA grades, consult the NHLA's "Rules for the Measurement and Inspection of Hardwoods and Cypress."

### 7.2.1 Hardwood—Measurement System

The NHLA lumber grading rules adopted by the U.S. hardwood industry are based on an imperial measurement system using inches and feet. In contrast, most export markets are more familiar with a metric standard. In addition, the grade rules were developed with random width and length lumber in mind. Any selection for particular specifications should be discussed prior to ordering.

## Board Foot

A board foot ( BF ) is the unit of measurement for hardwood lumber. A board foot is 1 foot long $\times 1$ foot wide $\times 1$ inch thick. $(1$ foot $=0.305$ meters, 1 inch $=25.4 \mathrm{~mm})$

The formula for determining board feet in a board is:
(Width in inches $\times$ length in feet $\times$ thickness in inches) divided by 12
The percentages of clear wood required for each grade are based on this $12^{\prime}$ unit of measure.

## Surface Measure

Surface measure (SM) is the surface area of a board in square feet. To determine surface measure, multiply the width of the board in inches by the length of the board in feet and divide the sum by 12 rounding up or down to the nearest whole number. The percentage of clear wood required for each grade is based on the surface measure, not the board feet, and because of this all boards, no matter what the thickness, are graded in the same way.

Some examples for surface measure calculations are as follows:

$$
\begin{aligned}
& 6^{1 / 2} \times 8^{\prime} \div 12=4^{1 / 3}=4^{\prime} \mathrm{SM} \\
& 8^{\prime \prime} \times 12^{\prime} \div 12=8^{\prime} \mathrm{SM} \\
& 10^{\prime \prime} \times 13^{\prime} \div 12=10 \%=11^{\prime} \mathrm{SM}
\end{aligned}
$$



Example of SM and BF:
The board is $2^{\prime \prime}$ thick, $6^{1 / 4^{\prime \prime}}$ wide, and $8^{\prime}$ long.
$6^{1 / 4 \prime} \times 8^{\prime} \div 12=41^{1 / 4}$, thus the SM is $4^{\prime}$. Multiply the SM by the thickness $2^{\prime \prime}$ and the BF is $8^{\prime}$.
When preparing a bundle tally for export, the boards are recorded by their width and length. Random widths above or below the half inch are rounded to the nearest whole inch. Board widths falling exactly on the half inch are alternatively rounded up or down. Lengths that fall between whole foot increments are always rounded down to the nearest whole foot. For example, a board $5^{1 / 4^{\prime \prime}}$ width and $8^{1 / 2}$ long is tallied $5^{\prime \prime}$ and $8^{\prime}$.

## Standard Thickness for Rough Sawn Lumber

Standard thickness for rough sawn lumber is expressed in quarters of an inch. For example, $1^{\prime \prime}=1 / 4$. The majority of U.S. hardwood lumber production is sawn between $1^{\prime \prime}$ and $2^{\prime \prime}$, although other thicknesses are available in more limited volumes. The standard thicknesses and their exact metric equivalent are shown below.

| $3 / 4$ | $\left(3 / 4^{\prime \prime}=19.0 \mathrm{~mm}\right)$ | $8 / 4$ | $\left(2^{\prime \prime}=50.8 \mathrm{~mm}\right)$ |
| :--- | :--- | :--- | :--- |
| $4 / 4$ | $\left(1^{\prime \prime}=25.4 \mathrm{~mm}\right)$ | $10 / 4$ | $\left(21 / 2^{\prime \prime}=63.5 \mathrm{~mm}\right)$ |
| $5 / 4$ | $\left(11 / 4^{\prime \prime}=31.8 \mathrm{~mm}\right)$ | $12 / 4$ | $\left(3^{\prime \prime}=76.2 \mathrm{~mm}\right)$ |
| $6 / 4$ | $\left(11 / 2^{\prime \prime}=38.1 \mathrm{~mm}\right)$ | $16 / 4$ | $\left(4^{\prime \prime}=10.1 \mathrm{~mm}\right)$ |

## Standard Thickness for Surfaced (Planed) Lumber

When rough sawn lumber is surfaced (planed) to a finished thickness, defects such as checks, stain, and warp are not considered when establishing the grade of a board, if they can be removed in the surfacing (planning) process. The finished thickness for lumber of $11 / 2^{\prime \prime}$ and less can be determined by subtracting $3 / 6^{\prime \prime}$ from the nominal thickness. For lumber $134^{\prime \prime}$ and thicker, subtract $1 / 4^{\prime \prime}$.

## Measurement of Kiln-dried Lumber

Net tally: The actual board feet of kiln-dried lumber measured after kiln drying.
Gross or green tally: The actual board feet measured before kiln drying. When kiln-dried lumber is sold on this basis, the buyer can expect to receive approximately $7 \%$ less board feet because of shrinkage in the kiln drying process.

## Estimating Board Feet in a Bundle of Lumber

To determine the board feet of one board, the procedure is to multiply the surface measure by the thickness. A bundle of lumber can be estimated in much the same manner. First, calculate the surface measure of one layer of boards. Do this by multiplying the width of the bundle, minus gaps, by the length of the bundle and divide the sum by 12 . If there are several lengths in the bundle, use an average length. Once one layer is estimated, multiply this sum by the total number of layers.

Example:
Average width of unit $40^{\prime \prime}$
(lumber only, after allowing for gaps between boards)
Length of unit $10^{\prime}$

$$
\begin{array}{lr}
40^{\prime \prime} \times 10^{\prime}=400 \div 12 & =33.33 \\
\text { Thickness of lumber } 8 / 4 & \times 2 \\
& =66.66 \\
\text { Number of layers : } & \times 10 \\
& =666.67
\end{array}
$$

Estimated board feet of the bundle 667 BF

## Conversion Factors

| $1 \prime \prime$ | 25.4 millimeters $(\mathrm{mm})$ | $1 \mathrm{~m}^{3}: 424$ board feet $(\mathrm{BF})$ |
| :--- | :--- | :--- |
| $1 \mathrm{~m}:$ | 3.281 feet | $1 \mathrm{~m}^{3}: 35.315$ cubic feet $(\mathrm{cu} \mathrm{ft})$ |
| $1.000 \mathrm{BF}:(1 \mathrm{MBF})$ | 2.36 cubic meters $\left(\mathrm{m}^{3}\right)$ |  |

## Additional Guidance

## Regional exceptions to the standard NHLA grades

The NHLA grades cover the majority of commercial hardwood species growing in the United States. The following is a brief summary of various species and color sorting that can be ordered from the American supplier.

## Red Alder

Grows exclusively in the Pacific Northwest between the vast stands of softwood timber such as Douglas fir and pine and is the most important commercial hardwood in this region. The grading rules for red alder are geared more toward specific end uses and appearance. The rules were developed on the West Coast of the United States with those manufacturers and exports in mind. An exceptional cabinet wood typically sold surfaced (planed) and
often cut to specific lengths and widths. Consult with your local supplier for a more detailed explanation of the alder grades and products available.

## Walnut

Considered the elite of the American hardwoods, walnut is the favorite of the darker woods for fine furniture, interiors, and gunstocks. Walnut grows in widely scattered stands throughout the eastern half of the United States, primarily in the Midwest. Historically, the grading rules for FAS walnut have been refined to encourage better use of this valuable species. Because of this, FAS Walnut grades allow for smaller boards, in both width and length. Natural characteristics are also admitted to a greater extent than the standard NHLA grade rules for other species. A detailed explanation can be found in the NHLA rules book. Consult with your local supplier for the walnut grades and products available.

## Color Sorting

In addition to sorting for grades or selecting specific widths, various species are commercially sold at an added value when color is also considered. It is important to note that color in this explanation refers to sapwood and heartwood.

## Number 1 and 2 White

A color selection is typically made on hard maple, but can be applied to any species where sapwood clear cuttings are desired, such as ash, birch, and soft maple.

Number 1 white means both faces and edges of the clear cuttings must be all sapwood.
Number 2 white means that one face and both edges of the clear cuttings must be sapwood and not less than $50 \%$ sapwood on the reverse face.

## Sap and Better

Commercially sold when only one face of the board needs to be sapwood. Usually applied to the same species as Number 1 and 2 White, although just a little less stringent. In Sap and Better, every board should have a minimum of one sapwood face in the clear cuttings.

## Red One Face and Better

Commercially sold when a minimum of one face of the board needs to be heartwood. Usually applied to species such as cherry, oak, walnut, gum, and even birch and maple in certain applications. What the producer is looking for in this specification is that all clear cuttings must have a minimum of one heartwood face.

A wide range of additional options is open to American hardwood producers in sorting and selecting specific lengths, widths, and even grain patterns. If these options can be agreed upon individually between producers and buyers, there can be benefits by making modifications to the standard grades shown in this guide. This may also assist with improving the yield from each $\log$ and thus contribute to the sustainability of the forest. It may also reduce costs to both sides or add value to the delivery.

## The Steps in Determining Grade

1. Determine species.
2. Calculate the surface measure (SM).
3. Determine the poor side of the board.
4. From this poor face, calculate the percentage of clear wood available.

Note: If Number 1 Common is the grade of the poor face, check the better face to see if it will grade FAS for the F1F or Selects grades to be achieved.
5. Once the grade is determined, check for any special features such as sapwood or heartwood cuttings for special color sorts.
6. Sort to bundles according to buyer and seller specifications.

### 7.2.2 Hardwoods—Selects and No. I and No. 2 Standards

## FAS

The FAS grade, which derives from an original grade "First And Seconds," will provide the user with long, clear cuttings best suited for high-quality furniture, interior joinery, and solid wood moldings. Minimum board size is $6^{\prime \prime}$ and wider and $8^{\prime}$ and longer. The FAS grade includes a range of boards that yield from 831/3\% (10/12ths) to $100 \%$ clear-wood cuttings over the entire surface of the board. The clear cuttings must be a minimum size of $3^{\prime \prime}$ wide by $7^{\prime}$ long or $4^{\prime \prime}$ wide by $5^{\prime}$ long. The number of these cuttings permitted depends on the size of the board, with most boards permitting one to two. The minimum width and length will vary, depending on species and whether the board is green or kiln dried. Both faces of the board must meet the minimum requirement for FAS.


## FAS One Face (F1F)

This grade is nearly always shipped with FAS. The better face must meet all FAS requirements, while the poor face must meet all the requirements of the Number 1 Common grade, thus ensuring the buyer with at least one FAS face. Often export shipments are assembled with an $80-20 \mathrm{mix}, 80 \%$ being the percentage of FAS boards and $20 \%$ being the percentage of F1F boards. These percentages are strictly left to individual buyer and seller agreement.

## Selects

This grade is virtually the same as F1F except for the minimum board size required. Selects allow boards $4^{\prime \prime}$ and wider and $6^{\prime}$ and longer in length. The Selects grade is generally associated with the northern regions of the United States and is also shipped in combination with the FAS grade.

Often export shipments of upper grades are simply referred to as FAS. The conventional business practice for American hardwoods is to ship these upper grades in some combination. Working closely with the supplier will enable the buyer to be sure that the expected quality will be received. Whether FAS is combined with F1F (Face And Better) or Selects (Sel And Better), every board in the shipment must have a minimum of one FAS face.

Prime grade: This grade has evolved from the NHLA grade of FAS for the export market. It is square edged and virtually wane free. The minimum clear yield will be select and better, with appearance being a major factor. Minimum size of the boards varies, depending on the species, region, and supplier.
Comsel grade: This grade has evolved from the NHLA grades of Number 1 Common and Selects. For the export market the minimum clear yield should be Number 1 Common or slightly better with appearance a main factor. Minimum size of the boards varies, depending on the species, region, and supplier.

Note: The terms Prime and Comsels are not standard NHLA definitions and therefore fall outside the official range of the NHLA grading rules.

### 7.2.3 American Standard Lumber Sizes for Yard and Structural Lumber for Construction

| Item | Thickness |  |  |  |  | Face Width |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Nominal (in.) | Minimum dressed |  |  |  | Nominal (in.) | Minimum dressed |  |  |  |
|  |  | Dry |  | Green |  |  | Dry |  | Green |  |
|  |  | (mm | (in.)) | (mm | (in.)) |  | (mm | (in.)) | (mm | (in.)) |
| Boards | 1 | 19 | (3/4) | 20 | (25/32) | 2 | 38 | (1-1/2) | 40 | (1-9/16) |
|  | 1-1/4 | 25 | (1) | 26 | (1-1/32) | 3 | 64 | (2-1/2) | 65 | (2-9/16) |
|  | 1-1/2 | 32 | (1-1/4) | 33 | (1-9/32) | 4 | 89 | (3-1/2) | 90 | (3-9/16) |
|  |  |  |  |  |  | 5 | 114 | (4-1/2) | 117 | (4-5/8) |
|  |  |  |  |  |  | 6 | 140 | (5-1/2) | 143 | (5-5/8) |
|  |  |  |  |  |  | 7 | 165 | (6-1/2) | 168 | (6-5/8) |
|  |  |  |  |  |  | 8 | 184 | (7-1/4) | 190 | (7-1/2) |
|  |  |  |  |  |  | 9 | 210 | (8-1/4) | 216 | (8-1/2) |
|  |  |  |  |  |  | 10 | 235 | (9-1/4) | 241 | (9-1/2) |
|  |  |  |  |  |  | 11 | 260 | (10-1/4) | 267 | (10-1/2) |
|  |  |  |  |  |  | 12 | 286 | (11-1/4) | 292 | (11-1/2) |
|  |  |  |  |  |  | 14 | 337 | (13-1/4) | 343 | (13-1/2) |
|  |  |  |  |  |  | 16 | 387 | (15-1/4) | 394 | (15-1/2) |
| Dimension | 2 | 38 | (1-1/2) | 40 | (1-9/16) | 2 | 38 | (1-1/2) | 40 | (1-9/16) |
|  | 2-1/2 | 51 | (2) | 52 | (2-1/16) | 3 | 64 | (2-1/2) | 65 | (2-9/16) |
|  | 3 | 64 | (2-1/2) | 65 | (2-9/16) | 4 | 89 | (3-1/2) | 90 | (3-9/16) |
|  | 3-1/2 | 76 | (3) | 78 | (3-1/16) | 5 | 114 | (4-1/2) | 117 | (4-5/8) |
|  | 4 | 89 | (3-1/2) | 90 | (3-9/16) | 6 | 140 | (5-1/2) | 143 | (5-5/8) |
|  | 4-1/2 | 102 | (4) | 103 | (4-1/16) | 8 | 184 | (7-1/4) | 190 | (7-1/2) |
|  |  |  |  |  |  | 10 | 235 | (9-1/4) | 241 | (9-1/2) |
|  |  |  |  |  |  | 12 | 286 | (11-1/4) | 292 | (11-1/2) |
|  |  |  |  |  |  | 14 | 337 | (13-1/4) | 343 | (13-1/2) |
|  |  |  |  |  |  | 16 | 387 | (15-1/4) | 394 | (15-1/2) |
| Timbers | $\geq 5$ | $\begin{aligned} & 13 \mathrm{~mm} \\ & \text { off } \end{aligned}$ | (1/2 in. off) | $\begin{aligned} & 13 \mathrm{~mm} \\ & \text { off } \end{aligned}$ | (1/2 in. off) | $\geq 5$ | $\begin{gathered} 13 \mathrm{~mm} \\ \text { off } \end{gathered}$ | (1/2 in. off) | $\begin{gathered} 13 \mathrm{~mm} \\ \text { off } \end{gathered}$ | (1/2 in. off) |

Factory and Shop lumber for remanufacture is offered in specified sizes to fit end-product requirements. Factory (Shop) grades for general cuttings are offered in thickness from standard 19 to 89 mm (nominal 1 to 4 in .). Thicknesses of door cuttings start at 35 mm (nominal 1-3/8 in.). Cuttings are of various lengths and widths. Laminating stock is sometimes offered oversize, compared with standard dimension sizes, to permit resurfacing prior to laminating. Industrial Clears can be offered rough or surfaced in a variety of sizes, starting from standard 38 mm (nominal 2 in.) and thinner and as narrow as standard 64 mm (nominal 3 in .). Sizes for special product grades such as molding stock and ladder stock are specified in appropriate grading rules or handled by purchase agreements.

## Surfacing

Lumber can be produced either rough or surfaced (dressed). Rough lumber has surface imperfections caused by the primary sawing operations. It may be greater than target size by variable amounts in both thickness and width, depending on the type of sawmill equipment. Rough lumber serves as a raw material for further manufacture and also for some decorative purposes. A rough-sawn surface is common in post and timber products. Because of surface roughness, grading of rough lumber is generally more difficult.

Surfaced lumber has been surfaced by a machine on one side (SIS), two sides (S2S), one edge (S1E), two edges (S2E), or combinations of sides and edges (S1S1E, S2S1E, S1S2, S4S). Lumber is surfaced to attain smoothness and uniformity of size.

Imperfections or blemishes defined in the grading rules and caused by machining are classified as "manufacturing imperfections." For example, chipped and torn grain are surface irregularities in which surface fibers have been torn out by the surfacing operation. Chipped grain is a "barely perceptible" characteristic, while torn grain is classified by depth. Raised grain, skip, machine burn and gouge, chip marks, and wavy surfacing are other manufacturing imperfections. Manufacturing imperfections are defined in the American Softwood.

### 7.2.4 Weights of Green and Kiln-Dried Hardwoods

| Common Name | Weights of Green and Kiln-Dried Lumber ${ }^{\text {a }}$ |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Green |  |  |  | Kiln-Dried ${ }^{\text {b }}$ |  |  |  |
|  | Latin Name | $\mathrm{lb} / \mathrm{ft} 3$ | kg/m3 | lb/MBF | kg/MBF | $\mathrm{lb} / \mathrm{ft} 3$ | kg/m3 | lb/MBF | kg/MBF |
| Alder, Red | Alnus rubra | 46 | 737 | 3833 | 1739 | 27.5 | 440 | 2288 | 1038 |
| Ash |  |  |  |  |  |  |  |  |  |
| Black | Fraxinus nigra | 52 | 833 | 4333 | 1966 | 34.1 | 546 | 2843 | 1290 |
| Green | Fraxinus pennsylvanica | 49 | 785 | 4083 | 1852 | 39.3 | 629 | 3274 | 1485 |
| White | Fraxinus americana | 48 | 769 | 4000 | 1814 | 41.0 | 657 | 3420 | 1551 |
| Aspen | Populus tremuloides | 43 | 689 | 3583 | 1625 | 26.5 | 425 | 2211 | 1003 |
| Basswood, Amer. | Tilia americana | 42 | 673 | 3500 | 1588 | 24.4 | 390 | 2031 | 921 |
| Beech, Amer. | Fagus grandifolia | 54 | 865 | 4500 | 2041 | 43.2 | 691 | 3597 | 1632 |
| Birch |  |  |  |  |  |  |  |  |  |
| Sweet | Betula lenta | 57 | 913 | 4750 | 2155 | 45.6 | 731 | 3803 | 1725 |
| White | Betula papyrifera | 50 | 801 | 4167 | 1890 | 36.8 | 590 | 3067 | 1391 |
| Yellow | Betula alleghaniensis | 57 | 913 | 4750 | 2155 | 42.2 | 677 | 3520 | 1597 |
| Black Gum/ Tupelo | Nyssa spp. | 45 | 721 | 3750 | 1701 | 34.6 | 555 | 2887 | 1310 |
| Black Locus | Robinia pseudoacacia | 58 | 929 | 4833 | 2192 | 48.0 | 769 | 4003 | 1816 |
| Black Walnut | Juglans nigra | 58 | 929 | 4833 | 2192 | 37.9 | 607 | 3159 | 1433 |
| Boxelder ${ }^{\text {c }}$ | Acer negundo | 32 | 513 | 2667 | 1210 | 27.5 | 508 | 2642 | 1198 |
| Buckeye, Yellow | Aesculus octandra | 49 | 785 | 4083 | 1852 | 24.5 | 392 | 2039 | 925 |
| Butternut | Juglans cinerea | 46 | 737 | 3833 | 1739 | 26.3 | 421 | 2191 | 994 |
| Cherry, Black | Prunus serotina | 45 | 721 | 3750 | 1701 | 34.6 | 554 | 2881 | 1307 |
| Chestnut, Amer. | Castanea dentate | 55 | 881 | 4583 | 2079 | 29.4 | 472 | 2453 | 1113 |
| Cottonwood ${ }^{\text {c }}$ | Populus deltoids | 49 | 785 | 4083 | 1852 | 27.5 | 444 | 2308 | 1047 |
| Cypress | Taxodium distichum | 60 | 961 | 5000 | 2268 | 30.6 | 491 | 2553 | 1158 |


| Common Name | Weights of Green and Kiln-Dried Lumber ${ }^{\text {- }}$ Cont'd |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Latin Name | Green |  |  |  | Kiln-Dried ${ }^{\text {b }}$ |  |  |  |
|  |  | lb/ft3 | kg/m3 | $\mathrm{lb} / \mathrm{MBF}$ | kg/MBF | lb/ft3 | kg/m3 | $\mathrm{lb} / \mathrm{MBF}$ | kg/MBF |
| Elm |  |  |  |  |  |  |  |  |  |
| Hard | Ulmus thomasil | 53 | 849 | 4417 | 2003 | 43.1 | 690 | 3591 | 1629 |
| Soft | Ulmus rubra | 56 | 897 | 4667 | 2117 | 36.0 | 576 | 2997 | 1359 |
| Hackberry | Celtis spp. | 50 | 801 | 4167 | 1890 | 36.7 | 588 | 3060 | 1388 |
| Hickory |  |  |  |  |  |  |  |  |  |
| True (average) | Carya spp. | 64 | 1025 | 5333 | 2449 | 49.6 | 795 | 4135 | 1876 |
| Pecan (bitternut) | Carya spp. | 61 | 977 | 5083 | 2306 | 27.5 | 724 | 3767 | 1709 |
| Honelylocust ${ }^{\text {c }}$ | Gleditsia triacanthos | 61 | 977 | 5083 | 2306 | 27.5 | 702 | 3650 | 1656 |
| Madrone | Arbutus spp. | 60 | 961 | 5000 | 2268 | 45.0 | 722 | 3754 | 1703 |
| Magnolia, So. ${ }^{\text {c }}$ | Magnolia grandiflora | 59 | 945 | 4917 | 2230 | 27.5 | 485 | 2525 | 1145 |
| Maple |  |  |  |  |  |  |  |  |  |
| Hard (sugar) | Acer spp. | 56 | 897 | 4667 | 2117 | 42.3 | 677 | 3523 | 1598 |
| Soft (red) | Acer spp. | 50 | 801 | 4167 | 1890 | 36.4 | 582 | 3030 | 1374 |
| Red Oak Group |  |  |  |  |  |  |  |  |  |
| Black | Quercus velutina | 62 | 993 | 5167 | 2344 | 42.4 | 679 | 3534 | 1603 |
| Cherrybark ${ }^{\text {c }}$ | Q. falcate v. pagodifolia | 68 | 1089 | 5667 | 2570 | 46.5 | 745 | 3875 | 1758 |
| Laurel | Q. laurifolia | 65 | 1041 | 5417 | 2457 | 43.8 | 702 | 3652 | 1657 |
| Northern red | Q. rubra | 63 | 1009 | 5250 | 2381 | 41.9 | 672 | 3494 | 1585 |
| Pin | Q. palustris | 63 | 1009 | 5250 | 2381 | 43.7 | 700 | 3642 | 1652 |
| Scarlet | $Q$. coccinea | 62 | 993 | 5167 | 2344 | 45.3 | 725 | 3774 | 1712 |
| Southern red | Q. faleata | 62 | 993 | 5167 | 2344 | 39.7 | 636 | 3309 | 1501 |
| Water | Q. nigra | 63 | 1009 | 5250 | 2381 | 42.8 | 685 | 3564 | 1617 |
| Willow | Q. phellos | 67 | 1073 | 5583 | 2533 | 43.8 | 701 | 3649 | 1655 |
| White Oak Group |  |  |  |  |  |  |  |  |  |
| Bur | Quercus macrocarpa | 62 | 993 | 5167 | 2344 | 43.1 | 690 | 3589 | 1628 |
| Chestnut | Q. prinus | 61 | 977 | 5083 | 2306 | 43.6 | 699 | 3636 | 1649 |
| Post | Q. stellata | 63 | 1009 | 5250 | 2381 | 45.9 | 734 | 3821 | 1733 |
| Swamp chestnut | Q. michauxii | 65 | 1041 | 5417 | 2457 | 45.9 | 736 | 3828 | 1736 |
| Swamp white ${ }^{\text {c }}$ | Q. bicolor | 69 | 1105 | 5750 | 2608 | 27.5 | 799 | 4158 | 1886 |
| White | Q. alba | 62 | 993 | 5167 | 2344 | 45.9 | 735 | 3825 | 1735 |
| Live Oak | Quercus virginiana | 76 | 1218 | 6333 | 2873 | 60.4 | 967 | 5032 | 2283 |
| Redcedar, Eastern | Juniperus virginia | 36 | 583 | 3033 | 1376 | 31.4 | 504 | 2620 | 1188 |
| Sassafras | Sassafras albidum | 44 | 705 | 3667 | 1663 | 27.5 | 497 | 2583 | 1172 |
| Sweetgum | Liquidambar styracifulua | 50 | 801 | 4167 | 1890 | 35.0 | 561 | 2920 | 1325 |
| Sycamore | Platanus occidentalis | 52 | 833 | 4333 | 1966 | 34.0 | 545 | 2836 | 1286 |
| Tanoak | Lithocarpus densiflorus | 65 | 1041 | 5417 | 2457 | 44.7 | 717 | 3728 | 1691 |


| Common Name | Weights of Green and Kiln-Dried Lumber ${ }^{\text {- }}$ Cont'd |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Latin Name | Green |  |  |  | Kiln-Dried ${ }^{\text {b }}$ |  |  |  |
|  |  | lb/ft3 | kg/m3 | lb/MBF | kg/MBF | lb/ft3 | kg/m3 | lb/MBF | kg/MBF |
| Yellow-poplar | Liriodendron tulipifera | 38 | 609 | 3167 | 1436 | 29.7 | 476 | 2475 | 1123 |
| Willow, Black | Salix nigra | 50 | 801 | 4167 | 1890 | 27.0 | 432 | 2249 | 1020 |

${ }^{\text {a }}$ Green weights directly from Hardwoods of North America, USDA Forest Service, FPL-GTR-83; and Wood Handbook, USDA Forest Service, Ag Handbook 72. Kiln-dried weights from Dry Kiln Operator's Manual, USDA Forest Service, except where noted. Weights are calculated based on average densities for each species. Thus, the weights given here should always be considered approximations because of the natural variation in anatomy, moisture content, and the ratio of heartwood to sapwood that occurs in a load. Board foot weights are based on exact board footage $\left(1 B F=1^{\prime} \times 1^{\prime} \times 1^{\prime \prime}\right)$. No allowance is given for over thickness.
${ }^{b}$ Kiln-dried weights are calculated at $8 \%$ moisture content.
${ }^{c}$ Kiln-dried weights extrapolated from Hardwoods of North America, USDA Forest Service, FPL-GTR-83.

### 7.2.5 National Hardwood Lumber Association Grading Rules

## Grading Rules

## SELECTS and BETTER:

Widths: $4^{\prime \prime}$ and wider, of which $5 \%$ of $3^{\prime \prime}$ width is admitted.
Lengths: Random $4^{\prime}$ and longer. Minimum cuttings: $4^{\prime \prime}$ wide by $3^{\prime}$ long, or $3^{\prime \prime}$ wide by $6^{\prime}$ long.
There is no limit to the number of cuttings. This grade admits all boards of $1^{\prime}$ and over, surface measure, that will yield not less than $83-1 / 3 \%$ of clear-face cuttings, the reverse side of the cuttings sound as defined in "SOUND CUTTING," except that boards of $1^{\prime}$ and over surface measure yielding not less than $83-1 / 3 \%$ clear-face cuttings on one face, the reverse side of the board grading not below No. 1 Shop.

Pith: No piece shall be admitted which contains pith exceeding in the aggregate in inches in length twice the surface measure in feet.

Splits: No piece shall be admitted which contains splits exceeding in the aggregate in inches in length twice the surface measure of the piece in feet, nor when diverging more than one inch to the foot in length, except when one foot or shorter and covered by Paragraph 59 of Standard Grades.

Wane: On the face side of Selects and Better, wane or its equivalent shall be limited to one-twelfth the surface measure of the piece.

On the No. 1 Shop side of Selects and Better, wane or its equivalent shall not exceed one-fourth the width by three-fourths the length in the aggregate, or pieces may alternately have wane one-third the width by one-half the length in the aggregate. Width of the wane may be divided and show on both edges. The reverse side of the cuttings in Selects and Better and No. 1 Shop are not required to be sound.

NO. 1 SHOP: Widths: $4^{\prime \prime}$ and wider, admitting $5 \%$ of $3^{\prime \prime}$ width.
Lengths: Random $4^{\prime}$ and longer.
Minimum cuttings: $3^{\prime \prime}$ wide by $3^{\prime}$ long, or $4^{\prime \prime}$ wide by $2^{\prime} 1^{\prime}$ and over surface measure shall yield not less than $66-2 / 3 \%$ clear-face cuttings, the reverse side of the cuttings sound as defined in "Sound Cutting."

NO. 2 SHOP: Widths: $4^{\prime \prime}$ and wider, admitting $5 \%$ of $3^{\prime \prime}$ width.
Lengths: Random $4^{\prime}$ and longer.
Minimum cuttings: $3^{\prime \prime}$ wide by $2^{\prime}$ long. There is no limit to the number of cuttings. Admits boards of $1^{\prime}$ and over surface measure that will yield not less than $50 \%$ clear-face cuttings, the reverse side of the cuttings sound as defined in "Sound Cutting."

NO. 3 SHOP: Widths: $3^{\prime \prime}$ and wider.
Lengths: Random $4^{\prime}$ and longer.
Minimum cuttings: $3^{\prime \prime}$ wide by $2^{\prime}$ long. There is no limit to the number of cuttings. Admits boards of $1^{\prime}$ and over surface measure that will yield not less than $33-1 / 3 \%$ of sound cuttings or better.

NOTE: Lumber poorer in cutting percentage, or less in width or length than admitted in No. 3 Shop described above, shall be tallied and reported below grade.

FRAME GRADE: Widths: $4^{\prime \prime}$ and wider
Lengths: 7 and longer
Minimum cuttings: Each piece must contain at least one cutting $4^{\prime \prime} \times 7^{\prime \prime}$; other cuttings, minimum size $4^{\prime \prime} \times 2^{\prime}$. Each piece shall yield not less than $83-1 / 3 \%$. There is no limit to the number of cuttings.

Wane: Wane shall not exceed one-fourth the width and one-half the length of the piece. Width of the wane may be divided and show on both edges.

Each cutting shall be reasonably flat and straight; will admit bark pockets, season checks, slight surface shake that does not impair the strength of the cutting, firm tight pith, stain, worm holes, and other holes or unsound knots that do not exceed in their greatest dimension $1-1 / 2^{\prime \prime}$ in $4^{\prime \prime}$ to $5^{\prime \prime}$ wide cuttings and $2^{\prime \prime}$ in $6^{\prime \prime}$ and wider cuttings. Sound knots that do not exceed in their greatest dimension one-half the width of the cutting and other defects that do not impair the strength of the cutting more than the above mentioned defects are admitted.

Rules apply to both faces of the piece.

## Quartered Sap Gum, Quartered Black Gum, and Quartered Tupelo

FAS: Standard. Except:
Widths $5^{\prime \prime}$ and wider; pieces $5^{\prime \prime}$ wide containing $3^{\prime}$ and $4^{\prime}$ surface measure shall be clear, pieces $5^{\prime \prime}$ wide containing $5^{\prime}$ to $7^{\prime}$ surface measure shall yield $11 / 12$ (91-2/3\%) clear-face in one cutting.

F1F:
SELECTS:
NO. 1 COMMON:
NO. 2A COMMON:
No figure is required.
Stain is admitted in all grades.
Pieces below the grade of No. 2A Common shall be graded as Sap Gum or Black Gum.

## Ribbon Stripe

When ribbon stripe figure is specified each piece shall be-selected for the stripe effect caused by the wavy grain brought out in quarter sawing. One face of each required cutting shall show $90 \%$ in the aggregate of such ribbon stripe figure.

## Plain Red Gum

Red Gum is lumber produced from the Sweet Gum tree, containing sufficient heartwood to be admitted into the grades defined under the caption of Red Gum.

Stain is admitted in the sapwood in all grades. Any part of the sapwood allowed may be included in the cuttings.
FAS: Standard, except:
FAS will admit $1^{\prime \prime}$ of sapwood in the aggregate on one face and one-fifth of the surface in the aggregate on the reverse side.
F1F: Standard.
SELECTS: Standard, except:

Pieces $4^{\prime \prime}$ and $5^{\prime \prime}$ wide and pieces $6^{\prime}$ and $7^{\prime}$ long shall be free of sapwood on one face; pieces $6^{\prime \prime}$ and wider $8^{\prime}$ and longer will admit $1^{\prime \prime}$ of sapwood in the aggregate on one face; such faces shall meet the grading requirements of Standard Selects. Unlimited sapwood is admitted on the reverse side.

NO. 1 COMMON: Standard, except:
Each cutting shall have one clear heartwood face.
NO. 2A COMMON: Standard, except:
Each cutting shall have one clear heartwood face.
Pieces below the grade of No. 2A Common shall be graded as Sap Gum.

## Plain Sawn Red Gum, Figured Wood

Each piece shall be especially selected for markings and color tones of spots and streaks producing a variegated effect on the surface.

One piece of each required cutting shall show $90 \%$ in the aggregate of such markings and color tones, with the exception that unfigured spaces not exceeding $1^{\prime \prime}$ by $24^{\prime \prime}$ or its equivalent in area between spots and streaks, shall be disregarded.

Otherwise the rules for Plain Red Gum shall apply.

## Quartered Red Gum

(No figure is required. Stain is admitted in the sapwood in all grades.)
FAS: Standard, except:
Widths $5^{\prime \prime}$ and wider; pieces $5^{\prime \prime}$ wide containing $3^{\prime}$ and $4^{\prime}$ surface measure shall be clear, pieces $5^{\prime \prime}$ wide containing $5^{\prime}$ to $7^{\prime}$ surface measure shall yield $11 / 12$ ( $91-2 / 3 \%$ ) clear-face in one cutting.

In FAS, pieces $5^{\prime \prime}$ wide shall be free of sapwood on one face; pieces $6^{\prime \prime}$ and $7^{\prime \prime}$ wide may have $3 / 4^{\prime \prime}$ of sapwood in the aggregate on one face; pieces $8^{\prime \prime}$ and wider may have $1^{\prime \prime}$ of sapwood in the aggregate on one face. The reverse side of any piece will admit sapwood aggregating one-fifth of its surface. Any part of the sapwood allowed may be included in the cuttings.

## SELECTS: Standard, except:

Pieces $4^{\prime \prime}$ and $5^{\prime \prime}$ wide shall be free of sapwood on one face; pieces $6^{\prime \prime}$ and $7^{\prime \prime}$ wide will admit $3 / 4^{\prime \prime}$ of sapwood and pieces $8^{\prime \prime}$ and wider $1^{\prime \prime}$ of sapwood in the aggregate on one face, which faces shall meet the grading requirements of Standard Selects. Unlimited sapwood is admitted on the reverse side.
NO. 1 COMMON: Standard, except:
Each cutting shall have one clear heartwood face.

## Bridge Plank and Crossing Plank

Widths: $6^{\prime \prime}$ and wider.
Lengths: $8^{\prime}$ to $16^{\prime}, 25 \%$ of the pieces in a shipment may be $1 / 4^{\prime \prime}$ scant in thickness.
Will admit pin, shot, and spot worm holes; an occasional grub or knot hole; sound knots; split in each end not exceeding in length the width of the piece.

PITH: Firm pith may be admitted on one face in pieces $2-1 / 4^{\prime \prime}$ or less in thickness. Firm pith may be admitted, either boxed or on one face, in pieces $2-1 / 2^{\prime \prime}$ and thicker.

SHAKE: Shake may be admitted on the pith face not to exceed one-third the length of the piece in the aggregate.

In this grade no shake shall be admitted that extends from edge to edge; from edge to either face; or from one face to the opposite face.

In planking $18^{\prime}$ and longer, pith may be admitted on one face. On the opposite face it may be admitted up to one-sixth of the length of the piece in the aggregate.

In planking $18^{\prime}$ and longer, shake may be admitted to the extent permitted in standard lengths.
WANE: One face and two edges shall be sound except that wane not exceeding one-third the length, one-third the width, and one-third the thickness of the piece will be admitted.

## Mine Lumber and Timber Products

## Mixed Hardwood

Cribbing Blocks, Mine Caps, Wedges, Mine Rails, Mine Ties, Headers, Bars
Will admit pith, boxed or showing on one face and one edge; knots; season checks; splits and other defects that do not impair the strength or prevent the use of the piece in its full size for purposes of strength. Wane not exceeding one-third the width or the thickness is admitted on one comer, or its aggregate equivalent on two or more comers, or it may extend across only one face for one-third the length to a depth not exceeding one-twelfth the distance to the opposite face.

## Sheet Piling, Sewer Sheathing, Hardwood Hearts

Will admit pith, boxed or showing on the surface; knots; checks; splits and defects commonly found in heart stock that do not seriously impair the strength or prevent the use of the piece in its full size. Wane not exceeding one-third the width or the thickness is admitted on one comer, or its aggregate equivalent on two or more comers, or it may extend across only one face for one-third the length to a depth not exceeding one-twelfth the distance to the opposite face.

## Military or Commercial Timbers and Planking

This designation of quality shall consist of the grades of "Select Car Stock"; " Dimension"; and "Sound Square Edge." Timbers or planking sold in accordance with this designation must contain not less than $50 \%$ of the quality of Common Dimension and Better, of which $50 \%$, one-half, must be of the grade of Select Car Stock. No material lower in quality than that defined under the caption of SOUND SQUARE EDGE shall be admitted.

## Hardwood Construction and Utility Boards

Finish and Dimension
GRADES: "A" finish—"B" finish, NO. 1, NO. 2, and NO. 3 Construction Boards and Utility Boards, No. 1 and No. 2 Dimension. Rough or dressed as specified.

| Nominal Rough and Dressed Thicknesses |  |  |  |  |  |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Nominal Rough | $1^{\prime \prime}$ | $1-1 / 4^{\prime \prime}$ | $1-1 / 2^{\prime \prime}$ | $2^{\prime \prime}$ | $1-1 / 2^{\prime \prime}$ | $3^{\prime \prime}$ |
| S1S or S2S | $25 / 32^{\prime \prime}$ | $1-1 / 16^{\prime \prime}$ | $1-5 / 16^{\prime \prime}$ | $1-5 / 8^{\prime \prime}$ | $2-1 / 8^{\prime \prime}$ | $2-5 / 8^{\prime \prime}$ |


| Nominal and Dressed Widths |  |  |  |
| :---: | :---: | :---: | :---: |
| Nominal | Finish <br> SIE or S2E | Construction and Utility Boards SIE or S2E | Dimension SIE or S2E |
| 3 in . | 2-5/8 in. | 2-5/8 in. |  |
| 4 in . | 3-5/8 in. | 3-5/8 in. | 3-5/8 in. |
| 5 in . | 4-5/8 in. | 4-5/8 in. |  |
| 6 in. | 5-5/8 in. | 5-5/8 in. | 5-5/8 in. |
| 7 in . | 6-5/8 in. | 6-5/8 in. |  |
| 8 in . | 7-1/2 in. | 7-1/2 in. | 7-1/2 in. |
| 9 in . | 8-1/2 in. | 8-1/2 in. |  |
| 10 in . | 9-1/2 in. | 9-1/2 in. | 9-1/2 in. |
| 11 in . | 10-1/2 in. | 10-1/2 in. |  |
| 12 in . | 11-1/2 in. | 11-1/2 in. | 11-1/2 in. |
| Over 12 in . | off $5 / 8 \mathrm{in}$. | off $5 / 8 \mathrm{in}$. | off 5/8 in. |

No. 1 Common admits pieces that will yield clear-face cuttings as follows:

| Surface Measure of Piece | Percentage of Yield | Number of Cuttings |
| :--- | :---: | :---: |
| $2^{\prime}$ | 75 | 1 |
| $3^{\prime}$ and $4^{\prime}$ | $66-2 / 3$ | 1 |
| $5^{\prime}$ to $7^{\prime}$ | 75 | 2 |
|  | $66-2 / 3$ | 2 |
| $8^{\prime}$ to $11^{\prime \prime}$ | 75 | 3 |
| $12^{\prime}$ and over | $66-2 / 3$ | 3 |
|  | $66-2 / 3$ | 4 |

NO. 2A COMMON: Standard, except
Lengths: 6 ft . and longer.
There is no limit to the number of cuttings.
NO. 2B COMMON: All the requirements for No. 2A Common Mahogany shall apply except cuttings to be sound as defined in Sound Cutting.
NO. 3 COMMON: Standard, to include No. 3A Common and No. 3B Common as one grade, except lengths are 6 ft . and longer.

## Tropical American, African Mahogany, and Spanish Cedar FAS 61 to 7-11" long (When Specified)

Widths: $6^{\prime \prime}$ and wider.
Pieces $3^{\prime}$ surface measure shall be clear; $4^{\prime}$ and over surface measure shall grade the same as Standard lengths in these woods.

## Pin Wormy Mahogany

SELECTED FAS PIN WORMY (N.O. GRADE):
Widths: $6^{\prime \prime}$ and wider.
Lengths: $6^{\prime}$ and longer.

One face of each board shall yield $75 \%$ clear of pin worm holes and other defects, in cuttings of not less than 144 square inches each and without limit to the number of cuttings. The reverse side shall grade No. 1 Common Pin Wormy (N Wormy) or better.

FAS PIN WORMY (A WORMY):
Widths: $6^{\prime \prime}$ and wider.
Lengths: $6^{\prime}$ and longer.
Shall grade FAS Mahogany except as to lengths, and except that pin worm holes or grooves, sapwood, and stain will be admitted in the cuttings. Black track worm grooves shall not be admitted in the cuttings.

## NO. 1 COMMON PIN WORMY (N WORMY);

Widths: $4^{\prime \prime}$ and wider.
Lengths: $6^{\prime}$ and longer.
Shall grade No. 1 Common except that pin worm holes or grooves, burls, stain, $3 / 4^{\prime \prime}$ sound knots, and equivalent defects are admitted in the cuttings, and that $50 \%$ of the required cuttings in the aggregate shall be free of black track worm grooves.

NO. 2 COMMON PIN WORMY (B WORMY):
Widths: $3^{\prime \prime}$ and wider.
Lengths: $6^{\prime}$ and longer.
Shall grade No. 2 Common except that pin worm holes or grooves, burls, stain, small checks, $3 / 4^{\prime \prime}$ sound knots, and equivalent defects are admitted in the cuttings. Black track worm grooves are admitted without limit in the cuttings.

## Mahogany Shorts

GRADES: FAS Shorts, Common Shorts and Pin Wormy Shorts.
Standard Lengths: $2^{\prime \prime}, 2-1 / 4^{\prime}, 2-1 / 2^{\prime}, 2-3 / 4^{\prime}, 3^{\prime}, 3-1 / 4^{\prime \prime}, 3-1 / 2^{\prime}, 3-3 / 4^{\prime}, 4^{\prime}, 4-1 / 4^{\prime}, 4-1 / 2^{\prime}, 4-3 / 4^{\prime}, 5^{\prime \prime}, 5-1 / 4^{\prime}, 5-1 / 2^{\prime}$.
Lengths other than standard shall be measured as of the next lower standard length.
Shorts shall be measured and tallied as if four times the actual standard length and the resulting total divided by four.

FAS:
Widths: $4^{\prime \prime}$ and wider.
Pieces $4^{\prime \prime}$ and $5^{\prime \prime}$ wide shall be clear.
Pieces $6^{\prime \prime}$ and wider will admit standard defects or their equivalent according to the above basis of surface measure (four times the actual surface measure) as follows:
$8^{\prime}, 1 ; 16^{\prime}, 2 ; 22^{\prime}, 3 ; 26^{\prime}, 4$.
COMMON SHORTS:
Widths: $3^{\prime \prime}$ and wider.
Shall yield $50 \%$ clear face in not over two cuttings.
No cutting containing less than 36 square inches shall be considered.
PIN WORMY SHORTS: Widths: $3^{\prime \prime}$ and wider.
Shall grade First and Seconds Shorts except as to minimum width, and pin worm holes or grooves, burls and stain are admitted.

## Mahogany Strips

Inspection shall be made from the better face of the piece.
Odd lengths are admitted without limit. Fractions over one-half foot in length shall be counted up, and fractions of one-half foot or less in length shall be dropped. This does not change the minimum length requirement of Strips.

The widths in Clear and No. 1 Common Strips are $2^{\prime \prime}, 2-1 / 2^{\prime \prime}, 3^{\prime \prime}, 3-1 / 2^{\prime \prime}, 4^{\prime \prime}, 4-1 / 2^{\prime \prime}, 5^{\prime \prime}$ and $5-1 / 2^{\prime \prime}$.
Strips may be $1 / 8^{\prime \prime}$ scant in width when shipping dry. In Clear Strips, tapering pieces shall be measured at the narrow end. In the grades of No. 1 Common and Wormy Strips, tapering pieces shall be measured one-third the length of the piece from the narrow end.

Sapwood is admitted without limit in all grades.
CLEAR:
Lengths: $6^{\prime}$ and longer.
Shall have one clear face, the reverse side will admit wane or its equivalent in other defects, not exceeding one-third the length, one-third the width and one-third the thickness of the piece and shall otherwise be sound as defined in "Sound Cutting."

NO. 1 COMMON:
Lengths: $6^{\prime}$ and longer.
Both edges of pieces $6^{\prime}$ and $7^{\prime}$ long and both edges of each cutting in $8^{\prime}$ and longer shall be clear. In addition to the above requirements, pieces $6^{\prime}$ and $7^{\prime}$ long will admit one standard defect; $8^{\prime}$ and longer shall yield $66-2 / 3 \%$ clear face in not over two cuttings in $8^{\prime}$ to $11^{\prime}$ and not over three cuttings in $12^{\prime}$ and longer. No cutting shall be less than $2^{\prime}$ long nor less than $2^{\prime \prime}$ wide in pieces $2^{\prime \prime}$ and $2-1 / 2^{\prime \prime}$ wide, nor less than $3^{\prime \prime}$ wide in pieces $3^{\prime \prime}$ and wider.

The reverse side of the cuttings to be sound as defined in "Sound Cutting."

## WORMY:

Lengths: $6^{\prime}$ and longer.
Widths: $1-1 / 2^{\prime \prime}, 2^{\prime \prime}, 3^{\prime \prime}, 3-1 / 2^{\prime \prime}$, and $3-3 / 4^{\prime \prime}$.
Shall yield $50 \%$ sound, no cutting to be considered that is less than $1-1 / 2^{\prime \prime}$ wide by $2^{\prime}$ long. The number of cuttings not limited. Pin worm holes, clear or stained pin worm grooves, burls, stain, small checks, sound knots not over $3 / 4^{\prime \prime}$ in average diameter, or other sound defects not exceeding in extent or damage the defects described will be admitted without limit.

One edge of each piece shall be square; the other edge will admit wane not exceeding in thickness or width the thickness of the piece and not exceeding one-third the length of the piece or the equivalent of such aggregate wane at one or both ends.

## Philippine Mahogany

"Philippine Red Mahogany" includes Tanguile, Red Lauan, and Tiaong.
"Light Red Philippine Mahogany" includes Almond, Bagtican, Mayapis, and White Lauan.
NOTE: National Hardwood Lumber Association inspectors will undertake to make distinction between Philippine Red Mahogany and Light Red Philippine Mahogany when required, but the Association does not assume financial liability with respect to color.

Odd lengths are admitted without limit.
In FAS, bright sapwood not exceeding in the aggregate one-third the width of the piece will be admitted on one face. Any part of the sapwood may be included in the cuttings.

COUNTERS (when specified):
Widths: $18^{\prime \prime}$ to $24^{\prime \prime}$.
Lengths: $12^{\prime}$ to $40^{\prime}$.

Counters shall be free of all defects on one face; the reverse side shall grade not below FAS. Splits shall be measured out.

FAS: Standard, except:
Pieces of $4^{\prime}$ and $5^{\prime}$ surface measure shall yield $11 / 12$ (91-2/3\%) clear face in one cutting.
F1F: Standard.
SELECTS: Standard, except:
Widths: $6^{\prime \prime}$ and wider.
Lengths: $8^{\prime}$ and longer.
NO. 1 COMMON: Standard, except:
Widths: $4^{\prime \prime}$ and wider
Lengths: $6^{\prime}$ and longer.
Splits exceeding in the aggregate in inches in length twice the surface measure of the piece in feet shall not be admitted.

NO. 2A COMMON: Standard, except:
Lengths: $6^{\prime}$ and longer.
There is no limit to the number of cuttings.
NO. 2B COMMON: All the requirements for No. 2A Common Philippine Mahogany shall apply except cuttings to be sound as defined in Sound Cutting.
NO. 3 COMMON: Standard, to include No. 3A Common and No. 3B Common as one grade, except lengths are $2^{\prime}$ and longer.

## Pin Wormy Philippine

FAS PIN WORMY: Same as FAS, except:
Pin worm holes are admitted without limit.
Widths: $6^{\prime \prime}$ and wider.
Lengths: $8^{\prime}$ and longer.
Scattered stained pin worm grooves not exceeding $25 \%$ of the required cutting area are admitted.
NO. 1 COMMON WORMY: Same as No. 1 Common except:
Pin worm holes, stained or otherwise, pin worm grooves, burls, sound pin knots not exceeding $1 / 2^{\prime \prime}$ in diameter or other sound defects that do not exceed in extent of damage the defects described, are admitted.

## Philippine Mahogany Shorts

FAS SHORTS:
Widths: $3^{\prime \prime}$ and wider.
Standard Lengths: $2^{\prime} 2-1 / 2^{\prime}, 3^{\prime}, 3-1 / 2^{\prime}, 4^{\prime}, 4-1 / 2^{\prime}, 5^{\prime}$ and $5-1 / 2^{\prime}$.
Lengths other than standard shall be measured as of the next lower standard length.
Shorts shall be measured and tallied as if four times the actual standard length, and the resulting total divided by four.

Pieces $3^{\prime \prime}$ to $5^{\prime \prime}$ wide shall be clear.
Pieces $6^{\prime \prime}$ and wider will admit standard defects or their equivalent according to the above basis of surface Measure (four times the actual surface measure) as follows:

Widths: $3^{\prime \prime}$ and wider.
Standard Lengths: $2^{\prime}, 2-1 / 2^{\prime}, 3^{\prime}, 3-1 / 2^{\prime}, 4^{\prime}, 4-1 / 2^{\prime}, 5^{\prime}$ and $5-1 / 2^{\prime}$.
Lengths other than standard shall be measured as of the next lower standard length.
Shorts shall be measured and tallied as if four times the actual length and the resulting total divided by four.
Shall yield $50 \%$ clear face in not over two cuttings. No cutting containing less than 36 square inches shall be considered.

## Strips

## STRIPS:

CLEAR: Standard Strip Grade, except:
Lengths: $6^{\prime}$ and longer, admitting $10 \%$ of $6^{\prime}$ and $7^{\prime}$.
NO. 1 COMMON: Standard Strip Grade, except:
$6^{\prime}$ and $7^{\prime}$ lengths may have one standard defect. Each cutting shall have clear edges.

## Flitches

CLEAR VENEER FLITCHES:
Thicknesses: $6^{\prime \prime}$ and thicker.
Widths: $8^{\prime \prime}$ and wider.
Lengths: $8^{\prime}$ and longer.
Shall be clear on one face and two edges; the reverse side shall grade not below FAS. Knots admitted on the reverse side shall be sound and not exceeding $3 / 4^{\prime \prime}$ in diameter. Sapwood and worm defects are not admitted.

PIN WORMY FLITCHES:
Thicknesses: $6^{\prime \prime}$ and thicker.
Widths: $8^{\prime \prime}$ and wider.
Lengths: $8^{\prime}$ and longer.
Will admit stained pin worm holes and grooves, pin knots not exceeding $5 / 8^{\prime \prime}$ in diameter without limit. Sapwood not exceeding one-sixth the thickness and showing on one face will be admitted.

NO. 1 COMMON FLITCHES:
Thicknesses: $6^{\prime \prime}$ and thicker.
Widths: $8^{\prime \prime}$ and wider.
Lengths: $8^{\prime}$ and longer.
Shall yield $66-2 / 3 \%$ in clear-face cuttings.
Minimum cutting $4^{\prime \prime}$ wide by $3^{\prime}$ long.

## Ribbon Stripe

When ribbon stripe figure is specified in lumber or flitches, each piece shall be selected for the stripe effect brought out in quarter sawing. One face of each required cutting shall show $90 \%$ or more in the aggregate of such ribbon stripe figure.

## Apitong and Other Philippine Hardwoods

All other Philippine hardwoods shall be graded under the rules for the inspection of Philippine Mahogany.

NOTE: National Hardwood Lumber Association inspectors will not accept responsibility for the distinction between Apitong and other Philippine hardwoods included in this classification and when issuing Association certificates for the grading of these woods will use the term "Said to be."

## North American, Tropical American, and African Hardwoods (Other than Mahogany and Spanish Cedar)

Unless otherwise specified, North American, Tropical American and African hardwoods for which there are no established grading rules, shall be graded under the STANDARD GRADES.

NOTE: National inspectors will not accept responsibility for distinguishing the species of woods included in this classification and when issuing certificates for the grading will use the term "Said to be."

## Aromatic Red Cedar

GRADES: NO. 1 COMMON AND BETTER and NO. 2A COMMON:
Will admit sound knots, white streaks and firm, tight pith in the cuttings, which otherwise shall be sound.
No cutting may contain sapwood in the aggregate exceeding one-sixth the heartwood side. Unlimited sapwood is admitted on the reverse side.

Variation in thickness may be $1 / 2^{\prime \prime}$ on $4 / 4^{\prime \prime}$ to $8 / 4^{\prime \prime}$.
Thicknesses: Standard.
NO. 1 COMMON AND BETTER:
Widths: $3^{\prime \prime}$ and wider, admitting $25 \%$ of $3^{\prime \prime}$ width.
Lengths: $3^{\prime}$ and longer.
Minimum cutting: $3^{\prime \prime}$ wide by $2^{\prime}$ long or $2^{\prime \prime}$ wide by $3^{\prime}$ long.
Each piece shall yield not less than $66-2 / 3 \%$ of cuttings.
There is no limit to the number of cuttings.
NO. 2A COMMON:
Widths: $2^{\prime \prime}$ and wider, admitting- $35 \%$ of $2^{\prime \prime}$ width.
Lengths: $2^{\prime}$ and longer.
Minimum Cutting: $2^{\prime \prime}$ or wider containing not less than 48 square inches.
Each piece shall yield not less than $50 \%$ of cuttings.
There is no limit to the number of cuttings.

## Pacific Coast Red Alder, Pacific Coast Maple

When Pacific Coast Alder or Maple is sold and specified "Pin knots no defect," knots or their equivalent, not exceeding $1 / 4^{\prime \prime}$ in their greatest dimension, sound or containing unsound centers not over $1 / 8^{\prime \prime}$ in diameter, shall be admitted in the cuttings.

The General Instructions and Standard Grades (pages 6-21) shall govern the measurement and inspection of all commercial hardwoods indigenous to the Northwest hardwood belt, with the exceptions as set forth under the respective species.

These rules shall apply to green, dry, rough, or surfaced lumber.
The better face of boards in all thicknesses shall yield not less than the minimum percentage of cuttings required for the grade; the reverse side of the cuttings in all thicknesses shall be sound as defined in "SOUND CUTTING," or better, unless otherwise specified.

No exception shall be made to these rules unless agreed to by the seller and the buyer and specifically stated in the purchase order.

Strength Properties of Some Commercially Important Woods Grown in the United States (inch-pound) ${ }^{\text {a }}$

| Common species names | Moisture content | Specific gravity ${ }^{\text {b }}$ | Modulus <br> of <br> rupture <br> ( $\mathrm{lbf} / \mathrm{in}^{2}$ ) | Static bending |  | Impact bending (in.) | Compression parallel to grain (lbf/in ${ }^{2}$ ) | Compression perpendicular to grain (lbf/ $i n^{2}$ ) | Shear parallel to grain (lbf/in ${ }^{2}$ ) | Tension perpendicular to grain (lbf/ $i n^{2}$ ) | Side harness (lbf) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Modulus of elasticity ${ }^{\text {c }}$ ( $\times 10^{6} \mathrm{lbf} /$ $i n^{2}$ ) | Work to maximum load (inlbf/in ${ }^{3}$ ) |  |  |  |  |  |  |
| Hickory, pecan | Green | 0.60 | 10,300 | 1.40 | 20.0 | 66 | 4,570 | 800 | 1,240 | - | - |
| Bitternut | 12\% | 0.66 | 17,100 | 1.79 | 18.2 | 66 | 9,040 | 1,680 | - | - | - |
| Nutmeg | Green | 0.56 | 9,100 | 1.29 | 22.8 | 54 | 3,980 | 760 | 1,030 | - | - |
|  | 12\% | 0.60 | 16,600 | 1.70 | 25.1 | - | 6,910 | 1,570 | - | - | - |
| Pecan | Green | 0.60 | 9,800 | 1.37 | 14.6 | 53 | 3,990 | 780 | 1,480 | 680 | 1,310 |
|  | 12\% | 0.66 | 13,700 | 1.73 | 13.8 | 44 | 7,850 | 1,720 | 2,080 | - | 1,820 |
| Water | Green | 0.61 | 10,700 | 1.56 | 18.8 | 56 | 4,660 | 880 | 1,440 | - |  |
|  | 12\% | 0.62 | 17,800 | 2.02 | 19.3 | 53 | 8,600 | 1,550 | - | - | - |
| Hickory, true | Green | 0.64 | 11,100 | 1.57 | 26.1 | 88 | 4,480 | 810 | 1,280 | - | - |
| Mockernut | 12\% | 0.72 | 19,200 | 2.22 | 22.6 | 77 | 8,940 | 1,730 | 1,740 | - | - |
| Pignut | Green | 0.66 | 11,700 | 1.65 | 31.7 | 89 | 4,810 | 920 | 1,370 | - | - |
|  | 12\% | 0.75 | 20,100 | 2.26 | 30.4 | 74 | 9,190 | 1,980 | 2,150 | - | - |
| Shagbark | Green | 0.64 | 11,000 | 1.57 | 23.7 | 74 | 4,580 | 840 | 1,520 | - | - |
|  | 12\% | 0.72 | 20,200 | 2.16 | 25.8 | 67 | 9,210 | 1,760 | 2,430 | - | - |
| Shellbark | Green | 0.62 | 10,500 | 1.34 | 29.9 | 104 | 3,920 | 810 | 1,190 | - | - |
|  | 12\% | 0.69 | 18,100 | 1.89 | 23.6 | 88 | 8,000 | 1,800 | 2,110 | - | - |
| Honeylocust | Green | 0.60 | 10,200 | 1.29 | 12.6 | 47 | 4,420 | 1,150 | 1,660 | 930 | 1,390 |
|  | 12\% | - | 14,700 | 1.63 | 13.3 | 47 | 7,500 | 1,840 | 2,250 | 900 | 1,580 |
| Locust, black | Green | 0.66 | 13,800 | 1.85 | 15.4 | 44 | 6,800 | 1,160 | 1,760 | 770 | 1,570 |
|  | 12\% | 6.69 | 19,400 | 2.05 | 18.4 | 57 | 10,180 | 1,830 | 2,480 | 640 | 1,700 |
| Magnolia |  |  |  |  |  |  |  |  |  |  |  |
| Cucumbertree | Green | 0.44 | 7,400 | 1.56 | 10.0 | 30 | 3,140 | 330 | 990 | 440 | 520 |
|  | 12\% | 0.48 | 12,300 | 1.82 | 12.2 | 35 | 6,310 | 570 | 1,340 | 660 | 700 |
| Southern | Green | 0.46 | 6,800 | 1.11 | 15.4 | 54 | 2,700 | 460 | 1,040 | 610 | 740 |
|  | 12\% | 0.50 | 11,200 | 1.40 | 12.8 | 29 | 5,460 | 860 | 1,530 | 740 | 1,020 |
| Maple |  |  |  |  |  |  |  |  |  |  |  |
| Bigleaf | Green | 0.44 | 7,400 | 1.10 | 8.7 | 23 | 3,240 | 450 | 1,110 | 600 | 620 |
|  | 12\% | 0.48 | 10,700 | 1.45 | 7.8 | 28 | 5,950 | 750 | 1,730 | 540 | 850 |
| Black | Green | 0.52 | 7,900 | 1.33 | 12.8 | 48 | 3,270 | 600 | 1,130 | 720 | 840 |
|  | 12\% | 0.57 | 13,300 | 1.62 | 12.5 | 40 | 6,680 | 1,020 | 1,820 | 670 | 1,180 |


| Red | Green | 0.49 | 7,700 | 1.39 | 11.4 | 32 | 3,280 | 400 | 1,150 | - | 700 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 12\% | 0.54 | 13,400 | 1.64 | 12.5 | 32 | 6,540 | 1,000 | 1,850 | - | 950 |
| Silver | Green | 0.44 | 5,800 | 0.94 | 11.0 | 29 | 2,490 | 370 | 1,050 | 560 | 590 |
|  | 12\% | 0.47 | 8,900 | 1.14 | 8.3 | 25 | 5,220 | 740 | 1,480 | 500 | 700 |
| Sugar | Green | 0.56 | 9,400 | 1.55 | 13.3 | 40 | 4,020 | 640 | 1,460 | - | 970 |
|  | 12\% | 0.63 | 15,800 | 1.83 | 16.5 | 39 | 7,830 | 1,470 | 2,330 | - | 1,450 |
| Oak, red |  |  |  |  |  |  |  |  |  |  |  |
| Black | Green | 0.56 | 8,200 | 1.18 | 12.2 | 40 | 3,470 | 710 | 1,220 | - | 1,060 |
|  | 12\% | 0.61 | 13,900 | 1.64 | 13.7 | 41 | 6,520 | 930 | 1,910 | - | 1,210 |
| Cherrybark | Green | 0.61 | 10,800 | 1.79 | 14.7 | 54 | 4,620 | 760 | 1,320 | 800 | 1,240 |
|  | 12\% | 0.68 | 18,100 | 2.28 | 18.3 | 49 | 8,740 | 1,250 | 2,000 | 840 | 1,480 |
| Laurel | Green | 0.56 | 7,900 | 1.39 | 11.2 | 39 | 3,170 | 570 | 1,180 | 770 | 1,000 |
|  | 12\% | 0.63 | 12,600 | 1.69 | 11.8 | 39 | 6,980 | 1,060 | 1,830 | 790 | 1,210 |
| Northern red | Green | 0.56 | 8,300 | 1.35 | 13.2 | 44 | 3,440 | 610 | 1,210 | 750 | 1,000 |
|  | 12\% | 0.63 | 14,300 | 1.82 | 14.5 | 43 | 6,760 | 1,010 | 1,780 | 800 | 1,290 |
| Pin | Green | 0.58 | 8,300 | 1.32 | 14.0 | 48 | 3,680 | 720 | 1,290 | 800 | 1,070 |
|  | 12\% | 0.63 | 14,000 | 1.73 | 14.8 | 45 | 6,820 | 1,020 | 2,080 | 1,050 | 1,510 |
| Scarlet | Green | 0.60 | 10,400 | 1.48 | 15.0 | 54 | 4,090 | 830 | 1,410 | 700 | 1,200 |
|  | 12\% | 0.67 | 17,400 | 1.91 | 20.5 | 53 | 8,330 | 1,120 | 1,890 | 870 | 1,400 |
| Southern red | Green | 0.52 | 6,900 | 1.14 | 8.0 | 29 | 3,030 | 550 | 930 | 480 | 860 |
|  | 12\% | 0.59 | 10,900 | 1.49 | 9.4 | 26 | 6,090 | 870 | 1,390 | 510 | 1,060 |
| Port-Orford | Green | 0.39 | 6,600 | 1.30 | 7.4 | 21 | 3,140 | 300 | 840 | 180 | 380 |
|  | 12\% | 0.43 | 12,700 | 1.70 | 9.1 | 28 | 6,250 | 720 | 1,370 | 400 | 630 |
| Western redeedar Yellow | Green | 0.31 | 5,200 | 0.94 | 5.0 | 17 | 2,770 | 240 | 770 | 230 | 260 |
|  | 12\% | 0.32 | 7,500 | 1.11 | 5.8 | 17 | 4,560 | 460 | 990 | 220 | 350 |
|  | Green | 0.42 | 6,400 | 1.14 | 9.2 | 27 | 3,050 | 350 | 840 | 330 | 440 |
|  | 12\% | 0.44 | 11,100 | 1.42 | 10.4 | 29 | 6,310 | 620 | 1,130 | 360 | 580 |
| Douglas-fir ${ }^{\text {d }}$ |  |  |  |  |  |  |  |  |  |  |  |
| Coast | Green | 0.45 | 7,700 | 1.56 | 7.6 | 26 | 3,780 | 380 | 900 | 300 | 500 |
|  | 12\% | 0.48 | 12,400 | 1.95 | 9.9 | 31 | 7,230 | 800 | 1,130 | 340 | 710 |
| Interior West | Green | 0.46 | 7,700 | 1.51 | 7.2 | 26 | 3,870 | 420 | 940 | 290 | 510 |
|  | 12\% | 0.50 | 12,600 | 1.83 | 10.6 | 32 | 7,430 | 760 | 1,290 | 350 | 660 |
| Interior North | Green | 0.45 | 7,400 | 1.41 | 8.1 | 22 | 3,470 | 360 | 950 | 340 | 420 |
|  | 12\% | 0.48 | 13,100 | 1.79 | 10.5 | 26 | 6,900 | 770 | 1,400 | 390 | 600 |
| Interior South | Green | 0.43 | 6,800 | 1.16 | 8.0 | 15 | 3,110 | 340 | 950 | 250 | 360 |
|  | 12\% | 0.46 | 11,900 | 1.49 | 9.0 | 20 | 6,230 | 740 | 1,510 | 330 | 510 |
| Fir |  |  |  |  |  |  |  |  |  |  |  |
| Balsam | Green | 0.33 | 5,500 | 1.25 | 4.7 | 16 | 2,630 | 190 | 662 | 180 | 290 |
|  | 12\% | 0.35 | 9,200 | 1.45 | 5.1 | 20 | 5,280 | 404 | 944 | 180 | 400 |
| California red | Green | 0.36 | 5,800 | 1.17 | 6.4 | 21 | 2,760 | 330 | 770 | 380 | 360 |
|  | 12\% | 0.38 | 10,500 | 1.50 | 8.9 | 24 | 5,460 | 610 | 1,040 | 390 | 500 |
| Grand | Green | 0.35 | 5,800 | 1.25 | 5.6 | 22 | 2,940 | 270 | 740 | 240 | 360 |
|  | 12\% | 0.37 | 8,900 | 1.57 | 7.5 | 28 | 5,290 | 500 | 900 | 240 | 490 |
| Noble | Green | 0.37 | 6,200 | 1.38 | 6.0 | 19 | 3,010 | 270 | 800 | 230 | 290 |

Strength Properties of Some Commercially Important Woods Grown in the United States (inch-pound) ${ }^{\text {a }}$-Cont'd

| Common species names | Moisture content | Specific gravity ${ }^{\text {b }}$ | Modulus of rupture ( $\mathrm{lbf} / \mathrm{in}^{2}$ ) | Static bending |  | Impact bending (in.) | Compression parallel to grain (lbf/in ${ }^{2}$ ) | Compression perpendicular to grain (lbf/ $i n^{2}$ ) | Shear parallel to grain ( $\mathrm{lbf} / \mathrm{in}^{2}$ ) | Tension perpendicular to grain (lbf/ $i n^{2}$ ) | Side harness (lbf) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Modulus of elasticity ${ }^{\text {c }}$ ( $\times 10^{6} \mathrm{lbf} /$ $i n^{2}$ ) | Work to maximum load (in$\mathrm{lbf} / \mathrm{in}^{3}$ ) |  |  |  |  |  |  |
| Pacific silver | 12\% | 0.39 | 10,700 | 1.72 | 8.8 | 23 | 6,100 | 520 | 1,050 | 220 | 410 |
|  | Green | 0.40 | 6,400 | 1.42 | 6.0 | 21 | 3,140 | 220 | 750 | 240 | 310 |
|  | 12\% | 0.43 | 11,000 | 1.76 | 9.3 | 24 | 6,410 | 450 | 1,220 | - | 430 |
| Subalpine | Green | 0.31 | 4,900 | 1.05 | - | - | 2,300 | 190 | 700 | - | 260 |
|  | 12\% | 0.32 | 8,600 | 1.29 | - | - | 4,860 | 390 | 1,070 | - | 350 |
| White | Green | 0.37 | 5,900 | 1.16 | 5.6 | 22 | 2,900 | 280 | 760 | 300 | 340 |
|  | 12\% | 0.39 | 9,800 | 1.50 | 7.2 | 20 | 5,800 | 530 | 1,100 | 300 | 480 |
| Hemlock |  |  |  |  |  |  |  |  |  |  |  |
| Eastern | Green | 0.38 | 6,400 | 1.07 | 6.7 | 21 | 3,080 | 360 | 850 | 230 | 400 |
|  | 12\% | 0.40 | 8,900 | 1.20 | 6.8 | 21 | 5,410 | 650 | 1,060 | - | 500 |
| Mountain | Green | 0.42 | 6,300 | 1.04 | 11.0 | 32 | 2,880 | 370 | 930 | 330 | 470 |
|  | 12\% | 0.45 | 11,500 | 1.33 | 10.4 | 32 | 6,440 | 860 | 1,540 | - | 680 |
| Western | Green | 0.42 | 6,600 | 1.31 | 6.9 | 22 | 3,360 | 280 | 860 | 290 | 410 |
|  | 12\% | 0.45 | 11,300 | 1.63 | 8.3 | 23 | 7,200 | 550 | 1,290 | 340 | 540 |
| Larch, western | Green | 0.48 | 7,700 | 1.46 | 10.3 | 29 | 3,760 | 400 | 870 | 330 | 510 |
|  | 12\% | 0.52 | 13,000 | 1.87 | 12.6 | 35 | 7,620 | 930 | 1,360 | 430 | 830 |
| Pine |  |  |  |  |  |  |  |  |  |  |  |
| Eastern white | Green | 0.34 | 4,900 | 0.99 | 5.2 | 17 | 2,440 | 220 | 680 | 250 | 290 |
|  | 12\% | 0.35 | 8,600 | 1.24 | 6.8 | 18 | 4,800 | 440 | 900 | 310 | 380 |
| Jack | Green | 0.40 | 6,000 | 1.07 | 7.2 | 26 | 2,950 | 300 | 750 | 360 | 400 |
|  | 12\% | 0.43 | 9,900 | 1.35 | 8.3 | 27 | 5,660 | 580 | 1,170 | 420 | 570 |
| Loblolly | Green | 0.47 | 7,300 | 1.40 | 8.2 | 30 | 3,510 | 390 | 860 | 260 | 450 |
|  | 12\% | 0.51 | 12,800 | 1.79 | 10.4 | 30 | 7,130 | 790 | 1,390 | 470 | 690 |
| Lodgepole | Green | 0.38 | 5,500 | 1.08 | 5.6 | 20 | 2,610 | 250 | 680 | 220 | 330 |
|  | 12\% | 0.41 | 9,400 | 1.34 | 6.8 | 20 | 5,370 | 610 | 880 | 290 | 480 |
| Longleaf | Green | 0.554 | 8,500 | 1.59 | 8.9 | 35 | 4,320 | 480 | 1,040 | 330 | 590 |
|  | 12\% | 0.59 | 14,500 | 1.98 | 11.8 | 34 | 8,470 | 960 | 1,510 | 470 | 870 |
| Pitch | Green | 0.47 | 6,800 | 1.20 | 9.2 | - | 2,950 | 360 | 860 | - | - |
|  | 12\% | 0.52 | 10,800 | 1.43 | 9.2 | - | 5,940 | 820 | 1,360 | - | - |
| Hardwoods |  |  |  |  |  |  |  |  |  |  |  |
| Alder, red | Green | 0.37 | 6,500 | 1.17 | 8.0 | 22 | 2,960 | 250 | 770 | 390 | 440 |
|  | 12\% | 0.41 | 9,800 | 1.38 | 8.4 | 20 | 5,820 | 440 | 1,080 | 420 | 590 |
| Ash |  |  |  |  |  |  |  |  |  |  |  |
| Black | Green | 0.45 | 6,000 | 1.04 | 12.1 | 33 | 2,300 | 350 | 860 | 490 | 520 |


| Blue | Green | 0.53 | 9,600 | 1.24 | 14.7 | - | 4,180 | 810 | 1,540 | - | - |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 12\% | 0.58 | 13,800 | 1.40 | 14.4 | - | 6,980 | 1,420 | 2,030 | - | - |
| Green | Green | 0.53 | 9,500 | 1.40 | 11.8 | 35 | 4,200 | 730 | 1,260 | 590 | 870 |
|  | 12\% | 0.56 | 14,100 | 1.66 | 13.4 | 32 | 7,080 | 1,310 | 1,910 | 700 | 1,200 |
| Oregon | Green | 0.50 | 7,600 | 1.13 | 12.2 | 39 | 3,510 | 530 | 1,190 | 590 | 790 |
|  | 12\% | 0.55 | 12,700 | 1.36 | 14.4 | 33 | 6,040 | 1,250 | 1,790 | 720 | 1,160 |
| White | Green | 0.55 | 9,500 | 1.44 | 15.7 | 38 | 3,990 | 670 | 1,350 | 590 | 960 |
|  | 12\% | 0.60 | 15,000 | 1.74 | 16.6 | 43 | 7,410 | 1,160 | 1,910 | 940 | 1,320 |
| Aspen |  |  |  |  |  |  |  |  |  |  |  |
| Bigtooth | Green | 0.36 | 5,400 | 1.12 | 5.7 | - | 2,500 | 210 | 730 | - | - |
|  | 12\% | 0.39 | 9,100 | 1.43 | 7.7 | - | 5,300 | 450 | 1,080 | - | - |
| Quaking | Green | 0.35 | 5,100 | 0.86 | 6.4 | 22 | 2,140 | 180 | 660 | 230 | 300 |
|  | 12\% | 0.38 | 8,400. | 1.18 | 7.6 | 21 | 4,250 | 370 | 850 | 260 | 350 |
| Basswood, | Green | 0.32 | 5,000 | 1.04 | 5.3 | 16 | 2,220 | 170 | 600 | 280 | 250 |
| American | 12\% | 0.37 | 8,700 | 1.46 | 7.2 | 16 | 4,730 | 370 | 990 | 350 | 410 |
| Beech, | Green | 0.56 | 8,600 | 1.38 | 11.9 | 43 | 3,550 | 540 | 1,290 | 720 | 850 |
| American | 12\% | 0.64 | 14,900 | 1.72 | 15.1 | 41 | 7,300 | 1,010 | 2,010 ${ }^{\prime}$ | 1,010 | 1,300 |
| Birch |  |  |  |  |  |  |  |  |  |  |  |
| Paper | Green | 0.48 | 6,400 | 1.17 | 16.2 | 49 | 2,360 | 270 | 840 | 380 | 560 |
|  | 12\% | 0.55 | 12,300 | 1.59 | 16.0 | 34 | 5,690 | 600 | 1,210 | - | 910 |
| Sweet | Green | 0.60 | 9,400 | 1.65 | 15.7 | 48 | 3,740 | 470 | 1,240 | 430 | 970 |
|  | 12\% | 0.65 | 16,900 | 2.17 | 18.0 | 47 | 8,540 | 1,080 | 2,240 | 950 | 1,470 |
| Yellow | Green | 0.55 | 8,300 | 1.50 | 16.1 | 48 | 3,380 | 430 | 1,110 | 430 | 780 |
|  | 12\% | 0.62 | 16,600 | 2.01 | 20.8 | 55 | 8,170 | 970 | 1,880 | 920 | 1,260 |
| Butternut | Green | 0.36 | 5,400 | 0.97 | 8.2 | 24 | 2,420 | 220 | 760 | 430 | 390 |
|  | 12\% | 0.38 | 8,100 | 1.18 | 8.2 | 24 | 5,110 | 460 | 1,170 | 440 | 490 |
| Cherry, black | Green | 0.47 | 8,000 | 1.31 | 12.8 | 33 | 3,540 | 360 | 1,130 | 570 | 660 |
|  | 12\% | 0.50 | 12,300 | 1.49 | 11.4 | 29 | 7,110 | 690 | 1,700 | 560 | 950 |
| Chestnut, | Green | 0.40 | 5,600 | 0.93 | 7.0 | 24 | 2,470 | 310 | 800 | 440 | 420 |
| American | 12\% | 0.43 | 8,600 | 1.23 | 6.5 | 19 | 5,320 | 620 | 1,080 | 460 | 540 |
| Cottonwood |  |  |  |  |  |  |  |  |  |  |  |
| Balsam, | Green | 0.31 | 3,900 | 0.75 | 4.2 | - | 1,690 | 140 | 500 | - | - |
| poplar | 12\% | 0.34 | 6,800 | 1.10 | 5.0 |  | 4,020 | 300 | 790 | - | - |
| Black | Green | 0.31 | 4,900 | 1.08 | 5.0 | 20 | 2,200 | 160 | 610 | 270 | 250 |
|  | 12\% | 0.35 | 8,500 | 1.27 | 6.7 | 22 | 4,500 | 300 | 1,040 | 330 | 350 |
| Eastern | Green | 0.37 | 5,300 | 1.01 | 7.3 | 21 | 2,280 | 200 | 680 | 410 | 340 |
|  | 12\% | 0.40 | 8,500 | 1.37 | 7.4 | 20 | 4,910 | 380 | 930 | 580 | 430 |

Strength Properties of Some Commercially Important Woods Grown in the United States (inch-pound) ${ }^{\text {a }}$-Cont'd

| Common species names | Moisture content | Specific gravity ${ }^{\text {b }}$ | Modulus <br> of <br> rupture <br> ( $\mathrm{lbf} / \mathrm{in}^{2}$ ) | Static bending |  | Impact bending (in.) | Compression parallel to grain (lbf/in ${ }^{2}$ ) | Compression perpendicular to grain (lbf/ $i n^{2}$ ) | Shear parallel to grain ( $\mathrm{lbf} / \mathrm{in}^{2}$ ) | Tension perpendicular to grain (lbf/ $i n^{2}$ ) | Side harness (lbf) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Modulus of elasticity ${ }^{\text {c }}$ ( $\times 10^{6} \mathrm{lbf} /$ $i n^{2}$ ) | Work to maximum load (in$\mathrm{lbf} / \mathrm{in}^{3}$ ) |  |  |  |  |  |  |
| Elm |  |  |  |  |  |  |  |  |  |  |  |
| American | Green | 0.46 | 7,200 | 1.11 | 11.8 | 38 | 2,910 | 360 | 1,000 | 590 | 620 |
|  | 12\% | 0.50 | 11,800 | 1.34 | 13.0 | 39 | 5,520 | 690 | 1,510 | 660 | 830 |
| Rock | Green | 0.57 | 9,500 | 1.19 | 19.8 | 54 | 3,780 | 610 | 1,270 | - | 940 |
|  | 12\% | 0.63 | 14,800 | 1.54 | 19.2 | 56 | 7,050 | 1,230 | 1,920 | - | 1,320 |
| Slippery | Green | 0.48 | 8,000 | 1.23 | 15.4 | 47 | 3,320 | 420 | 1,110 | 640 | 660 |
|  | 12\% | 0.53 | 13,000 | 1.49 | 16.9 | 45 | 6,360 | 820 | 1,630 | 530 | 860 |
| Hackberry | Green | 0.49 | 6,500 | 0.95 | 14.5 | 48 | 2,650 | 400 | 1,070 | 630 | 700 |
|  | 12\% | 0.53 | 11,000 | 1.19 | 12.8 | 43 | 5,440 | 890 | 1,590 | 580 | 880 |
| Pond | Green | 0.51 | 7,400 | 1.28 | 7.5 | - | 3,660 | 440 | 940 | - | - |
|  | 12\% | 0.56 | 11,600 | 1.75 | 8.6 | - | 7,540 | 910 | 1,380 | - | - |
| Ponderosa | Green | 0.38 | 5,100 | 1.00 | 5.2 | 21 | 2,450 | 280 | 700 | 310 | 320 |
|  | 12\% | 0.40 | 9,400 | 1.29 | 7.1 | 19 | 5,320 | 580 | 1,130 | 420 | 460 |
| Red | Green | 0.41 | 5,800 | 1.28 | 6.1 | 26 | 2,730 | 260 | 690 | 300 | 340 |
|  | 12\% | 0.46 | 11,000 | 1.63 | 9.9 | 26 | 6,070 | 600 | 1,210 | 460 | 560 |
| Sand | Green | 0.46 | 7,500 | 1.02 | 9.6 | - | 3,440 | 450 | 1,140 | - | - |
|  | 12\% | 0.48 | 11,600 | 1.41 | 9.6 | - | 6,920 | 836 | - | - | - |
| Shortleaf | Green | 0.47 | 7,400 | 1.39 | 8.2 | 30 | 3,530 | 350 | 910 | 320 | 440 |
|  | 12\% | 0.51 | 13,100 | 1.75 | 11.0 | 33 | 7,270 | 820 | 1,390 | 470 | 690 |
| Slash | Green | 0.54 | 8,700 | 1.53 | 9.6 | - | 3,820 | 530 | 960 | - | - |
|  | 12\% | 0.59 | 16,300 | 1.98 | 13.2 | - | 8,140 | 1020 | 1,680 | - | - |
| Spruce | Green | 0.41 | 6,000 | 1.00 | - | - | 2,840 | 280 | 900 | - | 450 |
|  | 12\% | 0.44 | 10,400 | 1.23 | - | - | 5,650 | 730 | 1,490 | - | 660 |
| Sugar | Green | 0.34 | 4,900 | 1.03 | 5.4 | 17 | 2,460 | 210 | 720 | 270 | 270 |
|  | 12\% | 0.36 | 8,200 | 1.19 | 5.5 | 18 | 4,460 | 500 | 1,130 | 350 | 380 |
| Virginia | Green | 0.45 | 7,300 | 1.22 | 10.9 | 34 | 3,420 | 390 | 890 | 400 | 540 |
|  | 12\% | 0.48 | 13,000 | 1.52 | 13.7 | 32 | 6,710 | 910 | 1,350 | 380 | 740 |
| Western white | Green | 0.35 | 4,700 | 1.19 | 5.0 | 19 | 2,430 | 190 | 680 | 260 | 260 |
|  | 12\% | 0.38 | 9,700 | 1.46 | 8.8 | 23 | 5,040 | 470 | 1,040 | - | 420 |
| Redwood |  |  |  |  |  |  |  |  |  |  |  |
| Old-growth | Green | 0.38 | 7,500 | 1.18 | 7.4 | 21 | 4,200 | 420 | 800 | 260 | 410 |
|  | 12\% | 0.40 | 10,000 | 1.34 | 6.9 | 19 | 6,150 | 700 | 940 | 240 | 480 |
| Young-growth | Green | 0.34 | 5,900 | 0.96 | 5.7 | 16 | 3,110 | 270 | 890 | 300 | 350 |
|  | 12\% | 0.35 | 7,900 | 1.10 | 5.2 | 15 | 5,220 | 520 | 1,110 | 250 | 420 |


| Spruce |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Black | Green | 0.38 | 6,100 | 1.38 | 7.4 | 24 | 2,840 | 240 | 739 | 100 | 370 |
|  | 12\% | 0.42 | 10,800 | 1.61 | 10.5 | 23 | 5,960 | 550 | 1,230 | - | 520 |
| Engelmann | Green | 0.33 | 4,700 | 1.03 | 5.1 | 16 | 2,180 | 200 | 640 | 240 | 260 |
|  | 12\% | 0.35 | 9,300 | 1.30 | 6.4 | 18 | 4,480 | 410 | 1,200 | 350 | 390 |
| Red | Green | 0.37 | 6,000 | 1.33 | 6.9 | 18 | 2,720 | 260 | 750 | 220 | 350 |
|  | 12\% | 0.40 | 10,800 | 1.61 | 8.4 | 25 | 5,540 | 550 | 1,290 | 350 | 490 |
| Sitka | Green | 0.37 | 5,700 | 1.23 | 6.3 | 24 | 2,670 | 280 | 760 | 250 | 350 |
|  | 12\% | 0.40 | 10,200 | 1.57 | 9.4 | 25 | 5,610 | 580 | 1,150 | 370 | 510 |
| White | Green | 0.33 | 5,000 | 1.14 | 6.0 | 22 | 2,350 | 210 | 640 | 220 | 320 |
|  | 12\% | 0.36 | 9,400 | 1.43 | 7.7 | 20 | 5,180 | 430 | 970 | 360 | 480 |
| Tamarack | Green | 0.49 | 7,200 | 1.24 | 7.2 | 28 | 3,480 | 390 | 860 | 260 | 380 |
|  | 12\% | 0.53 | 11,600 | 1.64 | 7.1 | 23 | 7,160 | 800 | 1,280 | 400 | 590 |


 tension is maximum tensile strength; and side hardness is hardness measured when load is perpendicular to grain.
${ }^{b}$ Specific gravity is based on weight when oven dry and volume when green or at $12 \%$ moisture content.
Modulus of elasticity measured from a simply supported, center-loaded beam, on a span depth ratio of 14/1. To correct for shear deflection, the modulus can be increased by $10 \%$.

 Arizona, and New Mexico

### 7.4.0 Equilibrium Moisture Content of Wood in Each of 50 States

Equilibrium Moisture Content of Wood, Exposed to Outdoor Atmosphere, in Several U.S. Locations in 1997

| State | City | Equilibrium moisture content ${ }^{\text {a }}$ (\%) |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Jan. | Feb. | Mar. | Apr. | May | June | July | Aug. | Sept. | Oct., | Nov. | Dec. |
| AK | Juneau | 16.5 | 16.0 | 15.1 | 13.9 | 13.6 | 13.9 | 15.1 | 16.5 | 18.1 | 18.0 | 17.7 | 18.1 |
| AL | Mobile | 13.8 | 13.1 | 13.3 | 13.3 | 13.4 | 13.3 | 14.2 | 14.4 | 13.9 | 13.0 | 13.7 | 14.0 |
| AZ | Flagstaff | 11.8 | 11.4 | 10.8 | 9.3 | 8.8 | 7.5 | 9.7 | 11.1 | 10.3 | 10.1 | 10.8 | 11.8 |
| AZ | Phoenix | 9.4 | 8.4 | 7.9 | 6.1 | 5.1 | 4.6 | 6.2 | 6.9 | 6.9 | 7.0 | 8.2 | 9.5 |
| AR | Little Rock | 13.8 | 13.2 | 12.8 | 13.1 | 13.7 | 13.1 | 13.3 | 13.5 | 13.9 | 13.1 | 13.5 | 13.9 |
| CA | Fresno | 16.4 | 14.1 | 12.6 | 10.6 | 9.1 | 8.2 | 7.8 | 8.4 | 9.2 | 10.3 | 13.4 | 16.6 |
| CA | Los Angeles | 12.2 | 13.0 | 13.8 | 13.8 | 14.4 | 14.8 | 15.0 | 15.1 | 14.5 | 13.8 | 12.4 | 12.1 |
| CO | Denver | 10.7 | 10.5 | 10.2 | 9.6 | 10.2 | 9.6 | 9.4 | 9.6 | 9.5 | 9.5 | 11.0 | 11.0 |
| DC | Washington | 11.8 | 11.5 | 11.3 | 11.1 | 11.6 | 11.7 | 11.7 | 12.3 | 12.6 | 12.5 | 12.2 | 12.2 |
| FL | Miami | 13.5 | 13.1 | 12.8 | 12.3 | 12.7 | 14.0 | 13.7 | 14.1 | 14.5 | 13.5 | 13.9 | 13.4 |
| GA | Atlanta | 13.3 | 12.3 | 12.0 | 11.8 | 12.5 | 13.0 | 13.8 | 14.2 | 13.9 | 13.0 | 12.9 | 13.2 |
| HI | Honolulu | 13.3 | 12.8 | 11.9 | 11.3 | 10.8 | 10.6 | 10.6 | 10.7 | 10.8 | 11.3 | 12.1 | 12.9 |
| ID | Boise | 15.2 | 13.5 | 11.1 | 10.0 | 9.7 | 9.0 | 7.3 | 7.3 | 8.4 | 10.0 | 13.3 | 15.2 |
| IL | Chicago | 14.2 | 13.7 | 13.4 | 12.5 | 12.2 | 12.4 | 12.8 | 13.3 | 13.3 | 12.9 | 14.0 | 14.9 |
| IN | Indianapolis | 15.1 | 14.6 | 13.8 | 12.8 | 13.0 | 12.8 | 13.9 | 14.5 | 14.2 | 13.7 | 14.8 | 15.7 |
| IA | Des Moines | 14.0 | 13.9 | 13.3 | 12.6 | 12.4 | 12.6 | 13.1 | 13.4 | 13.7 | 12.7 | 13.9 | 14.9 |
| KS | Wichita | 13.8 | 13.4 | 12.4 | 12.4 | 13.2 | 12.5 | 11.5 | 11.8 | 12.6 | 12.4 | 13.2 | 13.9 |
| KY | Louisville | 13.7 | 13.3 | 12.6 | 12.0 | 12.8 | 13.0 | 13.3 | 13.7 | 14.1 | 13.3 | 13.5 | 13.9 |
| LA | New Orleans | 14.9 | 14.3 | 14.0 | 14.2 | 14.1 | 14.6 | 15.2 | 15.3 | 14.8 | 14.0 | 14.2 | 15.0 |
| ME | Portland | 13.1 | 12.7 | 12.7 | 12.1 | 12.6 | 13.0 | 13.0 | 13.4 | 13.9 | 13.8 | 14.0 | 13.5 |
| MA | Boston | 11.8 | 11.6 | 11.9 | 11.7 | 12.2 | 12.1 | 11.9 | 12.5 | 13.1 | 12.8 | 12.6 | 12.2 |
| MI | Detroit | 14.7 | 14.1 | 13.5 | 12.6 | 12.3 | 12.3 | 12.6 | 13.3 | 13.7 | 13.5 | 14.4 | 15.1 |
| MN | Minneapolis-St. Paul | 13.7 | 13.6 | 13.3 | 12.0 | 11.9 | 12.3 | 12.5 | 13.2 | 13.8 | 13.3 | 14.3 | 14.6 |
| MS | Jackson | 15.1 | 14.4 | 13.7 | 13.8 | 14.1 | 13.9 | 14.6 | 14.6 | 14.6 | 14.1 | 14.3 | 14.9 |
| MO | St. Louis | 14.5 | 14.1 | 13.2 | 12.4 | 12.8 | 12.6 | 12.9 | 13.3 | 13.7 | 13.1 | 14.0 | 14.9 |
| MT | Missoula | 16.7 | 15.1 | 12.8 | 11.4 | 11.6 | 11.7 | 10.1 | 9.8 | 11.3 | 12.9 | 16.2 | 17.6 |
| NE | Omaha | 14.0 | 13.8 | 13.0 | 12.1 | 12.6 | 12.9 | 13.3 | 13.8 | 14.0 | 13.0 | 13.9 | 14.8 |
| NV | Las Vegas | 8.5 | 7.7 | 7.0 | 5.5 | 5.0 | 4.0 | 4.5 | 5.2 | 5.3 | 5.9 | 7.2 | 8.4 |
| NV | Reno | 12.3 | 10.7 | 9.7 | 8.8 | 8.8 | 8.2 | 7.7 | 7.9 | 8.4 | 9.4 | 10.9 | 12.3 |
| NM | Albuquerque | 10.4 | 9.3 | 8.0 | 6.9 | 6.8 | 6.4 | 8.0 | 8.9 | 8.7 | 8.6 | 9.6 | 10.7 |
| NY | New York | 12.2 | 11.9 | 11.5 | 11.0 | 11.5 | 11.8 | 11.8 | 12.4 | 12.6 | 12.3 | 12.5 | 12.3 |
| NC | Raleigh | 12.8 | 12.1 | 12.2 | 11.7 | 13.1 | 13.4 | 13.8 | 14.5 | 14.5 | 13.7 | 12.9 | 12.8 |
| ND | Fargo | 14.2 | 14.6 | 15.2 | 12.9 | 11.9 | 12.9 | 13.2 | 13.2 | 13.7 | 13.5 | 15.2 | 15.2 |
| OH | Cleveland | 14.6 | 14.2 | 13.7 | 12.6 | 12.7 | 12.7 | 12.8 | 13.7 | 13.8 | 13.3 | 13.8 | 14.6 |
| OK | Oklahoma City | 13.2 | 12.9 | 12.2 | 12.1 | 13.4 | 13.1 | 11.7 | 11.8 | 12.9 | 12.3 | 12.8 | 13.2 |
| OR | Pendleton | 15.8 | 14.0 | 11.6 | 10.6 | 9.9 | 9.1 | 7.4 | 7.7 | 8.8 | 11.0 | 14.6 | 16.5 |
| OR | Portland | 16.5 | 15.3 | 14.2 | 13.5 | 13.1 | 12.4 | 11.7 | 11.9 | 12.6 | 15.0 | 16.8 | 17.4 |
| PA | Philadelphia | 12.6 | 11.9 | 11.7 | 11.2 | 11.8 | 11.9 | 12.1 | 12.4 | 13.0 | 13.0 | 12.7 | 12.7 |
| SC | Charleston | 13.3 | 12.6 | 12.5 | 12.4 | 12.8 | 13.5 | 14.1 | 14.6 | 14.5 | 13.7 | 13.2 | 13.2 |
| SD | Sioux Falls | 14.2 | 14.6 | 14.2 | 12.9 | 12.6 | 12.8 | 12.6 | 13.3 | 13.6 | 13.0 | 14.6 | 15.3 |
| TN | Memphis | 13.8 | 13.1 | 12.4 | 12.2 | 12.7 | 12.8 | 13.0 | 13.1 | 13.2 | 12.5 | 12.9 | 13.6 |
| TX | Dallas-Ft.Worth | 13.6 | 13.1 | 12.9 | 13.2 | 13.9 | 13.0 | 11.6 | 11.7 | 12.9 | 12.8 | 13.1 | 13.5 |
| TX | El Paso | 9.6 | 8.2 | 7.0 | 5.8 | 6.1 | 6.3 | 8.3 | 9.1 | 9.3 | 8.8 | 9.0 | 9.8 |
| UT | Salt Lake City | 14.6 | 13.2 | 11.1 | 10.0 | 9.4 | 8.2 | 7.1 | 7.4 | 8.5 | 10.3 | 12.8 | 14.9 |
| VA | Richmond | 13.2 | 12.5 | 12.0 | 11.3 | 12.1 | 12.4 | 13.0 | 13.7 | 13.8 | 13.5 | 12.8 | 13.0 |
| WA | Seattle-Tacoma | 15.6 | 14.6 | 15.4 | 13.7 | 13.0 | 12.7 | 12.2 | 12.5 | 13.5 | 15.3 | 16.3 | 16.5 |

Equilibrium Moisture Content of Wood, Exposed to Outdoor Atmosphere, in Several U.S. Locations in 1997-Cont'd

Equilibrium moisture content ${ }^{\text {a }}$ (\%)

| State | City | Jan. | Feb. | Mar. | Apr. | May | June | July | Aug. | Sept. | Oct., | Nov. | Dec. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| WI | Madison | 14.5 | 14.3 | 14.1 | 12.8 | 12.5 | 12.8 | 13.4 | 14.4 | 14.9 | 14.1 | 15.2 | 15.7 |
| WV | Charleston | 13.7 | 13.0 | 12.1 | 11.4 | 12.5 | 13.3 | 14.1 | 14.3 | 14.0 | 13.6 | 13.0 | 13.5 |
| WY | Cheyenne | 10.2 | 10.4 | 10.7 | 10.4 | 10.8 | 10.5 | 9.9 | 9.9 | 9.7 | 9.7 | 10.6 | 10.6 |

${ }^{a}$ EMC values were determined from the average of 30 or more years of relative humidity and temperature data available from the National Climatic Data Center of the National Oceanic and Atmospheric Administration.

### 7.4.1 Coefficients for Dimensional Change as result of Shrinkage

Coefficients for Dimensional Change as a Result of Shrinking or Swelling within Moisture Content Limits of $6 \%$ to $14 \% ~\left(C_{\mathrm{T}}=\right.$ dimensional change coefficient for tangential direction; $C_{\mathrm{R}}=$ radial direction $)$

| Species | Dimensional change coefficient ${ }^{\text {a }}$ |  | Species | Dimensional change coefficient ${ }^{\text {a }}$ |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\mathrm{C}_{R}$ | $\mathrm{C}_{T}$ |  | $\mathrm{C}_{\mathrm{R}}$ | $\mathrm{C}_{T}$ |
| Hardwoods |  |  |  |  |  |
| Alder, red | 0.00151 | 0.00256 | Honeylocust | 0.00144 | 0.00230 |
| Apple | 0.00205 | 0.00376 | Locust, black | 0.00158 | 0.00252 |
| Ash, black | 0.00172 | 0.00274 | Madrone, Pacific | 0.00194 | 0.00451 |
| Ash, Oregon | 0.00141 | 0.00285 | Magnolia, cucumbertree | 0.00180 | 0.00312 |
| Ash, pumpkin | 0.00126 | 0.00219 | Magnolia, southern | 0.00187 | 0.00230 |
| Ash, white | 0.00169 | 0.00274 | Magnolia, sweetbay | 0.00162 | 0.00293 |
| Ash, green | 0.00169 | 0.00274 | Maple, bigleaf | 0.00126 | 0.00248 |
| Aspen, quaking | 0.00119 | 0.00234 | Maple, red | 0.00137 | 0.00289 |
| Basswood, American | 0.00230 | 0.00330 | Maple, silver | 0.00102 | 0.00252 |
| Beech, American | 0.00190 | 0.00431 | Maple, black | 0.00165 | 0.00353 |
| Birch, paper | 0.00219 | 0.00304 | Maple, sugar | 0.00165 | 0.00353 |
| Birch, river | 0.00162 | 0.00327 | Oak, black | 0.00123 | 0.00230 |
| Birch, yellow | 0.00256 | 0.00338 | Red Oak, commercial | 0.00158 | 0.00369 |
| Birch, sweet | 0.00256 | 0.00338 | Red oak, California | 0.00123 | 0.00230 |
| Buckeye, yellow | 0.00123 | 0.00285 | Red oak: water, laurel, willow | 0.00151 | 0.00350 |
| Butternut | 0.00116 | 0.00223 | White Oak, commercial | 0.00180 | 0.00365 |
| Catalpa, northern | 0.00085 | 0.00169 | White oak, live | 0.00230 | 0.00338 |
| Cherry, black | 0.00126 | 0.00248 | White oak, Oregon white | 0.00144 | 0.00327 |
| Chestnut, American | 0.00116 | 0.00234 | White oak, overcup | 0.00183 | 0.00462 |
| Cottonwood, black | 0.00123 | 0.00304 | Persimmon, common | 0.00278 | 0.00403 |
| Cottonwood, eastern | 0.00133 | 0.00327 | Sassafras | 0.00137 | 0.00216 |
| Elm, American | 0.00144 | 0.00338 | Sweet gum | 0.00183 | 0.00365 |
| Elm, rock | 0.00165 | 0.00285 | Sycamore, American | 0.00172 | 0.00296 |
| Elm, slippery | 0.00169 | 0.00315 | Tanoak | 0.00169 | 0.00423 |
| Elm, winged | 0.00183 | 0.00419 | Tupelo, black | 0.00176 | 0.00308 |
| Elm, cedar | 0.00183 | 0.00419 | Tupelo, water | 0.00144 | 0.00267 |
| Hackberry | 0.00165 | 0.00315 | Walnut, black | 0.00190 | 0.00274 |
| Hickory, pecan | 0.00169 | 0.00315 | Willow, black | 0.00112 | 0.00308 |
| Hickory, true | 0.00259 | 0.00411 | Willow, Pacific | 0.00099 | 0.00319 |

Coefficients for Dimensional Change as a Result of Shrinking or Swelling within Moisture Content Limits of $6 \%$ to $14 \% ~\left(C_{T}=\right.$ dimensional change coefficient for tangential direction; $C_{R}=$ radial direction $)-$ Cont'd

| Species | Dimensional change coefficient ${ }^{\text {a }}$ |  | Species | Dimensional change coefficient ${ }^{\text {a }}$ |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\mathrm{C}_{R}$ | $\mathrm{C}_{T}$ |  | $\mathrm{C}_{\mathrm{R}}$ | $\mathrm{C}_{T}$ |
| Holly, American | 0.00165 | 0.00353 | Yellow-poplar | 0.00158 | 0.00289 |
| Softwoods |  |  |  |  |  |
| Baldcypress | 0.00130 | 0.00216 | Pine, eastern white | 0.00071 | 0.00212 |
| Cedar, yellow | 0.00095 | 0.00208 | Pine, jack | 0.00126 | 0.00230 |
| Cedar, Atlantic white | 0.00099 | 0.00187 | Pine, loblolly | 0.00165 | 0.00259 |
| Cedar, eastern red | 0.00106 | 0.00162 | Pine, pond | 0.00165 | 0.00259 |
| Cedar, Incense | 0.00112 | 0.00180 | Pine, lodgepole | 0.00148 | 0.00234 |
| Cedar, Northern white" | 0.00101 | 0.00229 | Pine, Jeffrey | 0.00148 | 0.00234 |
| Cedar, Port-Orford | 0.00158 | 0.00241 | Pine, longleaf | 0.00176 | 0.00263 |
| Cedar, western red" | 0.00111 | 0.00234 | Pine, ponderosa | 0.00133 | 0.00216 |
| Douglas-fir, Coast-type | 0.00165 | 0.00267 | Pine, red | 0.00130 | 0.00252 |
| Douglas-fir, Interior north | 0.00130 | 0.00241 | Pine, shortleaf | 0.00158 | 0.00271 |
| Douglas-fir, Interior west | 0.00165 | 0.00263 | Pine, slash | 0.00187 | 0.00267 |
| Fir, balsam | 0.00099 | 0.00241 | Pine, sugar | 0.00099 | 0.00194 |
| Fir, California red | 0.00155 | 0.00278 | Pine, Virginia | 0.00144 | 0.00252 |
| Fir, noble | 0.00148 | 0.00293 | Pine, western white | 0.00141 | 0.00259 |
| Fir, Pacific silver | 0.00151 | 0.00327 | Redwood, old-growth ${ }^{\text {b }}$ | 0.00120 | 0.00205 |
| Fir, subalpine | 0.00088 | 0.00259 | Redwood, second-growth ${ }^{\text {b }}$ | 0.00101 | 0.00229 |
| Fir, grand | 0.00112 | 0.00245 | Spruce, black | 0.00141 | 0.00237 |
| Fir, white | 0.00112 | 0.00245 | Spruce, Engelmann | 0.00130 | 0.00248 |
| Hemlock, eastern | 0.00102 | 0.00237 | Spruce, red | 0.00130 | 0.00274 |
| Hemlock, western | 0.00144 | 0.00274 | Spruce, white | 0.00130 | 0.00274 |
| Larch, western | 0.00155' | 0.00323 | Spruce, Sitka | 0.00148 | 0.00263 |
|  |  |  | Tamarack | 0.00126 | 0.00259 |
| Imported Woods |  |  |  |  |  |
| Andiroba, crabwood | 0.00137 | 0.00274 | Light .red "Philippine mahogany" | 0.00126 | 0.00241 |
| Angelique | 0.00180 | 0.00312 | Limba | 0.00151 | 0.00187 |
| Apitong, keruing ${ }^{\text {b }}$ | 0.00243 | 0.00527 | Mahogany ${ }^{\text {b }}$ | 0.00172 | 0.00238 |
| (all Dipterocarpus spp.) |  |  | Meranti | 0.00126 | 0.00289 |
| Avodire | 0.00126 | 0.00226 | Obeche | 0.00106 | 0.00183 |
| Balsa | 0.00102 | 0.00267 | Okoume | 0.00194 | 0.00212 |
| Banak | 0.00158 | 0.00312 | Parana, pine | 0.00137 | 0.00278 |
| Cativo | 0.00078 | 0.00183 | Paumarfim | 0.00158 | 0.00312 |
| Cuangare | 0.00183 | 0.00342 | Primavera | 0.00106 | 0.00180 |
| Greenheart ${ }^{\text {b }}$ | 0.00390 | 0.00430 | Ramin | 0.00133 | 0.00308 |
| Iroko' ${ }^{\prime \prime}$ | 0.00153 | 0.00205 | Santa Maria | 0.00187 | 0.00278 |
| Khaya | 0.00141 | 0.00201 | Spanish-cedar | 0.00141 | 0.00219 |
| Kokrodua" | 0.00148 | 0.00297 | Teak ${ }^{\text {b }}$ | 0.00101 | 0.00186 |
| Lauans: dark red "Philippine mahogany" | 0.00133 | 0.00267 |  |  |  |

[^17]
## Calculation Based on Green Dimensions

Approximate dimensional changes associated with moisture content changes greater than $6 \%$ to $14 \%$, or when one moisture value is outside of those limits, can be calculated by

$$
\begin{equation*}
\Delta D=\frac{D_{\mathrm{I}}\left(M_{\mathrm{F}}-M_{\mathrm{I}}\right)}{30(100) / S_{\mathrm{T}}-30+M_{\mathrm{I}}} \tag{12-3}
\end{equation*}
$$

where $S_{\mathrm{T}}$ is tangential shrinkage (\%) from green to oven dry (Ch. 3, Tables 3-5 and 3-6) (use radial shrinkage $S_{\mathrm{R}}$ when appropriate).
Neither $M_{\mathrm{I}}$ nor $M_{\mathrm{F}}$ should exceed $30 \%$, the assumed moisture content value when shrinkage starts for most species.

### 7.4.2 Drying of Wood—Recommended Moisture Content



FIGURE 12-1 Recommended Average Moisture Content for Interior Use of Wood Products in Various Areas of the United States.

Hot-pressed plywood and other board products, such as particleboard and hardboard, usually do not have the same moisture content as lumber. The high temperatures used in hot presses cause these products to assume a lower moisture content for a given relative humidity. Because this lower equilibrium moisture content varies widely, depending on the specific type of hot-pressed product, it is recommended that such products be conditioned at 30 to $40 \%$ relative humidity for interior use and $65 \%$ for exterior use.

Lumber used in the manufacture of large laminated members should be dried to a moisture content slightly less than the moisture content expected in service so that moisture absorbed from the adhesive will not cause the moisture content of the product to exceed the service value. The range of moisture content between laminations assembled into a single member should not exceed 5 percentage points. Although laminated members are often massive and respond rather slowly to changes in environmental conditions, it is desirable to follow the recommendations in Table 12-2 for moisture content at time of installation.

## Recommended Moisture Content Values for Various Wood Items at Time of Installation

| Use of wood | Recommended moisture content (\%) in various climatological regions |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Most areas of the United States |  | Dry southwestern area ${ }^{\text {a }}$ |  | Damp, warm coastal area ${ }^{\text {a }}$ |  |
|  | Average ${ }^{\text {b }}$ | Individual pieces | Average ${ }^{\text {b }}$ | Individual pieces | Average ${ }^{\text {b }}$ | Individual pieces |
| Interior: woodwork, flooring, furniture, wood trim | 8 | 6-10 | 6 | 4-9 | 11 | 8-13 |
| Exterior: siding, wood trim, sheathing, laminated timbers | 12 | 9-14 | 9 | 7-12 | 12 | 9-14 |

${ }^{a}$ Major areas are indicated in Figure 12-1.
${ }^{b}$ To obtain a realistic average, test at least $10 \%$ of each item. If the quantity of a given item is small, make several tests. For example, in an ordinary dwelling having about 60 floor joists, at least 10 tests should be made on joists selected at random. Source: U.S. Department of Agriculture.

## Drying of Wood

Drying is required for wood to be used in most products. Dried lumber has many advantages over green lumber for producers and consumers. Removal of excess water reduces weight, thus lowering shipping and handling costs. Proper drying confines shrinking and swelling of wood in use to manageable amounts under all but extreme conditions of relative humidity or flooding. As wood dries, most of its strength properties increase, as well as its electrical and thermal insulating properties. Properly dried lumber can be cut to precise dimensions and machined more easily and efficiently; wood parts can be more securely fitted and fastened together with nails, screws, bolts, and adhesives; warping, splitting, checking, and other harmful effects of uncontrolled drying are largely eliminated; and paint, varnish, and other finishes are more effectively applied and maintained. Wood must be relatively dry before it can be glued or treated with decay-preventing and fire-retardant chemicals.

The key to successful and efficient drying is control of the drying process. Timely application of optimum or at least adequate temperature, relative humidity, and air circulation conditions is critical. Uncontrolled drying leads to drying defects that can adversely affect the serviceability and economics of the product. The usual strategy is to dry as fast as the particular species, thickness, and end-product requirements will allow without damaging the wood. Slower drying can be uneconomical as well as introduce the risk of stain.

Softwood lumber intended for framing in construction is usually targeted for drying to an average moisture content of $15 \%$, not to exceed $19 \%$. Softwood lumber for many other uses is dried to a low moisture content, 10 to $12 \%$ for many appearance grades to as low as 7 to $9 \%$ for furniture, cabinets, and millwork. Hardwood lumber for framing in construction, though not in common use, should also be dried to an average moisture content of $15 \%$, not to exceed $19 \%$. Hardwood lumber for furniture, cabinets, and millwork is usually dried to 6 to $8 \%$ moisture content.

A typical hardwood schedule might begin at $49^{\circ} \mathrm{C}\left(120^{\circ} \mathrm{F}\right)$ and $80 \%$ relative humidity when the lumber is green. By the time the lumber has reached $15 \%$ moisture content, the temperature is as high as $82^{\circ} \mathrm{C}\left(180^{\circ} \mathrm{F}\right)$. A typical hardwood drying schedule is shown in Table 12-3. Some method of monitoring moisture content during drying is required for schedules based on moisture content. One common method is the use of short kiln samples that are periodically weighed, usually manually but potentially remotely with load cells. Alternatively, electrodes are embedded in sample boards to sense the change in electrical conductivity with moisture content. This system is limited to moisture content values less than $30 \%$.

Softwood kiln schedules generally differ from hardwood schedules in that changes in kiln temperature and relative humidity are made at predetermined times rather than moisture content levels. Examples of time-based schedules, both conventional temperature ( $<100^{\circ} \mathrm{C}\left(<212^{\circ} \mathrm{F}\right)$ ) and high temperature $\left(>110^{\circ} \mathrm{C}\left(>230^{\circ} \mathrm{F}\right)\right.$ ), are given in Table 12-3.

## Drying Defects

Most drying defects or problems that develop in wood products during drying can be classified as fracture or distortion, warp, or discoloration. Defects in any one of these categories are caused by an interaction of wood properties with processing factors. Wood shrinkage is mainly responsible for wood ruptures and distortion of shape. Cell structure and chemical extractives in wood contribute to defects associated with uneven moisture content, undesirable color, and undesirable surface texture. Drying temperature is the most important processing factor because it can be responsible for defects in each category.


FIGURE 12-6 Package-loaded kiln with fans connected directly to motors.


| Butternut | Green | 0.36 | 5,400 | 0.97 | 8.2 | 24 | 2,420 | 220 | 760 | 430 | 390 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 12\% | 0.38 | 8,100 | 1.18 | 8.2 | 24 | 5,110 | 460 | 1,170 | 440 | 490 |
| Cherry, black | Green | 0.47 | 8,000 | 1.31 | 12.8 | 33 | 3,540 | 360 | 1,130 | 570 | 660 |
|  | 12\% | 0.50 | 12,300 | 1.49 | 11.4 | 29 | 7,110 | 690 | 1,700 | 560 | 950 |
| Chestnut, American | Green | 0.40 | 5,600 | 0.93 | 7.0 | 24 | 2,470 | 310 | 800 | 440 | 420 |
|  | 12\% | 0.43 | 8,600 | 1.23 | 6.5 | 19 | 5,320 | 620 | 1,080 | 460 | 540 |
| Cottonwood |  |  |  |  |  |  |  |  |  |  |  |
| Balsam, poplar | Green | 0.31 | 3,900 | 0.75 | 4.2 | - | 1,690 | 140 | 500 | - | - |
|  | 12\% | 0.34 | 6,800 | 1.10 | 5.0 | - | 4,020 | 300 | 790 | - | - |
| Black | Green | 0.31 | 4,900 | 1.08 | 5.0 | 20 | 2,200 | 160 | 610 | 270 | 250 |
|  | 12\% | 0.35 | 8,500 | 1.27 | 6.7 | 22 | 4,500 | 300 | 1,040 | 330 | 350 |
| Eastern | Green | 0.37 | 5,300 | 1.01 | 7.3 | 21 | 2,280 | 200 | 680 | 410 | 340 |
|  | 12\% | 0.40 | 8,500 | 1.37 | 7.4 | 20 | 4,910 | 380 | 930 | 580 | 430 |
| Elm American | Green | 0.46 | 7,200 | 1.11 | 11.8 | 38 | 2,910 | 360 | 1,000 | 590 | 620 |
|  |  |  |  |  |  |  |  |  |  |  |  |
|  | 12\% | 0.50 | 11,800 | 1.34 | 13.0 | 39 | 5,520 | 690 | 1,510 | 660 | 830 |
| Rock | Green | 0.57 | 9,500 | 1.19 | 19.8 | 54 | 3,780 | 610 | 1,270 | - | 940 |
|  | 1.2\% | 0.63 | 14,800 | 1.54 | 19.2 | 56 | 7,050 | 1,230 | 1,920 | - | 1,320 |
| Slippery | Green | 0.48 | 8,000 | 1.23 | 15.4 | 47 | 3,320 | 420 | 1,110 | 640 | 660 |
|  | 12\% | 0.53 | 13,000 | 1.49 | 16.9 | 45 | 6,360 | 820 | 1,630 | 530 | 860 |
| Hackberry | Green | 0.49 | 6,500 | 0.95 | 14.5 | 48 | 2,650 | 400 | 1,070 | 630 | 700 |
|  | 12\% | 0.53 | 11,000 | 1.19 | 12.8 | 43 | 5,440 | 890 | 1,590 | 580 | 880 |
| Hickory, pecan | Green | 0.60 | 10,300 | 1.40 | 20.0 | 66 | 4,570 | 800 | 1,240 | - | - |
| Bittnernut | 12\% | 0.66 | 17,100 | 1.79 | 18.2 | 66 | 9,040 | 1,680 | - | - | - |
| Nutmeg | Green | 0.56 | 9,100 | 1.29 | 22.8 | 54 | 3,980 | 760 | 1,030 | - | - |
|  | 12\% | 0.60 | 16,600 | 1.70 | 25.1 |  | 6,910 | 1,570 | - | - | - |
| Pecan | Green | 0.60 | 9,800 | 1.37 | 14.6 | 53 | 3,990 | 780 | 1,480 | 680 | 1,310 |
|  | 12\% | 0.66 | 13,700 | 1.73 | 13.8 | 44 | 7,850 | 1,720 | 2,080 | - | 1,820 |
| Water | Green | 0.61 | 10,700 | 1.56 | 18.8 | 56 | 4,660 | 880 | 1,440 | - | - |
|  | 12\% | 0.62 | 17,800 | 2.02 | 19.3 | 53 | 8,600 | 1,550 | - | - | - |
| Hickory, true |  |  |  |  |  |  |  |  |  |  |  |
| Mockernut | Green | 0.64 | 11,100 | 1.57 | 26.1 | 88 | 4,480 | 810 | 1,280 | - | - |
|  | 12\% | 0.72 | 19,200 | 2.22 | 22.6 | 77 | 8,940 | 1,730 | 1,740 | - | - |
| Pignut | Green | 0.66 | 11,700 | 1.65 | 31.7 | 89 | 4,810 | 920 | 1,370 | - | - |
|  | 12\% | 0.75 | 20,100 | 2.26 | 30.4 | 74 | 9,190 | 1,980 | 2,150 | - | - |
| Shagbark | Green | 0.64 | 11,000 | 1.57 | 23.7 | 74 | 4,580 | 840 | 1,520 | - | - |
|  | 12\% | 0.72 | 20,200 | 2.16 | 25.8 | 67 | 9,210 | 1,760 | 2,430 | - | - |
| Shellbark | Green | 0.62 | 10,500 | 1.34 | 29.9 | 104 | 3,920 | 810 | 1,190 | - | - |
|  | 12\% | 0.69 | 18,100 | 1.89 | 23.6 | 88 | 8,000 | 1,800 | 2,110 | - | - |
| Honeylocust | Green | 0.60 | 10,200 | 1.29 | 12.6 | 47 | 4,420 | 1,150 | 1,660 | 930 | 1,390 |
|  | 12\% |  | 14,700 | 1.63 | 13.3 | 47 | 7,500 | 1,840 | 2,250 | 900 | 1,580 |
| Locust, black | Green | 0.66 | 13,800 | 1.85 | 15.4 | 44 | 6,800 | 1,160 | 1,760 | 770 | 1,570 |
|  | 12\% | 0.69 | 19,400 | 2.05 | 18.4 | 57 | 10,180 | 1,830 | 2,480 | 640 | 1,700 |



| Willow | Green | 0.56 | 7,400 | 1.29 | 8.8 | 35 | 3,000 | 610 | 1,180 | 760 | 980 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 12\% | 0.69 | 14,500 | 1.90 | 14.6 | 42 | 7,040 | 1,130 | 1,650 | - | 1,460 |
| Oak, white |  |  |  |  |  |  |  |  |  |  |  |
| Bur | Green | 0.58 | 7,200 | 0.88 | 10.7 | 44 | 3,290 | 680 | 1,350 | 800 | 1,110 |
|  | 12\% | 0.64 | 10,300 | 1.03 | 9.8 | 29 | 6,060 | 1,200 | 1,820 | 680 | 1,370 |
| Chestnut | Green | 0.57 | 8,000 | 1.37 | 9.4 | 35 | 3,520 | 530 | 1,210 | 690 | 890 |
|  | 12\% | 0.66 | 13,300 | 1.59 | 11.0 | 40 | 6,830 | 840 | 1,490 | - | 1,130 |
| Live | Green | 0.80 | 11,900 | 1.58 | 12.3 | - | 5,430 | 2,040 | 2,210 | - | - |
|  | 12\% | 0.88 | 18,400 | 1.98 | 18.9 | - | 8,900 | 2,840 | 2,660 | - | - |
| Overcup | Green | 0.57 | 8,000 | 1.15 | 12.6 | 44 | 3,370 | 540 | 1,320 | 730 | 960 |
|  | 12\% | 0.63 | 12,600 | 1.42 | 15.7 | 38 | 6,200 | 810 | 2,000 | 940 | 1,190 |
| Post | Green | 0.60 | 8,100 | 1.09 | 11.0 | 44 | 3,480 | 860 | 1,280 | 790 | 1,130 |
|  | 12\% | 0.67 | 13,200 | 1.51 | 13.2 | 46 | 6,600 | 1,430 | 1,840 | 780 | 1,360 |
| Swamp chestnut | Green | 0.60 | 8,500 | 1.35 | 12.8 | 45 | 3,540 | 570 | 1,260 | 670 | 1,110 |
|  | 12\% | 0.67 | 13,900 | 1.77 | 12.0 | 41 | 7,270 | 1,110 | 1,990 | 690 | 1,240 |
| Swamp white | Green | 0.64 | 9,900 | 1.59 | 14.5 | 50 | 4,360 | 760 | 1,300 | 860 | 1,160 |
|  | 12\% | 0.72 | 17,700 | 2.05 | 19.2 | 49 | 8,600 | 1,190 | 2,000 | 830 | 1,620 |
| White | Green | 0.60 | 8,300 | 1.25 | 11.6 | 42 | 3,560 | 670 | 1,250 | 770 | 1,060 |
|  | 12\% | 0.68 | 15,200 | 1.78 | 14.8 | 37 | 7,440 | 1,070 | 2,000 | 800 | 1,360 |
| Sassafras | Green | 0.42 | 6,000 | 0.91 | 7.1 | - | 2,730 | 370 | 950 | - | - |
|  | 12\% | 0.46 | 9,000 | 1.12 | 8.7 | - | 4,760 | 850 | 1,240 | - | - |
| Sweetgum | Green | 0.46 | 7,100 | 1.20 | 10.1 | 36 | 3,040 | 370 | 990 | 540 | 600 |
|  | 12\% | 0.52 | 12,500 | 1.64 | 11.9 | 32 | 6,320 | 620 | 1,600 | 760 | 850 |
| Sycamore, American | Green | 0.46 | 6,500 | 1.06 | 7.5 | 26 | 2,920 | 360 | 1,000 | 630 | 610 |
|  | 12\% | 0.49 | 10,000 | 1.42 | 8.5 | 26 | 5,380 | 700 | 1,470 | 720 | 770 |
| Tanoak | Green | 0.58 | 10,500 | 1.55 | 13.4 | - | 4,650 | - | - | - | - |
|  | 12\% | - | - | - | - | - | - | - | - | - | - |
| Tupelo |  |  |  |  |  |  |  |  |  |  |  |
| Black | Green | 0.46 | 7,000 | 1.03 | 8.0 | 30 | 3,040 | 480 | 1,100 | 570 | 640 |
|  | 12\% | 0.50 | 9,600 | 1.20 | 6.2 | 22 | 5,520 | 930 | 1,340 | 500 | 810 |
| Water | Green | 0.46 | 7,300 | 1.05 | 8.3 | 30 | 3,370 | 480 | 1,190 | 600 | 710 |
|  | 12\% | 0.50 | 9,600 | 1.26 | 6.9 | 23 | 5,920 | 870 | 1,590 | 700 | 880 |
| Walnut, Black | Green | 0.51 | 9,500 | 1.42 | 14.6 | 37 | 4,300 | 490 | 1,220 | 570 | 900 |
|  | 12\% | 0.55 | 14,600 | 1.68 | 10.7 | 34 | 7,580 | 1,010 | 1,370 | 690 | 1,010 |
| Willow, Black | Green | 0.36 | 4,800 | 0.79 | 11.0 | - | 2,040 | 180 | 680 | - | - |
|  | 12\% | 0.39 | 7,800 | 1.01 | 8.8 | - | 4,100 | 430 | 1,250 | - | - |
| Yellow-poplar | Green | 0.40 | 6,000 | 1.22 | 7.5 | 26 | 2,660 | 270 | 790 | 510 | 440 |
|  | 12\% | 0.42 | 10,100 | 1.58 | 8.8 | 24 | 5,540 | 500 | 1,190 | 540 | 540 |
| Softwoods |  |  |  |  |  |  |  |  |  |  |  |
| Baldcypress | Green | 0.42 | 6,600 | 1.18 | 6.6 | 25 | 3,589 | 400 | 810 | 300 | 390 |
|  | 12\% | 0.46 | 10,600 | 1.44 | 8.2 | 24 | 6,360 | 730 | 1,000 | 270 | 510 |

## Strength properties of some commercially important woods grown in the United States (Inch-Ponud)-Cont'd

| Common species names | Moisture content | Specific gravity ${ }^{\text {b }}$ | Static bending |  |  | Impact bending (in.) | Com- <br> pression parallel to grain (lbf/in ${ }^{2}$ ) | Com- <br> pression perpendicular to grain (lbf/in ${ }^{2}$ ) | Shear parallel to grain ( $\mathrm{lbf} / \mathrm{in}^{2}$ ) | Tension perpendicular to grain (lbf/in ${ }^{2}$ ) | Side <br> hard- <br> ness <br> (lbf) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Modulus of rupture (lbf/in ${ }^{2}$ ) | Modulus of elasticity ${ }^{\text {c }}$ $\left(\times 10^{6} \mathrm{Ibf} / \mathrm{in}^{2}\right)$ | Work to maximum load (in-lbf/in ${ }^{3}$ ) |  |  |  |  |  |  |
| Cedar |  |  |  |  |  |  |  |  |  |  |  |
| Atlantic white | Green | 0.31 | 4,700 | 0.75 | 5.9 | 18 | 2,390 | 240 | 690 | 180 | 290 |
|  | 12\% | 0.32 | 6,800 | 0.93 | 4.1 | 13 | 4,700 | 410 | 800 | 220 | 350 |
| Eastern redcedar | Green | 0.44 | 7,000 | 0.65 | 15.0 | 35 | 3,570 | 700 | 1,010 | 330 | 650 |
|  | 12\% | 0.47 | 8,800 | 0.88 | 8.3 | 22 | 6,020 | 920 | - | - | - |
| Incense | Green | 0.35 | 6,200 | 0.84 | 6.4 | 17 | 3,150 | 370 | 830 | 280 | 390 |
|  | 12\% | 0.37 | 8,000 | 1.04 | 5.4 | 17 | 5,200 | 590 | 880 | 270 | 470 |
| Northern White | Green | 0.29 | 4,200 | 0.64 | 5.7 | 15 | 1,990 | 230 | 620 | 240 | 230 |
|  | 12\% | 0.31 | 6,500 | 0.80 | 4.8 | 12 | 3,960 | 310 | 850 | 240 | 320 |
| Port-Orford | Green | 0.39 | 6,600 | 1.30 | 7.4 | 21 | 3,140 | 300 | 840 | 180 | 380 |
|  | $12 \%$ | $0.43$ | $12,700$ | 1.70 | 9.1 | 28 | 6,250 | 720 | 1,370 | 400 | 630 |
| Western redcedar | Green | 0.31 | 5,200 | 0.94 | 5.0 | 17 | 2,770 | 240 | 770 | 230 | 260 |
|  | 12\% | 0.32 | 7,500 | 1.11 | 5.8 | 17 | 4,560 | 460 | 990 | 220 | 350 |
| Yellow | Green | 0.42 | 6,400 | 1.14 | 9.2 | 27 | 3,050 | 350 | 840 | 330 | 440 |
|  | 12\% | 0.44 | 11,100 | 1.42 | 10.4 | 29 | 6,310 | 620 | 1,130 | 360 | 580 |
|  |  |  |  |  |  |  |  |  |  |  |  |
| Coast | Green | 0.45 | 7,700 | 1.56 | 7.6 | 26 | 3,780 | 380 | 900 | 300 | 500 |
|  | 12\% | 0.48 | 12,400 | 1.95 | 9.9 | 31 | 7,230 | 800 | 1,130 | 340 | 710 |
| Interior West | Green | 0.46 | 7,700 | 1.51 | 7.2 | 26 | 3,870 | 420 | 940 | 290 | 510 |
|  | 12\% | 0.50 | 12,600 | 1.83 | 10.6 | 32 | 7,430 | 760 | 1,290 | 350 | 660 |
| Interior North | Green | 0.45 | 7,400 | 1.41 | 8.1 | 22 | 3,470 | 360 | 950 | 340 | 420 |
|  | 12\% | 0.48 | 13,100 | 1.79 | 10.5 | 26 | 6,900 | 770 | 1,400 | 390 | 600 |
| Interior South | Green | 0.43 | 6,800 | 1.16 | 8.0 | 15 | 3,110 | 340 | 950 | 250 | 360 |
|  | 12\% | 0.46 | 11,900 | 1.49 | 9.0 | 20 | 6,230 | 740 | 1,510 | 330 | 510 |
| Fir |  |  |  |  |  |  |  |  |  |  |  |
| Balsam | Green | 0.33 | 5,500 | 1.25 | 4.7 | 16 | 2,630 | 190 | 662 | 180 | 290 |
|  | 12\% | 0.35 | 9,200 | 1.45 | 5.1 | 20 | 5,280 | 404 | 944 | 180 | 400 |
| California red | Green | 0.36 | 5,800 | 1.17 | 6.4 | 21 | 2,760 | 330 | 770 | 380 | 360 |
|  | 12\% | 0.38 | 10,500 | 1.50 | 8.9 | 24 | 5,460 | 610 | 1,040 | 390 | 500 |
| Grand | Green | 0.35 | 5,800 | 1.25 | 5.6 | 22 | 2,940 | 270 | 740 | 240 | 360 |
|  | 12\% | 0.37 | 8,900 | 1.57 | 7.5 | 28 | 5,290 | 500 | 900 | 240 | 490 |
| Noble | Green | 0.37 | 6,200 | 1.38 | 6.0 | 19 | 3,010 | 270 | 800 | 230 | 290 |
|  | 12\% | 0.39 | 10,700 | 1.72 | 8.8 | 23 | 6,100 | 520 | 1,050 | 220 | 410 |


| Pacific silver | Green | 0.40 | 6,400 | 1.42 | 6.0 | 21 | 3,140 | 220 | 750 | 240 | 310 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 12\% | 0.43 | 11,000 | 1.76 | 9.3 | 24 | 6,410 | 450 | 1,220 | - | 430 |
| Subalpine | Green | 0.31 | 4,900 | 1.05 | - | - | 2,300 | 190 | 700 | - | 260 |
|  | 12\% | 0.32 | 8,600 | 1.29 | - | - | 4,860 | 390 | 1,070 | - | 350 |
| White | Green | 0.37 | 5,900 | 1.16 | 5.6 | 22 | 2,900 | 280 | 760 | 300 | 340 |
|  | 12\% | 0.39 | 9,800 | 1.50 | 1.1 | 20 | 5,800 | 530 | 1,100 | 300 | 480 |
| Hemlock |  |  |  |  |  |  |  |  |  |  |  |
| Eastern | Green | 0.38 | 6,400 | 1.07 | 6.7 | 21 | 3,080 | 360 | 850 | 230 | 400 |
|  | 12\% | 0.40 | 8,900 | 1.20 | 6.8 | 21 | 5,410 | 650 | 1,060 | - | 500 |
| Mountain | Green | 0.42 | 6,300 | 1.04 | 11.0 | 32 | 2,880 | 370 | 930 | 330 | 470 |
|  | 12\% | 0.45 | 11,500 | 1.33 | 10.4 | 32 | 6,440 | 860 | 1,540 | - | 680 |
| Western | Green | 0.42 | 6,600 | 1.31 | 6.9 | 22 | 3,360 | 280 | 860 | 290 | 410 |
|  | 12\% | 0.45 | 11,300 | 1.63 | 8.3 | 23 | 7,200 | 550 | 1,290 | 340 | 540 |
| Larch, western | Green | 0.48 | 7,700 | 1.46 | 10.3 | 29 | 3,760 | 400 | 870 | 330 | 510 |
|  | 12\% | 0.52 | 13,000 | 1.87 | 12.6 | 35 | 7,620 | 930 | 1,360 | 430 | 830 |
| Pine |  |  |  |  |  |  |  |  |  |  |  |
| Eastern white | Green | 0.34 | 4,900 | 0.99 | 5.2 | 17 | 2,440 | 220 | 680 | 250 | 290 |
|  | 12\% | 0.35 | 8,600 | 1.24 | 6.8 | 18 | 4,800 | 440 | 900 | 310 | 380 |
| Jack | Green | 0.40 | 6,000 | 1.07 | 7.2 | 26 | 2,950 | 300 | 750 | 360 | 400 |
|  | 12\% | 0.43 | 9,900 | 1.35 | 8.3 | 27 | 5,660 | 580 | 1,170 | 420 | 570 |
| Loblolly | Green | 0.47 | 7,300 | 1.40 | 8.2 | 30 | 3,510 | 390 | 860 | 260 | 450 |
|  | 12\% | 0.51 | 12,800 | 1.79 | 10.4 | 30 | 7,130 | 790 | 1,390 | 470 | 690 |
| Lodgepole | Green | 0.38 | 5,500 | 1.08 | 5.6 | 20 | 2,610 | 250 | 680 | 220 | 330 |
|  | 12\% | 0.41 | 9,400 | 1.34 | 6.8 | 20 | 5,370 | 610 | 880 | 290 | 480 |
| Longleaf | Green | 0.554 | 8,500 | 1.59 | 8.9 | 35 | 4,320 | 480 | 1,040 | 330 | 590 |
|  | 12\% | 0.59 | 14,500 | 1.98 | 11.8 | 34 | 8,470 | 960 | 1,510 | 470 | 870 |
| Pitch | Green | 0.47 | 6,800 | 1.20 | 9.2 | - | 2,950 | 360 | 860 | - | - |
|  | 12\% | 0.52 | 10,800 | 1.43 | 9.2 | - | 5,940 | 820 | 1,360 | - | - |

### 7.5.1 Mechanical Properties of Some Common Woods Grown in the United States-Metric

## Strength properties of some commercially important woods grown in the United States (Metric)

| Common species names | Moisture content | Specific gravity ${ }^{\text {b }}$ | Static bending |  |  | Impact bending (mm) | Compression parallel to grain ( kPa ) | Compression perpendicular to grain (kPa) | Shear parallel to grain ( kPa ) | Tension Side perpen- harddicular ness to grain ( N ) (kPa) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Modulus of rupture (kPa) | Modulus <br> of <br> elasticity ${ }^{\text {c }}$ <br> (MPa) | Work to maximum load (kJ/m ${ }^{3}$ ) |  |  |  |  |  |

## Hardwoods

| Alder, red | Green | 0.37 | 45,000 | 8,100 | 55 | 560 | 20,400 | 1,700 | 5,300 | 2,700 | 2,000 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 12\% | 0.41 | 68,000 | 9,500 | 58 | 510 | 40,100 | 3,000 | 7,400 | 2,900 | 2,600 |
| Ash |  |  |  |  |  |  |  |  |  |  |  |
| Black | Green | 0.45 | 41,000 | 7,200 | 83 | 840 | 15,900 | 2,400 | 5,900 | 3,400 | 2,300 |
|  | 12\% | 0.49 | 87,000 | 11,000 | 103 | 890 | 41,200 | 5,200 | 10,800 | 4,800 | 3,800 |
| Blue | Green | 0.53 | 66,000 | 8,500 | 101 | - | 24,800 | 5,600 | 10,600 | - | - |
|  | 12\% | 0.58 | 95,000 | 9,700 | 99 | - | 48,100 | 9,800 | 14,000 | - | - |
| Green | Green | 0.53 | 66,000 | 9,700 | 81 | 890 | 29,000 | 5,000 | 8,700 | 4,100 | 3,900 |
|  | 12\% | 0.56 | 97,000 | 11,400 | 92 | 810 | 48,800 | 9,000 | 13,200 | 4,800 | 5,300 |
| Oregon | Green | 0.50 | 52,000 | 7,800 | 84 | 990 | 24,200 | 3,700 | 8,200 | 4,100 | 3,500 |
|  | 12\% | 0.55 | 88,000 | 9,400 | 99 | 840 | 41,600 | 8,600 | 12,300 | 5,000 | 5,200 |
| White | Green | 0.55 | 66,000 | 9,900 | 108 | 970 | 27,500 | 4,600 | 9,300 | 4,100 | 4,300 |
|  | 12\% | 0.60 | 103,000 | 12,000 | 115 | 1,090 | 51,100 | 8,000 | 13,200 | 6,500 | 5,900 |
| Aspen |  |  |  |  |  |  |  |  |  |  |  |
| Bigtooth | Green | 0.36 | 37,000 | 7,700 | 39 | - | 17,200 | 1,400 | 5,000 | - | - |
|  | 12\% | 0.39 | 63,000 | 9,900 | 53 | - | 36,500 | 3,100 | 7,400 | - | - |
| Quaking | Green | 0.35 | 35,000 | 5,900 | 44 | 560 | 14,800 | 1,200 | 4,600 | 1,600 | 1,300 |
|  | 12\% | 0.38 | 58,000 | 8,100 | 52 | 530 | 29,300 | 2,600 | 5,900 | 1,800 | 1,600 |
| Basswood, | Green | 0.32 | 34,000 | 7,200 | 37 | 410 | 15,300 | 1,200 | 4,100 | 1,900 | 1,100 |
| American | 12\% | 0.37 | 60,000 | 10,100 | 50 | 410 | 32,600 | 2,600 | 6,800 | 2,400 | 1,800 |
| Beech, | Green | 0.56 | 59,000 | 9,500 | 82 | 1,090 | 24,500 | 3,700 | 8,900 | 5,000 | 3,800 |
| American | 12\% | 0.64 | 103,000 | 11,900 | 104 | 1,040 | 50,300 | 7,000 | 13,900 | 7,000 | 5,800 |
| Birch |  |  |  |  |  |  |  |  |  |  |  |
| Paper | Green | 0.48 | 44,000 | 8,100 | 112 | 1,240 | 16,300 | 1,900 | 5,800 | 2,600 | 2,500 |
|  | 12\% | 0.55 | 85,000 | 11,000 | 110 | 860 | 39,200 | 4,100 | 8,300 | - | 4,000 |
| Sweet | Green | 0.60 | 65,000 | 11,400 | 108 | 1,220 | 25,800 | 3,200 | 8,500 | 3,000 | 4,300 |
|  | 12\% | 0.65 | 117,000 | 15,000 | 124 | 1,190 | 58,900 | 7,400 | 15,400 | 6,600 | 6,500 |
| Yellow | Green | 0.55 | 57,000 | 10,300 | 111 | 1,220 | 23,300 | 3,000 | 7,700 | 3,000 | 3,600 |
|  | 12\% | 0.62 | 114,000 | 13,900 | 143 | 1,400 | 56,300 | 6,700 | 13,000 | 6,300 | 5,600 |
| Butternut | Green | 0.36 | 37,000 | 6,700 | 57 | 610 | 16,700 | 1,500 | 5,200 | 3,000 | 1,700 |
|  | 12\% | 0.38 | 56,000 | 8,100 | 57 | 610 | 36,200 | 3,200 | 8,100 | 3,000 | 2,200 |
| Cherry, black | Green | 0.47 | 55,000 | 9,000 | 88 | 840 | 24,400 | 2,500 | 7,800 | 3,900 | 2,900 |
|  | 12\% | 0.50 | 85,000 | 10,300 | 79 | 740 | 49,000 | 4,800 | 11,700 | 3,900 | 4,200 |
| Chestnut, American | Green | 0.40 | 39,000 | 6,400 | 48 | 610 | 17,000 | 2,100 | 5,500 | 3,000 | 1,900 |
|  | 12\% | 0.43 | 59,000 | 8,500 | 45 | 480 | 36,700 | 4,300 | 7,400 | 3,200 | 2,400 |
| Cottonwood |  |  |  |  |  |  |  |  |  |  |  |
| Balsam poplar Black | Green | 0.31 | 27,000 | 5,200 | 29 | - | 11,700 | 1,000 | 3,400 | - | - |
|  | 12\% | 0.34 | 47,000 | 7,600 | 34 | - | 27,700 | 2,100 | 5,400 | - | - |
|  | Green | 0.31 | 34,000 | 7,400 | 34 | 510 | 15,200 | 1,100 | 4,200 | 1,900 | 1,100 |
|  | 12\% | 0.35 | 59,000 | 8,800 | 46 | 560 | 31,000 | 2,100 | 7,200 | 2,300 | 1,600 |
| Eastern | Green | 0.37 | 37,000 | 7,000 | 50 | 530 | 15,700 | 1,400 | 4,700 | 2,800 | 1,500 |
|  | 12\% | 0.40 | 59,000 | 9,400 | 51 | 510 | 33,900 | 2,600 | 6,400 | 4,000 | 1,900 |

## Strength properties of some commercially important woods grown in the United States (Metric)-Cont'd

| Common species names | Moisture content | Specific gravity ${ }^{\text {b }}$ | Static bending |  |  | Impact bending (mm) | Compression parallel to grain (kPa) | Com- <br> pression <br> perpen- <br> dicular <br> to grain <br> (kPa) | Shear parallel to grain (kPa) | Tension <br> perpen- <br> dicular <br> to grain <br> (kPa) | Side hardness (N) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Modulus <br> of <br> rupture <br> (kPa) | Modulus <br> of <br> elasticity ${ }^{\text {c }}$ <br> (MPa) | Work to maximum load (kJ/m ${ }^{3}$ ) |  |  |  |  |  |  |
| Elm |  |  |  |  |  |  |  |  |  |  |  |
| American | Green | 0.46 | 50,000 | 7,700 | 81 | 970 | 20,100 | 2,500 | 6,900 | 4,100 | 2,800 |
|  | 12\% | 0.50 | 81,000 | 9,200 | 90 | 990 | 38,100 | 4,800 | 10,400 | 4,600 | 3,700 |
| Rock | Green | 0.57 | 66,000 | 8,200 | 137 | 1,370 | 26,100 | 4,200 | 8,800 | - | - |
|  | 12\% | 0.63 | 102,000 | 10,600 | 132 | 1,420 | 48,600 | 8,500 | 13,200 | - | - |
| Slippery | Green | 0.48 | 55,000 | 8,500 | 106 | 1,190 | 22,900 | 2,900 | 7,700 | 4,400 | 2,900 |
|  | 12\% | 0.53 | 90,000 | 10,300 | 117 | 1,140 | 43,900 | 5,700 | 11,200 | 3,700 | 3,800 |
| Hackberry | Green | 0.49 | 45,000 | 6,600 | 100 | 1,220 | 18,300 | 2,800 | 7,400 | 4,300 | 3,100 |
|  | 12\% | 0.53 | 76,000 | 8,200 | 88 | 1,090 | 37,500 | 6,100 | 11,000 | 4,000 | 3,900 |
| Hickory, pecan |  |  |  |  |  |  |  |  |  |  |  |
| Bitternut | Green | 0.60 | 71,000 | 9,700 | 138 | 1,680 | 31,500 | 5,500 | 8,500 | - | - |
|  | 12\% | 0.66 | 118,000 | 12,300 | 125 | 1,680 | 62,300 | 11,600 | - | - | - |
| Nutmeg | Green | 0.56 | 63,000 | 8,900 | 157 | 1,370 | 27,400 | 5,200 | 7,100 | - | - |
|  | 12\% | 0.60 | 114,000 | 11,700 | 173 | - | 47,600 | 10,800 | - | - | - |
| Pecan | Green | 0.60 | 68,000 | 9,400 | 101 | 1,350 | 27,500 | 5,400 | 10,200 | 4,700 | 5,800 |
|  | 12\% | 0.66 | 94,000 | 11,900 | 95 | 1,120 | 54,100 | 11,900 | 14,300 | - | 8,100 |
| Water | Green | 0.61 | 74,000 | 10,800 | 130 | 1,420 | 32,100 | 6,100 | 9,900 | - | - |
|  | 12\% | 0.62 | 123,000 | 13,900 | 133 | 1,350 | 59,300 | 10,700 | - | - | - |
| Hickory, true |  |  |  |  |  |  |  |  |  |  |  |
| Mockrnut | Green | 0.64 | 77,000 | 10,800 | 180 | 2,240 | 30,900 | 5,600 | 8,800 | - | - |
|  | 12\% | 0.72 | 132,00 | 15,300 | 156 | 1,960 | 61,600 | 11,900 | 12,000 | - | - |
| Pignut | Green | 0.66 | 81,000 | 11,400 | 219 | 2,260 | 33,200 | 6,300 | 9,400 | - | - |
|  | 12\% | 0.75 | 139,000 | 15,600 | 210 | 1,880 | 63,400 | 13,700 | 14,800 | - | - |
| Shagbark | Green | 0.64 | 76,000 | 10,800 | 163 | 1,880 | 31,600 | 5,800 | 10,500 | - | - |
|  | 12\% | 0.72 | 139,000 | 14,900 | 178 | 1,700 | 63,500 | 12,100 | 16,800 | - | - |
| Shellbark | Green | 0.62 | 72,000 | 9,200 | 206 | 2,640 | 27,000 | 5,600 | 8,200 | - | - |
|  | 12\% | 0.69 | 125,000 | 13,000 | 163 | 2,240 | 55,200 | 12,400 | 14,500 | - | - |
| Honeylocust | Green | 0.60 | 70,000 | 8,900 | 87 | 1,190 | 30,500 | 7,900 | 11,400 | 6,400 | 6,200 |
|  | 12\% | - | 101,000 | 11,200 | 92 | 1,190 | 51,700 | 12,700 | 15,500 | 6,200 | 7,000 |
| Locust, black |  | 0.66 | 95,000 | 12,800 | 106 | 1,120 | 46,900 | 8,000 | 12,100 | 5,300 | 7,000 |
|  | 12\% | 0.69 | 134,000 | 14,100 | 127 | 1,450 | 70,200 | 12,600 | 17,100 | 4,400 | 7,600 |
| Magnolia |  |  |  |  |  |  |  |  |  |  |  |
| Cucumber | Green | 0.44 | 51,000 | 10,800 | 69 | 760 | 21,600 | 2,300 | 6,800 | 3,000 | 2,300 |
| tree | 12\% | 0.48 | 85,000 | 12,500 | 84 | 890 | 43,500 | 3,900 | 9,200 | 4,600 | 3,100 |
| Southern | Green | 0.46 | 47,000 | 7,700 | 106 | 1,370 | 18,600 | 3,200 | 7,200 | 4,200 | 3,300 |
|  | 12\% | 0.50 | 77,000 | 9,700 | 88 | 740 | 37,600 | 5,900 | 10,500 | 5,100 | 4,500 |
| Maple |  |  |  |  |  |  |  |  |  |  |  |
| Bigleaf | Green | 0.44 | 51,000 | 7,600 | 60 | 580 | 22,300 | 3,100 | 7,700 | 4,100 | 2,800 |
|  | 12\% | 0.48 | 74,000 | 10,000 | 54 | 710 | 41,000 | 5,200 | 11,900 | 3,700 | 3,800 |
| Black | Green | 0.52 | 54,000 | 9,200 | 88 | 1,220 | 22,500 | 4,100 | 7,800 | 5,000 | 3,700 |
|  | 12\% | 0.57 | 92,000 | 11,200 | 86 | 1,020 | 46,100 | 7,000 | 12,500 | 4,600 | 5,200 |
| Red | Green | 0.49 | 53,000 | 9,600, | 79 | 810 | 22,600 | 2,800 | 7,900 | - | 3,100 |
|  | 12\% | 0.54 | 92,000 | 11,300 | 86 | 810 | 45,100 | 6,900 | 12,800 | - | 4,200 |
| Silver | Green | 0.44 | 40,000 | 6,500 | 76 | 740 | 17,200 | 2,600 | 7,200 | 3,900 | 2,600 |
|  | 12\% | 0.47 | 61,000 | 7,900 | 57 | 640 | 36,000 | 5,100 | 10,200 | 3,400 | 3,100 |
| Sugar | Green | 0.56 | 65,000 | 10,700 | 92 | 1,020 | 27,700 | 4,400 | 10,100 | - | 4,300 |
|  | 12\% | 0.63 | 109,000 | 12,600 | 114 | 990 | 54,000 | 10,100 | 16,100 | - | 6,400 |

Strength properties of some commercially important woods grown
in the United States (Metric) - Cont'd

| Common species names | Moisture content | Specific gravity ${ }^{\text {b }}$ | Static bending |  |  | Impact bending (mm) | Compression parallel to grain ( kPa ) | Com- <br> pression <br> perpen- <br> dicular <br> to grain (kPa) | Shear parallel to grain (kPa) | Tension perpendicular to grain (kPa) | Side hardness (N) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Modulus <br> of rupture (kPa) | Modulus of elasticity ${ }^{\text {c }}$ (MPa) | Work to maximum load (kJ/m ${ }^{3}$ ) |  |  |  |  |  |  |
| Oak, red |  |  |  |  |  |  |  |  |  |  |  |
| Black | Green | 0.56 | 57,000 | 8,100 | 84 | 1,020 | 23,900 | 4,900 | 8,400 | - | 4,700 |
|  | 12\% | 0.61 | 96,000 | 11,300 | 94 | 1,040 | 45,000 | 6,400 | 13,200 | - | 5,400 |
| Cherrybark | Green | 0.61 | 74,000 | 12,300 | 101 | 1,370 | 31,900 | 5,200 | 9,100 | 5,500 | 5,500 |
|  | 12\% | 0.68 | 125,000 | 15,700 | 126 | 1,240 | 60,300 | 8,600 | 13,800 | 5,800 | 6,600 |
| Laurel | Green | 0.56 | 54,000 | 9,600 | 77 | 990 | 21,900 | 3,900 | 8,100 | 5,300 | 4,400 |
|  | 12\% | 0.63 | 87,0000 | 11,700 | 81 | 990 | 48,100 | 7,300 | 12,600 | 5,400 | 5,400 |
| Northern | Green | 0.56 | 57,000 | 9,300 | 91 | 1,120 | 23,700 | 4,200 | 8,300 | 5,200 | 4,400 |
| red | 12\% | 0.63 | 99,000 | 12,500 | 100 | 1,090 | 46,600 | 7,000 | 12,300 | 5,500 | 5,700 |
| Pin | Green | 0.58 | 57,000 | 9,100 | 97 | 1,220 | 25,400 | 5,000 | 8,900 | 5,500 | 4,800 |
|  | 12\% | 0.63 | 97,000 | 11,900 | 102 | 1,140 | 47,000 | 7,000 | 14,300 | 7,200 | 6,700 |
| Scarlet | Green | 0.60 | 72,000 | 10,200 | 103 | 1,370 | 28,200 | 5,700 | 9,700 | 4,800 | 5,300 |
|  | 12\% | 0.67 | 120,000 | 13,200 | 141 | 1,350 | 57,400 | 7,700 | 13,000 | 6,000 | 6,200 |
| Southern <br> red <br> Water | Green | 0.52 | 48,000 | 7,900 | 55 | 740 | 20,900 | 3,800 | 6,400 | 3,300 | 3,800 |
|  | 12\% | 0.59 | 75,000 | 10,300 | 65 | 660 | 42,000 | 6,000 | 9,600 | 3,500 | 4,700 |
|  | Green | 0.56 | 61,000 | 10,700 | 77 | 990 | 25,800 | 4,300 | 8,500 | 5,700 | 4,500 |
|  | 12\% | 0.63 | 106,000 | 13,900 | 148 | 1,120 | 46,700 | 7,000 | 13,900 | 6,300 | 5,300 |
| Willow | Green | 0.56 | 51,000 | 8,900 | 61 | 890 | 20,700 | 4,200 | 8,100 | 5,200 | 4,400 |
|  | 12\% | 0.69 | 100,000 | 13,100 | 101 | 1,070 | 48,500 | 7,800 | 11,400 | - | 6,500 |
| Oak, white |  |  |  |  |  |  |  |  |  |  |  |
| Bur | Green | 0.58 | 50,000 | 6,100 | 74 | 1,120 | 22,700 | 4,700 | 9,300 | 5,500 | 4,900 |
|  | 12\% | 0.64 | 71,000 | 7,100 | 68 | 740 | 41,800 | 8,300 | 12,500 | 4,700 | 6,100 |
| Chestnut | Green | 0.57 | 55,000 | 9,400 | 65 | 890 | 24,300 | 3,700 | 8,300 | 4,800 | 4,000 |
|  | 12\% | 0.66 | 92,000 | 11,000 | 76 | 1,020 | 47,100 | 5,800 | 10,300 | - | 5,000 |
| Live | Green | 0.80 | 82,000 | 10,900 | 85 | - | 37,400 | 14,100 | 15,200 | - | - |
|  | 12\% | 0.88 | 127,000 | 13,700 | 130 | - | 61,400 | 19,600 | 18,300 | - | - |
| Overcup | Green | 0.57 | 55,000 | 7,900 | 87 | 1,120 | 23,200 | 3,700 | 9,100 | 5,000 | 4,300 |
|  | 12\% | 0.63 | 87,000 | 9,800 | 108 | 970 | 42,700 | 5,600 | 13,800 | 6,500 | 5,300 |
| Post | Green | 0.60 | 56,000 | 7,500 | 76 | 1,120 | 24,000 | 5,900 | 8,800 | 5,400 | 5,000 |
|  | 12\% | 0.67 | 91,000 | 10,400 | 91 | 1,170 | 45,300 | 9,900 | 12,700 | 5,400 | 6,000 |
| Swamp chestnut Swamp white White | Green | 0.60 | 59,000 | 9,300 | 88 | 1,140 | 24,400 | 3,900 | 8,700 | 4,600 | 4,900 |
|  | 12\% | 0.67 | 96,000 | 12,200 | 83 | 1,040 | 50,100 | 7,700 | 13,700 | 4,800 | 5,500 |
|  | Green | 0.64 | 68,000 | 11,000 | 100 | 1,270 | 30,100 | 5,200 | 9,000 | 5,900 | 5,200 |
|  | 12\% | 0.72 | 122,000 | 14,100 | 132 | 1,240 | 59,300 | 8,200 | 13,800 | 5,700 | 7,200 |
|  | Green | 0.60 | 57,000 | 8,600 | 80 | 1,070 | 24,500 | 4,600 | 8,600 | 5,300 | 4,700 |
|  | 12\% | 0.68 | 105,000 | 12,300 | 102 | 940 | 51,300 | 7,400 | 13,800 | 5,500 | 6,000 |
| Sassafras | Green | 0.42 | 41,000 | 6,300 | 49 | - | 18,800 | 2,600 | 6,600 | - | - |
|  | 12\% | 0.46 | 62,000 | 7,700 | 60 | - | 32,800 | 5,900 | 8,500 | - | - |
| Sweetgum | Green | 0.46 | 49,000 | 8,300 | 70 | 910 | 21,000 | 2,600 | 6,800 | 3,700 | 2,700 |
|  | 12\% | 0.52 | 86,000 | 11,300 | 82 | 810 | 43,600 | 4,300 | 11,000 | 5,200 | 3,800 |
| Sycamore, <br> American <br> Tanoak | Green | 0.46 | 45,000 | 7,300 | 52 | 660 | 20,100 | 2,500 | 6,900 | 4,300 | 2,700 |
|  | 12\% | 0.49 | 69,000 | 9,800 | 59 | 660 | 37,100 | 4,800 | 10,100 | 5,000 | 3,400 |
|  | Green | 0.58 | 72,000 | 10,700 | 92 | - | 32,100 | - | - | - | - |
|  | 12\% | - | - | - | - | - | - | - | - | - | - |
| Tupelo Black | Green | 0.46 | 48,000 | 7,100 | 55 | 760 | 21,000 | 3,300 | 7,600 | 3,900 | 2,800 |
|  | 12\% | 0.50 | 66,000 | 8,300 | 43 | 560 | 38,100 | 6,400 | 9,200 | 3,400 | 3,600 |

## Strength properties of some commercially important woods grown in the United States (Metric)-Cont'd

| Common species names | Moisture content | Specific gravity ${ }^{\text {b }}$ | Static bending |  |  | Impact bending (mm) | Compression parallel to grain (kPa) | Com- <br> pression <br> perpen- <br> dicular <br> to grain <br> (kPa) | Shear parallel to grain (kPa) | Tension Side perpen- harddicular ness to grain ( N ) (kPa) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Modulus of rupture (kPa) | Modulus <br> of <br> elasticity ${ }^{\text {c }}$ <br> (MPa) | Work to maximum load (kJ/m ${ }^{3}$ ) |  |  |  |  |  |  |
| Water | Green | 0.46 | 50,000 | 7,200 | 57 | 760 | 23,200 | 3,300 | 8,200 | 4,100 | 3,200 |
|  | 12\% | 0.50 | 66,000 | 8,700 | 48 | 580 | 40,800 | 6,000 | 11,000 | 4,800 | 3,900 |
| Walnut, black | Green | 0.51 | 66,000 | 9,800 | 101 | 940 | 29,600 | 3,400 | 8,400 | 3,900 | 4,000 |
|  | 12\% | 0.55 | 101,000 | 11,600 | 74 | 860 | 52,300 | 7,000 | 9,400 | 4,800 | 4,500 |
| Willow, black | Green | 0.36 | 33,000 | 5,400 | 76 | - | 14,100 | 1,200 | 4,700 | - | - |
|  | 12\% | 0.39 | 54,000 | 7,000 | 61 | - | 28,300 | 3,000 | 8,600 | - |  |
| Yellowpoplar | Green | 0.40 | 41,000 | 8,400 | 52 | 660 | 18,300 | 1,900 | 5,400 | 3,500 | 2,000 |
|  | 12\% | 0.42 | 70,000 | 10,900 | 61 | 610 | 38,200 | 3,400 | 8,200 | 3,700 | 2,400 |

## Softwoods

| Baldcypress | Green | 0.42 | 46,000 | 8,100 | 46 | 640 | 24,700 | 2,800 | 5,600 | 2,100 | 1,700 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 12\% | 0.46 | 73,000 | 9,900 | 57 | 610 | 43,900 | 5,000 | 6,900 | 1,900 | 2,300 |
| Cedar |  |  |  |  |  |  |  |  |  |  |  |
| Atlantic | Green | 0.31 | 32,000 | 5,200 | 41 | 460 | 16,500 | 1,700 | 4,800 | 1,200 | 1,300 |
| white | 12\% | 0.32 | 47,000 | 6,400 | 28 | 330 | 32,400 | 2,800 | 5,500 | 1,500 | 1,600 |
| Eastern | Green | 0.44 | 48,000 | 4,500 | 103 | 890 | 24,600 | 4,800 | 7,000 | 2,300 | 2,900 |
| redcedar | 12\% | 0.47 | 61,000 | 6,100 | 57 | 560 | 41,500 | 6,300 | - | - | 4,000 |
| Incense | Green | 0.35 | 43,000 | 5,800 | 44 | 430 | 21,700 | 2,600 | 5,700 | 1,900 | 1,700 |
|  | 12\% | 0.37 | 55,000 | 7,200 | 37 | 430 | 35,900 | 4,100 | 6,100 | 1,900 | 2,100 |
| Northern white | Green | 0.29 | 29,000 | 4,400 | 39 | 380 | 13,700 | 1,600 | 4,300 | 1,700 | 1,000 |
|  | 12\% | 0.31 | 45,000 | 5,500 | 33 | 300 | 27,300 | 2,100 | 5,900 | 1,700 | 1,400 |
| Port- | Green | 0.39 | 45,000 | 9,000 | 51 | 530 | 21,600 | 2,100 | 5,800 | 1,200 | 1,700 |
| Orford | 12\% | 0.43 | 88,000 | 11,700 | 63 | 710 | 43,100 | 5,000 | 9,400 | 2,800 | 2,800 |
| Western | Green | 0.31 | 35,900 | 6,500 | 34 | 430 | 19,100 | 1,700 | 5,300 | 1,600 | 1,200 |
| redcedar | 12\% | 0.32 | 51,700 | 7,700 | 40 | 430 | 31,400 | 3,200 | 6,800 | 1,500 | 1,600 |
| Yellow | Green | 0.42 | 44,000 | 7,900 | 63 | 690 | 21,000 | 2,400 | 5,800 | 2,300 | 2,000 |
|  | 12\% | 0.44 | 77,000 | 9,800 | 72 | 740 | 43,500 | 4,300 | 7,800 | 2,500 | 2,600 |
| Douglas-fir ${ }^{\text {d }}$ | Green | 0.45 | 53,000 | 10,800 | 52 | 660 | 26,100 | 2,600 | 6,200 | 2,100 | 2,200 |
| Coast | 12\% | 0.48 | 85,000 | 13,400 | 68 | 790 | 49,900 | 5,500 | 7,800 | 2,300 | 3,200 |
| Interior West | Green | 0.46 | 53,000 | 10,400 | 50 | 660 | 26,700 | 2,900 | 6,500 | 2,000 | 2,300 |
|  | 12\% | 0.50 | 87,000 | 12,600 | 73 | 810 | 51,200 | 5,200 | 8,900 | 2,400 | 2,900 |
| Interior | Green | 0.45 | 51,000 | 9,700 | 56 | 560 | 23,900 | 2,500 | 6,600 | 2,300 | 1,900 |
| North | 12\% | 0.48 | 90,000 | 12,300 | 72 | 660 | 47,600 | 5,300 | 9,700 | 2,700 | 2,700 |
| Interior | Green | 0.43 | 47,000 | 8,000 | 55 | 380 | 21,400 | 2,300 | 6,600 | 1,700 | 1,600 |
| South | 12\% | 0.46 | 82,000 | 10,300 | 62 | 510 | 43,000 | 5,100 | 10,400 | 2,300 | 2,300 |
| Fir |  |  |  |  |  |  |  |  |  |  |  |
| Balsam | Green | 0.33 | 38,000 | 8,600 | 32 | 410 | 18,100 | 1,300 | 4,600 | 1,200 | 1,300 |
|  | 12\% | 0.35 | 63,000 | 10,000 | 35 | 510 | 36,400 | 2,800 | 6,500 | 1,200 | 1,800 |
| California | Green | 0.36 | 40,000 | 8,100 | 44 | 530 | 19,000 | 2,300 | 5,300 | 2,600 | 1,600 |
| red | 12\% | 0.38 | 72,400 | 10,300 | 61 | 610 | 37,600 | 4,200 | 7,200 | 2,700 | 2,200 |
| Grand | Green | 0.35 | 40,000 | '8,600 | 39 | 560 | 20,300 | 1,900 | 5,100 | 1,700 | 1,600 |
|  | 12\% | 0.37 | 61,400 | 10,800 | 52 | 710 | 36,500 | 3,400 | 6,200 | 1,700 | 2,200 |
| Noble | Green | 0.37 | 43,000 | 9,500 | 41 | 480 | 20,800 | 1,900 | 5,500 | 1,600 | 1,300 |
|  | 12\% | 0.39 | 74,000 | 11,900 | 61 | 580 | 42,100 | 3,600 | 7,200 | 1,500 | 1,800 |
| Pacific silver | Green | 0.40 | 44,000 | 9,800 | 41 | 530 | 21,600 | 1,500 | 5,200 | 1,700 | 1,400 |
|  | 12\% | 0.43 | 75,800 | 12,100 | 64 | 610 | 44,200 | 3,100 | 8,400 | - | 1,900 |

Strength properties of some commercially important woods grown in the United States (Metric) - Cont'd

| Common species names | Moisture content | Specific gravity ${ }^{\text {b }}$ | Static bending |  |  | Impact bending (mm) | Com- <br> pression parallel to grain (kPa) | Compression perpendicular to grain ( kPa ) | Shear parallel to grain (kPa) | Tension perpendicular to grain (kPa) | Side <br> hardness (N) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Modulus <br> of rupture (kPa) | Modulus of elasticity ${ }^{\text {c }}$ (MPa) | Work to maximum load (kJ/m ${ }^{3}$ ) |  |  |  |  |  |  |
| Subalpine | Green | 0.31 | 34,000 | 7,200 | - | - | 15,900 | 1,300 | 4,800 | - | 1,200 |
|  | 12\% | 0.32 | 59,000 | 8,900 | - | - | 33,500 | 2,700 | 7,400 | - | 1,600 |
| White | Green | 0.37 | 41,000 | 8,000 | 39 | 560 | 20,000 | 1,900 | 5,200 | 2,100 | 1,500 |
|  | 12\% | 0.39 | 68,000 | 10,300 | 50 | 510 | 40,000 | 3,700 | 7,600 | 2,100 | 2,100 |
| Hemlock |  |  |  |  |  |  |  |  |  |  |  |
| Eastern | Green | 0.38 | 44,000 | 7,400 | 46 | 530 | 21,200 | 2,500 | 5,900 | 1,600 | 1,800 |
|  | 12\% | 0.40 | 61,000 | 8,300 | 47 | 530 | 37,300 | 4,500 | 7,300 | - | 2,200 |
| Mountain | Green | 0.42 | 43,000 | 7,200 | 76 | 810 | 19,900 | 2,600 | 6,400 | 2,300 | 2,100 |
|  | 12\% | 0.45 | 79,000 | 9,200 | 72 | 810 | 44,400 | 5,900 | 10,600 | - | 3,000 |
| Western | Green | 0.42 | 46,000 | 9,000 | 48 | 560 | 23,200 | 1,900 | 5,900 | 2,000 | 1,800 |
|  | 12\% | 0.45 | 78,000 | 11,300 | 57 | 580 | 49,000 | 3,800 | 8,600 | 2,300 | 2,400 |
| Larch, western | Green | 0.48 | 53,000 | 10,100 | 71 | 740 | 25,900 | 2,800 | 6,000 | 2,300 | 2,300 |
|  | 12\% | 0.52 | 90,000 | 12,900 | 87 | 890 | 52,500 | 6,400 | 9,400 | 3,000 | 3,700 |
| Pine |  |  |  |  |  |  |  |  |  |  |  |
| Eastern | Green | 0.34 | 34,000 | 6,800 | 36 | 430 | 16,800 | 1,500 | 4,700 | 1,700 | 1,300 |
| white | 12\% | 0.35 | 59,000 | 8,500 | 47 | 460 | 33,100 | 3,000 | 6,200 | 2,100 | 1,700 |
| Jack | Green | 0.40 | 41,000 | 7,400 | 50 | 660 | 20,300 | 2,100 | 5,200 | 2,500 | 1,800 |
|  | 12\% | 0.43 | 68,000 | 9,300 | 57 | 690 | 39,000 | 4,000 | 8,100 | 2,900 | 2,500 |
| Loblolly | Green | 0.47 | 50,000 | 9,700 | 57 | 760 | 24,200 | 2,700 | 5,900 | 1,800 | 2,000 |
|  | 12\% | 0.51 | 88,000 | 12,300 | 72 | 760 | 49,200 | 5,400 | 9,600 | 3,200 | 3,100 |
| Lodgepole | Green | 0.38 | 38,000 | 7,400 | 39 | 510 | 18,000 | 1,700 | 4,700 | 1,500 | 1,500 |
|  | 12\% | 0.41 | 65,000 | 9,200 | 47 | 510 | 37,000 | 4,200 | 6,100 | 2,000 | 2,100 |
| Longleaf | Green | 0.54 | 59,000 | 11,000 | 61 | 890 | 29,800 | 3,300 | 7,200 | 2,300 | 2,600 |
|  | 12\% | 0.59 | 1,00,000 | 13,700 | 81 | 860 | 58,400 | 6,600 | 10,400 | 3,200 | 3,900 |
| Pitch | Green | 0.47 | 47,000 | 8,300 | 63 | - | 20,300 | 2,500 | 5,900 | - | - |
|  | 12\% | 0.52 | 74,000 | 9,900 | 63 | - | 41,000 | 5,600 | 9,400 | - | - |

### 7.6.0 Mechanical Properties of Wood Imported from Other Countries—Inch-Pound

Mechanical Properties of Some Woods Imported into the United States other than
Canadian Imports (Inch-Pound) ${ }^{\text {a }}$

| Common and botanical names of species | Moisture content | Specific gravity | Static bending |  |  | Compression parallel to grain (lbf/in ${ }^{2}$ ) | Shear parallel to grain (lbf/ $i n^{2}$ ) | Side <br> hardness <br> (lbf) | Sample origin $^{\text {b }}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Modulus of rupture ( $\mathrm{lbf} / \mathrm{in}^{2}$ ) | Modulus of elasticity (x16 ${ }^{6} \mathrm{lbf} /$ $i n^{3}$ ) | Work to maximum load (in$\mathrm{lbf} / \mathrm{in}^{3}$ ) |  |  |  |  |
| Afrormosia (Pericopsis elata) | Green | 0.61 | 14,800 | 1.77 | 19.5 | 7,490 | 1,670 | 1,600 | AF |
|  | 12\% |  | 18,400 | 1.94 | 18.4 | 9,940 | 2,090 | 1,560 |  |
| Albarco (Cariniana spp.) | Green | 0.48 | - | - | - | - | - | - | AM |
|  | 12\% |  | 4,500 | 1.5 | 13.8 | 6,820 | 2,310 | 1,020 |  |

## Mechanical Properties of Some Woods Imported into the United States other than Canadian Imports (Inch-Pound) ${ }^{\text {a }}$ - Cont'd

| Common and botanical names of species | Moisture content | Specific gravity | Static bending |  |  | Compression parallel to grain (lbf/in ${ }^{2}$ ) | Shear parallel to grain (lbf/ $i n^{2}$ ) | Side hardness <br> (lbf) | Sample origin ${ }^{\text {b }}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Modulus of rupture (lbf/in ${ }^{2}$ ) | Modulus of elasticity (x16 ${ }^{6} \mathrm{lbf} /$ $i n^{3}$ ) | Work to maximum load (in$\mathrm{lbf} / \mathrm{in}^{3}$ ) |  |  |  |  |
| Andiroba (Carapa | Green | 0.54 | 10,000 | 1.69 | 9.8 | 4,780 | 1,220 | 880 | AM |
| guianensis) | 12\% | - | 15,500 | 2 | 14 | 8,120 | 1,510 | 1,130 |  |
| Angelin (Andira | Green | 0.6.5 | - | - | - | - | - | - | AF |
| inermis) | 12\% |  | 18,000 | 2.49 | - | 9,200 | 1,840 | 1,750 |  |
| Angelique (Dicorynia | Green | 0.6 | 11,400 | 1.84 | 12 | 5,590 | 1,340 | 1,100 | AM |
| guianensis) | 12\% | - | 17,400 | 2.19 | 15.2 | 8,770 | 1,660 | 1,290 |  |
| Avodire | Green | 0.48 | - | - | - | - | - | - | AF |
| (Turraeanthus africanus) | 12\% |  | 12,700 | 1.49 | 9.4 | 7,150 | 2,030 | 1,080 |  |
| Azobe (Lophira | Green | 0.87 | 16,900 | 2.16 | 12 | 9,520 | 2,040 | 2,890 | AF |
| alata) | 12\% |  | 24,500 | 2.47 | - | 12,600 | 2,960 | 3,350 |  |
| Balsa (Ochroma | Green | 0.16 | - | - | - | - | - | - | AM |
| pyramidale) | 12\% |  | 3,140 | 0.49 | 2.1 | 2,160 | 300 | - |  |
| Banak (Virola spp.) | Green | 0.42 | 5,600 | 1.64 | 4.1 | 2,390 | 720 | 320 | AM |
|  | 12\% | - | 10,900 | 2.04 | 10 | 5,140 | 980 | 510 |  |
| Benge (Guibourtia | Green | 0.65 | - | - | - | - | - | - | AF |
| arnoldiana) | 12\% |  | 21,400 | 2.04 | - | 11,400 | 2,090 | 1,750 |  |
| Bubinga | Green | 0.71 | - | - | - | - | - | - | AF |
| (Guibourtia spp.) | 12\% |  | 22,600 | 2.48 | - | 10,500 | 3,110 | 2,690 |  |
| Bulletwood | Green | 0.85 | 17,300 | 2.7 | 13.6 | 8,690 | 1,900 | 2,230 | AM |
| (Manilkara bidentata) | 12\% |  | 27,300 | 3.45 | 28.5 | 11,640 | 2,500 | 3,190 |  |
| Cativo (Prioria | Green | 0.4 | 5,900 | 0.94 | 5.4 | 2,460 | 860 | 440 | AM |
| copaifera) | 12\% | - | 8,600 | 1.11 | 7.2 | 4,290 | 1,060 | 630 |  |
| Ceiba (Ceiba | Green | 0.25 | 2,200 | 0.41 | 1,2 | 1,060 | 350 | 220 | AM |
| pentandra) | 12\% |  | 4,300 | 0.54 | 2.8 | 2,380 | 550 | 240 |  |
| Courbaril | Green | 0.71 | 12,900 | 1.84 | 14.6 | 5,800 | 1,770 | 1,970 | AM |
| (Hymenaea courbaril) | 12\% | - | 19,400 | 2.16 | 17.6 | 9,510 | 2,470 | 2,350 |  |
| Cuangare | Green | 0.31 | 4,000 | 1.01 | - | 2,080 | 590 | 230 | AM |
| (Dialyanthera spp.) | 12\% |  | 7,300 | 1.52 | - | 4,760 | 830 | 380 |  |
| Cypress, Mexican | Green | 0.93 | 6,200 | 0.92 | - | 2,880 | 950 | 340 | AF |
| (Cupressus lustianica) | 12\% |  | 10,300 | 1.02 | - | 5,380 | 1,580 | 460 |  |
| Degame | Green | 0.67 | 14,300 | 1.93 | 18.6 | 6,200 | 1,660 | 1,630 | AM |
| (Calycophyllum candidissimum) | 12\% |  | 22,300 | 2.27 | 27 | 9,670 | 2,120 | 1,940 |  |
| Determa (Ocotea | Green | 0.52 | 7,800 | 1.46 | 4.8 | 3,760 | 860 | 520 | AM |
| rubra) | 12\% |  | 10,500 | 1.82 | 6.4 | 5,800 | 980 | 660 |  |
| Ekop (Tetraberlinia | Green | 0.6 | - | - | - | - | - | - | AF |
| tubmaniana) | 12\% |  | 16,700 | 2.21 | - | 9,010 | - | - |  |
| Goncalo alves | Green | 0.84 | 12,100 | 1.94 | 6.7 | 6,580 | 1,760 | 1,910 | AM |
| (Astronium graveolens) | 12\% | - | 16,600 | 2.23 | 10.4 | 10,320 | 1,960 | 2,160 |  |

Mechanical Properties of Some Woods Imported into the United States other than
Canadian Imports (Inch-Pound) ${ }^{\text {a }}$ - Cont'd

| Common and botanical names of species | Moisture content | Specific gravity | Static bending |  |  | Compression parallel to grain (lbf/in ${ }^{2}$ ) | Shear parallel to grain (lbf/ $i n^{2}$ ) | Side <br> hardness <br> (lbf) | Sample origin ${ }^{\text {b }}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Modulus of rupture ( $\mathrm{lbf} / \mathrm{in}^{2}$ ) | Modulus of elasticity (x16 ${ }^{6} \mathrm{lbf} /$ $i n^{3}$ ) | Work to maximum load (in$\mathrm{lbf} / \mathrm{in}^{3}$ ) |  |  |  |  |
| Greenheart | Green | 0.8 | 19,300 | 2.47 | 10.5 | 9,380 | 1,930 | 1,880 | AM |
| (Chlorocardium rodiei) | 12\% |  | 24,900 | 3.25 | 25.3 | 12,510 | 2,620 | 2,350 |  |
| Hura (Hura | Green | 0.38 | 6,300 | 1.04 | 5.9 | 2,790 | 830 | 440 | AM |
| crepitans) | 12\% |  | 8,700 | 1.17 | 6.7 | 4,800 | 1,080 | 550 |  |
| llomba | Geen | 0.4 | 5,500 | 1.14 | - | 2,900 | 840 | 470 | AF |
| (Pycnanthus angolensis) | 12\% |  | 9,900 | 1.59 | - | 5,550 | 1,290 | 610 |  |
| Ipe (Tabebuia spp., | Green | 0.92 | 22,600 | 2.92 | 27.6 | 10,350 | 2,120 | 3,060 | AM |
| lapacho group) | 12\% |  | 25,400 | 3.14 | 22 | 13,010 | 2,060 | 3,680 |  |
| Iroko (Chlorophora | Green | 0.54 | 10,200 | 1.29 | 10.5 | 4,910 | 1,310 | 1,080 | AF |
| spp.) | 12\% |  | 12,400 | 1.46 | 9 | 7,590 | 1,800 | 1,260 |  |
| Jarrah (Eucalyptus | Green | 0.67 | 9,900 | 1.48 | - | 5,190 | 1,320 | 1,290 | AS |
| marginata) | 12\% | - | 16,200 | 1.88 | - | 8,870 | 2,130 | 1,910 |  |
| Jelutong (Dyera | Green | 0.36 | 5,600 | 1.16 | 5.6 | 3,050 | 760 | 330 | AS |
| costulata) | 15\% |  | 7,300 | 1.18 | 6.4 | 3,920 | 840 | 390 |  |
| Kaneelhart (Licaria | Green | 0.96 | 22,300 | 3.82 | 13.6 | 13,390 | 1,680 | 2,210 | AM |
| spp.) | 12\% |  | 29,900 | 4.06 | 17.5 | 17,400 | 1,970 | 2,900 |  |
| Kapur | Green | 0.64 | 12,800 | 1.6 | 15.7 | 6,220 | 1,170 | 980 | AS |
| (Dryobalanops spp.) | 12\% |  | 18,300 | 1.88 | 18.8 | 10,090 | 1,990 | 1,230 |  |
| Karri (Eucalyptus | Green | 0.82 | 11,200 | 1.94 | 11.6 | 5,450 | 1,510 | 1,360 | AS |
| diversicolor) | 12\% |  | 20,160 | 2.6 | 25.4 | 10,800 | 2,420 | 2,040 |  |
| Kempas | Green | 0.71 | 14,500 | 2.41 | 12.2 | 7,930 | 1,460 | 1,480 | AS |
| (Koompassia malaccensis) | 12\% |  | 17,700 | 2.69 | 15.3 | 9,520 | 1,790 | 1,710 |  |
| Keruing | Green | 0.69 | 11,900 | 1.71 | 13.9 | 5,680 | 1,170 | 1,060 | AS |
| (Dipterocarpus spp.) | 12\% |  | 19,900 | 2.07 | 23.5 | 10,500 | 2,070 | 1,270 |  |
| Lignumvitae | Green | 1.05 | - | - | - | - | - | - | AM |
| (Guaiacum spp.) | 12\% | - | - |  | - | 11,400 | - | 4,500 |  |
| Limba (Terminate | Green | 0.38 | 6,000 | 0.77 | 7.7 | 2,780 | 88 | 400 | AF |
| superba) | 12\% |  | 8,800 | 1.01 | 8.9 | 4,730 | 1,410 | 490 |  |
| Macawood | Green | 0.94 | 22,300 | 3.02 | - | 10,540 | 1,840 | 3,320 | AM |
| (Platymiscium spp.) | 12\% |  | 27,600 | 3.2 | - | 16,100 | 2,540 | 3,150 |  |
| Mahogany, African | Green | 0.42 | 7,400 | 1.15 | 7.1 | 3,730 | 931 | 640 | AF |
| (Khaya spp.) | 12\% |  | 10,700 | 1.4 | 8.3 | 6,460 | 1,500 | 830 |  |
| Mahogany, true | Green | 0.45 | 9,000 | 1.34 | 9.1 | 4,340 | 1,240 | 740 | AM |
| (Swietenia macrophylla) | 12\% | - | 11,500 | 1.5 | 7.5 | 6,780 | 1,230 | 800 |  |
| Manbarklak | Green | 0.87 | 17,100 | 2.7 | 17.4 | 7,340 | 1,630 | 2,280 | AM |
| (Eschweilera spp.) | 12\% |  | 26,500 | 3.14 | 33.3 | 11,210 | 2,070 | 3,480 |  |
| Manni (Symphonia | Green | 0.58 | 11,200 | 1.96 | 11.2 | 5,160 | 1,140 | 940 | AM |
| globulifera) | 12\% |  | 16,900 | 2.46 | 16.5 | 8,820 | 1,420 | 1,120 |  |

## Mechanical Properties of Some Woods Imported into the United States other than Canadian Imports (Inch-Pound) ${ }^{\text {a }}$ - Cont'd

| Common and botanical names of species | Moisture content | Specific gravity | Static bending |  |  | Compression parallel to grain (lbf/in ${ }^{2}$ ) | Shear parallel to grain (lbf/ $i n^{2}$ ) | Side <br> hardness <br> (lbf) | Sample origin $^{\text {b }}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Modulus <br> of rupture ( $\mathrm{lbf} / \mathrm{in}^{2}$ ) | Modulus of elasticity (x16 ${ }^{6} \mathrm{lbf} /$ $i n^{3}$ ) | Work to maximum load (in$\mathrm{lbf} / \mathrm{in}^{3}$ ) |  |  |  |  |
| Marishballi | Green | 0.88 | 17,100 | 2.93 | 13.4 | 7,580 | 1,620 | 2,250 | AM |
| (Lincania spp.) | 12\% |  | 27,700 | 3.34 | 14.2 | 13,390 | 1,750 | 3,570 |  |
| Merbau (Intsia | Green | 0.64 | 12,900 | 2.02 | 12.8 | 6,770 | 1,560 | 1,380 | AS |
| spp.) | 15\% | - | 16,800 | 2.23 | 14.8 | 8,440 | 1,810 | 1,500 |  |
| Mersawa | Green | 0.52 | 8,000 | 1.77 | - | 3,960 | 740 | 880 | AS |
| (Anisoptera spp.) | 12\% |  | 13,800 | 2.28 | - | 7,370 | 890 | 1,290 |  |
| Mora (Mora spp.) | Green | 0.78 | 12,600 | 2.33 | 13.5 | 6,400 | 1,400 | 1,450 | AM |
|  | 12\% |  | 22,100 | 2.96 | 18.5 | 11,840 | 1,900 | 2,300 |  |
| Oak (Quercus | Green | 0.76 | - | - | - | - | - | - | AM |
| spp.) | 12\% |  | 23,000 | 3.02 | 16.5 | - | - | 2,500 |  |
| Obeche | Green | 0.3 | 5,100 | 0.72 | 6.2 | 2,570 | 660 | 420 | AF |
| (Triplochiton scleroxylon) | 12\% |  | 7,400 | 0.86 | 6.9 | 3,930 | 990 | 430 |  |
| Okoume | Green | 0.33 | - | - | - | - | - | - | AF |
| (Aucoumea | 12\% |  | 7,400 | 1.14 | - | 3,970 | 970 | 380 |  |
| klaineana) |  |  |  |  |  |  |  |  |  |
| Opepe (Nauclea | Green | 0.63 | 13,600 | 1.73 | 12.2 | 7,480 | 1,900 | 1,520 | AF |
| diderrichii) | 12\% |  | 17,400 | 1.94 | 14.4 | 10,400 | 2,480 | 1,630 |  |
| Ovangkol | Green | 0.67 | - | - | - | - | - | - | AF |
| (Guibourtia ehie) | 12\% |  | 16,900 | 2.56 | - | 8,300 | - | - |  |
| Para-angelim | Green | 0.63 | 14,600 | 1.95 | 12.8 | 7,460 | 1,600 | 1,720 | AM |
| (Hymenolobium excelsum) | 12\% |  | 17,600 | 2.05 | 15.9 | 8,990 | 2,010 | 1,720 |  |
| Parana-pine | Green | 0.46 | 7,200 | 1.35 | 9.7 | 4,010 | 970 | 560 | AM |
| (Araucaria augustifolia) | 1.2\% | - | 13,500 | 1.61 | 12.2 | 7,660 | 1,730 | 780 |  |
| Pau marfim | Green | 0.73 | 14,400 | 1.66 | - | 6,070 | - | - | AM |
| (Balfourodendron riedelianum) | 15\% |  | 18,900 | - | - | 8,190 | - | - |  |
| Peroba de campos | Green | 0.62 | - | - | - | - | - | - | AM |
| (Paratecoma peroba) | 12\% |  | 15,400 | 1.77 | 10.1 | 8,880 | 2,130 | 1,600 |  |
| Peroba rosa | Green | 0.66 | 10,900 | 1.29 | 10.5 | 5,540 | 1,880 | 1,580 | AM |
| (Aspidosperma | 12\% |  | 12,100 | 1.53 | 9.2 | 7,920 | 2,490 | 1,730 |  |
| spp., peroba group) |  |  |  |  |  |  |  |  |  |
| Pilon (Hyeronima | Green | 0.65 | 10,700 | 1.88 | 8.3 | 4,960 | 1,200 | 1,220 | AM |
| spp.) | 12\% |  | 18,200 | 2.27 | 12.1 | 9,620 | 1,720 | 1,700 |  |
| Pine, Caribbean | Green | 0.68 | 11,200 | 1.88 | 10.7 | 4,900 | 1,170 | 980 | AM |
| (Pinus caribaea) | 12\% | - | 16,700 | 2.24 | 17.3 | 8,540 | 2,090 | 1,240 |  |
| Pine, ocote (Pinus | Green | 0.55 | 8,000 | 1.74 | 6.9 | 3,690 | 1,040 | 580 | AM |
| oocarpa) | 12\% | - | 14,900 | 2.25 | 10.9 | 7,680 | 1,720 | 910 |  |
| Pine, radiata (Pinus | Green | 0.42 | 6,100 | 1.18 | - | 2,790 | 750 | 480 | AS |
| radiata) | 12\% | - | 11,700 | 1.48 | - | 6,080 | 1,600 | 750 |  |

Mechanical Properties of Some Woods Imported into the United States other than
Canadian Imports (Inch-Pound) ${ }^{\text {a }}$ - Cont'd

| Common and botanical names of species | Moisture content | Specific gravity | Static bending |  |  | Compression parallel to grain (lbf/in ${ }^{2}$ ) | Shear parallel to grain (lbf/ $i n^{2}$ ) | Side <br> hardness (lbf) | Sample origin $^{\text {b }}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Modulus of rupture ( $\mathrm{lbf} / \mathrm{in}^{2}$ ) | Modulus of elasticity (x16 ${ }^{6} \mathrm{lbf} /$ $i n^{3}$ ) | Work to maximum load (in$\mathrm{lbf} / \mathrm{in}^{3}$ ) |  |  |  |  |
| Piquia (Caryocar spp.) | Green | 0.72 | 12,400 | 1.82 | 8.4 | 6,290 | 1,640 | 1,720 | AM |
|  | 12\% |  | 17,000 | 2.16 | 15.8 | 8,410 | 1,990 | 1,720 |  |
| Primavera | Green | 0.4 | 7,200 | 0.99 | 7.2 | 3,510 | 1,030 | 700 | AM |
| (Tabebuia donnell-smithii) | 12\% |  | 9,500 | 1.04 | 6.4 | 5,600 | 1,390 | 660 |  |
| Purpleheart | Green | 0.67 | 1,370 | 2 | 14.8 | 7,020 | 1,640 | 1,810 | AM |
| (Peltogyne spp.) | 12\% |  | 19,200 | 2.27 | 17.6 | 10,320 | 2,220 | 1,860 |  |
| Ramin (Gonystylus | Green | 0.52 | 9,800 | 1.57 | 9 | 5,390 | 990 | 640 | AS |
| bancanus) | 12\% | - | 18,500 | 2.17 | 17 | 10,080 | 1,520 | 1,300 |  |
| Robe (Tabebuia | Green | 0.52 | 10,800 | 1.45 | 11.7 | 4,910 | 1,250 | 910 | AM |
| spp., roble group) | 12\% |  | 13,800 | 1.6 | 12.5 | 7,340 | 1,450 | 960 |  |
| Rosewood, | Green | 0.8 | 14,100 | 1.84 | 13.2 | 5,510 | 2,360 | 2,440 | AM |
| Brazilian | 12\% | - | 19,000 | 1.88 | - | 9,600 | 2,110 | 2,720 |  |
| Rosewood, Indian | Green | 0.75 | 9,200 | 1.19 | 11.6 | 4,530 | 1,400 | 1,560 | AS |
| (Dalbergia latifolia) | 12\% |  | 16,900 | 1.78 | 13.1 | 9,220 | 2,090 | 3,170 |  |
| Sande (Brosimum | Green | 0.49 | 8,500 | 1.94 | - | 4,490 | 1,040 | 600 | AM |
| spp., utile group) | 12\% |  | 14,300 | 2.39 | - | 8,220 | 1,290 | 900 |  |
| Santa Maria | Green | 0.52 | 10,500 | 1.59 | 12.7 | 4,560 | 1,260 | 890 | AM |
| (Calophyllum brasiliense) | 12\% | - | 14,600 | 1.83 | 16.1 | 6,910 | 2,080 | 1,150 |  |
| Sapele | Green | 0.55 | 10,200 | 1.49 | 10.5 | 5,010 | 1,250 | 1,020 | AF |
| (Entandrophragma cylindricum) | 12\% | - | 15,300 | 1.82 | 15.7 | 8,160 | 2,260 | 1,510 |  |
| Sepetir | Green | 0.56 | 11,200 | 1.57 | 13.3 | 5,460 | 1,310 | 950 | AS |
| (Pseudosindora palustris) | 12\% |  | 17,200 | 1.97 | 13.3 | 8,880 | 2,030 | 1,410 |  |
| Shorea (Shorea | Green | 0.68 | 11,700 | 2.1 |  | 5,380 | 1,440 | 1,350 | AS |
| spp., bullau group) | 12\% |  | 18,800 | 2.61 | - | 10,180 | 2,190 | 1,780 |  |
| Shorea, lauanmeranti group |  |  |  |  |  |  |  |  |  |
| Dark red meranti | Green | 0.46 | 9,400 | 1.5 | 8.6 | 4,720 | 1,110 | 700 | AS |
|  | 12\% |  | 12,700 | 1.77 | 13.8 | 7,360 | 1,450 | 780 |  |
| Light red meranti | Green | 0.34 | 6,600 | 1.04 | 6.2 | 3,330 | 710 | 440 | AS |
|  | 12\% |  | 9,500 | 1.23 | 8.6 | 5,920 | 970 | 460 |  |
| White meranti | Green | 0.55 | 9,800 | 1.3 | 8.3 | 5,490 | 1,320 | 1,000 | AS |
|  | 15\% |  | 12,400 | 1.49 | 11.4 | 6,350 | 1,540 | 1,140 |  |
| Yellow meranti | Green | 0.46 | 8,000 | 1.3 | 8.1 | 3,880 | 1,030 | 750 | AS |
|  | 12\% |  | 11,400 | 1.55 | 10.1 | 5,900 | 1,520 | 770 |  |
| Spanish-cedar | Green | 0.41 | 7,500 | 1.31 | 7.1 | 3,370 | 990 | 550 | AM |
| (Cedrela spp.) | 12\% | - | 11,500 | 1.44 | 9.4 | 6,210 | 1,100 | 600 |  |
| Sucupira <br> (Bowdichia spp.) | Green | 0.74 | 17,200 | 2.27 | - | 9,730 | - | - | AM |
|  | 15\% |  | 19,400 | - | - | 11,100 | - | - |  |

Mechanical Properties of Some Woods Imported into the United States other than Canadian Imports (Inch-Pound) ${ }^{\text {a }}$ - Cont'd

| Common and botanical names of species | Moisture content | Specific gravity | Static bending |  |  | Compression parallel to grain (lbf/in ${ }^{2}$ ) | Shear parallel to grain (lbf/ $i n^{2}$ ) | Side <br> hardness <br> (lbf) | Sample origin ${ }^{\text {b }}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Modulus <br> of <br> rupture <br> ( $\mathrm{lbf} / \mathrm{in}^{2}$ ) | Modulus of elasticity (x16 ${ }^{6} \mathrm{lbf} /$ $i n^{3}$ ) | Work to maximum load (in$\mathrm{lbf} / \mathrm{in}^{3}$ ) |  |  |  |  |
| Sucupira (Diplotropis | Green | 0.78 | 17,400 | 2.68 | 13 | 8,020 | 1,800 | 1,980 | AM |
| purpurea) | 12\% |  | 20,600 | 2.87 | 14.8 | 12,140 | 1,960 | 2,140 |  |
| Teak (Tectona grandis) | Green | 0.55 | 11,600 | 1.37 | 13.4 | 5,960 | 1,290 | 930 | AS |
|  | 12\% |  | 14,600 | 1.55 | 12 | 8,410 | 1,890 | 1,000 |  |
| Tornillo (Cedrelinga cateniformis) Wallaba (Eperua spp.) | Green | 0.45 | 8,400 | - | - | 4,100 | 1,170 | 870 | AM |
|  | 12\% | - | - | - | - | - | - | - |  |
|  | Green | 0.78 | 14,300 | 2.33 | - | 8,040 | - | 1,540 | AM |
|  | 12\% | - | 19,100 | 2.28 | - | 10,760 | - | 2,040 |  |

[^18]
## Average Coefficients of Variation for Some Mechanical Properties of Clear Wood

| Property | Coefficient of Variation ${ }^{\text {a (\%) }}$ |
| :--- | :---: |
| Static bending |  |
| Modulus of rupture | 16 |
| Modulus of elasticity | 22 |
| Work to maximum load | 34 |
| Impact bending | 25 |
| Compression parallel to grain | 18 |
| Compression perpendicular to grain | 28 |
| Shear parallel to grain, maximum shearing strength | 14 |
| Tension parallel to grain | 25 |
| Side hardness | 20 |
| Toughness | 34 |
| Specific gravity | 10 |

${ }^{a}$ Values based on results of tests of green wood from approximately 50 species. Values for wood adjusted to. $12 \%$ moisture content may be assumed to be approximately of the same magnitude.

### 7.7.0 Mechanical Properties of Wood Imported from Other Countries—Metric

Mechanical Properties of Some Woods Imported into the United States other than Canadian Imports (Metric) ${ }^{\text {a }}$

| Common and botanical names of species | Moisture content | Specific gravity | Static bending |  |  | Compression parallel to grain ( kPa ) | Shear <br> parallel <br> to grain <br> ( kPa ) | Side hardness (N) | Sample origin ${ }^{\text {b }}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Modulus of rupture (kPa) | Modulus <br> of elasticity (MPa) | Work to maximum load $\left(k J / m^{3}\right)$ |  |  |  |  |
| Afrormosia (Pericopsis elata) | Green | 0.61 | 102,000 | 12,200 | 135 | 51,600 | 11,500 | 7,100 | AF |
|  | 12\% |  | 126,900 | 13,400 | 127 | 68,500 | 14,400 | 6,900 |  |
| Albarco (Cariniana spp.) | Green | 0.48 | - | - | - | - | - | - | AM |
|  | 12\% |  | 100,000 | 10,300 | 95 | 47,000 | 15,900 | 4,500 |  |
| Andiroba (Carapa guianensis) | Green | 0.54 | 71,000 | 11,700 | 68 | 33,000 | 8,400 | 3,900 | AM |
|  | 12\% | - | 106,900 | 13,800 | 97 | 56,000 | 10,400 | 5,000 |  |
| Angelin (Andira inermis) | Green | 0.65 | - | - | - | - | - | - | AF |
|  | 12\% |  | 124,100 | 17,200 | - | 63,400 | 12,700 | 7,800 |  |
| Angelique (Dicorynia guianensis) | Green | 0.6 | 78,600 | 12,700 | 83 | 38,500 | 9,200 | 4,900 | AM |
|  | 12\% | - | 120,000 | 15,100 | 105 | 60,500 | 11,400 | 5,700 |  |
| Avodire (Turraeanthus | Green | 0.48 | - | - | - | - | - | - | AF |
| africanus) | 12\% |  | 87,600 | 10,300 | 65 | 49,300 | 14,000 | 4,800 |  |
| Azobe (Lophira alata) | Green | 0.87 | 116,500 | 14,900 | 83 | 65,600 | 14,100 | 12,900 | AF |
|  | 12\% |  | 168,900 | 17,000 | - | 86,900 | 20,400 | 14,900 |  |
| Balsa (Ochrpma | Green | 0.16 | - | - | - | - | - | - | AM |
| pyramidale) | 12\% |  | 21,600 | 3,400 | 14 | 14,900 | 2,100 |  |  |
| Banak (Virola spp.) | Green | 0.42 | 38,600 | 11,300 | 28 | 16,500 | 5,000 | 1,400 | AM |
|  | 12\% | - | 75,200 | 14,100 | 69 | 35,400 | 6,800 | 2,300 |  |
| Benge (Guibourtia arnoldiana) | Green | 0.65 | - | - | - | - | - | - | AF |
|  | 12\% |  | 147,500 | 14,100 | - | 78,600 | 14,400 | 7,800 |  |
| Bubinga (Guibourtia spp.) | Green | 0.71 | - | - | - | - | - | - | AF |
|  | 12\% |  | 155,800 | 17,100 | - | 72,400 | 21,400 | 12,000 |  |
| Bulletwood (Manilkara bidentata) | Green | 0.85 | 119,300 | 18,600 | 94 | 59,900 | 13,100 | 9,900 | AM |
|  | 12\% |  | 188,200 | 23,800 | 197 | 80,300 | 17,200 | 14,200 |  |
| Cativo (Prioria copaifera) | Green | 0.4 | 40,700 | 6,500 | 37 | 17,000 | 5,900 | 2,000 | AM |
|  | 12\% | - | 59,300 | 7,700 | 50 | 29,600 | 7,300 | 2,800 |  |
| Ceiba (Ceiba pentandra) | Green | 0.25 | 15,200 | 2,800 | 8 | 7,300 | 2,400 | 1,000 | AM |
|  | 12\% |  | 29,600 | 3,700 | 19 | 16,400 | 3,800 | 1,100 |  |
| Courbaril (Hymenaea courbari) | Green | 0.71 | 88,900 | 12,700 | 101 | 40,000 | 12,200 | 8,800 | AM |
|  | 12\% | - | 133,800 | 14,900 | 121 | 65,600 | 17,000 | 10,500 |  |
| Cuangare (Dialyanthera spp.) | Green | 0.31 | 27,600 | 7,000 | - | 14,300 | 4,100 | 1,000 | AM |
|  | 12\% |  | 50,300 | 10,500 | - | 32,800 | 5,700 | 1,700 |  |
| Cypress, Mexican (Cupressus lustianica) | Green | 0.93 | 42,700 | 6,300 | - | 19,900 | 6,600 | 1,500 | AF |
|  | 12\% |  | 71,000 | 7,000 | - | 37,100 | 10,900 | 2,000 |  |
| Degame (Calycophyllum candidissimum) | Green | 0.67 | 98,600 | 13,300 | 128 | 42,700 | 11,400 | 7,300 | AM |
|  | 12\% |  | 153,800 | 15,700 | 186 | 66,700 | 14,600 | 8,600 |  |
| Determa (Ocotea rubra) | Green | 0.52 | 53,800 | 10,100 | 33 | 25,900 | 5,900 | 2,300 | AM |
|  | 12\% |  | 72,400 | 12,500 | 44 | 40,000 | 6,800 | 2,900 |  |
| Ekop (Tetraberlinia tubmaniana) | Green | 0.6 | - | - | - | - | - | - | AF |
|  | 12\% |  | 115,100 | 15,200 | - | 62,100 | - | - |  |
| Goncalo alves (Astronium graveolens) | Green | 0.84 | 83,400 | 13,400 | 46 | 45,400 | 12,100 | 8,500 | AM |
|  | 12\% | - | 114,500 | 15,400 | 72 | 71,200 | 13,500 | 9,600 |  |

Mechanical Properties of Some Woods Imported into the United States other than Canadian Imports (Metric) ${ }^{\text {a }}$ - Cont'd

| Common and botanical names of species | Moisture content | Specific gravity | Static bending |  |  | Compression parallel to grain (kPa) | Shear parallel to grain (kPa) | Side <br> hard- <br> ness <br> (N) | Sample origin ${ }^{\text {b }}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Modulus <br> of rupture (kPa) | Modulus <br> of <br> elasticity (MPa) | Work to maximum load (kJ/m ${ }^{3}$ ) |  |  |  |  |
| Greenheart | Green | 0.8 | 133,100 | 17,000 | 72 | 64,700 | 13,300 | 8,400 | AM |
| (Chlorocardium rodiei) | 12\% |  | 171,700 | 22,400 | 175 | 86,300 | 18,100 | 10,500 |  |
| Hura (Hura crepitans) | Green | 0.38 | 43,400 | 7,200 | 41 | 19,200 | 5,700 | 2,000 | AM |
|  | 12\% |  | 60,000 | 8,100 | 46 | 33,100 | 7,400 | 2,400 |  |
| llomba (Pycnanthus angolensis) | Geen | 0.4 | 37,900 | 7,900 |  | 20,000 | 5,800 | 2,100 | AF |
|  | 12\% |  | 68,300 | 11,000 | - | 38,300 | 8,900 | 2,700 |  |
| Ipe (Tabebuia spp., lapacho group) | Green | 0.92 | 155,800 | 20,100 | 190 | 71,400 | 14,600 | 13,600 | AM |
|  | 12\% |  | 175,100 | 21,600 | 152 | 89,700 | 14,200 | 16,400 |  |
| Iroko (Chlorophora spp.) | Green | 0.54 | 70,300 | 8,900 | 72 | 33,900 | 9,000 | 4,800 | AF |
|  | 12\% |  | 85,500 | 10,100 | 62 | 52,300 | 12,400 | 5,600 |  |
| Jarrah (Eucalyptus marginata) | Green | 0.67 | 68,300 | 10,200 | - | 35,800 | 9,100 | 5,700 | AS |
|  | 12\% | - | 111,700 | 13,000 | - | 61,200 | 14,700 | 8,500 |  |
| Jelutong (Dyera costulata) | Green | 0.36 | 38,600 | 8,000 | 39 | 21,000 | 5,200 | 1,500 | AS |
|  | 15\% |  | 50,300 | 8,100 | 44 | 27,000 | 5,800 | 1,700 |  |
| Kaneelhart (Licaria spp.) | Green | 0.96 | 153,800 | 26,300 | 94 | 92,300 | 11,600 | 9,800 | AM |
|  | 12\% |  | 206,200 | 28,000 | 121 | 120,000 | 13,600 | 12,900 |  |
| Kapur (Dryobalanops spp.) | Green | 0.64 | 88,300 | 11,000 | 108 | 42,900 | 8,100 | 4,400 | AS |
|  | 12\% |  | 126,200 | 13,000 | 130 | 69,600 | 13,700 | 5,500 |  |
| Karri (Eucalyptus diversicolor) | Green | 0.82 | 77,200 | 13,400 | 80 | 37,600 | 10,400 | 6,000 | AS |
|  | 12\% |  | 139,000 | 17,900 | 175 | 74,500 | 16,700 | 9,100 |  |
| Kempas (Koompassia malaccensis) | Green | 0.71 | 100,000 | 16,600 | 84 | 54,700 | 10,100 | 6,600 | AS |
|  | 12\% |  | 122,000 | 18,500 | 106 | 65,600 | 12,300 | 7,600 |  |
| Keruing (Dipterocarpus spp.) | Green | 0.69 | 82,000 | 11,800 | 96 | 39,200 | 8,100 | 4,700 | AS |
|  | 12\% |  | 137,200 | 14,300 | 162 | 72,400 | 14,300 | 5,600 |  |
| Lignumvitae | Green | 1.05 | - | - | - | - |  |  | AM |
| (Guaiacum spp.) | 12\% | - | - | - | - | 78,600 | - | 20,000 |  |
| Limba (Terminalia superba) | Green | 0.38 | 41,400 | 5,300 | 53 | 19,200 | 600 | 1,800 | AF |
|  | 12\% |  | 60,700 | 7,000 | 61 | 32,600 | 9,700 | 2,200 |  |
| Macawood (Platymiscium spp.) | Green | 0.94 | 153,800 | 20,800 | - | 72,700 | 12,700 | 14,800 | AM |
|  | 12\% |  | 190,300 | 22,100 | - | 111,000 | 17,500 | 14000 |  |
| Mahogany, African (Khaya spp.) | Green | 0.42 | 51,000 | 7,900 | 49 | 25,700 | 6,400 | 2,800 | AF |
|  | 12\%. |  | 73,800 | 9,700 | 57 | 44,500 | 10,300 | 3,700 |  |
| Mahogany, true | Green | 0.45 | 62,100 | 9,200 | 63 | 29,900 | 8,500 | 3,300 | AM |
| (Swietenia macrophylla) | 12\% | - | 79,300 | 10,300 | 52 | 46,700 | 8,500 | 3,600 |  |
| Manbarklak <br> (Eschweilera spp.) | Green | 0.87 | 117,900 | 18,600 | 120 | 50,600 | 11,200 | 10,100 | AM |
|  | 12\% |  | 182,700 | 21,600 | 230 | 77,300 | 14,300 | 15,500 |  |
| Manni (Symphonia globulifera) | Green | 0.58 | 77,200 | 13,500 | 77 | 35,600 | 7,900 | 4,200 | AM |
|  | 12\% |  | 116,500 | 17,000 | 114 | 60,800 | 9,800 | 5,000 |  |
| Marishballi (Lincania spp.) | Green | 0.88 | 117,900 | 20,200 | 92 | 52,300 | 11,200 | 10,000 | AM |
|  | 12\% |  | 191,000 | 23,000 | 98 | 92,300 | 12,100 | 15,900 |  |
| Merbau (Intsia spp.) | Green | 0.64 | 88,900 | 13,900 | 88 | 46,700 | 10,800 | 6,100 | AS |
|  | 15\% | - | 115,800 | 15,400 | 102 | 58,200 | 12,500 | 6,700 |  |
| Mersawa (Anisoptera spp.) | Green | 0.52 | 55,200 | 12,200 | - | 27,300 | 5,100 | 3,900 | AS |
|  | 12\% |  | 95,100 | 15,700 | - | 50,800 | 6,100 | 5,700 |  |

Mechanical Properties of Some Woods Imported into the United States other than Canadian Imports (Metric) ${ }^{\text {a }}$ - Cont'd

| Common and botanical names of species | Moisture content | Specific gravity | Static bending |  |  | Compression parallel to grain ( kPa ) | Shear parallel to grain (kPa) | Side <br> hard- <br> ness <br> (N) | Sample origin ${ }^{\text {b }}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Modulus of rupture (kPa) | Modulus <br> of <br> elasticity <br> (MPa) | Work to maximum load $\left(\mathrm{kJ} / \mathrm{m}^{3}\right)$ |  |  |  |  |
| Mora (Mora spp.) | Green | 0.78 | 86,900 | 16,100 | 93 | 44,100 | 9,700 | 6,400 | AM |
|  | 12\% |  | 152,400 | 20,400 | 128 | 81,600 | 13,100 | 10,200 |  |
| Oak (Quercus spp.) | Green | 0.76 | - | - | - | - | - |  | AM |
|  | 12\% |  | 158,600 | 20,800 | 114 |  |  | 11,100 |  |
| Obeche (Triplochiton scleroxylon) | Green | 0.3 | 35,200 | 5,000 | 43 | 17,700 | 4,600 | 1,900 | AF |
|  | 12\% |  | 51,000 | 5,900 | 48 | 27,100 | 6,800 | 1,900 |  |
| Okoume (Aucoumea | Green | 0.33 | - | - | - | - | - | - | AF |
| klaineana) | 12\% |  | 51,000 | 7,900 | - | 27,400 | 6,700 | 1,700 |  |
| Opepe (Nauclea | Green | 0.63 | 93,800 | 11,900 | 84 | 51,600 | 13,100 | 6,800 | AF |
| diderrichii) | 12\% |  | 120,000 | 13,400 | 99 | 71,700 | 17,100 | 7,300 |  |
| Ovangkol (Guibourtia | Green | 0.67 | - | - | - | - | - | - | AF |
| ehie) | 12\% |  | 116,500 | 17,700 | - | 57,200 | - | - |  |
| (Hymenolobium excelsum) | Green | 0.63 | 100,700 | 13,400 | 88 | 51,400 | 11,000 | 7,700 | AM |
|  | 12\% |  | 121,300 | 14,100 | 110 | 62,000 | 13,900 | 7,700 |  |
| Parana-pine (Araucaria augustifolia) | Green | 0.46 | 49,600 | 9,300 | 67 | 27,600 | 6,700 | 2,500 | AM |
|  | 12\% | - | 93,100 | 11,100 | 84 | 52,800 | 11,900 | 3,500 |  |
| Pau marfim (Balfourodendron riedelianum) | Green | 0.73 | 99,300 | 11,400 | - | 41,900 | - | - | AM |
|  | 15\% |  | 130,300 | - | - | 56,500 |  |  |  |
| Peroba de campos | Green | 0.62 | - | - | - |  |  |  | AM |
| (Paratecoma peroba) | 12\% |  | 106,200 | 12,200 | 70 | 61,200 | 14,700 | 7,100 |  |
| (Aspidosperma | Green | 0.66 | 75,200 | 8,900 | 72 | 38,200 | 13,000 | 7,000 | AM |
| spp., peroba group) | 12\% |  | 83,400 | 10,500 | 63 | 54,600 | 17,200 | 7,700 |  |
| Pilon (Hyeronima spp.) | Green | 0.65 | 73,800 | 13,000 | 57 | 34,200 | 8,300 | 5,400 | AM |
|  | 12\% | - | 125,500 | 15,700 | 83 | 66,300 | 11,900 | 7,600 |  |
| Pine, Caribbean (Pinus caribaea) | Green | 0.68 | 77,200 | 13,000 | 74 | 33,800 | 8,100 | 4,400 | AM |
|  | 12\% | - | 115,100 | 15,400 | 119 | 58,900 | 14,400 | 5,500 |  |
| Pine, ocote (Pinusoocarpa) | Green | 0.55 | 55,200 | 12,000 | 48 | 25,400 | 7,200 | 2,600 | AM |
|  | 12\% | - | 102,700 | 15,500 | 75 | 53,000 | 11,900 | 4,000 |  |
| Pine, radiata (Pinus radiata) | Green | 0.42 | 42,100 | 8,100 | - | 19,200 | 5,200 | 2,100 | AS |
|  | 12\% | - | 80,700 | 10,200 | - | 41,900 | 11,000 | 3,300 |  |
| Piquia (Caryocar spp.) | Green | 0.72 | 85,500 | 12,500 | 58 | 43,400 | 11,300 | 7,700 | AM |
|  | 12\% |  | 117,200 | 14,900 | 109 | 58,000 | 13,700 | 7,700 |  |
| Primavera (Tabebuia donnell-smithii) | Green | 0.4 | 49,600 | 6,800 | 50 | 24,200 | 7,100 | 3,100 | AM |
|  | 12\% |  | 65,500 | 7,200 | 44 | 38,600 | 9,600 | 2,900 |  |
| Purpleheart (Peltogyne spp.) | Green | 0.67 | 9,400 | 13,800 | 102 | 48,400 | 11,300 | 8,100 | AM |
|  | 12\% |  | 132,400 | 15,700 | 121 | 71,200 | 15,300 | 8,300 |  |
| Ramin (Gonystylus bancanus) | Green | 0.52 | 67,600 | 10,800 | 62 | 37,200 | 6,800 | 2,800 | AS |
|  | 12\% | - | 127,600 | 15,000 | 117 | 69,500 | 10,500 | 5,800 |  |
| Robe (Tabebuia spp., roble group) Rosewood, Brazilian (Dalbergia nigra) | Green | 0.52 | 74,500 | 10,000 | 81 | 33,900 | 8,600 | 4,000 | AM |
|  | 12\% | - | 95,100 | 11,000 | 86 | 50,600 | 10,000 | 4,300 |  |
|  | Green | 0.8 | 97,200 | 12,700 | 91 | 38,000 | 16,300 | 10,900 | AM |
|  | 12\% | - | 131,000 | 13,000 | - | 66,200 | 14,500 | 12,100 |  |

Mechanical Properties of Some Woods Imported into the United States other than Canadian
Imports (Metric) ${ }^{\text {a }}$ - Cont'd

| Common and botanical names of species | Moisture content | Specific gravity | Static bending |  |  | Compression parallel to grain (kPa) | Shear parallel to grain ( kPa ) | Side <br> hard- <br> ness <br> (N) | Sample origin ${ }^{\text {b }}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Modulus of rupture (kPa) | Modulus of elasticity (MPa) | Work to maximum load (kJ/m ${ }^{3}$ ) |  |  |  |  |
| Rosewood, Indian (Dalbergia latifolia) | Green | 0.75 | 63,400 | 8,200 | 80 | 31,200 | 9,700 | 6,900 | AS |
|  | 12\% |  | 116,500 | 12,300 | 90 | 63,600 | 14,400 | 14,100 |  |
| Sande (Brosimum spp., utile group) | Green | 0.49 | 58,600 | 13,400 | - | 31,000 | 7,200 | 2,700 | AM |
|  | 12\% |  | 98,600 | 16,500 | - | 56,700 | 8,900 | 4,000 |  |
| Santa Maria | Green | 0.52 | 72,400 | 11,000 | 88 | 31,400 | 8,700 | 4,000 | AM |
| brasiliense) |  |  |  |  |  |  |  |  |  |
| Sapele | Green | 0.55 | 70,300 | 10,300 | 72 | 34,500 | 8,600 | 4,500 | AF |
| (Entandrophragma cylindricum) | 12\% | cylindricum) |  |  |  |  |  |  |  |
| Sepetir (Pseudosindora | Green | 0.56 | 77,200 | 10,800 | 92 | 37,600 | 9,000 | 4,200 | AS |
| palustris) | 12\% | - | 118,600 | 13,600 | 92 | 61,200 | 14,000 | 6,300 |  |
| Shorea (Shorea spp., | Green | 0.68 | 80,700 | 14,500 | - | 37,100 | 9,900 | 6,000 | AS |
| baulau group) | 12\% |  | 129,600 | 18,000 | - | 70,200 | 15,100 | 7,900 |  |
| Shorea, lauan-meranti group |  |  |  |  |  |  |  |  |  |
| Dark red meranti | Green | 0.46 | 64,800 | 10,300 | 59 | 32,500 | 7,700 | 3,100 | AS |
|  | 12\% |  | 87,600 | 12,200 | 95 | 50,700 | 10,000 | 3,500 |  |
| Light red meranti | Green | 0.34 | 45,500 | 7,200 | 43 | 23,000 | 4,900 | 2,000 | AS |
|  | 12\% |  | 65,500 | 8,500 | 59 | 40,800 | 6,700 | 2,000 |  |
| White meranti | Green | 0.55 | 67,600 | 9,000 | 57 | 37,900 | 9,100 | 4,400 | AS |
|  | 15\% |  | 85,500 | 10,300 | 79 | 43,800 | 10,600 | 5,100 |  |
| Yellow meranti | Green | 0.46 | 55,200 | 9,000 | 56 | 26,800 | 7,100 | 3,300 | AS |
|  | 12\% |  | 78,600 | 10,700 | 70 | 40,700 | 10,500 | 3,400 |  |
| Spanish-cedar (Cedrela spp.) | Green | 0.41 | 51,700 | 9,000 | 49 | 23,200 | 6,800 | 2,400 | AM |
|  | 12\% | - | 79,300 | 9,900 | 65 | 42,800 | 7,600 | 2,700 |  |
| Sucupira (Bowdichia spp.) | Green | 0.74 | 118,600 | 15,700 | - | 67,100 | - | - | AM |
|  | 15\% |  | 133,800 | - | - | 76,500 | - | - |  |
| Sucupira (Diplotropis purpurea) | Green | 0.78 | 120,000 | 18,500 | 90 | 55,300 | 12,400 | 8,800 | AM |
|  | 12\% |  | 142,000 | 19,800 | 102 | 83,700 | 13,500 | 9,500 |  |
| Teak (Tectona grandis) | Green | 0.55 | 80,000 | 9,400 | 92 | 41,100 | 8,900 | 4,100 | AS |
|  | 12\% |  | 100,700 | 10,700 | 83 | 58,000 | 13,000 | 4,400 |  |
| Tornillo (Cedrelinga cateniformis) Wallaba (Eperua spp.) | Green | 0.45 | 57,900 | - | - | 28,300 | 8,100 | 3,900 | AM |
|  | 12\% | - | - | - | - | - |  |  |  |
|  | Green | 0.78 | 98,600 | 16,100 | - | 55,400 |  | 6,900 | AM |
|  | 12\% | - | 131,700 | 15,700 | - | 74,200 | - | 9,100 |  |

[^19]7.8.0 Thermal Qualities of Wood

| Species | Specific gravity | Conductivity (W/m-K (Btuin/h-ft ${ }^{2} .^{\circ} \mathrm{F}$ )) |  | Resistivity (W/m.K (h.ft ${ }^{2} .{ }^{\circ}$ F/Btu.in)) |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Ovendry | 12\% MC | Ovendry | 12\% MC |
| Softwoods |  |  |  |  |  |
| Baldcypress | 0.47 | 0.11 (0.76) | 0.13 (0.92) | 9.1 (1.3) | 7.5 (1.1) |
| Cedar |  |  |  |  |  |
| Atlantic white | 0.34 | 0.085 (0.59) | 0.10 (0.70) | 12 (1.7) | 9.9 (1.4) |
| Eastern red | 0.48 | 0.11 (0.77) | 0.14 (0.94) | 8.9 (1.3) | 7.4 (1.1) |
| Northern white | 0.31 | 0.079 (0.55) | 0.094 (0.65) | 13 (1.8) | 11(1.5) |
| Port-Orford | 0.43 | 0.10 (0.71) | 0.12 (0.85) | 9.8 (1.4) | 8.1 (1.2) |
| Western red | 0.33 | 0.083 (0.57) | 0.10 (0.68) | 12 (1.7) | 10 (1.5) |
| Yellow | 0.46 | 0.11 (0.75) | 0.13 (0.90) | 9.3 (1.3) | 7.7 (1.1) |
| Douglas-fir |  |  |  |  |  |
| Coast | 0.51 | 0.12 (0.82) | 0.14 (0.99) | 8.5 (1.2) | 7.0 (1.0) |
| Interior north | 0.50 | 0.12 (0.80) | 0.14 (0.97) | 8.6 (1.2) | 7.1 (1.0) |
| Interior west | 0.52 | 0.12 (0.83) | 0.14 (1.0) | 8.4 (1.2) | 6.9 (1.0) |
| Fir |  |  |  |  |  |
| Balsam | 0.37 | 0.090 (0.63) | 0.11 (0.75) | 11 (1.6) | 9.2 (1.3) |
| White | 0.41 | 0.10 (0.68) | 0.12 (0.82) | 10 (1.5) | 8.5 (1.2) |
| Hemlock |  |  |  |  |  |
| Eastern | 0.42 | 0.10 (0.69) | 0.12 (0.84) | 10 (1.4) | 8.3 (1.2) |
| Western | 0.48 | 0.11 (0.77) | 0.14 (0.94) | 8.9 (1.3) | 7.4 (1.1) |
| Larch, western | 0.56 | 0.13 (0.88) | 0.15 (1.1) | 7.9 (1.1) | 6.5 (0.93) |
| Pine |  |  |  |  |  |
| Eastern white | 0.37 | 0.090 (0.63) | 0.11 (0.75) | 11 (1.6) | 9.2 (1.3) |
| Jack | 0.45 | 0.11 (0.73) | 0.13 (0.89) | 9.4 (1.4) | 7.8 (1.1) |
| Loblolly | 0.54 | 0.12 (0.86) | 0.15 (1.0) | 8.1 (1.2) | 6.7 (0.96) |
| Lodgepole | 0.43 | 0.10 (0.71) | 0.12 (0.85) | 9.8 (1.4) | 8.1 (1.2) |
| Longleaf | 0.62 | 0.14 (0.96) | 0.17 (1.2) | 7.2 (1.0) | 5.9 (0.85) |
| Pitch | 0.53 | 0.12 (0.84) | 0.15 (1.0) | 8.2 (1.2) | 6.8 (0.98) |
| Ponderosa | 0.42 | 0.10 (0.69) | 0.12 (0.84) | 10 (1.4) | 8.3 (1.2) |
| Red | 0.46 | 0.11 (0.75) | 0.13 (0.90) | 9.3 (1.3) | 7.7 (1.1) |
| Shortleaf | 0.54 | 0.12 (0.86) | 0.15 (1.0) | 8.1 (1.2) | 6.7 (0.96) |
| Slash | 0.61 | 0.14 (0.95) | 0.17 (1.2) | 7.3 (1.1) | 6.0 (0.86) |
| Western white | 0.37 | 0.090 (0.63) | 0.11 (0.75) | 11 (1.6) | 9.2 (1.3) |
| Redwood | 0.40 | 0.10 (0.67) | 0.12 (0.80) | 10 (1.5) | 8.6 (1.2) |
| Old growth | 0.41 | 0.10 (0.68) | 0.12 (0.82) | 10 (1.5) | 8.5 (1.2) |
| Young growth | 0.37 | 0.090 (0.63) | 0.11 (0.75) | 11 (1.6) | 9.2 (1.3) |
| Spruce 0.43 |  |  |  |  |  |
| Black | 0.43 | 0.10 (0.71) | 0.12 (0.85) | 9.8 (1.4) | 8.1 (1.2) |
| Engelmann | 0.37 | 0.090 (0.63) | 0.11 (0.75) | 11 (1.6) | 9.2 (1.3) |
| Red | 0.42 | 0.10 (0.69) | 0.12 (0.84) | 10 (1.4) | 8.3 (1.2) |
| Sitka | 0.42 | 0.10 (0.69) | 0.12 (0.84) | 10 (1.4) | 8.3 (1.2) |
| White | 0.37 | 0.090 (0.63) | 0.11 (0.75) | 11 (1.6) | 9.2 (1.3) |
| Values in this table are approximate and should be used with caution; actual conductivities may vary by as much as $20 \%$. The specific gravities also do not represent species averages. <br> Source: U.S. Department of Agriculture. |  |  |  |  |  |

### 7.9.0 Machine Stress-Rated Lumber-Calculating Design Values

## Design Values of MSR Lumber

MSR lumber assures the performance and reliability of your engineered components and structures. The machine grading process sorts dimension lumber by strength and stiffness to improve consistency. Daily quality control testing for strength and stiffness ensures that products meet structural requirements. Machine graded lumber certification and quality control procedures are based on approved American or Canadian lumber standards (ALS or CLS).

Design values in pounds per square inch (psi)

| Grade Designation | Bending <br> $\mathrm{F}_{\mathbf{b}}$ | Tension Parallel <br> to Grain $\mathrm{F}_{\mathrm{t}}$ | Compression Parallel <br> to Grain $\mathrm{F}_{\mathrm{e} / /}$ | Modulus of Elasticity <br> E |
| :--- | :---: | :---: | :---: | :---: |
| $1650 \mathrm{f}-1.5 \mathrm{E}$ | 1650 | 1020 | 1700 | $1,500,000$ |
| $1800 \mathrm{f}-1.6 \mathrm{E}$ | 1800 | 1175 | 1750 | $1,600,000$ |
| $2100 \mathrm{f}-1.8 \mathrm{E}$ | 2100 | 1575 | 1875 | $1,800,000$ |
| $2400 \mathrm{f}-2.0 \mathrm{E}$ | 2400 | 1925 | 1975 | $2,000,000$ |

## Notes:

(1) Other grades: The grades listed above are meant as examples of commonly produced MSR grades. This is not intended to be a complete list. The Supplement to the National Design Specification (NDS) for Wood Construction (AF\&PA, 1997) provides a good summary of the established grades in Machine Stress-Rated (MSR) and Machine Evaluated Lumber (MEL) in Table 4C on page 35.
(2) Quality Control: QC testing takes place on a daily basis for all machine-graded lumber products. Depending on the grade requirements, testing takes place for one or more of the following properties: $\mathrm{E}, \mathrm{F}_{\mathrm{b}}, \mathrm{F}_{\mathrm{t}}$. This testing process is established to verify that production meets design requirements for all products shipped to customers as MSR, MEL, or E-rated laminating grades.
(3) Common Species: The MSR Lumber Production Survey (MSR Lumber Producers Council, 1998) identifies Spruce-Pine-Fir (SPF) as the species in which the majority of MSR is produced ( $69 \%$ of total MSR in 1998). Douglas Fir-Larch (DFL), Hem-Fir (HF), and Southern Pine (SYP) share significant volumes as well (5 to $14 \%$ of total MSR in 1998). Consult the MSR Lumber Production Survey for more information.
(4) Species-specific design values: MSR lumber simplifies many design considerations since a grade like 1650f-1.5E maintains the same $\mathrm{F}_{\mathrm{b}}, \mathrm{F}_{\mathrm{t}}, \mathrm{F}_{\mathrm{c} / /}$, and E values no matter what species or size is considered. Other properties, however, change by species as they relate to the specific gravity (density) of wood. A couple of examples are listed below. Please refer to NDS Supplement footnote \#2 for Table 4C for more detailed information. Grade rules writing agencies are another good source of up-to-date information on this topic (see NDS Supplement Section 1.1 on page 2).

| Grade Designation | Species | SG | Shear Parallel to <br> Grain Fv | Compression Perpendicular <br> to Grain $F_{c \perp \perp}$ |
| :--- | :---: | :---: | :---: | :---: |
| $1650 \mathrm{f}-1.5 \mathrm{E}$ | SPF | 0.42 | 135 | 425 |
| $1650 \mathrm{f}-1.5 \mathrm{E}$ | HF | 0.46 | 145 | 405 |
| $2400 \mathrm{f}-2.0 \mathrm{E}$ | SPF | 0.50 | 170 | 615 |
| $2400 \mathrm{f}-2.0 \mathrm{E}$ | SYP | 0.57 | 190 | 805 |

(5) Adjustment Factors: Adjustment factors must be applied to the allowable design values presented here. Adjustments to values are taken for duration of load, repetitive member situations, beam and column stability, and other factors as summarized in NDS Table 2.3.1 on page 9. Machine graded lumber enjoys specific advantages in beam stability, column stability, and buckling stiffness factors as a result of the consistency of E compared to visually graded lumber.

## Lumber Producers Council

## Educational Information

## Design Values of Machine Stress-Rated Lumber

Machine Stress-Rated lumber (MSR Lumber) assures the performance and reliability of your engineered components and structures. The machine grading process sorts dimension lumber by strength and stiffness to improve consistency. Daily quality control testing for strength and stiffness ensures that products meet structural requirements. Machine-graded lumber certification and quality control procedures are based on approved American or Canadian lumber standards (ALS or CLS).

Design Values in Pounds per Square Inch (psi)

| Grade Designation | Bending Fb | Tension Parallel to <br> Grain Ft | Compression Parallel <br> to Grain Fc// | Modulus of Elasticity E |
| :--- | :---: | :---: | :---: | :---: |
| $1650 \mathrm{f}-1.5 \mathrm{E}$ | 1650 | 1020 | 1700 | $1,500,000$ |
| $1800 \mathrm{f}-1.6 \mathrm{E}$ | 1800 | 1175 | 1750 | $1,600,000$ |
| $210 \mathrm{f}-1.8 \mathrm{E}$ | 2100 | 1575 | 1875 | $1,800,000$ |
| $2400 \mathrm{f}-2.0 \mathrm{E}$ | 2400 | 1925 | 1975 | $2,000,000$ |

## Notes:

(1) Other grades of Machine Stress-Rated Lumber: The grades listed above are meant as examples of commonly produced Machine Stress-Rated (MSR) grades. This is not intended to be a complete list. The Supplement to the National Design Specification (NDS) for Wood Construction (AF\& PA, 1997) provides a good summary of the established grades in Machine Stress-Rated (MSR) and Machine Evaluated Lumber (MEL) in Table 4C on page 35.
(2) Quality Control of Machine Stress-Rated Lumber: QC testing takes place on a daily basis for all ma-chine-graded lumber products. Depending on the grade requirements, testing takes place for one or more of the following properties: $\mathrm{E}, \mathrm{Fb}, \mathrm{Ft}$. This testing process is established to verify that production meets design requirements for all products shipped to customers as MSR, MEL, or E-rated laminating grades.
(3) Common Species used in Machine Stress-Rated Lumber: The Machine Stress-Rated Lumber Production Survey (MSR Lumber Producers Council, 1998) identifies Spruce-Pine-Fir (SPF) as the species in which the majority of Machine Stress-Rated Lumber (MSR Lumber) is produced ( $69 \%$ of total Machine Stress Rated in 1998). Douglas Fir-Larch (DFL), Hem-Fir (HF), and Southern Pine (SYP) and share significant volumes as well ( $5 \%$ to $14 \%$ of total Machine Stress-Rated Lumber (MSR Lumber) in 1998). Consult the MSR Lumber Production Survey for more information.
(4) Species-specific design values for Machine Stress-Rated Lumber: Machine Stress-Rated lumber (MSR lumber) simplifies many design considerations since a grade like 1650 M .5 E maintains the same $\mathrm{Fb}, \mathrm{Ft}, \mathrm{Fc} / /$,
and E values no matter what species or size is considered. Other properties, however, change by species as they relate to the specific gravity (density) of wood. A couple of examples are listed below. Please refer to NDS Supplement footnotes \#2 for Table 4C for more detailed information. Grade rules writing agencies are another good source of up-to-date information on this topic (see NDS Supplement Section 1.1 on page 2).

| Grade Designation | Species | SG | Shear Parallel to <br> Grain Fv | Compression Perpendicular <br> to Grain Fc.L |
| :---: | :---: | :---: | :---: | :---: |
| $1650 f-1.5 \mathrm{E}$ | SPF | 0.42 | 135 | 425 |
| $1650 \mathrm{f}-1.5 \mathrm{E}$ | HF | 0.43 | 145 | 405 |
| $2400 \mathrm{f}-2.0 \mathrm{E}$ | SPF | 0.50 | 170 | 615 |
| $2400 \mathrm{f}-2.0 \mathrm{E}$ | SYP | 0.57 | 190 | 805 |

(5) Adjustment Factors: Adjustment factors must be applied to the allowable design values presented here. Adjustments to values are taken for duration of load, repetitive member situations, beam and column stability, and other factors as summarized in NDS Table 2.3.1 on page 9. Machine-graded lumber enjoys specific advantages in beam stability, column stability, and buckling stiffness factors as a result of the consistency of E compared to visually graded lumber.

### 7.10.0 Grade Names for Interior and Exterior Plywood

| Panel Grade Designation | Minimum <br> Face | Veneer <br> Back | Quality Inner Plies | Surface |
| :---: | :---: | :---: | :---: | :---: |
| $\mathrm{N}-\mathrm{N}$ | N | N | C | S2S ${ }^{\text {a }}$ |
| N-A | N | A | C | S2S |
| N-B | N | B | C | S2S |
| N-D | N | D | D | S2S |
| A-A | A | A | D | S2S |
| A-B | A | B | D | S2S |
| A-D | A | D | D | S2S |
| B-B | B | B | D | S2S |
| B-D | B | D | D | S2S |
| Underlayment | C plugged | D | C \& D | Touch sanded |
| C-D plugged | C plugged | D | D | Touch sanded |
| Structural 1 C-D |  |  |  | Unsanded |
| Structural 1 C-D plugged, underlayment |  |  |  | Touch sanded |
| C-D | C | D | D | Unsanded |
| C-D with exterior adhesive | C | D | D | Unsanded |

[^20]Grade Names for Exterior Plywood Grades ${ }^{\text {a }}$

| Panel Grade Designation | Minimum <br> Face | Veneer <br> Back | Quality inner <br> Plies | Surface |
| :--- | :--- | :--- | :--- | :--- |
| Marine, A-A, A-B. B-B, HDO, MDO <br> Special exterior, A-A, A-B, B-B, HDO, MDO |  |  |  | See regular grades <br> A-A |
| A-B | A | A | C | See regular grades |
| A-C | A | B | C | S2S |
| B-B (concrete form) | A | C | C | S2S |
| B-B |  |  | S2S |  |
| B-C | B | B | C | S2S |
| C-C plugged | B | C | C | S2S |
| C-C | C plugged | C | C | Touch sanded |
| A-A high-density overlay | C | C | C | Unsanded |
| B-B high-density overlay | A | A | C plugged | - |
| B-B high-density concrete form overlay | B | B | C plugged | - |
| B-B medium-density overlay | B | B | C plugged | - |
| Special overlays | B | B | C | - |

${ }^{a}$ NIST 1995.
${ }^{b}$ Sanded on two sides.

### 7.10.1 Softwood Plywood Species Grouped by Stiffness and Strength

| Group 1 | Group 2 | Group 3 | Group 4 | Group 5 |
| :---: | :---: | :---: | :---: | :---: |
| Apitong | Cedar, Port Orford | Alder, red | Aspen | Basswood |
| Beech, American | Cypress | Birch, paper | Bigtooth | Poplar |
| Birch | Douglas-fir | Cedar, yellow | Quaking | Balsam |
| Sweet | Fir | Fir, subalpine | Cativo |  |
| Yellow | Balsam | Hemlock, eastern | Cedar |  |
| Douglas-fir | California red | Maple, bigleaf | Incense |  |
| Kapur | Grand | Pine | Western |  |
| Keruing | Noble | Jack | Red |  |
| Larch, western | Pacific silver | Lodgepole | Cottonwood |  |
| Maple, sugar | White | Ponderosa | Eastern |  |
| Pine | Hemlock, western | Spruce | Black |  |
| Caribbean | Lauan | Redwood | (Western Poplar) |  |
| Ocofe | Almon | Spruce | Pine, eastern |  |
| Pine, Southern | Bagtikan | Engelman | White, sugar |  |
| Loblolly | Mayapis | White |  |  |
| Longleaf | Red lauan |  |  |  |
| Shortleaf | Tangile |  |  |  |
| Slash | White lauan |  |  |  |
| Tanoak | Maple, black |  |  |  |
|  | Mengkulang |  |  |  |
|  | Meranti, red |  |  |  |
|  | Mersawa |  |  |  |
|  | Pine |  |  |  |
|  | Pond |  |  |  |
|  | Red |  |  |  |
|  | Virginia |  |  |  |
|  | Western white |  |  |  |


$\left.\begin{array}{|cccc|}\hline \text { Group 1 } & \text { Group 2 } & \text { Group 3 } & \text { Group 4 }\end{array}\right]$| Group 5 |
| :--- |
| Spruce |
| Black |
| Red |
| Sitka |
| Sweetgum |
| Tamarack |
| Yellow poplar |

### 7.10.2 Typical Grade Stamps for Plywood and Oriented Stand Board (OSB)

1. Product Standard that governs specifics of production for construction and industrial plywood.
2. Nominal panel thickness subject to acceptable tolerances.
3. Panel grade designation indicating minimum veneer grade used for panel face and back, or grade name based on panel use.
4. Performance-rated panel standard indicating structural-use panel test procedure recognized by National Evaluation Service (NES).
5. NES report number from Council of American Building Officials (CABO).
6. Exposure durability classification: Exposure 1 indicates interior panel bonded with exterior glue suitable for uses not permanently exposed to weather.
7. Span rating indicating maximum spacing of roof and floor supports for ordinary residential construction applications; 32/16 rating identifies a panel rated for use on roof supports spaced up to 813 mm ( 32 in .) o.c, or floor supports spaced up to 406 mm ( 16 in.) o.c.
8. Sized for spacing denotes panels that have been sized to allow for spacing of panel edges during installation to reduce the possibility of buckling.


FIGURE 10-4 Typical grade stamps for plywood and OSB.

### 7.11.0 Classification of Wood Composite Boards by Particle Size and Process Type



FIGURE 10-2 Classification of wood composite boards by particle size, density, and process type (Suchsland and Woodson 1986).

### 7.11.1 Particleboard Grade Requirements

Particleboard grade requirement ${ }^{\text {a,b,c }}$

| Grade ${ }^{\text {d }}$ | MOR <br> (MPa) | MOE <br> (MPa) | Internal Bond (MPa) | Hardness <br> (N) | Linear <br> Expansion max avg (\%) | Screwholding ( N ) |  | Formaldehyde Maximum Emission (ppm) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  | Face | Edge |  |
| H-1 | 16.5 | 2,400 | 0.90 | 2,225 | NS | 1,800 | 1,325 | 0.30 |
| H-2 | 20.5 | 2,400 | 0.90 | 4,450 | NS | 1,900 | 1,550 | 0.30 |
| H-3 | 23.5 | 2,750 | 1.00 | 6,675 | NS | 2,000 | 1,550 | 0.30 |
| H-1 | 11.0 | 1,725 | 0.40 | 2,225 | 0.35 | NS | NS | 0.30 |
| H-S | 12.5 | 1,900 | 0.40 | 2,225 | 0.35 | 900 | 800 | 0.30 |
| M-2 | 14.5 | 2,225 | 0.45 | 2,225 | 0.35 | 1,000 | 900 | 0.30 |
| M-3 | 16.5 | 2,750 | 0.55 | 2,225 | 0.35 | 1,100 | 1,000 | 0.30 |
| LD-1 | 3.0 | 550 | 0.10 | NS | 0.35 | 400 | NS | 0.30 |
| LD-2 | 5.0 | 1,025 | 0.15 | NS | 0.35 | 550 | NS | 0.30 |

[^21]
### 7.11.2 Particleboard Flooring Grade Requirements

| Grade $^{\text {a }}$ | MOR <br> $(\mathbf{M P a})$ | MOE <br> $(\mathbf{M P a})$ | Internal Bond <br> $(\mathbf{M P a})$ | Hardness <br> $(\mathbf{N})$ | Linear Expansion <br> max avg (\%) | Formaldehyde Maximum <br> Emission (ppm) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| PBU | 11.0 | 1,725 | 0.40 | 2,225 | 0.35 | 0.20 |
| D-2 | 16.5 | 2,750 | 0.55 | 2,225 | 0.30 | 0.20 |
| D-3 | 19.5 | 3,100 | 0.55 | 2,225 | 0.30 | 0.20 |

${ }^{a} P B U=$ underlayment $; D=$ manufactured home decking.

Attrition milling is an age-old concept whereby material is fed between two disks, one rotating and the other stationary. As the material is forced through the preset gap between the disks, it is sheared, cut, and abraded into fibers and fiber bundles. Grain has been ground in this way for centuries.

Attrition milling, or refining as it is commonly called, can be augmented by water soaking, steam cooking, or chemical treatments. Steaming the lignocellulosic weakens the lignin bonds between the cellulosic fibers. As a result, the fibers are more readily separated and usually are less damaged than fibers processed by dry processing methods. Chemical treatments, usually alkali, are also used to weaken the lignin bonds. All of these treatments help increase fiber quality and reduce energy requirements, but they may reduce yield as well. Refiners are available with single- or double-rotating disks, as well as steam-pressurized and unpressurized configurations. For medium-density fiberboard (MDF), steam-pressurized refining is typical.

Fiberboard is normally classified by density and can be made by either dry or wet processes (Fig. 10-2). Dry processes are applicable to boards with high density (hardboard) and medium density (MDF). Wet processes are applicable to both high-density hardboard and low-density insulation board. The following subsections briefly describe the manufacturing of high-and medium-density dry-process fiberboard, wet-process hardboard, and wet-process low-density insulation board. Suchsland and Woodson (1986) and Maloney (1993) provide more detailed information.


Examples of grade stamps for particleboard.

### 7.12.0 Medium-Density Fiberboard and Hardboard Property Requirements

| Product <br> Class ${ }^{\text {a }}$ | Nominal <br> Thickness (mm) | MOR <br> (MPa) | MOE <br> (MPa) | Internal Bond (MPa) | Screw-holding <br> (N) |  | Formaldehyde Emission ${ }^{\text {b }}$ (ppm) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | Face | Edge |  |
| Interior MDF |  |  |  |  |  |  |  |
| HD |  | 34.5 | 3,450 | 0.75 | 1,555 | 1,335 | 0.30 |
| MD | $\leq 21$ | 24.0 | 2,400 | 0.60 | 1,445 | 1,110 | 0.30 |
|  | >21 | 24.0 | 2,400 | 0.55 | 1,335 | 1,000 | 0.30 |
| LD |  | 14.0 | 1,400 | 0.30 | 780 | 670 | 0.30 |
| Exterior MDF |  |  |  |  |  |  |  |
| MD-Exterior | $\leq 21$ | 34.5 | 3,450 | 0.90 | 1,445 | 1,110 | 0.30 |
| adhesive | $>21$ | 31.0 | 3,100 | 0.70 | 1,335 | 1,000 | 0.30 |

${ }^{a}$ MD-Exterior adhesive panels shall maintain at least $50 \%$ of listed MOR after ASTM D1037-1991, accelerated aging (3.3.4). HD $=$ density $>800$ $\mathrm{kg} / \mathrm{m}^{3}\left(>50 \mathrm{lb} / \mathrm{ft}^{3}\right), M D=$ density 640 to $800 \mathrm{~kg} / \mathrm{m}^{3}\left(40\right.$ to $\left.50 \mathrm{lb} / \mathrm{ft}^{3}\right), L D=$ density $<640 \mathrm{~kg} / \mathrm{m}^{3}\left(<40 \mathrm{lb} / \mathrm{ft}^{3}\right)$.
${ }^{b}$ Maximum emission when tested in accordance with ASTM E1333-1990. Standard test method for determining formaldehyde levels from wood products under defined test conditions using a larger chamber (ASTM).

Hardboard Physical Property Requirements ${ }^{\text {a }}$

| Product <br> Class | Normal Thickness (mm) | Water Resistance (max avg/panel) |  | MOR (min avg/ Panel) (MPa) | Tensile Strength (min avg/ panel) (MPa) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Water Absorption Based on Weight (\%) | Thickness Swelling (\%) |  | Parallel to Surface | Perpendicular to Surface |
| Tempered | 2.1 | 30 | 25 | 41.4 | 20.7 | 0.90 |
|  | 2.5 | 25 | 20 | 41.4 | 20.7 | 0.90 |
|  | 3.2 | 25 | 20 | 41.4 | 20.7 | 0.90 |
|  | 4.8 | 25 | 20 | 41.4 | 20.7 | 0.90 |
|  | 6.4 | 20 | 15 | 41.4 | 20.7 | 0.90 |
|  | 7.9 | 15 | 10 | 41.4 | 20.7 | 0.90 |
|  | 9.5 | 10 | 9 | 41.4 | 20.7 | 0.90 |
| Standard | 2.1 | 40 | 30 | 31.0 | 15.2 | 0.62 |
|  | 2.5 | 35 | 25 | 31.0 | 15.2 | 0.62 |
|  | 3.2 | 35 | 25 | 31.0 | 15.2 | 0.62 |
|  | 4.8 | 35 | 25 | 31.0 | 15.2 | 0.62 |
|  | 6.4 | 25 | 20 | 31.0 | 15.2 | 0.62 |
|  | 7.9 | 20 | 15 | 31.0 | 15.2 | 0.62 |
|  | 9.5 | 15 | 10 | 31.0 | 15.2 | 0.62 |
| Servicetempered | 3.2 | 35 | 30 | 31.0 | 3.8 | 0.52 |
|  | 4.8 | 30 | 30 | 31.0 | 3.8 | 0.52 |
|  | 6.4 | 30 | 25 | 31.0 | 3.8 | 0.52 |
|  | 9.5 | 20 | 15 | 31.0 | 3.8 | 0.52 |

${ }^{a}$ AHA 1995a.

### 7.13.0 Physical and Mechanical Properties of Hardboard Siding

| Property ${ }^{\text {a }}$ | Requirement |
| :---: | :---: |
| Water absorption (based on weight) | 12\% (max avg/panel) |
| Thickness swelling | 8\% (max avg/panel) |
| Weatherability of substrate (max residual swell) | 20\% |
| Weatherability of primed substrate | No checking, erosion, flaking, or objectionable fiber raising; adhesion, less than 3.2 mm ( 0.125 in.) of coating picked up |
| Linear expansion 30\% to 90\% RH (max) | Thickness range Maximum linear <br> (cm) <br> expansion (\%)  |
|  | 0.220-0.324 0.36 |
|  | 0.325-0.375 0.38 |
|  | 0.376-0.450 0.40 |
|  | $>0.451 \quad 0.40$ |
| Nail-head pull-through | 667 N (150 lb) (min avg/panel) |
| Lateral nail resistance | 667 N (150 lb) (min avg/panel) |
| Modulus of rupture | $12.4 \mathrm{MPa}\left(1,800 \mathrm{lb} / \mathrm{in}^{2}\right)$ for $9.5,11$, and 12.7 mm (3/8, 7/16, and $1 / 2 \mathrm{in}$.) thick (min avg/panel) 20.7 MPa ( $3,000 \mathrm{lb} / \mathrm{in}^{2}$ ) for 6.4 mm ( $1 / 4 \mathrm{in}$.) thick (min avg/panel) |
| Hardness | $2002 \mathrm{~N}(450 \mathrm{lb})(\mathrm{min}$ avg/panel) |
| Impact | 229 mm (9 in.) (min avg/panel) |
| Moisture contend ${ }^{\text {b }}$ | $4 \%$ to $9 \%$ included, and not more than $3 \%$ variance between any two boards in any one shipment or order |
| ${ }^{a}$ Refer to ANSI/AHA A135.6 1-1990 for test method for determining information on properties. <br> ${ }^{b}$ Since hardboard is a wood-based material, its moisture content varies with environmental humidity conditions. When the environmental humidity conditions in the area of intended use are a critical factor, the purchaser should specify a moisture content range more restrictive than 4 to $9 \%$ so that fluctuation in the moisture content of the siding will be kept to a minimum. |  |

Properties of hardboard siding; hardboard siding products come in a great variety of finishes and textures (smooth or embossed) and in different sizes. For application purposes, the AHA classifies siding into three basic types:

Lap siding-boards applied horizontally, with each board overlapping the board below it
Square edge panels-siding intended for vertical application in full sheets
Shiplap edge panel siding—siding intended for vertical application, with the long edges incorporating shiplap joints

The type of panel dictates the application method. The AHA administers a quality conformance program for hardboard for both panel and lap siding. Participation in this program is voluntary and is open to all (not restricted to AHA members). Under this program, hardboard siding products are tested by an independent laboratory in accordance with product standard ANSI/AHA A135.6. Figure 10-13a provides an example of a grade stamp for a siding product meeting this standard.

Insulation Board-Physical and mechanical properties of insulation board are published in the ASTM C208 standard specification for cellulosic fiber insulation board.


FIGURE 10-13 Examples of grade stamps: (a) grade stamp for siding conforming to ANSI/AHA A135.6 standard, and (b) grade mark stamp for cellulosic fiberboard products conforming to ANSI/AHA A194.1 standard.

## Section 8

## Fasteners for Wood and Steel-Calculations for Selection

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### 8.1.0 Nail Sizes-Common Wire Nails

| Size | Length (inches) | Gauge | Number in a Pound | Safe Lateral Load* |  | Resistance to withdrawal in pounds per inch of penetration perpendicular to the grain, into the main member, in Douglas Fir |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Required Penetration (inches) | Load (pounds) in Douglas Fir |  |
| 2d | 1 | 15 | 876 |  |  |  |
| 3d | $11 / 4$ | 14 | 568 |  |  |  |
| 4d | $11 / 2$ | $12^{1 / 2}$ | 316 |  |  |  |
| 5 d | $13 / 4$ | 121/2 | 271 |  |  |  |
| 6 d | 2 | 111/2 | 181 | 1 | 70 | 27 |
| 7 d | $21 / 4$ | 111/2 | 161 |  |  |  |
| 8d | 21/2 | $10^{1 / 4}$ | 106 | $11 / 4$ | 100 | 32 |
| 9d | 23/4 | 101/2 | 96 |  |  | 32 |
| 10d | 3 | 9 | 69 | 11/2 | 120 | 36 |
| 12d | $31 / 4$ | 9 | 64 | $15 / 8$ | 130 | 36 |
| 16d | $31 / 2$ | 8 | 49 | $15 / 8$ | 160 | 40 |
| 20d | 4 | 6 | 31 | 2 | 190 | 48 |
| 30d | $4^{1 / 2}$ | 5 | 24 | $2^{1 / 4}$ | 230 | 52 |
| 40d | 5 | 4 | 18 | $21 / 3$ | 270 | 56 |
| 50d | $51 / 2$ | 3 | 14 | 23/4 | 310 | 61 |
| 60d | 6 | 2 | 11 | 3 | 360 | 67 |

*For nails inserted perpendicular to the grain. For nails driven parallel to the grain or toe-nailed, the load should not be more than $2 / 3$ of the value in column 6.
Source: U.S. Department of Agriculture.

### 8.1.1 Bright Common Nails, Box Nails, Annular Nails—Length and Diameter in United States and Metric

## Nails

Nails are the most common mechanical fastenings used in wood construction. There are many types, sizes, and forms of nails (Fig. 8-1). The load equations presented in this chapter apply for bright, smooth, common steel wire nails driven into wood when there is no visible splitting. For nails other than common wire nails, the loads can be adjusted by factors given later in the chapter.


FIGURE 8-1 Various types of nails: (left to right) bright smooth wire nail, cement coated, zinc-coated, annularly threaded, helically threaded, helically threaded and barbed, and barbed.

Nails in use resist withdrawal loads, lateral loads, or a combination of the two. Both withdrawal and lateral resistance are affected by the wood, the nail, and the condition of use. In general, however, any variation in these factors has a more pronounced effect on withdrawal resistance than on lateral resistance. The serviceability of joints with nails laterally loaded does not depend greatly on withdrawal resistance unless large joint distortion is tolerable.

The diameters of various penny or gauge sizes of bright common nails are given in Table 8-1. The penny size designation should be used cautiously. International nail producers sometimes do not adhere to the dimensions of Table $8-1$. Thus penny sizes, though still widely used, are obsolete. Specifying nail sizes by length and diameter dimensions is recommended. Bright box nails are generally of the same length but slightly smaller diameter (Table 8-2), while cement-coated nails such as coolers, sinkers, and coated box nails are slightly shorter ( $3.2 \mathrm{~mm}[1 / 8 \mathrm{in}$.$] ) and of smaller diameter than common nails of the same penny size. Helically and annularly$ threaded nails generally have smaller diameters than common nails for the same penny size (Table 8-3).

## Withdrawal Resistance

The resistance of a nail shank to direct withdrawal from a piece of wood depends on the density of the wood, the diameter of the nail, and the depth of penetration. The surface condition of the nail at the time of driving also influences the initial withdrawal resistance.

TABLE 8-1 Sizes of Bright Common Wire Nails

| Size | Gauge | Length (mm [in.] ) |
| :---: | :---: | :---: |
| 6d | $11-1 / 2$ | $50.8(2)$ |
| 8d | $10-1 / 4$ | $63.5(2-1 / 2)$ |
| 10d | 9 | $76.2(3)$ |
| 12d | $82.6(3-1 / 4)$ | $2.87(0.113)$ |
| 16d | 8 | $88.9(3-1 / 2)$ |
| 20d | $101.6(4)$ | $3.33(0.131)$ |
| 30d | 6 | $114.3(4-1 / 2)$ |
| 40d | 5 | $127.0(5)$ |
| 50d | 4 | $139.7(5-1 / 2)$ |
| 60d | 3 | $152.4(6)$ |

TABLE 8-2 Sizes of Smooth Box Nails

| Size | Gauge | Length (mm [in.)]) | Diameter (mm [in.] ) |
| :---: | :---: | :---: | :---: |
| 3d | $14-1 / 2$ | $31.8(1-1 / 4)$ | $1.93(0.076)$ |
| 4d | 14 | $38.1(1-1 / 2)$ | $2.03(0.080)$ |
| 5d | 14 | $44.5(1-3 / 4)$ | $2.03(0.080)$ |
| 6d | $12-1 / 2$ | $50.8(2)$ | $2.49(0.098)$ |
| 7d | $12-1 / 2$ | $57.2(2-1 / 4)$ | $2.49(0.098)$ |
| 8d | $11-1 / 2$ | $63.5(2-1 / 2)$ | $2.87(0.113)$ |
| 10d | $10-1 / 2$ | $76.2(3)$ | $3.25(0.128)$ |
| 16d | 10 | $88.9(3-1 / 2)$ | $3.43(0.135)$ |
| 20d | 9 | $101.6(4)$ | $3.76(0.148)$ |

TABLE 8-3 Sizes of Helically and Annularly Threaded Nails

| Size | Length (mm [in.] ) | Diameter (mm [in.] ) |
| :---: | :---: | :---: |
| 6d | $50.8(2)$ | $3.05(0.120)$ |
| 8d | $63.5(2-1 / 2)$ | $3.05(0.120)$ |
| 10d | $76.2(3)$ | $3.43(0.135)$ |
| 12d | $82.6(3-1 / 4)$ | $3.43(0.135)$ |
| 16d | $88.9(3-1 / 2)$ | $3.76(0.148)$ |
| 20d | $101.6(4)$ | $4.50(0.177)$ |
| 30d | $114.3(4-1 / 2)$ | $4.50(0.177)$ |
| 40d | $127.0(5)$ | $4.50(0.177)$ |
| 50d | $139.7(5-1 / 2)$ | $4.50(0.177)$ |
| 60d | $152.4(6)$ | $4.50(0.177)$ |
| 70d | $177.8(7)$ | $5.26(0.207)$ |
| 80d | $203.2(8)$ | $5.26(0.207)$ |
| 90d | $228.6(9)$ | $5.26(0.207)$ |
| Sol |  |  |

Source: U.S. Department of Agriculture.

### 8.1.2 Nail Sizes-Common Wire Spikes

| Size | Length (inches) | Gauge | Number per Pound |
| :---: | :---: | :---: | :---: |
| 10d | 3 | 6 | 41 |
| 12d | $31 / 4$ | 6 | 38 |
| 16d | $31 / 2$ | 5 | 30 |
| 20d | 4 | 4 | 23 |
| 30d | $41 / 2$ | 3 | 17 |
| 40d | 5 | 1 | 13 |
| 50d | $51 / 2$ | 1 | 10 |
| 60d | 6 |  | 8 |
| By permission: tools@sizes.com |  |  |  |

Formerly $7^{\prime \prime}$ wire spikes were 0 gauge ( $0.3065^{\prime \prime}$ ), and $8^{\prime \prime}$ and $9^{\prime \prime}$ spikes were 00 gauge ( $0.331^{\prime \prime}$ ). Now $7^{\prime \prime}, 8^{\prime \prime}$, and $9^{\prime \prime}$ spikes are $5 / 16$ inch in diameter and $10^{\prime \prime}$ and $12^{\prime \prime}$ spikes are $3 / 8$ inch.

### 8.1.3 Nail Sizes-Casing Nails

These nails are used where the nail head must be hidden. They have small heads and smaller diameters than common nails.

Casing nails have a conical head, sometimes cupped, and are somewhat thicker than a finishing nail. They are sometimes sold already painted and are used to attach trim.

| Size | Length (inches) | Gauge | Number per Pound |
| :---: | :---: | :---: | :---: |
| $2 d$ | 1 | $15^{1 ⁄ 2}$ | 1,010 |
| $3 d$ | $11 / 4$ | $141 / 2$ | 635 |
| 4d | $11 / 2$ | 14 | 473 |
| 5d | $13 / 4$ | $12^{1 ⁄ 2}$ | 406 |
| 6d | 2 | 236 |  |


| Size | Length (inches) | Gauge | Number per Pound |
| :---: | :---: | :---: | :---: |
| 7d | 21/4 | $12^{1 / 2}$ | 210 |
| 8d | 21/2 | $111 / 2$ | 145 |
| 9d | 23/4 | $111 / 2$ | 132 |
| 10d | 3 | 101/2 | 94 |
| 12d | $31 / 4$ | 101/2 | 87 |
| 16d | $31 / 2$ | 10 | 71 |
| 20d | 4 | 9 | 52 |
| 30d | $41 / 2$ | 9 | 46 |
| 40d | 5 | 8 | 35 |

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### 8.1.4 Nail Sizes-Finishing Nails

fasteners $>$ nails
Finishing nails are used where the nail head must be hidden. They have small heads and smaller diameters than common nails.

Finishing nails, seen in profile, have a barrel-shaped head with a small diameter and a dimple on the top. After the nail is driven almost flush with the surface, the point of a nail set is placed in the dimple and the head driven below the surface. The resulting small hole can be filled with putty. Outdoors, in time the hole will tend to close by itself when the wood fibers swell.

| Size | Length (inches) | Gauge | Number per Pound |
| :---: | :---: | :---: | :---: |
| 2d | 1 | 161/2 | 1,351 |
| 3d | $11 / 4$ | 151/2 | 807 |
| 4d | $11 / 2$ | 15 | 584 |
| 5 d | $13 / 4$ | 15 | 500 |
| 6d | 2 | 13 | 309 |
| 7 d | $21 / 4$ | 13 | 238 |
| 8d | $21 / 2$ | $12^{1 / 2}$ | 189 |
| 9d | 23/4 | 121/2 | 172 |
| 10d | 3 | 111/2 | 121 |
| 12d | $31 / 4$ | 111/2 | 113 |
| 16d | $31 / 2$ | 11 | 90 |
| 20d | 4 | 10 | 62 |

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### 8.1.5 Nail Sizes—Deformed Shank Nails

Deformed shank nails are those that have helical shanks, resembling a screw, or that have shanks covered by rings and grooves. Deformed shanks are used in the hope of increasing the resistance of the nail to being pulled out, compared to plain shank nails.

Lawrence Soltis reports that the withdrawal resistance of ring shank nails is about $40 \%$ greater than that of common nails. The benefit is much greater in wood subject to repeated changes in moisture content; under those conditions deformed shank nails can be four times better than a plain shank nail of the same diameter. He also suggests a basis for choosing between a ring or a helical shank: "In general, annularly threaded nails sustain larger withdrawal loads, and helically threaded nails sustain greater impact withdrawal work values than do other nail forms."

Besides this series, many other types of nails are available with ring or spiral shanks.

| Size | Length (inches) | Gauge | Diameter (inches) |
| :---: | :---: | :---: | :---: |
| 3d | $1 \frac{1}{4}$ | $12^{1 / 2} 2$ | 0.099 |
| 4d | $1 \frac{1}{2}$ | $12^{1 / 2}$ | 0.099 |
| 6d | 2 | 11 | 0.120 |
| 8d | $21 / 2$ | 11 | 0.120 |
| 10d | 3 | 10 | 0.135 |
| 12d | $31 / 4$ | 10 | 0.135 |
| 16d | $31 / 2$ | 9 | 0.148 |
| 20d | 4 | 7 | 0.177 |
| 30d | $41 / 2$ | 7 | 0.177 |
| 40d | 5 | 7 | 0.177 |
| 50d | $51 / 2$ | 7 | 0.177 |
| 60d | 6 | 7 | 0.177 |
| 70d | 7 | 5 | 0.207 |
| 80d | 8 | 5 | 0.207 |

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### 8.1.6 Nail Sizes—Roofing Nails

Roofing nails are short, diamond-pointed steel nails with a wide flat head. The shank may be barbed, and they are often galvanized.

A peculiarity of the roofing nails standard is that it is the only type of nail with zero tolerance for undersize heads.

| Length (inches) | Gauge | Head diameter (inches) |
| :--- | :---: | :---: |
| $3 / 4$ | 12 | 0.375 |
|  | 11 | 0.375 |
|  | 11 | 0.438 |
|  | 10 | 0.469 |
|  | $91 / 2$ | 0.484 |
|  | 9 | 0.500 |
|  | 8 | 0.500 |
| $7 / 8$ | 12 | 0.375 |
|  | 11 | 0.375 |
|  | 11 | 0.438 |
|  | 11 | 0.500 |
|  | 10 | 0.469 |
|  | $91 / 2$ | 0.484 |
|  | 9 | 0.500 |
| 1 | 8 | 0.281 |
|  | 12 | 0.375 |
|  | 12 | 0.375 |
|  | 11 | 0.438 |
|  | 11 | 0.500 |
|  | 11 | 0.469 |
|  | 10 | 0.484 |
|  | $91 / 2$ | 0.500 |
|  | 9 | 0.500 |


| Length (inches) | Gauge | Head diameter (inches) |
| :---: | :---: | :---: |
| $11 / 8$ | 12 | 0.375 |
|  | 11 | 0.438 |
|  | 10 | 0.469 |
|  | $91 / 2$ | 0.484 |
|  | 9 | 0.500 |
|  | 8 | 0.500 |
| $11 / 4$ | 12 | 0.375 |
|  | 11 | 0.312 |
|  | 11 | 0.375 |
|  | 11 | 0.438 |
|  | 11 | 0.500 |
|  | 10 | 0.469 |
|  | $91 / 2$ | 0.484 |
|  | 9 | 0.500 |
|  | 8 | 0.500 |
| $11 / 2$ | 12 | 0.375 |
|  | 11 | 0.375 |
|  | 11 | 0.438 |
|  | 11 | 0.500 |
|  | 10 | 0.469 |
|  | $91 / 2$ | 0.484 |
|  | 9 | 0.500 |
|  | 8 | 0.500 |
| $13 / 4$ | 12 | 0.375 |
|  |  | 0.375 |
|  | 11 | 0.438 |
|  | 11 | 0.500 |
|  | 11 | 0.469 |
|  | 10 | 0.484 |

### 8.1.7 Nail Sizes—Joist Hanger Nails

## fasteners $>\underline{\text { nails }}$

These short, stout nails are used to install sheet metal connectors, including joist hangers. The appropriate size is specified by the connector manufacturer, and frequently the connector is packaged with the necessary number of nails.

They are galvanized, and versions in stainless steel are also available.

Smooth Shanks

|  | Length <br> (inches) | Gauge | Head Diameter <br> (inches) | No. in a Pound |
| ---: | :---: | :---: | :---: | :---: |
| 8 d | $11 / 2$ | $101 / 4$ | $9 / 32$ | 147 |
| 10 d | $11 / 2$ | 9 | $5 / 16$ | 123 |

By permission: tools@sizes.com

Joist hanger nails are also available with deformed shanks for greater holding power.

### 8.1.8 Cut Nails—Various Configurations

Cut nails possess great durability. They are hard to pull out because the wood fibers are pushed downward and wedge against the nails, thus greatly reducing loosening. Once your customers have used them, they will prefer them for all kinds of work.

Click on any of the products listed below for details.



# 8.2.0 Instructions on Nail Selection and Usage for Exposed Wood Structures Fasteners for Exposed Wood Structures 

Robert H. Falk, P.E. and Andrew J. Baker


#### Abstract

This paper provides an overview of the use of fasteners that are appropriate for exposed wood structures. Several types of fasteners are reviewed, and physical and chemical explanations for fastener corrosion are provided. Recommendations for long-term performance are given.

\section*{Introduction}

One of the most important considerations in building a wood outdoor structure is its performance as a structural system. Unlike the skeleton of a conventionally framed wood house, many outdoor structures are built without the sheathing, siding, and roof that provide structural stability and protection from the environment. Fully exposed to the degrading effects of the weather, structural members must be properly designed and connected to


ensure long-term, safe performance. Structural support comes from not only the proper sizing and placement of the posts, beams, joists, and other members used in the construction, but from the connection of these members. This paper discusses the fasteners recommended for use in outdoor structures.

## Fastener Types

The overall integrity of any wood structure depends on how its components are held together. Therefore, it makes little sense to properly size the wood members only to improperly fasten them. The most common fasteners for wood construction are nails, screws, lag screws, and bolts. Metal straps and hangers of various types are also available. Fasteners used for wood construction are typically manufactured from mild steel, although many types and sizes can be made from stainless steel, brass, and bronze. Nails and screws are the most common type of fasteners for attaching members in light-frame structures. For fastening heavy members of an outdoor structure, such as the beam to the posts, lag screws or bolts are the fasteners of choice.

Holding power and corrosion protection are probably the two most important concerns when choosing fasteners. Improperly specified fasteners can loosen when the wood shrinks and swells as a result of moisture cycling of exposed lumber. Rusting of steel fasteners not only weakens the fastener, but the chemical reactions involved in corrosion can also weaken the wood surrounding the fastener.

Nails. Smooth-shanked nails can lose some of their withdrawal resistance when exposed to wetting and drying cycles, resulting in nail pop-up and loosening of connections. Through the use of a wet-service, strengthreduction factor, the National Design Specification (NDS) (AFPA, 1991) accounts for wood shrinkage from around smooth-shanked nails as the moisture content changes from wet to dry. However, better performance in withdrawal can be expected by using deformed shank nails to resist the effects of severe wetting and drying of exposed wood structures. Two commonly available deformed shank fasteners with the capacity to retain withdrawal resistance are spirally grooved and annularly grooved (ring-shanked) nails.

Screws. Common wood screws have been used to fasten wood for decades; however, the more recently developed multipurpose screw has found common use in wood-deck construction, primarily to fasten deck boards to joists. These fasteners have a thread design that can be driven fast, and they have good holding power. Unlike common wood screws, they are straight shanked. Commonly available in $2-$ to $3-\mathrm{in}$. ( $50-\mathrm{to} 75-\mathrm{mm}$ ) lengths, multipurpose screws are available with a Phillips head or square recess head and are most easily driven with a power drill.

Multipurpose screws are not intended to fasten joist hangers to beams and will not equal the design capacity of the hanger. Only manufacturer-specified hanger nails should be used to attach hangers.

Screws have an advantage over nails in that they are more easily withdrawn to remove defective or damaged members. They are also effective in drawing down cupped or twisted decking boards into a flat position, and will resist withdrawal over time.

Lag Screws and Bolts. Lag screws are commonly used to fasten one member to a thicker member where a through bolt cannot be used. Pilot holes must be drilled for lag screws, and the screw must be fully inserted to be effective. According to the NDS (AFPA, 1991), for softwood species typically used in outdoor structures, pilot holes should be about 60 to $70 \%$ of the diameter of the screw for the threaded portion, and the full diameter for the unthreaded shank. Make sure the lag screw is long enough so that at least half of its length penetrates the thicker member.

Bolts offer more rigidity and typically more load-carrying capacity than lag screws. However, their use is obviously limited to situations where a hole can be drilled completely through the members to be connected.

Holes drilled for bolts should be no more than $1 / 16 \mathrm{in}$. ( 2 mm ) larger in diameter than the size of the bolt used. As with lag screws, washers should be used under both the head of the bolt and nut to distribute the bolt force over a larger area and limit crushing of the wood. Machine bolts are a better choice than carriage bolts because carriage bolts are manufactured for use without washers. Dome-head bolts, typically used in heavy timber construction, are also a good choice. After drilling holes for fasteners, it is important to immediately saturate the holes with a preservative, such as copper napthenate. After about one year, bolts should be retightened, and thereafter checked for tightness every year or so.

Joist Hangers and Metal Straps. Joist hangers, metal straps, and other hardware are often used in outdoor wood construction; however, most are intended for indoor use. Although typically electroplated with zinc, their long-term corrosion resistance in exposed environments is unknown. Some manufacturers make these products from stainless steel or apply heavy coatings of galvanizing to increase longevity.

Source: USDA Forest Service.

## Wood/Metal Interaction

Wood and metal are compatible in most construction; however, if there is sufficient moisture at the wood-tometal interface, some corrosion can be expected with susceptible metals. The corrosion of metal in contact with moist wood is an electrochemical process. The rate and amount of corrosion depend on the metal, the conductivity of the wood, and the duration and temperature of the surrounding environmental conditions. The risk of corrosion depends somewhat on the wood species, presence of external corrosive contaminants, and condition of the wood (untreated or treated with certain chemicals). Not only does moist wood in contact with metal cause some corrosion, but the chemical byproducts of corrosion can result in a slow deterioration of the wood adjacent to the metal. As a result, the fastener will lose cross section, and there will be some enlargement of the hole around the fastener. Additionally, most woods are slightly acidic, which may accelerate the corrosion of the steel (or galvanized coating).

If the moisture content of the wood is less than about $18 \%$, the metal corrosion rate is minimal (Baker, 1988). Remember, however, that only the moisture of the wood in contact with the metal is important. This means that metal, such as a fastener that has been cooled by ambient conditions and kept cool by the surrounding wood, can corrode when it becomes wet by condensation, such as on a warm, humid day following a cool night in the early spring. The condensed moisture wets the wood at the wood-to-metal interface. At first, this results in only an iron stain on the surrounding wood surface, but in time, the iron will chemically damage the wood structure and weaken the joint. This type of corrosion is responsible for the failure of many unheated wood structures, such as barns and sheds, in areas with humid days and cool nights.

Some fasteners are corrosion resistant because of a protective coating, and some are resistant because of the properties of the metal or alloy. A fastener can be resistant to corrosion in one environment, but corrode in another. A good example is aluminum, which will perform well in untreated wood exposed to the environment, but will corrode rapidly in wood treated with preservatives that contain copper.

## Coated Steel Fasteners

Most steel fasteners are uncoated because they are intended to be used in protected environments (indoors). Obviously, if these fasteners are exposed to the weather, they can rapidly corrode. In the mildest of cases, this corrosion can lead to unsightly staining of the wood. In more severe cases, it can cause complete disintegration of the fastener and a total loss of structural strength (Baker, 1988).

Several types of coatings are used to protect steel fasteners. These include chromate paint, plastic, ceramic, and metal coatings (galvanizing). Adhesive-type coatings (e.g., paint and plastic) can flake off when driving the fastener, compromising the protection it was intended to provide.

## General Guidelines for Fastener Use

1. At a minimum, use hot-dipped galvanized metal in outdoor wood structures.
2. Use stainless steel fasteners for added durability in severe exposures.
3. Always fasten a thinner member to a thicker one.
(a) A nail should be long enough to penetrate the receiving member a distance twice the thickness of the thinner member.
(b) A lag screw should penetrate the larger member by at least half the length of the screw.
4. Reduce splitting of boards when nailing by:
(a) Placing nails no closer to the edge than $1 / 2$ the board thickness and no closer to the end than the board thickness.
(b) Pre-drilling nail holes.
(c) Blunting the nail point.
(d) Using greater spacing between nails.
(e) Staggering nails in each row to prevent splitting along the grain.
5. Avoid end-grain nailing when possible.
6. When drilling holes for lag screws or bolts, saturate the hole with a preservative such as copper napthenate to prevent the migration of decay fungi into the untreated part of the member.
7. Use washers with bolts and lag screws to reduce crushing of the wood.
8. Tighten bolts and lag screws one year after construction and thereafter check tightness periodically.

While the coating protects by providing a barrier between the steel and the environment, galvanized coatings sacrificially corrode to protect the steel. When the coating is gone, the steel will begin to corrode. The galvanizing can be applied by electroplating, mechanical plating, or single- or double-dipping the fastener in molten zinc (hot dipped). The thickness of the coating is very important; a thicker coating provides additional protection.

Most manufacturers coat fasteners to the standard ASTM A153-87 (ASTM, 1987), which specifies a minimum coverage of $0.85 \mathrm{oz} / \mathrm{ft}^{2}\left(259 \mathrm{~g} / \mathrm{m}^{2}\right)$ of zinc. This is probably thick enough for most outdoor structures in dry-weather areas. Although the corrosion process typically proceeds over many years, our research shows that commonly available coated fasteners simply do not have a thick enough plating for long-term protection ( 20 yr ) in severe (underground or high humidity) environments (Baker, 1992). Thicker coatings [ $1.0 \mathrm{oz} / \mathrm{ft}^{2}\left(305 \mathrm{~g} / \mathrm{m}^{2}\right)$ ] are available and should be used in wetter situations. Coating specifications should be available from the fastener manufacturer.

Unfortunately, many building contractors use only electroplated nails for outdoor construction because they are readily available for use in pneumatic nail guns. Hot-dipped galvanized fasteners are produced for pneumatic nail guns, but their availability is limited.

## Stainless Steel, Copper, and Aluminum Fasteners

The chemical properties of stainless steel make it resistant to corrosion. Although more expensive than hot-dipped galvanized fasteners, stainless steel is a more durable option, particularly for outdoor structures that are located in high-humidity areas or that remain wet for much of the time. Research has shown that
little long-term degradation of stainless steel fasteners occurs even in the most severe exposure conditions (Baker, 1992). Also, the use of stainless steel fasteners reduces the possibility of staining around the fastener.

Although stainless steel fasteners are available in several grades, the American Iron and Steel Institute's (AISI) 300 series (e.g., 302,303,304, and 316) is appropriate for use in outdoor wood structures. While the price of stainless steel fasteners and hardware can be several times higher than the price of mild steel, their use is justified. The relatively small cost increase to the overall structure adds significantly to its reliability and long-term performance in severe conditions.

Copper, usually of rather high purity, and an alloy, silicon bronze, are often used to fasten wood (wood shakes, shingles, and in boat construction), although usually not in conventional structural applications.

Aluminum is suitable for use with untreated wood and wood treated with an oil-type preservative. However, aluminum should never be used in contact with wood that is treated with a waterborne preservative that contains copper, such as chromated copper arsenate (CCA), ammoniacal copper zinc arsenate (ACZA), or ammoniacal copper arsenate (ACA).

## Corrosion in Untreated Wood

For an isolated fastener in moist wood, crevice corrosion can occur (Baker, 1988). This is the type of corrosion observed in crevices along riveted and welded seams of metal tanks and pipes. The head of a steel fastener in moist wood usually acts as a cathode, and the shank in the "crevice" serves as the anode. At the anode, iron goes into solution in the form of ferrous ions. As corrosion proceeds, hydroxides of iron precipitate. This leaves an excess of hydrogen ions in the surrounding water and the pH decreases. In addition, the wood fiber is chemically degraded as the ferrous ions are oxidized to ferric ions (Baker, 1988).

Dissimilar metals that are in physical contact with each other can result in galvanic corrosion, in which the corrosion of the least corrosion-resistant metal increases and the corrosion of the most corrosion-resistant metal decreases. Because of this, the washer, nut, and bolt or lag screw should be manufactured from the same metal.

## Corrosion in Preservative-Treated Wood

Oil-type preservatives. Corrosion of metals in wood treated with oil-type preservatives is usually not a problem because the presence of heavy oils tends to inhibit corrosion. This is especially true in construction situations in which the holes for the fasteners are bored prior to treating.

When the preservative has not penetrated to the center of the wood member, such as in large beams or posts, fasteners are driven into moist, untreated wood, and fastener corrosion can occur.

Waterborne preservatives. Waterborne preservatives that contain copper cause corrosion of some metals. Corrosion in moist copper-treated wood is directly related to the presence of copper ions because they will "plate out" on a fastener that is more electronegative than copper. When this happens, a galvanic corrosion cell consisting of the fastener and the deposited copper is formed and the fastener corrodes (Baker, 1988).

## Geographic Location

Because a variety of climates and exposure conditions exist, local conditions should dictate proper fastener selection. In the United States, climates range from subtropical to desert to arctic. This has a large effect on the corrosion rates of metal fasteners in wood. Where the climate is moist and warm, corrosion rates are the highest; where it is cold and/or dry, corrosion rates are the lowest. The corrosion rates can differ by a factor of five to ten.

The average outdoor humidity in North America varies depending on location and season. In areas of higher average humidity and warmer temperatures (e.g., the southeastern United States, portions of the Midwest, and along the coasts), the hazard of fastener corrosion (and wood decay fungi attack) is greatest. Even in dry areas of the country, an outdoor structure that is very near or over water, or for some reason is wetted much of the time, can have a high moisture content, thus promoting corrosion and decay.

## Recommended Fasteners and Hardware

For treated or untreated wood that is above grade in structures exposed to weather, we suggest hot-dipped galvanized steel fasteners with at least 0.85 oz of zinc per $\mathrm{ft}^{2}\left(259 \mathrm{~g} / \mathrm{m}^{2}\right)$. This recommendation also applies for hardware used within the structure (joist hangers, straps). For wood that is below grade and treated with a preservative that contains copper, or that is in contact with saltwater, we suggest the use of AISI stainless steel Type 304, copper, or silicon bronze. Note that engineering design values are not published for copper and silicon bronze.

## References

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### 8.3.0 How Did the "Penny Weight" Nail Designation Originate? The Penny Nail

This article in The Ironmonger from 1915 tells us the story of the "penny" nail.
In this case, the researcher examined the records dated 1477 from the Church of St Mary-at-Hill in the City of London.

Although many different handmade nails were in use at the time which had specific names, a large proportion were named simply by the number of pence paid for a hundred nails.

For example, four penny nails were those of which a hundred were purchased for 4 d . (The ' d ' stands for pence in the days when sterling was denominated in pounds, shillings and pence $-£ \mathrm{sd}$ ).

The account records of the Church of St Mary-at-Hill show
'ffor a c of v peny nayle vd'
The ' $c$ ' is the Roman numeral for 'hundred' and ' $v$ ' is the Roman numeral for 'five'.
The amount of money paid for a hundred nails-fourpence, fivepence, sixpence-is thought to depend on the size of the nail. The larger the nail, the more expensive it was. The largest nail appears to have been the tenpenny nail, also referred to as the 'fyve stroke nayle'-possibly because it took five strokes of the hammer to get it home.

This nomenclature for nails 4d, 5d, 6d, etc., is still in use today particularly in the United States but relates only to the size of the nails, not the price!

| Size | Length | Size | Length | Size | Length |
| :--- | :--- | :--- | :--- | :--- | :--- |
| 2 d | $1^{\prime \prime}$ | 8 d | $21 / 2^{\prime \prime}$ | 30 d | $41 / 2^{\prime \prime}$ |
| 3 d | $11 / 4^{\prime \prime}$ | 9 d | $23 / 4^{\prime \prime}$ | 40 d | $5^{\prime \prime}$ |
| 4 d | $11 / 2^{\prime \prime}$ | 10 d | $3^{\prime \prime}$ | 50 d | $51 / 2^{\prime \prime}$ |
| 5 d | $13 / 4^{\prime \prime}$ | 12 d | $31 / 4^{\prime \prime}$ | 60 d | $6^{\prime \prime}$ |
| 6 d | $2^{\prime \prime}$ | 16 d | $31 / 2^{\prime \prime}$ | 80 d | $7^{\prime \prime}$ |
| 7 d | $21 / 4^{\prime \prime}$ | 20 d | $4^{\prime \prime}$ | 100 d | $8^{\prime \prime}$ |
| By permission: Glasgow Steel Nail, Glasgow, Scotland |  |  |  |  |  |

For those who would like to work out the true cost today, the article tells us that the medieval penny would have been the equivalent of around 1s 6 d in value in 1915. Government sources suggest that prices have risen over 61 -fold since 1914 , so a medieval penny might be worth around $£ 4.50$ today.

### 8.4.0 About Nails—Historic and Otherwise

## About Nails

Hand-forged nails were the first manufactured nails, and they date back to biblical times. As people first used hewn beams, timbers, planks, and whole logs to build with, the early hand-made nails were spikes. With the development of the split wood shingle, nails of about $1^{\prime \prime}$ long came into use. When sawyers, and then sawmills, began cutting dimension lumber, the sizes and varieties of nails greatly expanded. Thus, over time, nails developed in different sizes, shapes, and used different heads to fasten lumber and wood.

Nails have always been in demand. Some blacksmiths made only nails, and they were called "Nailers." Nails were so scarce (and expensive) in pre-1850 America that people would burn dilapidated buildings just to sift the ashes for nails.[1] They did so because pulling the nails would have damaged most of them. After the nails were recovered, a blacksmith could easily straighten any nails that had been bent during construction.

We still use the term "penny" when referring to a nail's size. It is believed that this term came into use in the early 1600s in England.[2] The English monetary unit was the Pound Sterling (£), which was divided into Shillings and Pence. The cost of 100 nails in Pence in the 1600 s is how we refer to nail sizes to this day. For example, 100 small nails that sold for 4 pence were called 4 d nails ( 4 d is the abbreviation of 4 pence). 100 larger nails that sold for 16 pence are 16 d nails. And so on.

Source: Appalachian Blacksmiths Association.
Setting the price of nails did not standardize their size. But it is apparent that the price of nails was constant, or near constant, for a long period of time, and thus, led to standard sizes as a result. For quite some time, nails have been sold by the pound-usually 1 lb . and 5 lb . boxes for small finishing and specialty nails and 50 lb . cartons for framing nails such as 8 d and 16 d . Nails are also sold by keg weight.

The cut nail made its appearance in the mid-1700s. For example, Thomas Jefferson established a nail factory at his Monticello plantation as a way to increase his farm income. His nail factory made both hand-forged and cut nails. It would not be until the middle-1800s that cut nails began dominating the marketplace. Cut nails are not actually "cut"-they are sheared from steel plate that is the thickness of the nail shank. Although routinely referred to as "square nails," the cutting machine tapers the nail shank as it is sheared from the steel plate. A second machine forms the head of a cut nail. The square nails in the above photograph are made in this manner. With the
hand-forged nail, all four sides are tapered. With the cut nail, two sides are parallel because they represent the thickness of the plate they were sheared from.

Cut nails could be manufactured much faster than hand-forged nails. As the process was mechanized, the cost per nail was less. However, cut nail factories employed operators and attendants for each machine so the process was still labor-intensive. The noise in those mills was deafening as well. Cut nails had their heyday from about 1820 (development of the Type B nail) to 1910, the advent of the wire nail.

Wire nails are round. Steel wire is fed into a machine that grips the wire, cuts it, makes the head, and chisels the point, all in one operation. This process is totally mechanized, requiring only someone to turn the machine on and off. Wire nail machines can make thousands of nails per minute.

Wire nails have all but replaced the cut nail. Cut nails are still used but mainly for restoration and masonry work. Though wire nails are cheaper to produce, the cut nail has a holding power of approximately four times to its modern, round cousin. Compared on that basis, cut nails win the day easily.

In modern construction, more and more nail-driving is being done with air-operated nail guns. Nails of nearly all sizes are available. However, since the air-nailing gun is large and cumbersome, it is most often used to fasten sheathing, such as plywood, to the framing. The nails are prepared to fit in the air gun's clip or nail sleeve (much like a stapler and the way staples are loaded) and are driven one-at-a-time. The air gun nail resembles the cut nail of old with the exception that the head is " T "-shaped rather than battened on all four sides.

### 8.4.1 Cut Floor Brads



|  | Dimensions and Tolerances (mm) |  |  |  |  |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :---: |
| $\mathbf{A}$ | $+/-$ | $\mathbf{B}$ | $+/-$ | $\mathbf{D}$ | $+/-$ |  |
| 40 | 1.0 | 2.5 | .18 | 7.5 | 1.0 |  |
| 45 | 1.0 | 2.5 | .18 | 7.5 | 1.0 |  |
| 50 | 1.0 | 2.5 | .18 | 7.5 | 1.0 |  |
| 60 | 1.5 | 3.0 | .22 | 9.0 | 1.0 |  |
| 65 | 1.5 | 3.2 | .22 | 9.6 | 1.0 |  |
| 75 | 1.5 | 3.2 | .22 | 9.6 | 1.0 |  |

Material Specification: Mild Steel EN 10111:1998:DD11, DD13
Galvanizing Specification: BS EN ISO 1461:1999
The above meets the requirements for Cut Floor Brads to BS1202:Part 1:1974 as amended

The tolerance on the " B " dimension refers to the thickness of the steel prior to manufacture. The process of manufacture may cause these tolerances to vary.

By permission: Glasgow Steel Nail, Glasgow, Scotland


### 8.4.2 Palm Holdfast



### 8.4.3 Moulder Brad



| Dimensions and Tolerances (mm) |  |  |  |  |  |
| :--- | :--- | :--- | :--- | :--- | :--- |
| $\mathbf{A}$ | $+/-$ | B | $+/-$ | $\mathbf{D}$ | $+/-$ |
| 50 | 1.0 | 2.0 | .18 | 4.5 | 1.0 |
| 60 | 1.0 | 2.0 | .18 | 6.5 | 1.0 |

Material Specification: Mild Steel EN 10111:1998 :DD11, DD13
Galvanizing Specification: BS EN ISO 1461:1999
The tolerance on the " $B$ " dimension refers to the thickness of the steel prior to manufacture. The process of manufacture may cause these tolerances to vary.

By permission: Glasgow Steel Nail, Glasgow, Scotland


### 8.4.4 Flat Countersunk Head Spike



* APPROX. ONLY


8.4.5 Rosehead Fine Shank


| Dimensions and Tolerances (mm) |  |  |  |  |  |  |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| $\mathbf{A}$ | $+/-$ | B | $+/-$ | C | $+/-$ | D | $+/-$ |
| 50 | 1.5 | 3.0 | .22 | 4.0 | 0.5 | 7.5 | 1.0 |
| 65 | 1.5 | 3.0 | .22 | 4.0 | 0.5 | 7.5 | 1.0 |
| 75 | 1.5 | 3.2 | .22 | 4.2 | 0.5 | 7.5 | 1.0 |
| 100 | 1.5 | 4.0 | .22 | 4.7 | 0.5 | 8.5 | 1.0 |

Material Specification: Mild Steel EN 10111:1998:DD11, DD13
Galvanizing Specification: BS EN ISO 1461:1999
The tolerance on the " $B$ " dimension refers to the thickness of the steel prior to manufacture. The process of manufacture may cause these tolerances to vary.

By permission: Glasgow Steel Nail, Glasgow, Scotland


### 8.4.6 Rosehead Square Shank Spike



| Dimensions and Tolerances (mm) |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | +/- | B | +/- | C | +/- | D | +/- |
| 40 | 1.0 | 3.2 | . 18 | 3.2 | 0.5 | 6.4 | 0.6 |
| 50 | 1.0 | 4.0 | . 22 | 4.0 | 0.5 | 8.0 | 0.8 |
| 65 | 1.5 | 4.0 | . 22 | 4.0 | 0.5 | 8.0 | 0.8 |
| 65 | 1.5 | 5.0 | . 24 | 5.0 | 0.5 | 10.0 | 1.0 |
| 75 | 1.5 | 5.0 | . 24 | 5.0 | 0.5 | 10.0 | 1.0 |
| 75 | 1.5 | 6.0 | . 27 | 6.0 | 0.5 | 12.0 | 1.5 |
| 90 | 1.5 | 5.0 | . 24 | 5.0 | 1.0 | 10.0 | 1.0 |
| 90 | 1.5 | 6.0 | . 27 | 6.0 | 1.0 | 12.0 | 1.5 |
| 100 | 1.5 | 6.0 | . 27 | 6.0 | 1.0 | 12.0 | 1.5 |
| 100 | 1.5 | 8.0 | . 30 | 8.0 | 1.0 | 16.0 | 2.0 |
| 115 | 2.0 | 8.0 | . 30 | 8.0 | 1.0 | 16.0 | 2.0 |
| 125 | 2.0 | 6.0 | . 27 | 6.0 | 1.0 | 12.0 | 1.5 |
| 125 | 2.0 | 8.0 | . 30 | 8.0 | 1.0 | 16.0 | 2.0 |
| 150 | 2.0 | 8.0 | . 30 | 8.0 | 1.0 | 16.0 | 2.0 |
| 150 | 2.0 | 10.0 | . 33 | 10.0 | 1.5 | 20.0 | 2.0 |
| 175 | 3.0 | 8.0 | . 30 | 8.0 | 1.0 | 16.0 | 2.5 |
| 175 | 3.0 | 10.0 | . 33 | 10.0 | 1.5 | 20.0 | 2.5 |
| 200 | 3.0 | 8.0 | . 30 | 8.0 | 1.0 | 16.0 | 2.5 |
| 200 | 3.0 | 10.0 | . 33 | 10.0 | 1.5 | 20.0 | 3.0 |
| 225 | 3.0 | 12.0 | . 36 | 12.0 | 1.5 | 24.0 | 3.5 |
| 243 | 3.0 | 12.0 | . 36 | 12.0 | 1.5 | 24.0 | 3.5 |

Material Specification: Mild Steel Up to 6 mm EN10111:1998:DD11, DD13 Over 6mm EN 10025: 2004: S275

Galvanizing Specification: BS EN ISO 1461:1999
The tolerance on the " B " dimension refers to the thickness of the steel prior to manufacture. The process of manufacture may cause these tolerances to vary.

[^22]

### 8.4.7 Décor Nail

$\xrightarrow{\text { B }}$

Material Specification : Mild Steel Up to 6 mm EN 10111:1998 DD11, DD13 Over 6mm EN 10025:2004:S275
Galvanizing Specification: BS EN ISO 1461:1999
The tolerance on the " B " dimension refers to the thickness of the steel prior to manufacture. The process of manufacture may cause these tolerances to vary.

By permission: Glasgow Steel Nail, Glasgow, Scotland


### 8.4.8 Boat Nail



Dimensions and Tolerances (mm)

| A | $+/-$ | B | $+/-$ | C | $+/-$ | D | $+/-$ |
| ---: | :--- | ---: | :--- | :--- | :--- | :--- | :--- |
| 50 | 1.0 | 5.0 | .24 | 4.0 | 1.0 | 10.0 | 1.0 |
| 65 | 1.5 | 5.0 | .24 | 4.0 | 1.0 | 10.0 | 1.0 |
| 75 | 1.5 | 6.0 | .27 | 4.0 | 1.0 | 12.0 | 1.5 |
| 100 | 1.5 | 8.0 | .27 | 6.0 | 1.0 | 15.0 | 1.5 |
| 125 | 2.0 | 10.0 | .30 | 8.0 | 1.5 | 20.0 | 2.0 |
| 150 | 2.0 | 10.0 | .33 | 8.0 | 1.5 | 20.0 | 2.5 |
| 175 | 3.0 | 10.0 | .33 | 8.0 | 1.5 | 20.0 | 2.5 |
| 200 | 3.0 | 10.0 | .33 | 8.0 | 1.5 | 20.0 | 2.5 |

Material Specification: Mild Steel Up to 6 mm EN 10111:1998:DD11, DD13 Over 6mm EN 10025 : 2004 : S275

Galvanizing Specification: BS EN ISO 1461:1999
The tolerance on the " B " dimension refers to the thickness of the steel prior to manufacture. The process of manufacture may cause these tolerances to vary.

By permission: Glasgow Steel Nail, Glasgow, Scotland


### 8.4.9 CLYDE Rail Spike



| Dimensions and Tolerances (mm) |  |  |  |  |  |  |  |
| :--- | :--- | ---: | :--- | ---: | :--- | :--- | :--- |
| A | $+/-$ | B | $+/-$ | C | $+/-$ | D | $+/-$ |
| 50 | 1.0 | 8.0 | .30 | 8.0 | 1.0 | 27.0 | 3.0 |
| 65 | 1.5 | 8.0 | .30 | 8.0 | 1.0 | 27.0 | 3.0 |
| 65 | 1.5 | 10.0 | .33 | 10.0 | 1.5 | 30.0 | 3.0 |
| 75 | 1.5 | 8.0 | .30 | 8.0 | 1.0 | 27.0 | 3.0 |
| 75 | 1.5 | 10.0 | .33 | 10.0 | 1.5 | 30.0 | 3.0 |
| 75 | 1.5 | 12.0 | .36 | 12.0 | 1.5 | 35.0 | 3.5 |
| 90 | 1.5 | 10.0 | .33 | 10.0 | 1.5 | 30.0 | 3.0 |
| 90 | 1.5 | 12.0 | .36 | 12.0 | 1.5 | 35.0 | 3.5 |
| 100 | 1.5 | 10.0 | .33 | 10.0 | 1.5 | 30.0 | 3.0 |
| 100 | 2.0 | 12.0 | .36 | 12.0 | 1.5 | 35.0 | 3.5 |
| 100 | 2.0 | 15.0 | .36 | 15.0 | 1.5 | 45.0 | 4.5 |
| 115 | 2.0 | 12.0 | .36 | 12.0 | 1.5 | 35.0 | 3.5 |
| 115 | 2.0 | 15.0 | .36 | 15.0 | 1.5 | 45.0 | 4.5 |
| 125 | 2.0 | 12.0 | .36 | 12.0 | 1.5 | 35.0 | 3.5 |
| 125 | 2.0 | 15.0 | .36 | 15.0 | 1.5 | 45.0 | 4.5 |
| 150 | 2.0 | 12.0 | .36 | 12.0 | 1.5 | 35.0 | 3.5 |
| 150 | 2.0 | 15.0 | .36 | 15.0 | 1.5 | 45.0 | 4.5 |

Material Specification: Mild Steel To EN 10025 : 2004 : S275
Galvanizing Specification: BS EN ISO 1461:1999
The tolerance on the " $B$ " dimension refers to the thickness of the steel prior to manufacture. The process of manufacture may cause these tolerances to vary.

## By permission: Glasgow Steel Nail, Glasgow, Scotland



### 8.5.0 Withdrawal Resistance of Nails

The general equation indicates that the dense, heavy woods offer greater nail-withdrawal resistance than the ones of lighter weight. This does not mean that the lighter species are not qualified for uses requiring highwithdrawal resistance. As a rule, the lighter species do not split as readily as the dense ones; thus lighter woods offer an opportunity for increasing the diameter, length, and number of the nails to compensate for the wood's lower nail-holding properties.

In practically all species, nails driven into green wood and pulled before any seasoning takes place will offer about the same withdrawal resistance as nails driven into seasoned wood and pulled soon after driving. However, if common smooth-shank nails are driven into green wood that is allowed to season or into seasoned wood that is subjected to cycles of wetting and drying before the nails are pulled, they lose a major part of their withdrawal resistance. In seasoned wood that is subjected only to moisture changes from normal atmospheric variations, the withdrawal resistance of smooth-shank nails also diminishes in time. On the other hand, tests indicate that, when moisture conditions cause nails to rust, withdrawal resistance is very erratic; it may be regained or even
increased over the immediate withdrawal resistance, Under all conditions of use, the withdrawal resistance of nails varies so widely that it is difficult to evaluate their behavior. The withdrawal loads for plain nails driven into wood that is subjected to wide alternating changes in moisture content may be as much as $75 \%$ below the values given by the general formula.

The specific gravity of various species of wood and their relative resistance to the withdrawal of smoothshank nails are given in Table 1. The numerical value of $6900 \mathrm{G}^{5 / 2}$ has been calculated for each species. The load per inch of penetration immediately after driving may be obtained by multiplying this nail-withdrawal factor by the diameter, $\underline{\mathrm{D}}$. For example, Table 1 shows a value of 790 D for an eightpenny common nail ( 0.131 -inch diameter) in ponderosa pine. Multiplying 790 times 0.131 gives a value of 103 pounds per inch of penetration.

Source: U.S. Department of Agriculture-Forest Service.

TABLE 1 Nail-Withdrawal Resistance

|  | Hardwoods |  |
| :--- | :---: | :---: |
|  | Specific Gravity ${ }^{1}$ | Relative Nail Load |

TABLE 1 Nail-Withdrawal Resistance-Cont'd Hardwoods

|  | Specific Gravity | Relative Nail Load |
| :--- | :---: | :---: |
| Sycamore, American | .54 | 1450 D |
| Tupelo, black | .55 | 1520 D |
| Tupelo, water | .52 | 1310 D |
| Yellow-poplar | .43 | 830 D |


| Softwoods |  |
| :--- | :---: | :---: |
| Specific Gravity ${ }^{1}$ | Relative Nail Load $^{2}$ |


| Alaska-cedar | 0.46 | 970 D |
| :---: | :---: | :---: |
| Baldcypress | . 48 | 1100 D |
| Douglas-fir, Coast-type | . 51 | 1310 D |
| Douglas-fir, Rocky Mountain-type | . 45 | 970 D |
| Fir, balsam | . 41 | 740 D |
| Fir, commercial white | .41 | 740 D |
| Hemlock, eastern | . 43 | 830 D |
| Hemlock, western | . 44 | 900 D |
| Larch, western | . 59 | 1860 D |
| Pine, eastern white | . 37 | 550 D |
| Pine, lodgepole | . 43 | 830 D |
| Pine, Ponderosa | . 42 | 790 D |
| Pine, red | . 51 | 1310 D |
| Pine, southern yellow | . 59 | 1860 D |
| Pine, sugar | . 38 | 620 D |
| Pine, western white | . 42 | 790 D |
| Port-Orford-cedar | . 44 | 900 D |
| Redcedar, western | . 34 | 470 D |
| Redwood (old-growth) | . 42 | 790 D |
| Spruce, Engelmann | . 35 | 500 D |
| Spruce, red | . 41 | 740 D |
| Spruce, Sitka | . 42 | 790 D |
| Spruce, white | . 45 | 970 D |
| White-cedar, Atlantic | . 35 | 500 D |
| White-cedar, northern | . 32 | 410 D |

${ }^{1}$ Based on weight and volume when ovendry.
${ }^{2}$ Load in pounds. $D=$ nail diameter in inches.

Nail diameter varies for different types of nails. Here are diameters of bright common wire nails:

| Penny | Gage | Diameter | Penny | Gage | Diameter |
| :---: | :---: | :---: | :---: | :---: | ---: |
| 4 | $12-1 / 2$ | 0.098 | 12 | 9 | 0.148 |
| 6 | $11-1 / 2$ | .113 | 16 | 8 | .162 |
| 8 | $10-1 / 4$ | .131 | 20 | 6 | .192 |
| 10 | 9 | .148 |  |  |  |



FIGURE 1 Load required to withdraw common nails from wood of different specific gravities immediately after nails were driven. Specific gravity is based on weight and volume of ovendry wood.

### 8.6.0 Wood Screws-Common Types and Withdrawal Resistance

## Wood Screws

The common types of wood screws have flat, oval, or round heads. The flathead screw is most commonly used if a flush surface is desired. Ovalhead and roundhead screws are used for appearance, and roundhead screws are used when countersinking is objectionable. The principal parts of a screw are the head, shank, thread, and core (Fig. 8-5). The root diameter for most sizes of screws averages about two-thirds the shank diameter. Wood screws are usually made of steel, brass, other metals, or alloys, and may have specific finishes such as nickel, blued, chromium, or cadmium. They are classified according to material, type, finish, shape of head, and diameter or gauge of the shank.

Current trends in fastenings for wood also include tapping screws. Tapping screws have threads the full length of the shank and may have some advantage for certain specific uses.

## Withdrawal Resistance

## Experimental Loads

The resistance of wood screw shanks to withdrawal from the side grain of seasoned wood varies directly with the square of the specific gravity of the wood. Within limits, the withdrawal load varies directly with the depth of penetration of the threaded portion and the diameter of the screw, provided the screw does not fail in tension. The screw will fail in tension when its strength is exceeded by the withdrawal strength from the wood. The limiting length to cause a tension failure decreases as the density of the wood increases since the withdrawal strength of the wood increases with density. The longer lengths of standard screws are therefore superfluous in dense hardwoods.

Source: U.S. Department of Agriculture.

### 8.6.1 Wood Screw Sizing

## Screw Sizing

The general size of a screw is given a number. As the number increases, so does the size of the entire screw, both head size and shaft size-but not length. Therefore, a \#8 screw is about twice the size of a \#4 screw, but may be the same length. Wake Up!-This is important.

Most wood screws have a common "pitch" to the threads, but some have a thread with a steeper incline. We simply call this a "fast" thread, but they are technically Type A screws. Think of this as a road going up a mountain; the steeper the road, the sooner you get to the top. Most of the screws for mounting hinges are self-tapping


FIGURE 8-5 Common types of wood screws: A, flathead; B, roundhead; and C, ovalhead.
(they tap their own mating threads in wood) type AB (they have more threads per inch and are more effective in brittle materials like wood than Type A). For more information on this, see screw types section.

Also in reference to threads you will see " $8-32$." This is the common knob and pull screw thread. The " 8 " refers to the size (diameter) of the screw, and the " 32 " means it has 32 threads to the inch. The diameter is measured at the shank of the screw.

Screws are sized by gauge number and length. For example, an 8 -gauge screw with 32 threads per inch and $1^{\prime \prime}$ in length would be written as: $8-32 \times 1^{\prime \prime}$. However, most wood screws do not include the threads per inch measurement and would just be listed as $8 \times 1^{\prime \prime}$. If the gauge number is not known, simply measure the diameter of the shank in inches and round to nearest listed number on chart below for screw number identification.

Source: D.Lawless Hardware- hingeddummy.info.

### 8.6.2 Dimensions of Wood Screws Chart

## Dimensions of Wood Screws Chart

| ---Shank* |  | Diameter ${ }^{\dagger}$ - |  |  | Root Diameter |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Gauge <br> Number | Max. Head Diameter | Basic <br> Decimal Size | Nearest Fractional Equivalent | Average <br> Decimal Size | Nearest Fractional Equivalent | Threads per Inch |
| 0 | . 119 | . 060 | 1/16 | . 040 | 3/64 | 32.00 |
| 1 | . 146 | . 073 | 5/64 | . 046 | 3/64 | 28.00 |
| 2 | . 172 | . 086 | 3/32 | . 054 | 1/16 | 26.00 |
| 3 | . 199 | . 099 | 7/64 | . 065 | 1/16 | 24.00 |
| 4 | . 225 | . 112 | 7/64 | . 075 | 5/64 | 22.00 |
| 5 | . 252 | . 125 | 1/8 | . 085 | 5/64 | 20.00 |
| 6 | . 279 | . 138 | 9/64 | . 094 | 3/32 | 18.00 |
| 7 | . 305 | . 151 | 5/32 | . 102 | 7/64 | 16.00 |
| 8 | . 332 | . 164 | 5/32 | . 112 | 7/64 | 15.00 |
| 9 | . 358 | . 177 | 11/64 | . 122 | 1/8 | 14.00 |
| 10 | . 385 | . 190 | 3/16 | . 130 | 1/8 | 13.00 |
| 11 | . 411 | . 203 | 13/64 | . 139 | 9/64 | 12.00 |
| 12 | . 438 | . 216 | 7/32 | . 148 | 9/64 | 11.00 |
| 14 | . 491 | . 242 | 1/4 | . 165 | 5/32 | 10.00 |
| 16 | . 544 | . 268 | 17/64 | . 184 | 3/16 | 9.00 |
| 18 | . 597 | . 294 | 19/64 | . 204 | 13/64 | 8.00 |
| 20 | . 650 | . 320 | 5/16 | . 233 | 7/32 | 8.00 |
| 24 | . 756 | . 372 | 3/8 | . 260 | 1/4 | 7.00 |

[^23]- The length of screw is taken from the surface of the material to the point of the screw. See illustration below.

- Thus, a 6-gauge screw with 15 threads per inch and $11 / 4^{\prime \prime}$ in length would be written as: 6-15 $\times 11 / 4^{\prime \prime}$

And here is an extra tid bit: this is a formula for obtaining the diameter when you only have the screw number.

Multiply the screw Number by 13 and add ". 060 .
Examples -
(No.) $8 \times 13=.104+$ ". $060=$ ". 164
(No.) $2 \times 13=.026+" .060=$ ". 086 .


### 8.6.3 Basic Types of Wood Screw Drives

## Basic Types of Wood Screws

Wood screws are classified by the type of drive, the shape of head, its length, and whether it is designed for wood or metal; this page refers to wood screws only.

## Types of Drives

Driver refers to the indented shape on the screw head used to turn the screw. There are many different types of drives. Here we are only covering the four most used drives: slotted (flathead), Phillips (crosshead), square, and pozidriv. A brief description of each drive is presented below with a picture at the bottom to illustrate each one's unique design.

SLOTTED/FLATHEAD: This is the original screw drive. You find these everywhere, though the practice of using screws with slotted drives is on the decline because the screwdriver slips out of the slot, particularly when you are applying heavy torque to really tighten down (or loosen, for that matter) these types of screws.

PHILLIPS/CROSSHEAD: This screw drive type is very popular-and again, you find them in a very wide range of applications. Common sizes are Phillips \#1, \#2, and \#3. The most common Phillips size is \#2.

SQUARE: Square recess screw drives are being used more and more as they are very resistant to cam-out, which is a fancy way of saying the tip of the tool does not slip out and mar the screwhead very easily. It is commonly found in two sizes: Square \#2 and \#3.

POZIDRIV: This screwhead isn't seen very often in the United States, though it is very common in Europe. It looks a lot like a Phillips screwhead, but it includes four more contact points. Common sizes are Pozidriv \#1, \#2, and \#3.


SLOTTED/FLATHEAD PHILLIPS/CROSSHEAD SQUARE POZIDRIV

### 8.6.4 Screw Head Types and Shapes

Screw head types refer to the shape of the head at the top of the screw. There are many different head types as well. Again, we will adequately cover head types, but only the most generally used. The shape of the screw head can be described as flat (countersunk), oval, round, pan, truss, button (dome), and so on. Detailed descriptions for each of the ones listed are presented below. Looking at the drawings should make the designs of each selfexplanatory.

FLAT/COUNTERSUNK: Supplied to standard dimensions with an $80^{\prime}$ to $82^{\prime}$ angle to be used where finished surfaces require a flush fastening unit (concealed below woods surface). The countersunk portion offers good centering possibilities.


OVAL: Fully specified as "oval countersunk," this head is identical to the standard flat head, but possesses, in addition, a rounded, neat-appearing upper surface for attractiveness of design.


ROUND: Not recommended for new design (see pan head). The round head rests on the surface of wood. This was the most universally used in the past.


PAN: Recommended for new designs to replace round, truss, and binding heads. Provides a low large diameter head, but with characteristically high outer edge along the outer periphery of the head where driving action is most effective for high tightening torques. Slightly different head contour where supplied with recessed head.

## 2 2 2

TRUSS: Also known as oven head, stove head, and oval binding head. A low, neat-appearing, large-diameter head having excellent design qualities, and as illustrated can be used to cover larger diameter clearance holes in sheet metal when additional play in assembly tolerance is required. Suggest pan head as a substitute.


BUTTON/DOME: Cylindrical with a rounded top.


### 8.6.5 Screw Thread and Point Types

Most wood screws have a common "pitch" to the threads, but some have a thread with a steeper incline. We simply call this a "fast" thread, but they are technically Type A screws. Think of this as a road going up a mountain; the steeper the road, the sooner you get to the top.

Also in reference to threads you will see " $8-32$." This is the common knob and pull screw thread. The " 8 " refers to the size (diameter) of the screw, and the " 32 " means it has 32 threads to the inch. Most of the screws for mounting hinges are self-tapping (they tap their own mating threads in wood) type AB (they have more threads per inch and are more effective in brittle materials like wood than Type A)

## Self-Tapping Screws



> TYPE A POINT: A THREAD FORMING SCREW WITH SHARP POINT AND COARSE THREAD (FEWER THREADS PER INCH) FOR USE WITH LIGHT MATERIAL.


TYPE AB POINT: A THREAD FORMING SCREW WITH
SHARP POINT AND FINE THREAD (MORE THREADS
PER INCH) FOR USE ON LIGHT AND HEAVY MATERIAL.


## TYPE B POINT: LARGER ROOT DIAMETER WITH FINER THREAD PITCH FOR HEAVIER USE

Threading on the shank is designed specifically for wood; wood threads have a tapped screw whereas sheet metal screws have mainly a parallel thread. Wood-type screws are also normally used for securing into wall plugs. Screws for chipboard usually have two threads the full length of the shank.

## Miscellaneous

The most interesting screw here, if there is such a thing, is a variable-length break-off knob screw. If you do not know exactly what length screw will work, order these and break them off where you need them. A unique design allows this to be done without damaging the threads.

- And here is that famous break-off knob screw. Break-off screw $8-32 \times 13 / 4^{\prime \prime}$ zinc plated. Break-off points $1^{\prime \prime}, 11 / 4^{\prime \prime}$, and $11 / 2^{\prime \prime}$.

- Perfect solution when you're not quite sure which screw length you require.
- Simply grip the section below the break point you choose with a pair of pliers and break off.
- Do not grip threaded section you want to use with pliers as this would damage the threads.

Source: D. Lawless Hardware- hinged dummy.info.

### 8.6.6 Type 316 Stainless Steel Deck Screws

## Woodpeckers Flat Head Deck Screw with Nibs

- Revolutionary 4-corner thread with raised ridge point for fast penetration, reduced drive torque, and outstanding holding power
- Self-countersinking Flat Head with Nibs
- Star Drive 6 Lobe
- Type 316 Stainless Steel for the ultimate in corrosion resistance
- Approved for use in the new ACQ lumber
- Square recess driver bits
- Click here for Nail Gun Reference Chart
- $\quad$ Smart-Bit ${ }^{\text {TM }}$ pre-drill \& counters inking bit

| Type 316 Stainless Steel-SALT WATER SAFE |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Length | Screw Size | Star Drive | Box of | Weight | Code | Price | Qty |
| 1-5/8" | \#8 | T20 | 4000 | 28 lb | 158FXN86A | \$ |  |
|  |  |  | 350 | 2.5 lb | 158FXN86B | \$ |  |
| $2^{\prime \prime}$ | \#8 | T20 | 3000 | 25 lb | 200FXN86A | \$ |  |
|  |  |  | 350 | 3 lb | 200FXN86B | \$ |  |
| 2-1/2" | \#10 | T25 | 1750 | 23 lb | 212FXN106A | \$ |  |
|  |  |  | 350 | 6 lb | 212FXN106B | \$ |  |
| $3 \prime$ | \#10 | T25 | 1750 | 27 lb | 300FXN106A | \$ |  |
|  |  |  | 350 | 5.5 lb | 300FXN106B | \$ |  |
| $3-1 / 2^{\prime \prime}$ | \#10 | T25 | 1000 | 19 lb | 312FXN106A | \$ |  |
|  |  |  | 250 | 6.7 lb | 312FXN106B | \$ |  |
| $4 \prime$ | \#12 | T27 | 750 | 28 lb | 400FXN126A | \$ |  |
|  |  |  | 100 | 3.8 lb | 400FXN126B | \$ |  |

*AISI Grade 316 Stainless Steel has lower carbon content than 305 or 302HQ and contains molybdenum for superior corrosion resistance in salt water and other highly corrosive environments. It is the grade of stainless steel to be specified in any seaside application.

By permission: Manasquanfasteners.com

### 8.6.7 Type 302 Stainless Steel Bugle Head Screws with Square Drive

## Type 302HQ Stainless Steel Bugle Head Square Drive Wood Screws



- Type 17 notched point for fast penetration
- Self-countersinking bugle head
- Square drive recess reduces driver cam-out
- Threaded approximately $2 / 3$ of shank on most sizes
- Approved for the new pressure treated lumber
- Available in Type 302HQ and 316 stainless steel
- Deep coarse threads for better hold
- Approved for use in the new ACQ lumber
- Click here for Fastener Estimator
- Smart-Bit ${ }^{\mathrm{TM}}$ pre-drill and countersinking bit
- Square recess driver bits

| Type 302HQ Stainless Steel |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Length | Screw <br> Size | Square <br> Drive | Carton Count | Code | Price pr/100 | Price <br> pr/100 min <br> 1000 | Full <br> Carton | Enter Qty in 100s |
| 1-1/4 ${ }^{\prime \prime}$ | \#6 | \#1 | 5000 | 114B6Q | \$ | \$ | \$ |  |
| 1-5/8" | \#6 | \#1 | 4000 | 158B6Q | \$ | \$ | \$ |  |
| $2^{\prime \prime}$ | \#6 | \#1 | 3000 | 200B6Q | \$ | \$ | \$ |  |
| 1-5/8" | \#8 | \#2 | 4000 | 158B8Q | \$ | \$ | \$ |  |
| $2^{\prime \prime}$ | \#8 | \#2 | 3000 | 200B8Q | \$ | \$ | \$ |  |
| 2-1/4" | \#8 | \#2 | 3000 | 214B8Q | \$ | \$ | \$ |  |
| 2-1/2" | \#8 | \#2 | 2000 | 212B8Q | \$ | \$ | \$ |  |
| $3 \prime$ | \#8 | \#2 | 1500 | 300B8Q | \$ | \$ | \$ |  |


| Type 302HQ Stainless Steel-Cont'd |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Length | Screw <br> Size | Square <br> Drive | Carton Count | Code | Price pr/100 | Price <br> pr/100 min $1000$ | Full Carton | Enter Qty <br> in 100s |
| $2^{\prime \prime}$ | \#10 | \#2 | 2500 | 200B10Q | \$ | \$ | \$ |  |
| 2-1/2" | \#10 | \#2 | 2000 | 212B10Q | \$ | \$ | \$ |  |
| $3 \prime$ | \#10 | \#2 | 1500 | 300B10Q | \$ | \$ | \$ |  |
| $3-1 / 2^{\prime \prime}$ | \#10 | \#2 | 1000 | 312B10Q | \$ | \$ | \$ |  |
| $4 \prime$ | \#10 | \#2 | 1000 | 400B10Q | \$ | \$ | \$ |  |
| 2" | \#12 | \#3 | 2000 | 200B12Q | \$ | \$ | \$ |  |
| $2-1 / 2^{\prime \prime}$ | \#12 | \#3 | 1500 | 212B12Q | \$ | \$ | \$ |  |
| $3 \prime$ | \#12 | \#3 | 1500 | 300B12Q | \$ | \$ | \$ |  |
| $3-1 / 2^{\prime \prime}$ | \#12 | \#3 | 1000 | 312B12Q | \$ | \$ | \$ |  |
| $2^{\prime \prime}$ | \#14 | \#3 | 1500 | 200B14Q | \$ | \$ | \$ |  |
| $2-1 / 2^{\prime \prime}$ | \#14 | \#3 | 1000 | 212B14Q | \$ | \$ | \$ |  |
| $3 \prime$ | \#14 | \#3 | 1000 | 300B14Q | \$ | \$ | \$ |  |
| $3-1 / 2^{\prime \prime}$ | \#14 | \#3 | 500 | 312B14Q | \$ | \$ | \$ |  |
| $4 \prime$ | \#14 | \#3 | 500 | 400B14Q | \$ | \$ | \$ |  |

By permission: Manasquanfasteners.com

### 8.6.8 Lateral and Withdrawal Resistance of Lag Screws

## Lag Screws

Lag screws are commonly used because of their convenience, particularly where it would be difficult to fasten a bolt or where a nut on the surface would be objectionable. Commonly available lag screws range from about 5.1 to 25.4 mm ( 0.2 to 1 in .) in diameter and from 25.4 to 406 mm ( 1 to 16 in .) in length. The length of the threaded part varies with the length of the screw and ranges from $19.0 \mathrm{~mm}(3 / 4 \mathrm{in}$.) with the 25.4 - and $31.8-\mathrm{mm}$ (1- and $1-1 / 4-\mathrm{in}$.) screws to half the length for all lengths greater than 254 mm ( 10 in .). Lag screws have a hexagonal-shaped head and are tightened by a wrench (as opposed to wood screws, which have a slotted
head and are tightened by a screw driver). The following equations for withdrawal and lateral loads are based on lag screws having a base metal average tensile yield strength of about $310.3 \mathrm{MPa}\left(45,000 \mathrm{lb} / \mathrm{in}^{2}\right)$ and an average ultimate tensile strength of $530.9 \mathrm{MPa}\left(77,000 \mathrm{lb} / \mathrm{in}^{2}\right)$.

## Withdrawal Resistance

The results of withdrawal tests have shown that the maximum direct withdrawal load of lag screws from the side grain of seasoned wood may be computed as

$$
\begin{array}{ll}
p=125.4 G^{3 / 2} D^{3 / 4} L & \text { (metric) } \\
p=8,100 G^{3 / 2} D^{3 / 4} L & (\text { inch }- \text { pound }) \tag{8-14b}
\end{array}
$$

where $p$ is maximum withdrawal load ( $\mathrm{N}, \mathrm{lb}$ ), $D$ shank diameter ( mm , in.), $G$ specific gravity of the wood based on ovendry weight and volume at $12 \%$ moisture content, and $L$ length ( mm , in.) of penetration of the threaded part. (The NDS and LRFD use ovendry weight and volume as a basis.) Equation (8-14) was developed independently of Equation ( $8-10$ ) but gives approximately the same results.
Source: U.S. Department of Agriculture.
Lag screws, like wood screws, require prebored holes of the proper size (Fig. 8-6). The lead hole for the shank should be the same diameter as the shank. The diameter of the lead hole for the threaded part varies with the density of the wood: for low-density softwoods, such as the cedars and white pines, 40 to $70 \%$ of the shank diameter; for Douglas-fir and Southern Pine, $60 \%$ to $75 \%$; and for dense hardwoods, such as oaks, $65 \%$ to $85 \%$. The smaller percentage in each range applies to lag screws of the smaller diameters, and the larger percentage to lag screws of larger diameters. Soap or similar lubricants should be used on the screw to facilitate turning, and lead holes slightly larger than those recommended for maximum efficiency should be used with long screws.


FIGURE 8-6 A, Clean-cut, deep penetration of thread made by lag screw turned into a lead hole of proper size, and B, rough, shallow penetration of thread made by lag screw turned into oversized lead hole.

In determining the withdrawal resistance, the allowable tensile strength of the lag screw at the net (root) section should not be exceeded. Penetration of the threaded part to a distance about seven times the shank diameter in the denser species (specific gravity greater than 0.61 ) and 10 to 12 times the shank diameter in the less dense species (specific gravity less than 0.42 ) will develop approximately the ultimate tensile strength of the lag screw. Penetrations at intermediate densities may be found by straight-line interpolation.

The resistance to withdrawal of a lag screw from the end-grain surface of a piece of wood is about threefourths as great as its resistance to withdrawal from the side-grain surface of the same piece.

## Lateral Resistance

Pre-1991
The experimentally determined lateral loads for lag screws inserted in the side grain and loaded parallel to the grain of a piece of seasoned wood can be computed as

$$
\begin{equation*}
p=K D^{2} \tag{8-15}
\end{equation*}
$$

where $p$ is the proportional limit lateral load $(\mathrm{N}, \mathrm{lb})$ parallel to the grain, $K$ a coefficient depending on the species-specific gravity, and $D$ shank diameter of the lag screw ( mm , in.). Values of $K$ for a number of specific gravity ranges can be found in Table 8-4. These coefficients are based on average results for several ranges of specific gravity for hardwoods and softwoods. The loads given by this equation apply when the thickness of the side member is 3.5 times the shank diameter of the lag screw, and the depth of penetration in the main member is 7 times the diameter in the harder woods and 11 times the diameter in the softer woods. For other thicknesses, the computed loads should be multiplied by the factors listed in Table 8-10.
The thickness of a solid wood side member should be about one-half the depth of penetration in the main member.

When the lag screw is inserted in the side grain of wood and the load is applied perpendicular to the grain, the load given by the lateral resistance equation should be multiplied by the factors listed in Table 8-11.

| TABLE 8-10 Multiplication Factors for Loads Computed <br> from Equation (7-15) |  |
| :--- | :---: |
| Ratio of Thickness of Side Member | Factor |
| to Shank Diameter of Lag Screw |  |
| 2 | 0.62 |
| 2.5 | 0.77 |
| 3 | 0.93 |
| 3.5 | 1.00 |
| 4 | 1.07 |
| 4.5 | 1.13 |
| 5 | 1.18 |
| 5.5 | 1.21 |
| 6 | 1.22 |
| 6.5 | 1.22 |


| TABLE 8-11 Multiplication Factors for Loads Applied <br> Perpendicular to Grain Computed from Equation (7-15) <br> with Lag Screw in Side grain of Wood |  |
| :--- | :---: |
| Shank diameter of Lag Screw (mm [in.] ) | Factor |
| $4.8(3 / 16)$ | 1.00 |
| $6.4(1 / 4)$ | 0.97 |
| $7.9(5 / 16)$ | 0.85 |
| $9.5(3 / 8)$ | 0.76 |
| $11.1(7 / 16)$ | 0.70 |
| $12.7(1 / 2)$ | 0.65 |
| $15.9(5 / 8)$ | 0.60 |
| $19.0(3 / 4)$ | 0.55 |
| $22.2(7 / 8)$ | 0.52 |
| $25.4(1)$ | 0.50 |

### 8.7.0 Bolts in Wood

## Bolts

## Bearing Stress of Wood under Bolts

The bearing stress under a bolt is computed by dividing the load on a bolt by the product $L D$, where $L$ is the length of a bolt in the main member and $D$ is the bolt diameter. Basic parallel-to-grain and perpendicular-tograin bearing stresses have been obtained from tests of three-member wood joints where each side member is half the thickness of the main member. The side members were loaded parallel to grain for both parallel- and perpendicular-to-grain tests. Prior to 1991, bearing stress was based on test results at the proportional limit. Since 1991, bearing stress has been based on test results at a yield limit state, which is defined as the $5 \%$ diameter offset on the load-deformation curve.

The bearing stress at proportional limit load is largest when the bolt does not bend, that is, for joints with small L/D values. The curves of Figures $8-8$ and $8-9$ show the reduction in proportional limit bolt-bearing stress as $L / D$ increases. The bearing stress at maximum load does not decrease as $L / D$ increases, but remains fairly constant, which means that the ratio of maximum load to proportional limit load increases as $L / D$ increases.


FIGURE 8-8 Variation in bolt-bearing stress at the proportional limit parallel to grain with $L / D$ ratio. Curve A, relation obtained from experimental evaluation; curve B , modified relation used for establishing design loads.


FIGURE 8-9 Variation in bolt-bearing stress at the proportional limit perpendicular to grain with $L / D$ ratio. Relations obtained from experimental evaluation for materials with average compression perpendicular stress of $7,860 \mathrm{kPa}\left(1,140 \mathrm{lb} / \mathrm{in}^{2}\right)$ (curve A-1) and $3,930 \mathrm{kPa}$ ( $570 \mathrm{lb} / \mathrm{in}^{2}$ ) (curve A-2). Curves B-1 and B-2, modified relations used for establishing design loads.

To maintain a fairly constant ratio between maximum load and design load for bolts, the relations between bearing stress and $L / D$ ratio have been adjusted as indicated in Figures 8-8 and 8-9.

The proportional limit bolt-bearing stress parallel to grain for small $L / D$ ratios is approximately $50 \%$ of the small clear crushing strength for softwoods and approximately $60 \%$ for hardwoods. For bearing stress perpendicular to the grain, the ratio between bearing stress at proportional limit load and the small clear proportional limit stress in compression perpendicular to grain depends on bolt diameter (Fig. 8-10) for small $L / D$ ratios.

Species compressive strength also affects the $L / D$ ratio relationship, as indicated in Figure 8-9. Relatively higher bolt proportional-limit stress perpendicular to grain is obtained with wood low in strength (proportional limit stress of $3,930 \mathrm{kPa}\left(570 \mathrm{lb} / \mathrm{in}^{2}\right.$ ) than with material of high strength (proportional limit stress of $7,860 \mathrm{kPa}$ $\left[1,140 \mathrm{lb} / \mathrm{in}^{2}\right]$ ). This effect also occurs for bolt-bearing stress parallel to grain, but not to the same extent as for perpendicular-to-grain loading.


FIGURE 8-10 Bearing stress perpendicular to the grain as affected by bolt diameter.

## Steel Side Plates

When steel side plates are used, the bolt-bearing stress parallel to grain at joint proportional limit is approximately $25 \%$ greater than that for wood side plates. The joint deformation at proportional limit is much smaller with steel side plates. If loads at equivalent joint deformation are compared, the load for joints with steel side plates is approximately $75 \%$ greater than that for wood side plates. Pre-1991 design criteria included increases in connection strength with steel side plates; post-1991 design criteria include steel side plate behavior in the yield model equations.

For perpendicular-to-grain loading, the same loads are obtained for wood and steel side plates.

## Bolt Quality

Both the properties of the wood and the quality of the bolt are factors in determining the strength of a bolted joint. The percentages given in Figures 8-8 and 8-9 for calculating bearing stress apply to steel machine bolts with a yield stress of $310 \mathrm{MPa}\left(45,000 \mathrm{lb} / \mathrm{in}^{2}\right)$. Figure $8-11$ indicates the increase in bearing stress parallel to grain for bolts with a yield stress of $862 \mathrm{MPa}\left(125,00 \mathrm{lb} / \mathrm{in}^{2}\right)$.

## Effect of Member Thickness

The proportional limit load is affected by the ratio of the side member thickness to the main member thickness (Fig. 8-12).

Pre-1991 design values for bolts are based on joints with the side member half the thickness of the main member. The usual practice in design of bolted joints is to take no increase in design load when the side members are greater than half the thickness of the main member. When the side members are less than half the thickness of the main member, a design load for a main member that is twice the thickness of the side member is used. Post1991 design values include member thickness directly in the yield model equations.

## Two-Member, Multiple-Member Joints

In pre-1991 design, the proportional limit load was taken as half the load for a three-member joint, with a main member the same thickness as the thinnest member for two-member joints.


FIGURE 8-11 Variation in the proportional limit bolt-bearing stress parallel to grain with $L / D$ ratio. Curve A, bolts with yield stress of $861.84 \mathrm{MPa}\left(125,000 \mathrm{lb} / \mathrm{in}^{2}\right)$; curve B, bolts with yield stress of $310.26 \mathrm{MPa}\left(45,000 \mathrm{lb} / \mathrm{in}^{2}\right)$.


FIGURE 8-12 Proportional limit load related to side member thickness for three-member joints. Center member thickness was 50.8 mm ( 2 in .).

For four or more members in a joint, the proportional limit load was taken as the sum of the loads for the individual shear planes by treating each shear plane as an equivalent two-member joint.

### 8.8.0 Wood Adhesives Characterized as to Expected Performance

TABLE 8-2 Wood Adhesives Categorized According to Their Expected Structural Performance at Varying Levels of Environmental Exposure ${ }^{\mathrm{a}, \mathrm{b}}$

| Structural Integrity | Service Environment | Adhesive Type |
| :--- | :--- | :--- |
| Structural | Fully exterior (withstands long-term water | Phenol-formaldehyde |
|  |  | Resorcinol-formaldehyde |
|  |  | Phenol-resorcinol-formaldehyde |
|  |  | Emulsion polymer/isocyanate |
|  | Limited exterior (withstands short-term | Melamine-formaldehyde |
|  | water soaking) | Melamine-urea-formaldehyde |
|  | Isocyanate |  |
|  | Interior (withstands short-term high humidity) | Epoxy |
|  | Urea-formaldehyde |  |
| Semistructural | Casein |  |
|  | Limited exterior | Cross-linked polyvinyl acetate |
|  |  | Polyurethane |
|  | Polyvinyl acetate |  |
|  | Interior | Animal |
|  |  | Soybean |
|  | Elastomeric construction |  |
|  | Elastomeric contact |  |
|  | Hot-melt |  |
|  |  | Starch |

[^24]
### 8.8.1 Categories of Selected Wood Species According to Ease of Bonding

| U.S. Hardwoods | U.S. Softwoods |  | Imported Woods |
| :---: | :---: | :---: | :---: |
| Bond easily ${ }^{\text {a }}$ |  |  |  |
| Alder | Fir | Balsa | Hura |
| Aspen | White | Cativo | Purpleheart |
| Basswood | Grand | Courbaril | Roble |
| Cottonwood | Noble | Determa ${ }^{\text {b }}$ |  |
| Chestnut, American | Pacific |  |  |
| Magnolia | Pine |  |  |
| Willow, black | Eastern white |  |  |
|  | Western white |  |  |
|  | Redcedar, western |  |  |
|  | Redwood |  |  |
|  | Spruce, Sitka |  |  |
| Bond well ${ }^{\text {c }}$ |  |  |  |
| Butternut | Douglas-fir | Afromosia | Meranti (lauan) |
| Elm | Larch, western ${ }^{\text {d }}$ | Andiroba | Light red |
| American | Pine | Angelique | White |
| Rock | Sugar | Avodire | Yellow |
| Hackberry | Ponderosa | Banak | Obeche |
| Maple, soft | Redcedar, eastern | Iroko | Okoume |
| Sweetgum |  | Jarrah | Opepe |
| Sycamore |  | Limba | Peroba rosa |
| Tupelo |  | Mahogany | Sapele |
| Walnut, black |  | African | Spanish-cedar |
| Yellow-poplar |  | American | Sucupira |
|  |  |  | Wallaba |
| Bond satisfactorily ${ }^{\text {e }}$ |  |  |  |
| Ash, white | Yellow-cedar | Angelin | Meranti (lauan), dark red |
| Beech, American | Port-Orford-cedar | Azobe | Pau marfim |
| Birch | Pines, southern | Benge | Parana-pine |
| Sweet |  | Bubinga | Pine |
| Yellow |  | Karri | Caribbean |
| Cherry Hickory |  |  | Radiata |
|  |  |  | Ramin |
| Pecan |  |  |  |
| True |  |  |  |
| Madrone |  |  |  |
| Maple, hard |  |  |  |
| Oak |  |  |  |
| Red ${ }^{\text {b }}$ |  |  |  |
| White ${ }^{\text {b }}$ |  |  |  |
| Bond with difficulty ${ }^{\text {f }}$ |  |  |  |
| Osage-orange |  | Balata | Keruing |
| Persimmon |  | Balau | Lapacho |
|  |  | Greenheart | Lignumvitae |
|  |  | Kaneelhart | Rosewood |
|  |  | Kapur | Teak |
| ${ }^{\text {a }}$ Bond very easily with adhesives of a wide range of properties and under a wide range of bonding conditions. <br> ${ }^{b}$ Difficult to bond with phenol-formaldehyde adhesive. |  |  |  |
| ${ }^{\text {c }}$ Bond well with a fairly wide range of adhesives under a moderately wide range of bonding conditions. <br> ${ }^{d}$ Wood from butt logs with high extractive content is difficult to bond. |  |  |  |
|  |  |  |  |
| ${ }^{e}$ Bond satisfactorily with good-quality adhesives under well-controlled bonding conditions. |  |  |  |
| ${ }^{\text {f }}$ Satisfactory results require careful selection of adhesives and very close control of bonding conditions; may require special surface treatment. |  |  |  |

### 8.8.2 Strength Properties of Various Types of Adhesives

| Type | Form and Color | Preparation and Application | Strength Properties | Typical Uses |
| :---: | :---: | :---: | :---: | :---: |
| Natural origin |  |  |  |  |
| Animal, protein | Solid and liquid; brown to white bondline | Solid form added to water, soaked, and melted; adhesive kept warm during application; liquid form applied directly; both pressed at room temperature; bonding process must be adjusted for small changes in temperature | High dry strength; low resistance to water and damp atmosphere | Assembly of furniture and stringed instruments; repairs of antique furniture |
| Blood, protein | Solid and partially dried whole blood; dark red to black bondline | Mixed with cold water, lime, caustic soda, and other chemicals; applied at room temperature; pressed either at room temperature or $120^{\circ} \mathrm{C}\left(250^{\circ} \mathrm{F}\right)$ and higher | High dry strength; moderate resistance to water and damp atmosphere and to microorganisms | Interior-type softwood plywood, sometimes in combination with soybean adhesive; mostly replaced by phenolic adhesive |
| Casein, protein | Powder with added chemicals; white to tan bondline | Mixed with water; applied and pressed at room temperature | High dry strength; moderate resistance to water, damp atmospheres, and intermediate temperatures; not suitable for exterior uses | Interior doors; discontinued use in laminated timbers |
| Soybean, protein | Powder with added chemicals; white to tan, similar color in bondline | Mixed with cold water, lime, caustic soda, and other chemicals; applied and pressed at room temperatures, but more frequently hot pressed when blended with blood adhesive | Moderate to low dry strength; moderate to low resistance to water and damp atmospheres; moderate resistance to intermediate temperatures | Softwood plywood for interior use, now replaced by phenolic adhesive. New fastsetting resorcinolsoybean adhesives for finger jointing of lumber being developed |
| Lignocellulosic residues and extracts | Powder or liquid; may be blended with phenolic adhesive; dark brown bondline | Blended with extender and filler by user; adhesive cured in hot-press $130^{\circ} \mathrm{C}$ to $150^{\circ} \mathrm{C}$ ( $266^{\circ} \mathrm{F}$ to $300^{\circ} \mathrm{F}$ ) similar to phenolic adhesive | Good dry strength; moderate to good wet strength; durability improved by blending with phenolic adhesive | Partial replacement for phenolic adhesive in composite and plywood panel products |
| Synthetic origin |  |  |  |  |
| Cross-linkable polyvinyl acetate emulsion | Liquid, similar to polyvinyl acetate emulsions but includes copolymers capable of cross-linking with a separate catalyst; white to tan with colorless bondline | Liquid emulsion mixed with catalyst; cure at room temperature or at elevated temperature in hot press and radio-frequency press | High dry strength; improved resistance to moisture and elevated temperatures, particularly long-term performance in moist environment | Interior and exterior doors; moulding and architectural woodwork; cellulosic overlays |


| Type | Form and Color | Preparation and Application | Strength Properties | Typical Uses |
| :---: | :---: | :---: | :---: | :---: |
| Elastomeric contact | Viscous liquid, typically neoprene or styrene-butadine elastomers in organic solvent or water emulsion; tan to yellow | Liquid applied directly to both surfaces, partially dried after spreading and before pressing; roller-pressing at room temperature produces instant bonding | Strength develops immediately upon pressing, increases slowly over a period of weeks; dry strengths much lower than those of conventional wood adhesives; low resistance to water and damp atmospheres; adhesive film readily yields under static load | On-the-job bonding of decorative tops to kitchen counters; factory lamination of wood, paper, metal, and plastic sheet materials |
| Elastomeric mastic (construction adhesive) | Putty-like consistency, synthetic or natural elastomers in organic solvent or latex emulsions; tan, yellow, gray | Mastic extruded in bead to framing members by caulking gun or like pressure equipment; nailing required to hold materials in place during setting and service | Strength develops slowly over several weeks; dry strength lower than conventional wood adhesives; resistant to water and moist atmospheres; tolerant of outdoor assembly conditions; gap-filling; nailing required to ensure structural integrity | Lumber to plywood in floor and wall systems; laminating gypsum board and rigid foam insulating; assembly of panel system in manufactured homes |
| Emulsion polymer/ isocyanate | Liquid emulsion and separate isocyanate hardener; white with hardener; colorless bondline | Emulsion and hardener mixed by user; reactive on mixing with controllable pot-life and curing time; cured at room and elevated temperatures; radio-frequency curable; high pressure required | High dry and wet strength; very resistant to water and damp atmosphere; very resistant to prolonged and repeated wetting and drying; adheres to metals and plastics | Laminated beams for interior and exterior use; lamination of plywood to steel metals and plastics; doors and architectural materials |
| Epoxy | Liquid resin and hardener supplied as two parts; completely reactive leaving no free solvent; clear to amber; colorless bondline | Resin and hardener mixed by user; reactive with limited pot-life; cured at room or elevated temperatures; only low pressure required for bond development | High dry and wet strength to wood, metal, glass, and plastic; formulations for wood resist water and damp atmospheres; delaminate with repeated wetting and drying; gap-filling | Laminating veneer and lumber in cold-molded wood boat hulls; assembly of wood components in aircraft; lamination of architectural railings and posts; repair of laminated wood beams and architectural building components; laminating sports equipment; general purpose home and shop |


| Type | Form and Color | Preparation and <br> Application | Strength Properties | Typical Uses |
| :--- | :--- | :--- | :--- | :--- |


| Type | Form and Color | Preparation and Application | Strength Properties | Typical Uses |
| :---: | :---: | :---: | :---: | :---: |
| Resorcinol and phenolresorcinol | Liquid resin and powdered hardener supplied as two parts; phenol may be copolymerized with resorcinol; dark red bondline | Liquid mixed with powdered or liquid hardener; resorcinol adhesives cure at room temperatures; phenol-resorcinols cure at temperatures from $21^{\circ} \mathrm{C}$ to $66^{\circ} \mathrm{C}\left(70^{\circ} \mathrm{F}\right.$ to $\left.150^{\circ} \mathrm{F}\right)$ | High dry and wet strength; very resistant to moisture and damp atmospheres; more resistant than wood to high temperature and chemical aging. | Primary adhesives for laminated timbers and assembly joints that must withstand severe service conditions. |
| Urea | Powder and liquid forms; may be blended with melamine or other more durable resins; white to tan resin with colorless bondline | Powder mixed with water, hardener, filler, and extender by user; some formulations cure at room temperatures, others require hot pressing at $120^{\circ} \mathrm{C}\left(250^{\circ} \mathrm{F}\right)$; curable with high-frequency heating | High dry and wet strength; moderately durable under damp atmospheres; moderate to low resistance to temperatures in excess of $50^{\circ} \mathrm{C}\left(122^{\circ} \mathrm{F}\right)$ | Hardwood plywood; furniture; fiberboard; particle-board; underlayment; flush doors; furniture cores |

### 8.9.0 Fasteners Installed in Hollow Masonry Units



Fasteners Installed in Hollow Units


Source: Brick Industry Association.

### 8.9.1 Fasteners Installed in Solid Masonry Units



Fasteners Installed in Solid Masonry Units


Source: Brick industry Association.

### 8.9.2 Power-Driven Fasteners for Masonry Units



Power-Driven Fastening Tool


FIG. 10 Power-Driven Pins

Power-driven fasteners require special installation equipment, safety equipment, and inspection procedures. For this reason, the manufacturer should be contacted to determine proper equipment and installation specifications

Source: Brick Industry Association.

### 8.9.3 Masonry Fastener Selection Chart

|  | Brick Type |  |  | Installation Location |  |  | Fixture Weight |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Fastener | Solid brick (cored) | Solid Brick (uncored) | Hollow Brick | Head Joint | Bed Joint | Unit Face | Light | Medium | Heavy |
| Wooden Blocks | X | X | X | X |  |  | X | X |  |
| Metal Wall Plugs | X | X | X | X | X |  | X | X |  |
| Screw Shields and Plugs | X | X | X | X | X |  | X |  |  |
| Toggle Bolts |  |  | X |  | X | X | X | X | X |
| Hollow Wall Screws |  |  | X |  | X | X | X | X |  |
| Screws | X | X | X | X | X | X | X | X |  |
| Sleeve Anchors | X | X | X | X | X | X | X | X | X |
| Wedge Anchors | X | X |  | X | X |  | X | X | X |
| Lag Shields | X | X |  | X | X |  | X | X | X |
| Masonry Nails | X | X | X | X | X |  | X | X |  |
| Powder-Driven Fasteners | X | X |  | X | X |  | X | X |  |
| Adhesives | X | X | X | Surfa | Appl |  | X |  |  |

## Environment

Environmental factors may have a definite impact on the long-term service life of fasteners and should be considered in their selection. Environmental factors do not, in general, influence the type of fastener selected, but should affect the choice of fastener based on the material from which the fastener is made. Corrosion is a major concern, especially when fasteners are exposed to the elements or when fasteners are used in areas where contact with corrosive agents is likely.

Steel fasteners used for applications under normal exposure conditions should be galvanized (zinc-coated) to resist corrosion. Lead, copper-coated or brass fasteners also provide adequate corrosion resistance for normal exposures. In applications where fasteners are subject to severe exposure conditions or exposed to chemicals, stainless steel fasteners should be used.

## Aesthetics

In most applications, the fastener or fasteners installed will be hidden by the attachment (i.e., cabinets, baseboards, electrical boxes or furring), and the physical appearance of the fastener (usually the head of a screw or bolt) will not be of importance. However, when fasteners are used to attach privacy partitions, lighting fixtures or rails, the head of the fastener is usually visible and required to match or accent the finish of the fixture. In these cases, finished screws or bolts (i.e., chrome or brass-plated, solid brass or painted) can be purchased to match the fixtures. The manufacturers should be contacted to determine the availability and range of finishes available in their products.

Source: Brick Industry Association.

### 8.10.0 Bolt Fasteners for Structural Steel—Identification Markings and Strength Requirements

Fastener Identification Markings Bolt Strength Requirements

|  |  |  |  |  |  |  |  |  |  |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Hardness |  |  |  |  |  |  |  |  |  |  |
| Grade Marking |  |  |  |  |  |  |  |  |  |  |



| Low or Medium <br> Carbon Steel | $1 / 4^{\prime \prime}-1 \frac{1}{2 \prime}$ | 33 | 36 | 60 | B70 | B100 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- |



Fastener Identification Markings Bolt Strength Requirements-Cont'd

|  |  |  |  | Mec | anical Prop | rties | Hardness Rockwell |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Grade Marking | Specification | Material and Treatment | Nominal Size (In.) | Proof <br> Load Min <br> (ksi) | Yield <br> Strength <br> Min (ksi) | Tensile <br> Strength <br> Min (ksi) | Min | Max |


$36 \quad 58$ min 80 max

One End Green



$55 \quad$| $75 \min$ |
| :--- |
| $95 \max$ |

One End Yellow

| SAE J429 | $1 / 4^{\prime \prime}-3 / 4^{\prime \prime}$ |  |  |  |  |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Grade 2 |  |  |  |  |  |  |
| $7 / 8^{\prime \prime}-1 \frac{1}{2 \prime \prime}$ | 55 | 57 | 74 | B80 | 36 | 60 |



Fastener Identification Markings Bolt Strength Requirements-Cont'd

| Grade Marking | Specification | Material and Treatment | Nominal <br> Size (In.) | Mechanical Properties |  |  | Hardness Rockwell |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Proof <br> Load Min <br> (ksi) | Yield <br> Strength <br> Min (ksi) | Tensile <br> Strength <br> Min (ksi) | Min | Max |
|  | $\frac{\text { A325 }}{\text { Type } 1}$ | Medium Carbon Steel, Q \& T | $\begin{aligned} & 1 / 2^{\prime \prime}-1^{\prime \prime} \\ & 1^{\prime \prime}-1^{1 / 2 \prime} \end{aligned}$ | 85 74 | 92 81 | 120 105 | C 24 C 19 | C35 C31 |

PB | Atmospheric |
| :--- |
| Corrosion |
| Resistant Steel, |
| $Q \& T$ |



SAE J429
Grade 5
Q \& T
$1^{\prime \prime}-11 / 2^{\prime \prime}$
C19 C31

$85 \quad 92$

| 74 | 81 | 105 |
| :--- | :--- | :--- |

55
58
90
$1^{\prime \prime}-3^{\prime \prime}$

105
125

| $\sim$ |  |  | $1 / 4^{\prime \prime}-21 / 2^{\prime \prime}$ |  | 105 | 125 | C35 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| B7 | A193 | Medium | $2^{\prime \prime}-4^{\prime \prime}$ |  | 95 | 115 | C35 |
| PB | Grade B7 | Carbon Alloy | $2^{\prime \prime}-4$ |  |  |  | C35 |
|  |  | Steel, Q \& T | $4^{\prime \prime}-7^{\prime \prime}$ |  | 75 | 100 | C35 |



Fastener Identification Markings Bolt Strength Requirements-Cont'd

| Grade Marking | Specification | Material and Treatment | Nominal Size (In.) | Mechanical Properties |  |  | Hardness Rockwell |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Proof <br> Load Min <br> (ksi) | Yield <br> Strength <br> Min (ksi) | Tensile <br> Strength <br> Min (ksi) | Min | Max |


$\frac{\text { F1554 }}{\text { Grade } 105}$

- $105 \quad 125 \mathrm{~min}$
$1 / 4^{\prime \prime}-3^{\prime \prime} 150 \max$

One End Red




SAE J429

| Medium | $1 / 4^{\prime \prime}-11 / 2^{\prime \prime}$ | 120 | 130 | 150 |
| :--- | :--- | :--- | :--- | :--- |
| Carbon |  |  |  |  |
| Alloy Steel, |  |  |  |  |
| Q \& T |  |  |  |  |

C33 C39

Q \& T
(Continued)

Fastener Identification Markings Bolt Strength Requirements-Cont'd

| Grade Marking | Specification | Material and Treatment | Nominal <br> Size (In.) | Mechanical Properties |  |  | Hardness Rockwell |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Proof <br> Load Min <br> (ksi) | Yield <br> Strength <br> Min (ksi) | Tensile <br> Strength <br> Min (ksi) | Min | Max |
|  | $\frac{\mathrm{A} 354}{\text { Grade BD }}$ |  | $\begin{aligned} & 1 / 4^{\prime \prime}-2^{1 / 2 \prime \prime} \\ & 2^{\prime \prime}-4^{\prime \prime} \end{aligned}$ | $\begin{aligned} & 120 \\ & 105 \end{aligned}$ | $\begin{aligned} & 130 \\ & 115 \end{aligned}$ | $\begin{aligned} & 150 \\ & 140 \end{aligned}$ | C33 C31 | C39 C39 |

1. All specifications are ASTM unless otherwise noted.
2. All specifications shall be marked by the manufacturer with a unique identifier to identify the manufacturer or private label distributor, as appropriate.
3. $Q \& T$-Quenched and Tempered.
4. Stamping of F1554 and A307 grade C bolts is a supplemental requirement while color coding is required.
5. Although markings are shown on hex heads, grade markings apply equally to products with other head configurations.
6. All Grade BD products shall be marked "BD." In addition to the "BD" marking, the product may be marked with six radial lines $60 \AA \AA^{\circ}$ apart.

### 8.10.1 Standard Thread Pitches

## Standard Thread Pitches

Thread series cover designations of diameter/pitch combinations that are measured by the number of threads per inch (TPI) applied to a single diameter.


Coarse Thread Series (UNC/UNRC) is the most common designation for general application bolts and nuts. Coarse thread is beneficial because it is less likely to cross thread, more tolerant in adverse conditions, and facilitates quick assembly.
Fine Thread Series (UNF/UNRF) is commonly used in precision applications. Because of the larger tensile stress areas, fine threads have high tension strength. However, a longer engagement is required for fine thread applications than for coarse series threads to prevent stripping.
8 - Thread Series (8UN) is the specified thread forming method for several ASTM standards, including A193 B7, A193 B8/B8M, and A320. This series is used for diameters one inch and above.

| Coarse Thread Series-UNC |  |  |  | Fine Thread Series-UNF |  |  |  | 8-Thread Series-8UN |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Nominal Size and Threads Per In. | Basic | Section at | Tensile | Nominal | Basic | Section at | Tensile | Nominal | Basic | Section at | Tensile |
|  | Pitch | Minor Dia. | Stress | Size and | Pitch | Minor Dia. | Stress | Size and | Pitch | Minor Dia. | Stress |
|  | Dia. |  | Area | Threads | Dia. |  | Area | Threads | Dia. |  | Area |
|  | In. | Sq in. | Sq in. |  | In. | Sq in. | Sq in. |  | In. | Sq in. | Sq in. |
| - - | - | - | - | 0-80 | 0.0519 | 0.00151 | 0.00180 | - - | - | - | - |
| 1-64 | 0.0629 | 0.00218 | 0.00263 | 1-72 | 0.0640 | 0.00237 | 0.00278 |  |  |  |  |
| 2-56 | 0.0744 | 0.00310 | 0.00370 | 2-64 | 0.0759 | 0.00339 | 0.00394 |  |  |  |  |
| 3-48 | 0.0855 | 0.00406 | 0.00487 | 3-56 | 0.0874 | 0.00451 | 0.00523 |  |  |  |  |
| 4-40 | 0.0958 | 0.00496 | 0.00604 | 4-48 | 0.0985 | 0.00566 | 0.00661 | - - | - | - | - |
| 5-40 | 0.1088 | 0.00672 | 0.00796 | 5-44 | 0.1102 | 0.00716 | 0.00830 |  |  |  |  |
| 6-32 | 0.1177 | 0.00745 | 0.00909 | 6-40 | 0.1218 | 0.00874 | 0.01015 |  |  |  |  |
| 8-32 | 0.1437 | 0.01196 | 0.0140 | 8-36 | 0.1460 | 0.01285 | 0.01474 |  |  |  |  |
| 10-24 | 0.1629 | 0.01450 | 0.0175 | 10-32 | 0.1697 | 0.0175 | 0.0200 | -- | - | - | - |
| 12-24 | 0.1889 | 0.0206 | 0.0242 | 12-28 | 0.1928 | 0.0226 | 0.0258 | -- | - | - | - |
| 1/4-20 | 0.2175 | 0.0269 | 0.0318 | 1/4-28 | 0.2268 | 0.0326 | 0.0364 | -- | - | - | - |
| 5/16-18 | 0.2764 | 0.0454 | 0.0524 | 5/16-24 | 0.2854 | 0.0524 | 0.0580 | -- | - | - | - |
| 3/8-16 | 0.3344 | 0.0678 | 0.0775 | 3/8-24 | 0.3479 | 0.0809 | 0.0878 | -- | - | - | - |
| 7/16-14 | 0.3911 | 0.0933 | 0.1063 | 7/16-20 | 0.4050 | 0.1090 | 0.1187 | -- | - | - | - |
| 1/2-13 | 0.4500 | 0.1257 | 0.1419 | 1/2-20 | 0.4675 | 0.1486 | 0.1599 | -- | - | - | - |
| 9/16-12 | 0.5084 | 0.162 | 0.182 | 9/16-18 | 0.5264 | 0.189 | 0.203 | -- | - | - | - |
| 5/8-11 | 0.5660 | 0.202 | 0.226 | 5/8-18 | 0.5889 | 0.240 | 0.256 | -- | - | - | - |
| 3/4-10 | 0.6850 | 0.302 | 0.334 | 3/4-16 | 0.7094 | 0.351 | 0.373 | -- | - | - | - |
| 7/8-9 | 0.8028 | 0.419 | 0.462 | 7/8-14 | 0.8286 | 0.180 | 0.509 | -- | - | - | - |
| 1-8 | 0.9188 | 0.551 | 0.606 | 1-12 | 0.9459 | 0.625 | 0.663 | 1-8 | 0.9188 | 0.551 | 0.606 |
| 1-1/8-7 | 1.0322 | 0.693 | 0.763 | 1-1/8-12 | 1.0709 | 0.812 | 0.856 | 1-1/8-8 | 1.0438 | 0.728 | 0.790 |
| 1-1/4-7 | 1.1572 | 0.890 | 0.969 | 1-1/4-12 | 1.1959 | 1.024 | 1.073 | 1-1/4-8 | 1.1688 | 0.929 | 1.000 |
| 1-3/8-6 | 1.2667 | 1.054 | 1.155 | 1-3/8-12 | 1.3209 | 1.260 | 1.315 | 1-3/8-8 | 1.2938 | 1.155 | 1.233 |
| 1-1/2-6 | 1.3917 | 1.294 | 1.405 | 1-1/2-12 | 1.4459 | 1.521 | 1.581 | 1-1/2-8 | 1.4188 | 1.405 | 1.492 |
| -- | - | - | - | -- | - | - | - | 1-5/8-8 | 1.5438 | 1.68 | 1.78 |
| 1-3/4-5 | 1.6201 | 1.74 | 1.90 | -- | - | - | - | 1-3/4-8 | 1.6688 | 1.98 | 2.08 |
| -- | - | - | - | -- | - | - | - | 1-7/8-8 | 1.7938 | 2.30 | 2.41 |
| 2-4-1/2 | 1.8557 | 2.30 | 2.50 | -- | - | - | - | 2-8 | 1.9188 | 2.65 | 2.77 |
| $2-1 / 4-4-1 / 2$ | 2.1057 | 3.02 | 3.25 | -- | - | - | - | 2-1/4-8 | 2.1688 | 3.42 | 3.56 |
| 2-1/2-4 | 2.3376 | 3.72 | 4.00 | -- | - | - | - | 2-1/2-8 | 2.4188 | 4.29 | 4.44 |
| 2-3/4-4 | 2.5876 | 4.62 | 4.93 | -- | - | - | - | 2-3/4-8 | 2.6688 | 5.26 | 5.43 |
| 3-4 | 2.8376 | 5.62 | 5.97 | -- | - | - | - | 3-8 | 2.9188 | 6.32 | 6.51 |
| 3-1/4-4 | 3.0876 | 6.72 | 7.10 | -- | - | - | - | 3-1/4-8 | 3.1688 | 7.49 | 7.69 |
| 3-1/2-4 | 3.3376 | 7.92 | 8.33 | -- | - | - | - | 3-1/2-8 | 3.4188 | 8.75 | 8.96 |
| 3-3/4-4 | 3.5876 | 9.21 | 9.66 | -- | - | - | - | 3-3/4-8 | 3.6688 | 10.11 | 10.34 |
| 4-4 | 3.8376 | 10.61 | 11.08 | -- | - | - | - | 4-8 | 3.9188 | 11.57 | 11.81 |

### 8.10.2 Suggested Starting Torque Values for ASTM and SAE Grade Bolts

| Bolt Size | TPI | Proof Load (lbs) | Clamp Load (lbs) | Tightening Torque (ft lbs) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Waxed | Galv | Plain |
| 1/4 | 20 | 1145 | 859 | 2 | 4 | 4 |
| 5/16 | 18 | 1886 | 1415 | 4 | 9 | 7 |
| 3/8 | 16 | 2790 | 2093 | 7 | 16 | 13 |
| 7/16 | 14 | 3827 | 2870 | 10 | 26 | 21 |
| 1/2 | 13 | 5108 | 3831 | 16 | 40 | 32 |
| 9/16 | 12 | 6552 | 4914 | 23 | 58 | 46 |
| 5/8 | 11 | 8136 | 6102 | 32 | 79 | 64 |
| 3/4 | 10 | 12024 | 9018 | 56 | 141 | 113 |
| 7/8 | 9 | 15200 | 11400 | 83 | 208 | 166 |
| 1 | 8 | 20000 | 15000 | 125 | 313 | 250 |
| $11 / 8$ | 7 | 25200 | 18900 | 177 | 443 | 354 |
| $11 / 4$ | 7 | 32000 | 24000 | 250 | 625 | 500 |
| $13 / 8$ | 6 | 38100 | 28575 | 327 | 819 | 655 |
| $11 / 2$ | 6 | 46400 | 34800 | 435 | 1088 | 870 |
| $13 / 4$ | 5 | 68400 | 51300 | 748 | 1870 | 1496 |
| 2 | $41 / 2$ | 90000 | 67500 | 1125 | 2813 | 2250 |
| $21 / 4$ | $41 / 2$ | 117000 | 87750 | 1645 | 4113 | 3291 |
| $21 / 2$ | 4 | 144000 | 108000 | 2250 | 5625 | 4500 |
| $23 / 4$ | 4 | 177480 | 133110 | 3050 | 7626 | 6101 |
| 3 | 4 | 214920 | 161190 | 4030 | 10074 | 8060 |
| $31 / 4$ | 4 | 255600 | 191700 | 5192 | 12980 | 10384 |
| $31 / 2$ | 4 | 299880 | 224910 | 6560 | 16400 | 13120 |
| $33 / 4$ | 4 | 347760 | 260820 | 8151 | 20377 | 16301 |
| 4 | 4 | 398880 | 299160 | 9972 | 24930 | 19944 |

## SAE Grade 2

| Bolt Size | TPI | Proof Load (lbs) | Clamp Load (lbs) | Tightening Torque (ft lbs) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Waxed | Galv | Plain |
| 1/4 | 20 | 1750 | 1313 | 3 | 7 | 5 |
| 5/16 | 18 | 2900 | 2175 | 6 | 14 | 11 |
| 3/8 | 16 | 4250 | 3188 | 10 | 25 | 20 |
| 7/16 | 14 | 5850 | 4388 | 16 | 40 | 32 |
| 1/2 | 13 | 7800 | 5850 | 24 | 61 | 49 |
| 9/16 | 12 | 10000 | 7500 | 35 | 88 | 70 |
| 5/8 | 11 | 12400 | 9300 | 48 | 121 | 97 |
| 3/4 | 10 | 18400 | 13800 | 86 | 216 | 173 |
| 7/8 | 9 | 15200 | 11400 | 83 | 208 | 166 |
| 1 | 8 | 20000 | 15000 | 125 | 313 | 250 |
| $11 / 8$ | 7 | 25200 | 18900 | 177 | 443 | 354 |
| 1 1/4 | 7 | 32000 | 24000 | 250 | 625 | 500 |
| $13 / 8$ | 6 | 38100 | 28575 | 327 | 819 | 655 |
| $11 / 2$ | 6 | 46400 | 34800 | 435 | 1088 | 870 |

## ASTM A325 / ASTM A449 / SAE Grade 5

| Bolt Size | TPI | Proof Load (lbs) | Clamp Load (lbs) | Tightening Torque (ft lbs) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Waxed | Galv | Plain |
| 1/4 | 20 | 2700 | 2025 | 4 | 11 | 8 |
| 5/16 | 18 | 4450 | 3338 | 9 | 22 | 17 |
| 3/8 | 16 | 6600 | 4950 | 15 | 39 | 31 |
| 7/16 | 14 | 9050 | 6788 | 25 | 62 | 49 |
| 1/2 | 13 | 12050 | 9038 | 38 | 94 | 75 |
| 9/16 | 12 | 15450 | 11588 | 54 | 136 | 109 |
| 5/8 | 11 | 19200 | 14400 | 75 | 188 | 150 |
| 3/4 | 10 | 28400 | 21300 | 133 | 333 | 266 |
| 7/8 | 9 | 39250 | 29438 | 215 | 537 | 429 |
| 1 | 8 | 51500 | 38625 | 322 | 805 | 644 |
| $11 / 8$ | 7 | 56450 | 42338 | 397 | 992 | 794 |
| $11 / 4$ | 7 | 71700 | 53775 | 560 | 1400 | 1120 |
| $13 / 8$ | 6 | 85450 | 64088 | 734 | 1836 | 1469 |
| $11 / 2$ | 6 | 104000 | 78000 | 975 | 2438 | 1950 |
| $13 / 4$ | 5 | 104500 | 78375 | 1143 | 2857 | 2286 |
| 2 | $41 / 2$ | 137500 | 103125 | 1719 | 4297 | 3438 |
| $21 / 4$ | $41 / 2$ | 178750 | 134063 | 2514 | 6284 | 5027 |
| $21 / 2$ | 4 | 220000 | 165000 | 3438 | 8594 | 6875 |
| $23 / 4$ | 4 | 271150 | 203363 | 4660 | 11651 | 9321 |
| 3 | 4 | 328350 | 246263 | 6157 | 15391 | 12313 |

## ASTM A193 B7

| Bolt Size | TPI | Proof Load (lbs) | Clamp Load (lbs) | Tightening Torque (ft lbs) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Waxed | Galv | Plain |
| 1/4 | 20 | 3350 | 2513 | 5 | 13 | 10 |
| 5/16 | 18 | 5500 | 4125 | 11 | 27 | 21 |
| 3/8 | 16 | 8150 | 6113 | 19 | 48 | 38 |
| 7/16 | 14 | 11150 | 8363 | 30 | 76 | 61 |
| 1/2 | 13 | 14900 | 11175 | 47 | 116 | 93 |
| 9/16 | 12 | 19100 | 14325 | 67 | 168 | 134 |
| 5/8 | 11 | 23750 | 17813 | 93 | 232 | 186 |
| 3/4 | 10 | 35050 | 25288 | 164 | 411 | 329 |
| 7/8 | 9 | 48500 | 36375 | 265 | 663 | 530 |
| 1 | 8 | 63650 | 47738 | 398 | 995 | 796 |
| $11 / 8$ | 7 | 80100 | 60075 | 563 | 1408 | 1126 |
| $11 / 4$ | 7 | 101750 | 76313 | 795 | 1987 | 1590 |
| $13 / 8$ | 6 | 121300 | 90975 | 1042 | 2606 | 2085 |
| $11 / 2$ | 6 | 147550 | 110663 | 1383 | 3458 | 2767 |
| $13 / 4$ | 5 | 199500 | 149625 | 2182 | 5455 | 4364 |
| 2 | $41 / 2$ | 262500 | 196875 | 3281 | 8203 | 6563 |
| $21 / 4$ | $41 / 2$ | 341250 | 255938 | 4799 | 11997 | 9598 |
| $21 / 2$ | 4 | 420000 | 315000 | 6563 | 16406 | 13125 |
| $23 / 4$ | 4 | 468500 | 351263 | 8050 | 20124 | 16100 |
| 3 | 4 | 567150 | 425363 | 10634 | 26585 | 21268 |
| $31 / 4$ | 4 | 674500 | 505875 | 13701 | 34252 | 27402 |

ASTM A193 B7-Cont'd

|  |  |  | Tightening Torque (ft lbs) |  |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| Bolt Size | TPI | Proof Load (lbs) | Clamp Load (lbs) | Waxed | Galv | Plain |
| 3 | $1 / 2$ | 4 | 791350 | 593513 | 17311 | 43277 |
| $33 / 4$ | 4 | 917700 | 688275 | 21509 | 53771 | 43622 |
| 4 | 4 | 1052600 | 789450 | 26315 | 65788 | 52630 |
|  |  |  |  |  |  |  |

## ASTM A354-BD / ASTM A490 / SAE Grade 8

| Bolt Size | TPI | Proof Load (lbs) | Clamp Load (lbs) | Tightening Torque |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Waxed | Plain |
| 1/4 | 20 | 3800 | 2850 | 6 | 12 |
| 5/16 | 18 | 6300 | 4725 | 12 | 25 |
| 3/8 | 16 | 9300 | 6975 | 22 | 44 |
| 7/16 | 14 | 12750 | 9563 | 35 | 70 |
| 1/2 | 13 | 17050 | 12788 | 53 | 107 |
| 9/16 | 12 | 21850 | 16388 | 77 | 154 |
| 5/8 | 11 | 27100 | 20325 | 106 | 212 |
| 3/4 | 10 | 40100 | 30075 | 188 | 376 |
| 7/8 | 9 | 55450 | 41588 | 303 | 606 |
| 1 | 8 | 72700 | 54525 | 454 | 909 |
| $11 / 8$ | 7 | 91550 | 68663 | 644 | 1287 |
| $11 / 4$ | 7 | 120000 | 90000 | 938 | 1875 |
| $13 / 8$ | 6 | 138600 | 103950 | 1191 | 2382 |
| $11 / 2$ | 6 | 168600 | 126450 | 1581 | 3161 |
| $13 / 4$ | 5 | 228000 | 171000 | 2494 | 4988 |
| 2 | $41 / 2$ | 300000 | 225000 | 3750 | 7500 |
| $21 / 4$ | $41 / 2$ | 390000 | 292500 | 5484 | 10969 |
| $21 / 2$ | 4 | 480000 | 360000 | 7500 | 15000 |
| $23 / 4$ | 4 | 517650 | 388238 | 8897 | 17794 |
| 3 | 4 | 626850 | 470138 | 11753 | 23507 |
| $31 / 4$ | 4 | 745500 | 559125 | 15143 | 30286 |
| $31 / 2$ | 4 | 874650 | 655988 | 19133 | 38266 |
| $33 / 4$ | 4 | 1014300 | 760725 | 23773 | 47545 |
| 4 | 4 | 1052600 | 789450 | 26315 | 52630 |

## Notes:

1. Values calculated using industry accepted formula $\mathrm{T}=\mathrm{KDP}$ where $\mathrm{T}=$ Torque, $\mathrm{K}=$ torque coefficient (dimensionless), $\mathrm{D}=$ nominal diameter (inches), $\mathrm{P}=$ bolt clamp load, lb.
2. K values: waxed (e.g., pressure wax as supplied on high strength nuts) $=.10$, hot dip galvanized $=.25$, and plain non-plated bolts (as received) $=0.20$.
3. Torque has been converted into $\mathrm{ft} / \mathrm{lbs}$ by dividing the result of the formula by 12 .
4. All calculations are for Coarse Thread Series (UNC).
5. Grade 2 calculations only cover fasteners $1 / 4^{\prime \prime}-3 / 4^{\prime \prime}$ in diameter up to $6^{\prime \prime}$ long; for longer fasteners the torque is reduced significantly.
6. Clamp loads are based on $75 \%$ of the minimum proof loads for each grade and size.
7. Proof load, stress area, yield strength, and other data are based on IFI 7th Edition (2003). Technical Data N-68, SAE J429, ASTM A307, A325, A354, A449, and A490.

The above estimated torque calculations are only offered as a guide. Use of its content by anyone is the sole responsibility of that person and they assume all risk. Due to many variables that affect the torque-tension relationship like human error, surface texture, lubrication, etc., the only way to determine the correct torque is through experimentation under actual joint and assembly conditions.

### 8.10.3 AASHTO to ASTM Conversion Chart

## AASHTO to ASTM Conversions

AASHTO is an acronym for American Association of State Highway and Transportation Officials. It is common for bolt specifications to be called out on construction plans with an AASHTO designation for state highway projects. Many of these designations can be directly converted to an ASTM equivalent. The following table lists some of the more common fastener-related specifications and their ASTM equivalents.

| AASHTO Grade | ASTM Equivalent | Description |
| :---: | :---: | :---: |
| M-111 | A123 | Hot-dip galvanizing of iron and steel products |
| M-164 | A325 | Structural bolt |
| M-183 | A36 | Raw material, low carbon steel |
| M-222 | A588 | Raw material, weathering steel |
| M-223 | A572 | Raw material, high strength / low alloy |
| M-232 | A153 | Hot-dip galvanizing of fasteners |
| M-253 | A490 | Structural bolt |
| M-291 | A563 | Nut specification possessing many grades |
| M-292 | $\stackrel{\text { A194 }}{ }$ | High strength, heavy hex nut |
| M-293 | F436 | Hardened washer |
| M-314 | F1554 | Anchor bolt specification with 3 grades |

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### 8.10.4 ASTM A563 Nut Compatibility Chart ASTM A563 Nut Compatibility Chart

Download a print version

| Grade of Bolt | Surface Finish | Nominal Size Inches | A563 Grade and ANSI Nut Style |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Recommended |  | Suitable |  |
|  |  |  | Hex | Heavy Hex | Hex | Heavy Hex |
| A307 Grade A \& C | Any | $\begin{aligned} & 1 / 4 \text { to } 11 / 2 \\ & >11 / 2 \text { to } 2 \\ & >2 \text { to } 4 \end{aligned}$ | A | $\begin{aligned} & \mathrm{A} \\ & \mathrm{~A} \end{aligned}$ | $\begin{aligned} & \mathrm{B}, \mathrm{D}, \mathrm{DH} \\ & \mathrm{~A} \end{aligned}$ | $\begin{aligned} & \text { A,B,C,D,DH,DH3 } \\ & \text { C,D,DH,DH3 } \\ & \text { C,D,DH,DH3 } \end{aligned}$ |
| A307 Grade B | Any | $\begin{aligned} & 1 / 4 \text { to } 11 / 2 \\ & >11 / 2 \text { to } 2 \\ & >2 \text { to } 4 \end{aligned}$ |  | $\begin{aligned} & \mathrm{A} \\ & \mathrm{~A} \\ & \mathrm{~A} \end{aligned}$ | $\begin{aligned} & \mathrm{B}, \mathrm{D}, \mathrm{DH} \\ & \mathrm{~A} \end{aligned}$ | A,B,C,D,DH,DH3 |
| A325 Type 1 | Plain <br> Galvanized | $\begin{aligned} & 1 / 2-11 / 2 \\ & 1 / 2-11 / 2 \end{aligned}$ |  | $\begin{aligned} & \mathrm{C} \\ & \mathrm{DH} \end{aligned}$ |  | C3, D, DH, DH3 |
| A325 Type 3 | Plain | 1/2-11/2 |  | C3 |  | DH3 |


| Download a print version-Cont'd |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Grade of Bolt | Surface Finish | Nominal Size Inches | A563 Grade and ANSI Nut Style |  |  |  |
|  |  |  | Recommended |  | Suitable |  |
|  |  |  | Hex | Heavy Hex | Hex | Heavy Hex |
| A354 Grade BC | Plain |  |  | C | D, DH | $\begin{aligned} & \mathrm{C} 3, \mathrm{D}, \mathrm{DH}, \mathrm{DH} 3 \\ & \mathrm{C} 3, \mathrm{D}, \mathrm{DH}, \mathrm{DH} 3 \end{aligned}$ |
|  |  | $>11 / 2 \text { to } 4$ |  | C |  |  |
|  | Galvanized | $1 / 4$ to $11 / 2$ |  | DH |  |  |
|  |  | $>11 / 2$ to 4 |  | DH |  |  |
| A354 Grade BD | Plain | $1 / 4 \text { to } 11 / 2$ |  | DH | DH | $\mathrm{D}, \mathrm{DH}, \mathrm{DH} 3$ |
|  |  | $>11 / 2 \text { to } 4$ |  | $\mathrm{DH}$ |  | DH3 |
| A449 | Plain | $1 / 4$ to $11 / 2$ | B |  | D, DH | B,C,C3,D,DH,DH3 |
|  |  | $>11 / 2$ to 3 |  | A |  | C,C3,D,DH,DH3 |
|  | Galvanized | $1 / 4 \text { to } 1 / 1 / 2$ |  | $\mathrm{DH}$ | D, DH |  |
|  |  | $>1 \frac{1}{2} \text { to } 3$ |  | $\mathrm{DH}$ |  | D |
| A490 Type 1 | Plain | $1 / 2-11 / 2$ |  | DH |  | DH3 |
| A490 Type 3 | Plain | 1/2-11/2 |  | DH3 |  |  |
| A687 | Plain Galvanized | - 3 |  | D |  | DH,DH3 |
|  |  | - 3 |  | DH |  |  |
| F1554 Grade 36 | Any | $1 / 4$ to $11 / 2$ | A |  | B,D,DH | A,B,C,D,DH,DH3 |
|  |  | $>11 / 2$ to 4 |  | A |  | C,D,DH,DH3 |
| F1554 Grade 55 | Plain | $1 / 4$ to $11 / 2$ | A |  | B,D,DH | A,B,C,D,DH,DH3 |
|  |  | $>11 / 2 \text { to } 4$ |  | A |  | C,D,DH,DH3 |
|  | Galvanized | $1 / 4$ to 4 |  | A |  | C,D,DH,DH3 |
| F1554 Grade 105 | Plain | $1 / 4$ to $11 / 2$ | D |  |  | DH, DH3 |
|  |  | $>11 / 2$ to 3 |  | DH |  | DH3 |
|  | Galvanized | $1 / 4$ to 3 |  | DH |  | DH3 |

By permission: Portland Bolt and Manufacturing, Portland, OR

### 8.11.0 Tension Shear-TC Bolts

## Structural Steel Fastening System

Unytite Inc., a QS 9000/ISO 9002 registered facility located in Peru, Illinois, is a manufacturer of "Structural Fastening Systems" for the Heavy Construction (High Rise, Bridge, Road, and Industrial Building applications), Petro Chemical (Refinery, Pipeline, and Chemical Industries), Heavy Equipment, Rail Car, and Tractor-Trailer O.E.M.'s (Original Equipment Manufacturer).

The unique Tension Control Fastening System is a three-piece fastening assembly comprised of a button head design bolt with a 12-point pintail, a high-strength heavy hex nut, and a hardened flat washer. When installed with a dual socket electric shear wrench, the outer socket applies the turning force to the nut, while the inner socket holds the bolt in place by gripping the 12 -point spline tip. When the forces reach or exceed the designed torque-tension coefficient, the 12-point spline tip will shear off, leaving the bolt and nut securing the application at the proper tension.

## Installation Procedure

1. Fit the inner socket of the shear wrench over the spline on the bolt and push forward until the outer socket engages completely with the nut.
2. Pull the larger trigger on the wrench. The inner socket will hold the bolt in place, while the outer socket tightens the nut. The spline will shear off when proper tension is reached.
3. Remove the wrench from the nut and pull the ejection trigger. This will eject the spline from the inner socket of the wrench. The installation is now complete and may be verified visually.

## Handling-Storage-Installation



1. All structural fasteners should be protected from dirt and moisture at the job site. No more than the amount of bolts to be used that day should be removed from the container, or protected storage. Remaining bolts at the end of the day should be returned to the correct container. Dirty or rusted bolts should not be used.
2. Place all the bolts into the connection, with a washer under the nut in standard and short slotted holes. For long slotted and oversize holes, a washer should be placed under the head of the bolt and under the nut. Washer and nut identification markings should always face the opposite direction of the connection.
3. Bring all the fasteners in the connection to a snug tight condition, starting with the most rigid part of the connection.
(The above recommendations by AISC apply to all A325 and A490 fasteners regardless of Installation methods)

## Determining Proper Bolt Length

To determine the proper length of fastener that is needed, refer to the chart at right for the proper length to add to the grip. The bolt length should be adjusted to the next $1 / 4$ inch for washer thickness.
(NOTE: 3-5 bolt threads should be within the structural member to prevent thread from running out.)

| Nominal Bolt Size | Length Added to Grip |
| :--- | :---: |
| $5 / 8^{\prime \prime}$ | $7 / 8^{\prime \prime}$ |
| $3 / 4^{\prime \prime}$ | $1^{\prime \prime}$ |
| $7 / 8^{\prime \prime}$ | $1-1 / 8^{\prime \prime}$ |
| $1^{\prime \prime}$ | $1-1 / 4^{\prime \prime}$ |
| $1-1 / 8^{\prime \prime}$ | $1-1 / 2^{\prime \prime}$ |
| L = Bolt Length |  |
| LG = Grip Length |  |
| LA $=$ Length Added to Grip |  |
| Source: UNYTITE, Peru, ILL. |  |



UNYTITE, INC. tension control bolts are designed, manufactured, and tested to conform to ASTM (American Society for Testing and Materials) F-1852, A-325 and A-490, AISC (American Institute of Steel Construction), FHWA* (Federal Highway Administration) and the most demanding customer specifications.


## ASTM F-1852 Dimensions for Twist-Off Structural Bolt

| Normal Size or Basic Product |
| :--- | :--- | :--- | :--- | :--- | :--- |
| Diameter |$\quad$| H | D |
| :--- | :--- |

## ASTM A325 (F1852) Mechanical Properties

|  | Bolt |  |  |  | Nut |  | Washer |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | ASTM A325 Type 1 |  |  |  | A563 DH |  | F436 |
|  | Proof Load | Tensile Strength | Hardness |  | Proof Load | Hardness | Hardness |
|  |  |  | Brinell | Rockwell |  |  |  |
| 5/8"-11 | 19,200 | 27,100 | $253 \sim 319$ | HRC 25~34 | 39,550 | HRC 24~38 | HRC 38~45 |
| 3/4"-10 | 28,400 | 40,100 |  |  | 58,450 |  |  |
| 7/8"-9 | 39,250 | 55,450 |  |  | 80,850 |  |  |
| 1"-8 | 51,500 | 72,700 |  |  | 106,050 |  |  |
| 1-1/8"-7 | 56,450 | 80,100 | 223~286 | 19~30 | 133,525 |  |  |

[^25]
## A325 (F1852) Fastener Tension

| Nominal Diameter | 1 | 2 | 3 |
| :---: | :---: | :---: | :---: |
|  | AISC Table 4 Design | AISC Installed Fastener | UNYTITE Fastener |
|  | Tension Min lbf | Tension Min lbf | Tension Min lbf |
| 5/8"-11 | 19,000 | 19,950 | 23,000 |
| 3/4"-10 | 28,000 | 29,400 | 33,000 |
| 7/8"-9 | 39,000 | 40,950 | 44,000 |
| 1"-8 | 51,000 | 53,550 | 57,000 |
| 1-1/8"-7 | 56,000 | 58,800 | 65,000 |

## ASTM A490 Mechanical Properties

|  | Bolt |  |  |  | Nut |  | Washer |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | ASTM A490 Type 1 |  |  |  | A563 DH |  | F436 |
|  | Proof Load | Tensile Strength |  | Hardness <br> Rockwell | Proof Load | Hardness | Hardness |
|  |  | Max | Min |  |  |  |  |
| 3/4"-10 | 40,100 | 56,800 | 50,100 | HRC 33~38 | 58,450 | HRC 24~38 | HRC 38~45 |
| 7/8"-9 | 55,450 | 78,550 | 69,300 |  | 80,850 |  |  |
| 1"-8 | 72,700 | 103,000 | 90,900 |  | 106,050 |  |  |
| 1-1/8"-7 | 91,550 | 129,700 | 114,450 |  | 133,525 |  |  |

## A490 Fastener Tension



## Calculations to Determine the Effectiveness and Control of Thermal and Sound Transmission

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### 9.0.1 Heat Transmission Modes

It is important to know how heat is transferred in fish holds. Heat is transferred by conduction, convection, or radiation, or by a combination of all three. Heat always moves from warmer to colder areas; it seeks a balance. If the interior of an insulated fish hold is colder than the outside air, the fish hold draws heat from the outside. The greater the temperature difference, the faster the heat flows to the colder area.

Conduction. By this mode, heat energy is passed through a solid, liquid, or gas from molecule to molecule in a material. In order for the heat to be conducted, there should be physical contact between particles and some temperature difference. Therefore, thermal conductivity is the measure of the speed of heat flow passed from particle to particle. The rate of heat flow through a specific material will be influenced by the difference of temperature and by its thermal conductivity.

Convection. By this mode, heat is transferred when a heated air/gas or liquid moves from one place to another, carrying its heat with it. The rate of heat flow will depend on the temperature of the moving gas or liquid and on its rate of flow.

Radiation. Heat energy is transmitted in the form of light, as infrared radiation or another form of electromagnetic waves. This energy emanates from a hot body and can travel freely only through completely transparent media. The atmosphere, glass, and translucent materials pass a significant amount of radiant heat, which can be absorbed when it falls on a surface (e.g., the ship's deck surface on a sunny day absorbs radiant heat and becomes hot). It is a well-known fact that light-colored or shiny surfaces reflect more radiant heat than black or dark surfaces; therefore the former will be heated more slowly.

In practice, the entry of heat into fish holds/fish containers is the result of a mixture of the three modes mentioned above, but the most significant mode is by conduction through walls and flooring.

Source: Food and Agriculture Organization of the United Nations.

### 9.0.2 Definitions and Thermal Property Symbols

The thermal properties of insulating materials and other common fishing vessel construction materials are known or can be accurately measured. The amount of heat transmission (flow) through any combination of materials can be calculated. However, it is necessary to know and understand certain technical terms to be able to calculate heat losses and understand the factors that are involved.

By convention, the ending -ity means the property of a material, regardless of its thickness, and the ending -ance refers to the property of a specific body of given thickness.

## Heat Energy

One kilocalorie ( 1 kcal or 1000 calories) is the amount of heat (energy) needed to raise the temperature of one kg of water by one degree Celsius ( ${ }^{\circ} \mathrm{C}$ ). The SI standard unit for energy is Joule (J). One kcal is approximately 4.18 kJ (this varies slightly with temperature). Another unit is the Btu (British thermal unit). One Btu corresponds roughly to 1 kJ .

## Thermal Conductivity

In simple terms this is a measure of the capacity of a material to conduct heat through its mass. Different insulating materials and other types of material have specific thermal conductivity values that can be used to measure their insulating effectiveness. It can be defined as the amount of heat/energy (expressed in kcal, Btu or J) that can be conducted in unit time through unit area of unit thickness of material, when there is a unit temperature difference. Thermal conductivity can be expressed in $\mathrm{kcal} \mathrm{m}^{-1}{ }^{\circ} \mathrm{C}^{-1}$, $\mathrm{Btu} \mathrm{ft}^{-1}{ }^{\circ} \mathrm{F}^{-1}$, and in the SI system in watt (W) $\mathrm{m}^{-1}{ }^{\circ} \mathrm{C}^{-1}$. Thermal conductivity is also known as the k -value.

## Coefficient of Thermal Conductance " ${ }^{\prime \prime}\left(\mathbf{k c a l ~ m}^{-2} \mathbf{h}^{-1}{ }^{\circ} \mathbf{C}^{-1}\right)$

This is designated as I (the Greek letter lambda) and defined as the amount of heat (in kcal) conducted in one hour through $1 \mathrm{~m}^{2}$ of material, with a thickness of 1 m , when the temperature drop through the material under conditions of steady heat flow is $1^{\circ} \mathrm{C}$. The thermal conductance is established by tests and is the basic rating for any material. I can also be expressed in $\mathrm{Btu} \mathrm{ft}^{-2} \mathrm{~h}^{-1}{ }^{\circ} \mathrm{F}^{-1}$ (British thermal unit per square foot, hour, and degree Fahrenheit) or in SI units in $\mathrm{W} \mathrm{m}^{-2} \operatorname{Kelvin}(\mathrm{~K})^{-1}$.

## Thermal Resistivity

The thermal resistivity is the reciprocal of the k -value $(1 / \mathrm{k})$.

## Thermal Resistance (R-value)

The thermal resistance ( R -value) is the reciprocal of $1(1 / 1)$ and is used for calculating the thermal resistance of any material or composite material. The R -value can be defined in simple terms as the resistance that any specific material offers to the heat flow. A good insulation material will have a high R-value. For thicknesses other than 1 m , the R -value increases in direct proportion to the increase in thickness of the insulation material. This is $\mathrm{x} / \mathrm{l}$, where $x$ stands for the thickness of the material in meters.

## Coefficient of Heat Transmission (U) (kcal m ${ }^{-2} \mathbf{h}^{-1}{ }^{\circ} \mathbf{C}^{-1}$ )

The symbol U designates the overall coefficient of heat transmission for any section of a material or a composite of materials. The SI units for $U$ are kcal per square meter of section per hour per degree Celsius, the difference between inside air temperature and outside air temperature. It can also be expressed in other unit systems. The U coefficient includes the thermal resistances of both surfaces of walls or flooring, as well as the thermal resistance of individual layers and air spaces that may be contained within the wall or flooring itself.

## Permeance to Water Vapor (pv)

This is defined as the quantity of water vapor that passes through the unit of area of a material of unit thickness, when the difference of water pressure between both faces of the material is the unit. It can be expressed as g cm $\mathrm{mmHg}^{-1} \mathrm{~m}^{-2}$ day $^{-1}$ or in the SI system as $\mathrm{g} \mathrm{m} \mathrm{MN}^{-1} \mathrm{~s}^{-1}$ (grams meter per mega Newton per second).

## Resistance to Water Vapor (rv)

This is the reciprocal of the permeance to water vapor and is defined as rv $=1 / \mathrm{pv}$.
Source: Food and Agriculture Organization of the United Nations.

### 9.0.3 R- and U-Values-Measuring the Resistance to the Flow of Heat and the Measure of Heat Conductivity

A measure of the resistance of building materials and structures to the flow of heat; the higher the R-value, the better the substance is as thermal insulation.

$$
\mathrm{R}-\text { value }=\frac{\text { temperature difference } \times \text { area } \times \text { time }}{\text { heat loss }}
$$

where the temperature difference is in degrees Fahrenheit, the area is in square feet, the time is in hours, and the heat loss is in Btus. If you know the R-value of a partition, you can use this formula to find the heat loss.

## Relation to U-value

The reciprocal of the $R$-value $(1 / R)$ is known as the $U$-value. The higher the $U$-value, the better the conduction of heat.

In Europe it is customary to use U-values instead of R-values. There, U-values are defined by the equation:

$$
\mathrm{U}-\text { value }=\frac{\text { watts }}{\text { kelvin } \times \text { meters }^{2}}
$$

This is not the reciprocal of the American R-value (kelvin instead of degrees Fahrenheit, meters instead of feet, etc.). To convert an American R-value into a European U-value, divide 1 by the R-value, then multiply the result by 5.682. To convert a European $U$-value to an American R-value, multiply by 0.176 , then divide 1 by the result.

### 9.0.4 Basic Types of Insulation—Where Applicable—Installation Methods—Advantages

| Form | Insulation <br> Materials | Where Applicable | Installation Method(s) | Advantages |
| :--- | :--- | :--- | :--- | :--- |
| Blanket: <br> batts and <br> rolls | Fiberglass Mineral <br> (rock or slag) wool <br> plastic fibers Natural <br> fibers | Unfinished walls, <br> including foundation <br> walls, and floors and <br> ceilings. | Fitted between studs, joists, <br> and beams. | Do-it-yourself. Suited for <br> standard stud and joist <br> spacing, which is relatively <br> free from obstructions. |
| Concrete <br> block <br> insulation | Foam beads or <br> liquid foam: <br> - Polystyrene <br> - Polyisocyanurate <br> or polyiso | Unfinished walls, <br> including foundation <br> walls, for new <br> construction or major <br> renovations. | Involves masonry skills. | Autoclaved aerated concrete <br> and autoclaved cellular |
|  | Vermiculite or <br> perlite pellets |  | concrete masonry units have <br> 10 times the insulating value <br> of conventional concrete. |  |
|  |  |  |  |  |


| Form | Insulation Materials | Where Applicable | Installation Method(s) | Advantages |
| :---: | :---: | :---: | :---: | :---: |
| Foam board or rigid foam | Polystyrene <br> Polyisocyanurate or polyiso Polyurethane | Unfinished walls, including foundation walls; floors and ceilings; unvented lowslope roofs. | Interior applications: must be covered with $1 / 2$-inch gypsum board or other building-code approved material for fire safety. <br> Exterior applications: must be covered with weatherproof facing. | High insulating value for relatively little thickness. <br> Can block thermal short circuits when installed continously over frames or joists. |
| Insulating concrete forms (ICFS) | Foam boards or form blocks | Unfinished walls, including foundation walls, for new construction. | Installed as part of the building structure. | Insulation is literally built into the home's walls, creating high thermal resistance. |
| Loose-fill | Cellulose Fiberglass Mineral (rock or slag) wool | Enclosed existing wall or open new wall cavities; unfinished attic floors; hard-toreach places. | Blown into place using special equipment; sometimes poured in. | Good for adding insulation to existing finished areas, irregularly shaped areas, and around obstructions. |
| Reflective system | Foil-faced kraft paper, plastic film, polyethlene bubbles, or cardboard | Unfinished walls, ceilings, and floors. | Foils, films, or papers: fitted between wood-frame studs, joists, and beams | Do-it-yourself. <br> All suitable for framing at standard spacing. Bubbleform suitable if framing is irregular or if obstructions are present. <br> Most effective at preventing downward heat flow; however, effectiveness depends on spacing. |
| Rigid <br> fibrous or fiber insulation | Fiberglass Mineral (rock or slag) wool | Ducts in unconditioned spaces and other places requiring insulation that can withstand high temperatures. | HVAC contractors fabricate the insulation into ducts either at their shops or at the job sites. | Can withstand high temperatures. |
| Sprayed foam and foamed-in-place | Cementitious <br> phenolic <br> Polyisocyanurate <br> Polyurethane | Enclosed existing wall or open new wall cavities; unfinished attic floors. | Applied using small spray containers or in larger quantities as a pressure sprayed (foamed-in-place) product. | Good for adding insulation to existing finished areas, irregularly shaped areas, and around obstructions. |
| Structural insulated panels (SIPs) | Foam board or liquid foam insulation core Straw core insulation | Unfinished walls, ceilings, floors, and roofs for new construction. | Builders connect them together to construct a house. | SIP-built houses provide superior and uniform insulation compared to more traditional construction methods; they also take less time to build. |

[^26]
### 9.1.0 Sample R-Value of Materials

| Properties of Materials |  |  |
| :---: | :---: | :---: |
| Material | R-Value Per Inch of Thickness | R-Value for Thickness Listed |
| 4" Clay Brick |  | 0.44 |
| $4^{\prime \prime}$ Block (115\#/ft ${ }^{3}$ ) $=72 \%$ solid |  | 1.19 |
| $6^{\prime \prime}$ Block (115\#/ft ${ }^{3}$ ) $=59 \%$ solid |  | 1.25 |
| $8^{\prime \prime}$ Block $\left(115 \# / \mathrm{ft}^{3}\right)=54 \%$ solid |  | 1.45 |
| 10" Block (115\#/ft ${ }^{2}$ ) $=52 \%$ solid |  | 1.55 |
| 12' Block ( $115 \# / \mathrm{ft}{ }^{2}$ ) $=48 \%$ solid |  | 1.65 |
| $6^{\prime \prime}$ Block ( $115 \# / \mathrm{ft}^{2}$ ) $=59 \%$ solid/filled with perlite |  | 3.95 |
| $8^{\prime \prime}$ Block (115\#/ft ${ }^{2}$ ) $=54 \%$ solid/filled with perlite |  | 4.65 |
| 10" Block ( $\left.115 \# / / \mathrm{ft}^{2}\right)=52 \%$ solid/filled with perlite |  | 5.65 |
| $12^{\prime \prime}$ Block (115\#/ft ${ }^{2}$ ) $=48 \%$ solid/filled with perlite |  | 7.05 |
| $1^{\prime \prime}$ Polyisocyanurate | 8.0 |  |
| 1" Extruded polystyrene | 5.0 |  |
| 1" Expanded polystyrene | 4.0 |  |
| $1^{\prime \prime}$ of Perlite | 2.70 |  |
| Exterior air film (winter) |  | 0.17 |
| Interior air film |  | 0.68 |
| Dead air space (3/4" to $4^{\prime \prime}$ ) (winter) |  | 0.97 |
| $3 / 4^{\prime \prime}$ reflective air space |  | 2.89 |
| 1/2" dry wall |  | 0.45 |
| $31 / 2^{\prime \prime}$ Batt (R-11) |  | 11.00 |
| $35 / 8^{\prime \prime}(R-13)$ |  | 13.00 |
| $11 / 2^{\prime \prime}(R-5)$ |  | 5.00 |
| $6^{\prime \prime}$ Batt (R-19) |  | 19.00 |
| $61 / 2^{\prime \prime}$ Batt (R-22) |  | 22.00 |
| $9^{\prime \prime}$ Batt (R-30) |  | 30.00 |
| 12" Batt (R-38) |  | 38.00 |

[^27]
### 9.1.1 Sample R-Value Calculations for Masonry Wall Assemblies

## Sample R-Value Calculations

Brick Veneer on Wood Frame (residential and single family usage)


|  |  |
| :--- | ---: |
| $R$ of the outside air film | 0.17 |
| $R$ of a $4^{\prime \prime}$ brick | 0.44 |
| $R$ of $1^{\prime \prime}$ reflective air space | 2.89 |
| $R$ of $3 / 4^{\prime \prime}$ polyisocyanurate | 5.60 |
| $R$ of $31 / 2^{\prime \prime}$ batt insulation | 11.00 |
| $R$ of $1 / 2^{\prime \prime}$ drywall | 0.45 |
| $R$ of the inside air film | 0.68 |
| $R$ of the total wall | 21.23 |
| $U$ of the wall | 0.047 |

## Solid Loadbearing Masonry Wall (midrise and multifamily usage)



|  |  |
| :--- | ---: |
| $R$ of the outside air film | 0.17 |
| $R$ of a $4^{\prime \prime}$ brick | 0.44 |
| $R$ of $6^{\prime \prime}$ block | 1.25 |
| $R$ of $3^{\prime \prime}$ expanded polystyrene | 12.00 |
| $R$ of $1 / 2^{\prime \prime}$ drywall | 0.45 |
| $R$ of the inside air film | 0.68 |
| $R$ of the total wall | 14.99 |
| $U$ of the wall | 0.066 |

## Brick and Block Cavity Wall (Quality construction for schools, commercial/industrial multifamily and high-rises)



If you were designing the wall shown on the left-a wall composed of a $35 / 8^{\prime \prime}$ brick, $3 / 4^{\prime \prime}$ air space, an unknown thickness of an unspecified Type of Rigid insulation, a 5 $5 / 8^{\prime \prime}$ block, $11 / 2^{\prime \prime}$ furring for $1 / 2^{\prime \prime}$ plaster dry-wall-what insulation would you select? The insulation that gives you the best dollar value for the R -value desired.

### 9.1.2 R-Values for Blanket-Batt Insulation

## Blanket (Batt and Roll) Insulation

Blanket insulation-the most common and widely available type of insulation-comes in the form of batts or rolls. It consists of flexible fibers, most commonly fiberglass. You also can find batts and rolls made from mineral (rock and slag) wool, plastic fibers, and natural fibers, such as cotton and sheep's wool.

Batts and rolls are available in widths suited to standard spacing of wall studs and attic or floor joists. Continuous rolls can be hand-cut and trimmed to fit. They are available with or without facings. Manufacturers often attach a facing (such as kraft paper, foil-kraft paper, or vinyl) to act as a vapor barrier and/or air barrier. Batts with a special flame-resistant facing are available in various widths for basement walls where the insulation will be left exposed. A facing also helps facilitate fastening during installation. However, it's recommended that you use unfaced batts if you're reinsulating over existing insulation.

Standard fiberglass blankets and batts have a thermal resistance or R-values between R-2.9 and R-3.8 per inch of thickness. High-performance (medium-density and high-density) fiberglass blankets and batts have R-values between R-3.7 and R-4.3 per inch of thickness. See the following table for an overview of these characteristics.

TABLE 1 Fiberglass Batt Insulation Characteristics*

| Thickness (inches) | R-Value | Cost (cents/sq. ft.) |
| :--- | :---: | :---: |
| $31 / 2$ | 11 | $12-16$ |
| $35 / 8$ | 13 | $15-20$ |
| $31 / 2$ (high density) | 15 | $34-40$ |
| 6 to $61 / 4$ | 19 | $27-34$ |
| $51 / 4$ (high density) | 21 | $33-39$ |
| 8 to $81 / 2$ | 25 | $37-45$ |
| 8 (high density) | 30 | $45-49$ |
| $91 / 2$ (standard) $j$ | 30 | $39-43$ |
| 12 | 30 | $55-60$ |

*This table is for comparison only. Determine actual thickness, $R$-value, and cost from manufacturer and/or local building supplier.

### 9.1.3 Calculating the R-Values for Wall Assemblies

Use the following R -value table to help you determine the R -value of your wall or ceiling assemblies. To obtain a wall or ceiling assembly R-value, you must add the R-values of the individual components together. See the following example:

| Calculating Assembly Wall R-Value* |  |  |  |
| :--- | :--- | :--- | :--- |
| Component | R-Value <br> Studs | R-Value <br> Cavity | Assembly <br> R-Value |
| Wall—Outside Air Film | 0.17 | 0.17 |  |
| Siding—Wood Bevel | 0.80 | 0.80 |  |
| Plywood Sheathing—1/2" | 0.63 | 0.63 |  |
| $31 / 2^{\prime \prime}$ Fiberglass Batt | 4.38 | 11.00 |  |
| $31 / 2^{\prime \prime}$ Stud | 0.45 | 0.45 |  |
| $1 / 2^{\prime \prime}$ Drywall |  |  |  |


| Component | R-Value <br> Studs | R-Value <br> Cavity | Assembly <br> R-Value |
| :--- | :---: | :---: | :---: |
| Inside Air Film | 0.68 | 0.68 |  |
| Percent for $16^{\prime \prime}$ o.c. + Additional studs | $15 \%$ | $85 \%$ |  |
| Total Wall Component R-Values | 7.12 | $\mathbf{1 3 . 7 3}$ |  |
| Wall Component U-Values | $\mathbf{0 . 1 4 0 4}$ | $\mathbf{0 . 0 7 2 8}$ |  |
| Total Wall Assembly R-Value |  |  | $\mathbf{1 2 . 0 5}$ |

Formula: Assembly R-value $=1 /$ (Assembly $U$-value) $=1 /(U$-studs $\times \%+U$-cavity $\times \%$ ) *This example is just for wood frame construction. Steel studs are a more complicated calculation.

## R-Value Table

| Material | R/Inch | R/Thickness |
| :---: | :---: | :---: |
| Insulation Materials |  |  |
| Fiberglass Batt | 3.14-4.30 |  |
| Fiberglass Blown (attic) | 2.20-4.30 |  |
| Fiberglass Blown (wall) | 3.70-4.30 |  |
| Rock Wool Batt | 3.14-4.00 |  |
| Rock Wool Blown (attic) | 3.10-4.00 |  |
| Rock Wool Blown (wall) | 3.10-4.00 |  |
| Cellulose Blown (attic) | 3.13 |  |
| Cellulose Blown (wall) | 3.70 |  |
| Vermiculite | 2.13 |  |
| Autoclaved Aerated Concrete | 1.05 |  |
| Urea Terpolymer Foam | 4.48 |  |
| Rigid Fiberglass ( $>4 \mathrm{lb} / \mathrm{ft} 3$ ) | 4.00 |  |
| Expanded Polystyrene (bead board) | 4.00 |  |
| Extruded Polystyrene | 5.00 |  |
| Polyurethane (foamed-in-place) | 6.25 |  |
| Polyisocyanurate (foil-faced) | 7.20 |  |
| Construction Materials |  |  |
| Concrete Block 4" |  | 0.80 |
| Concrete Block 8" |  | 1.11 |
| Concrete Block 12" |  | 1.28 |
| Brick 4" Common |  | 1.80 |
| Brick 4" Face |  | 0.44 |
| Poured Concrete | 0.08 |  |
| Soft Wood Lumber | 1.25 |  |
| $2^{\prime \prime}$ nominal ( $11 / 2^{\prime \prime}$ ) |  | 1.88 |
| $2 \times 4$ ( $31 / 2^{\prime \prime}$ ) |  | 4.38 |
| $2 \times 6$ ( $51 / 2^{\prime \prime}$ ) |  | 6.88 |
| Cedar Logs and Lumber | 1.33 |  |
| Sheathing Materials |  |  |
| Plywood | 1.25 |  |
| 1/4" |  | 0.31 |
| 3/8' ${ }^{\prime \prime}$ |  | 0.47 |
| 1/2" |  | 0.63 |
| 5/8' |  | 0.77 |


| Material | R/Inch | R/Thickness |
| :---: | :---: | :---: |
| 3/4" |  | 0.94 |
| Fiberboard | 2.64 |  |
| 1/2" |  | 1.32 |
| 25/32 ${ }^{\prime \prime}$ |  | 2.06 |
| Fiberglass (3/4") |  | 3.00 |
| (1") |  | 4.00 |
| (1 1/2") |  | 6.00 |
| Extruded Polystyrene (3/4") |  | 3.75 |
| (1") |  | 5.00 |
| (1 1/2") |  | 7.50 |
| Foil-faced Polyisocyanurate (3/4") |  | 5.40 |
| (1") |  | 7.20 |
| (1 1/2") |  | 10.80 |
| Siding Materials |  |  |
| Hardboard (1/2") |  | 0.34 |
| Plywood (5/8") |  | 0.77 |
| (3/4") |  | 0.93 |
| Wood Bevel Lapped |  | 0.80 |
| Aluminum, Steel, Vinyl (hollow backed) |  | 0.61 |
| (w/ 1/2" Insulating board) |  | 1.80 |
| Brick 4" |  | 0.44 |
| Interior Finish Materials |  |  |
| Gypsum Board (drywall 1/2") |  | 0.45 |
| (5/8") |  | 0.56 |
| Paneling (3/8") |  | 0.47 |
| Flooring Materials |  |  |
| Plywood | 1.25 |  |
| (3/4") |  | 0.93 |
| Particle Board (underlayment) | 1.31 |  |
| (5/8') |  | 0.82 |
| Hardwood Flooring | 0.91 |  |
| (3/4") |  | 0.68 |
| Tile, Linoleum |  | 0.05 |
| Carpet (fibrous pad) |  | 2.08 |
| (rubber pad) |  | 1.23 |
| Roofing Materials |  |  |
| Asphalt Shingles |  | 0.44 |
| Wood Shingles |  | 0.97 |
| Windows |  |  |
| Single Glass |  | 0.91 |
| w/storm |  | 2.00 |
| Double insulating glass (3/16) air space |  | 1.61 |
| (1/4" air space) |  | 1.69 |
| (1/2" air space) |  | 2.04 |
| (3/4" air space) |  | 2.38 |
| (1/2" w/ Low-E 0.20) |  | 3.13 |
| ( $\mathrm{w} / \mathrm{suspended}$ film) |  | 2.77 |
| ( $\mathrm{w} / 2$ suspended films) |  | 3.85 |
| ( $\mathrm{w} / \mathrm{s}$ suspended film and low-E) |  | 4.05 |
| Triple insulating glass |  | 2.56 |
| (1/4" air spaces) |  |  |
| (1/2" air spaces) |  | 3.23 |
| Addition for tight fitting drapes or shades, or closed blinds |  | 0.29 |


| Material | R/Inch | R/Thickness |
| :--- | :---: | :---: |
| Doors |  |  |
| Wood Hollow Core Flush $\left(13 / 4^{\prime \prime}\right)$ | 2.17 |  |
| Solid Core Flush $\left(13 / 4^{\prime \prime}\right)$ | 3.03 |  |
| Solid Core Flush $\left(21 / 4^{\prime \prime}\right)$ | 3.70 |  |
| Panel Door w/ $7 / 16^{\prime \prime}$ Panels (1 3/4") | 1.85 |  |
| Storm Door (wood 50\% glass) | 1.25 |  |
| (metal) | 1.00 |  |
| Metal Insulating $\left(2^{\prime \prime}\right.$ w/ urethane) | 15.00 |  |
| Air Films |  |  |
| Interior Ceiling | 0.61 |  |
| Interior Wall | 0.68 |  |
| Exterior | 0.17 |  |
| Air Spaces |  |  |
| $1 / 2^{\prime \prime}$ to $4^{\prime \prime}$ approximately | 1.00 |  |
| By permission: www.coloradoenergy.org |  |  |

### 9.1.4 Properties of Solid Unit Masonry and Concrete Walls

| Type |  | Layer Thickness, inches |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 |
| LW CMU | U | na | 0.71 | 0.64 | Na | na | na | na | na | na | na |
|  | Rw | na | 1.4 | 1.6 | Na | na | na | na | na | na | na |
|  | HC | na | 7.00 | 8.75 | Na | na | na | na | na | na | na |
| MW CMU | U | na | 0.76 | 0.70 | Na | na | na | na | na | na | na |
|  | Rw | na | 1.3 | 1.4 | Na | na | na | na | na | na | na |
|  | HC | na | 7.67 | 9.58 | Na | na | na | na | na | na | na |
| NW CMU | U | 0.89 | 0.82 | 0.76 | Na | na | na | na | na | na | na |
|  | Rw | 1.1 | 1.2 | 1.3 | Na | na | na | na | na | na | na |
|  | HC | 6.25 | 8.33 | 10.42 | Na | na | na | na | na | na | na |
| Clay Brick | U | 0.80 | 0.72 | 0.66 | Na | na | na | na | na | na | na |
|  | Rw | 1.3 | 1.4 | 1.5 | Na | na | na | na | na | na | na |
|  | HC | 6.30 | 8.40 | 10.43 | Na | na | na | na | na | na | na |
| Concrete | U | 0.96 | 0.91 | 0.86 | 0.82 | 0.78 | 0.74 | 0.71 | 0.68 | 0.65 | 0.63 |
|  | Rw | 1.0 | 1.1 | 1.2 | 1.2 | 1.3 | 1.4 | 1.4 | 1.5 | 1.5 | 1.6 |
|  | HC | 7.20 | 9.60 | 12.00 | 14.40 | 16.80 | 19.20 | 21.60 | 24.00 | 26.40 | 28.80 |

Notes:
LW CMU is a Light Weight Concrete Masonry Unit per ASTM C 90 or 55, Calculated at 105 PCF density
MW CMU is a Medium Weight Concrete Masonry Unit per ASTM C 90 or 55 , Calculated at 115 PCF density
NW CMU is a Normal Weight Concrete Masonry Unit per ASTM C 90 or 55, Calculated at 125 PCF density
Clay Brick is a Clay Unit per ASTM C 62, Calculated at 130 PCF density
Calculations based on Energy Calculations and Data, CMACN, 1986.
Values include air films on Inner and outer surfaces.
Source: Berkeley Solar Group; Concrete Masonry Association of California and Nevada.

### 9.1.5 Properties of Hollow Unit Masonry Walls

|  | Type |  | Core Treatment |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Solid Grout | Partly Grouted with Ungrouted Cells |  |
|  |  |  |  | Empty | Insulated |
| $12^{\prime \prime}$ | LW CMU | U | 0.51 | 0.43 | 0.30 |
|  |  | Rw | 2.0 | 2.3 | 3.3 |
|  |  | HC | 23 | 14.8 | 14.8 |
|  | MW CMU | U | 0.54 | 0.46 | 0.33 |
|  |  | Rw | 1.9 | 2.2 | 3.0 |
|  |  | HC | 23.9 | 15.6 | 15.6 |
|  | NW CMU | U | 0.57 | 0.49 | 0.36 |
|  |  | Rw | 1.8 | 2.0 | 2.8 |
|  |  | HC | 24.8 | 16.5 | 16.5 |
| $10^{\prime \prime}$ | LW CMU | U | 0.55 | 0.46 | 0.34 |
|  |  | Rw | 1.8 | 2.2 | 2.9 |
|  |  | HC | 18.9 | 12.6 | 12.6 |
|  | MW CMU | U | 0.59 | 0.49 | 0.37 |
|  |  | Rw | 1.7 | 2.1 | 2.7 |
|  |  | HC | 19.7 | 13.4 | 13.4 |
|  | NW CMU | U | 0.62 | 0.52 | 0.14 |
|  |  | Rw | 1.6 | 1.9 | 2.4 |
|  |  | HC | 20.5 | 14.2 | 14.2 |
| $8^{\prime \prime}$ | LW CMU | U | 0.62 | 0.50 | 0.37 |
|  |  | Rw | 1.6 | 2.0 | 2.7 |
|  |  | HC | 15.1 | 9.9 | 9.9 |
|  | MW CMU | U | 0.65 | 0.53 | 0.41 |
|  |  | Rw | 1.5 | 1.9 | 2.4 |
|  |  | HC | 15.7 | 10.5 | 10.5 |
|  | NW CMU | U | 0.69 | 0.56 | 0.44 |
|  |  | Rw | 1.4 | 1.8 | 2.3 |
|  |  | HC | 16.3 | 11.1 | 11.1 |
|  | Clay Unit | U | 0.57 | 0.47 | 0.39 |
|  |  | Rw | 1.8 | 2.1 | 2.6 |
|  |  | HC | 15.1 | 11.4 | 11.4 |
| $6^{\prime \prime}$ | LW CMU | U | 0.68 | 0.54 | 0.44 |
|  |  | Rw | 1.5 | 1.9 | 2.3 |
|  |  | HC | 10.9 | 7.9 | 7.9 |
|  | MW CMU | U | 0.72 | 0.58 | 0.48 |
|  |  | Rw | 1.4 | 1.7 | 2.1 |
|  |  | HC | 11.4 | 8.4 | 8.4 |


| Type |  | Core Treatment |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  |  | Solid Grout | Partly Grouted with Ungrouted Cells |  |
|  |  |  | Empty | Insulated |
| NW CMU | U | 0.76 | 0.61 | 0.52 |
|  | Rw | 1.3 | 1.6 | 1.9 |
|  | HC | 11.9 | 8.9 | 8.9 |
| Clay Unit | U | 0.65 | 0.52 | 0.45 |
|  | Rw | 1.5 | 1.9 | 2.2 |
|  | HC | 11.1 | 8.6 | 8.6 |

Notes:
LW CMU is a Light Weight Concrete Masonry Unit per ASTM C 90, Calculated at 105 PCF density
MW CMU is a Medium Weight Concrete Masonry Unit per ASTM C 90, Calculated at 115 PCF density
NW CMU is a Normal Weight Concrete Masonry Unit per ASTM C 90, Calculated at 125 PCF density
Clay Unit is a Hollow Clay Unit per ASTM C 652, Calculated at 130 PCF density
Values include air films on inner and outer surfaces.
Calculations based on Energy Calculations and Data, CMACN, 1986.
Grouted Cells at $32^{\prime \prime} \times 48^{\prime \prime}$ in Partly Grouted Walls
Source: Berkeley Solar Group; Concrete Masonry Association of California and Nevada.

### 9.2.0 Exterior Brick and Block Cavity Wall R-Values



| $4^{\prime \prime}$ Clay Brick | 0.44 |
| :--- | :--- |
| $4^{\prime \prime}$ Block $\left(115 \# . \mathrm{ft}^{3}\right)=72 \%$ solid | 1.19 |
| $6^{\prime \prime}$ Block $\left(115 \# . \mathrm{ft}^{3}\right)=59 \%$ solid | 1.34 |
| $8^{\prime \prime}$ Block $\left(115 \# . \mathrm{ft}^{3}\right)=54 \%$ solid | 1.51 |
| $10^{\prime \prime}$ Block $\left(115 \# . \mathrm{ft}^{3}\right)=52 \%$ solid | 1.61 |
| $12^{\prime \prime}$ Block $\left(115 \# . \mathrm{ft}^{3}\right)=48 \%$ solid | 1.72 |
| $1 / 2^{\prime \prime}$ Drywall | 0.45 |
| Exterior air film $($ winter $)$ | 0.17 |


| Interior air film | 0.68 |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Dead air space (3/4" to $4^{\prime \prime}$ ) (winter) | 0.97 |  |  |  |  |  |
| * Reflective air space | 2.8 |  |  |  |  |  |
| Insulation type thickness (inches) | 1/2" | 1" | $11 / 2^{\prime \prime}$ | $2^{\prime \prime}$ | $21 / 2^{\prime \prime}$ | 3" |
| Polyisocyanurate (foil face) Dow Tuff $\mathrm{R}^{\text {TM }} /$ Thermax $^{\text {TM }}$ | 4.0 | 8 | 12 | 16 | 18 | 21.6 |
| Extruded Polystyrene Dow, Owens Corning | - | 5 | 7.5 | 10 | 12.5 | 15 |
| *Use this value when insulation has a foil backing directly adjacent to air space. |  |  |  |  |  |  |


|  |  |
| :--- | :---: |
| Exterior air film | 0.17 |
| $4^{\prime \prime}$ Brick | 0.44 |
| R of the reflective air space | 2.80 |
| R of 2" Dow, Tuff R-C, polyisocyanurate insulation | 16.0 |
| $6^{\prime \prime}$ CMU | 1.34 |
| $11 / 2^{\prime \prime}$ air space | 0.97 |
| $1 / 2^{\prime \prime}$ Drywall | 0.45 |
| Interior Air Film | 0.68 |

Source: Berkeley Solar Group, Concrete Masonsry Association of California and Nevada.

### 9.2.1 An Exterior Masonry Wall Assembly with a Total R-Value of 20.21



### 9.2.2 An Exterior Masonry Wall Assembly with a Total R-Value of $\mathbf{2 8 . 2 1}$



Source: Masonry Advisory Council, Park Ridge, ILL.

### 9.2.3 Concrete Block Walls Utilizing Perlite Cavity Fill as an Insulator

Insulation is essential in all construction for energy conservation. The original cost of installing perlite loose fill insulation can be recovered quickly due to substantial reductions in heating and air condition energy consumption. In addition, perlite loose fill insulation cuts installation costs since it is lightweight and pours easily and quickly in place without need for special installation equipment or skills. The insulation may be poured directly into walls or emptied into a simply wood or metal hopper, which can be slid along the wall to direct the freeflowing perlite into cores of cavities, thus insulating all voids and air pockets.

Concrete Block Walls-U-Values and R-Values

| Block Thickness in Inches | Density of Block | Block Only |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Uninsulated |  | Insulated |  |
|  |  | $R$ | $U$ | $R$ | $U$ |
| 6 | 80 pcf Lightweight | 2.64 | . 38 | 6.75 | . 15 |
|  | 125 pcf Sand \& Gravel | 2.05 | . 49 | 3.86 | . 26 |
| 8 | 80 pcf Lightweight | 2.86 | . 35 | 9.07 | . 11 |
|  | 125 pcf Sand \& Gravel | 2.21 | . 45 | 5.06 | . 20 |


| Block Thickness in Inches | Density of Block | Block Only |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Uninsulated |  | Insulated |  |
|  |  | $R$ | $U$ | $R$ | $U$ |
| 10 | 80 pcf Lightweight | 3.00 | . 33 | 11.02 | . 09 |
|  | 125 pcf Sand \& Gravel | 2.31 | . 45 | 5.95 | . 17 |
| 12 | 80 pcf Lightweight | 3.12 | . 32 | 13.44 | . 08 |
|  | 125 pcf Sand \& Gravel | 2.38 | . 42 | 7.17 | . 15 |

Values given are approximate and include the effect of inside and outside air film resistances. Source: The Schundler Company, Metuchen, NJ.

## Installation

a. The insulation must be installed in the following locations:

- In the cores of all exterior (and interior) hollow masonry units.
- In the cavity between all exterior (and interior) masonry walls.
- Between exterior masonry walls and interior furring.
b. The insulation shall be poured directly into the wall at any convenient internal. Wall sections under doors and windows shall be filled before sills are placed.
c. All holes and openings in the wall through which insulation can escape shall be permanently sealed or caulked prior to installation of the insulation. Copper, galvanized steel, or fiberglass screening shall be used in all weep holes.
(The inclusion of weep holes is considered good construction design practice to allow passage of any water that might penetrate the cavities or core spaces of wall construction.)

| Approximate Coverage $^{(\mathrm{a})}$ |  |  |  |  |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| Sq. Ft. of Wall Area ${ }^{(\mathrm{b})}$ | $\mathbf{1}^{\prime \prime}$ Cavity | $\mathbf{2}^{\prime \prime}$ Cavity | $\mathbf{3}^{\prime \prime}$ Cavity | $\mathbf{6}^{\prime \prime}$ Block | $\mathbf{8}^{\prime \prime}$ Block | $\mathbf{1 2}^{\prime \prime}$ Block |
| 100 | 2 | 4 | 6 | 5 | 7 | 12 |
| 500 | 10 | 21 | 31 | 23 | 33 | 58 |
| 1,000 | 21 | 42 | 62 | 46 | 65 | 118 |
| 2,000 | 42 | 84 | 124 | 96 | 130 | 236 |
| 3,000 | 63 | 126 | 186 | 138 | 195 | 354 |
| 5,000 | 105 | 210 | 310 | 230 | 325 | 590 |
| 7,000 | 147 | 294 | 434 | 322 | 455 | 826 |
| 10,000 | 210 | 420 | 620 | 460 | 650 | 1,180 |

[^28]
### 9.3.0 Effective R-Values on Wood-Metal Framing Assemblies

| Type Actual Thick | Frame | Furring Space R-Value without Framing Effects |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 0 | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | 15 | 16 | 17 | 18 | 19 | 20 | 21 |
| Any | None | 0.5 | 1.5 | 2.5 | 3.5 | 4.5 | 5.5 | 6.5 | 7.5 | 8.5 | 9.5 | 10 | 11.5 | 12.5 | 13.5 | 14.5 | 15.5 | 16.5 | 17.5 | 18.5 | 19.5 | 20.5 | 21.5 |
| 0.5" | Wood | 1.3 | 1.3 | 1.9 | 2.4 | 2.7 | na | na | na | na | na | na | na | na | na | na | na | na | na | na | na | na | na |
|  | Metal | 0.9 | 0.9 | 1.1 | 1.1 | 1.2 | na | na | na | na | na | na | na | na | na | na | na | na | na | na | na | na | na |
| 0.75" | Wood | 1.4 | 1.4 | 2.1 | 2.7 | 3.1 | 3.5 | 3.8 | na | na | na | na | na | na | na | na | na | na | na | na | na | na | na |
|  | Metal | 1.0 | 1.0 | 1.3 | 1.4 | 1.5 | 1.5 | 1.6 | na | na | na | na | na | na | na | na | na | na | na | na | na | na | na |
| $1.0^{\prime \prime}$ | Wood | 1.3 | 1.5 | 2.2 | 2.9 | 3.4 | 3.9 | 4.3 | 4.6 | 4.9 | na | na | na | na | na | na | na | na | na | na | na | na | na |
|  | Metal | 1.0 | 1.1 | 1.4 | 1.6 | 1.7 | 1.8 | 1.8 | 1.9 | 1.9 | na | na | na | na | na | na | na | na | na | na | na | na | na |
| 1.5" | Wood | 1.3 | 1.5 | 2.4 | 3.1 | 3.8 | 4.4 | 4.9 | 5.4 | 5.8 | 6.2 | 6.5 | 6.8 | 7.1 | na | na | na | na | na | na | na | na | na |
|  | Metal | 1.1 | 1.2 | 1.6 | 1.9 | 2.1 | 2.2 | 2.3 | 2.4 | 2.5 | 2.5 | 2.6 | 2.6 | 2.7 | na | na | na | na | na | na | na | na | na |
| $2{ }^{\prime \prime}$ | Wood | 1.4 | 1.5 | 2.5 | 3.3 | 4.0 | 4.7 | 5.3 | 5.9 | 6.4 | 6.9 | 7.3 | 7.7 | 8.1 | 8.4 | 8.7 | 9.0 | 9.3 | na | na | na | na | na |
|  | Metal | 1.1 | 1.2 | 1.7 | 2.1 | 2.3 | 2.5 | 2.7 | 2.8 | 2.9 | 3.0 | 3.1 | 3.2 | 3.2 | 3.3 | 3.3 | 3.4 | 3.4 | na | na | na | na | na |
| 2.5 ${ }^{\prime \prime}$ | Wood | 1.4 | 1.5 | 2.5 | 3.4 | 4.2 | 4.9 | 5.6 | 6.3 | 6.8 | 7.4 | 7.9 | 8.4 | 8.8 | 9.2 | 9.6 | 10.0 | 10.3 | 10.6 | 10.9 | 11.2 | 11.5 | na |
|  | Metal | 1.2 | 1.3 | 1.8 | 2.3 | 2.6 | 2.8 | 3.0 | 3.2 | 3.3 | 3.5 | 3.6 | 3.6 | 3.7 | 3.8 | 3.9 | 3.9 | 4.0 | 4.0 | 4.1 | 4.1 | 4.1 | na |
| $3{ }^{\prime \prime}$ | Wood | 1.4 | 1.5 | 2.5 | 3.5 | 4.3 | 5.1 | 5.8 | 6.5 | 7.2 | 7.8 | 8.3 | 8.9 | 9.4 | 9.9 | 10.3 | 10.7 | 11.1 | 11.5 | 11.9 | 12.2 | 12.5 | 12.9 |
|  | Metal | 1.2 | 1.3 | 1.9 | 2.4 | 2.8 | 3.1 | 3.3 | 3.5 | 3.7 | 3.8 | 4.0 | 4.1 | 4.2 | 4.3 | 4.4 | 4.4 | 4.5 | 4.6 | 4.6 | 4.7 | 4.7 | 4.8 |
| $3.5^{\prime \prime}$ | Wood | 1.4 | 1.5 | 2.6 | 3.5 | 4.4 | 5.2 | 6.0 | 6.7 | 7.4 | 8.1 | 8.7 | 9.3 | 9.8 | 10.4 | 10.9 | 11.3 | 11.8 | 12.2 | 12.6 | 13.0 | 13.4 | 13.8 |
|  | Metal | 1.2 | 1.3 | 2.0 | 2.5 | 2.9 | 3.2 | 3.5 | 3.8 | 4.0 | 4.2 | 4.3 | 4.5 | 4.6 | 4.7 | 4.8 | 4.9 | 5.0 | 5.1 | 5.1 | 5.2 | 5.2 | 5.3 |
| $4 \prime$ | Wood | 1.4 | 1.6 | 2.6 | 3.6 | 4.5 | 5.3 | 6.1 | 6.9 | 7.6 | 8.3 | 9.0 | 9.6 | 10.2 | 10.8 | 11.3 | 11.9 | 12.4 | 12.8 | 13.3 | 13.7 | 14.2 | 14.6 |
|  | Metal | 1.2 | 1.3 | 2.0 | 2.6 | 3.0 | 3.4 | 3.7 | 4.0 | 4.2 | 4.5 | 4.6 | 4.8 | 5.0 | 5.1 | 5.2 | 5.3 | 5.4 | 5.5 | 5.6 | 5.7 | 5.8 | 5.8 |
| $4.5^{\prime \prime}$ | Wood | 1.4 | 1.6 | 2.6 | 3.6 | 4.5 | 5.4 | 6.2 | 7.1 | 7.8 | 8.5 | 9.2 | 9.9 | 10.5 | 11.2 | 11.7 | 12.3 | 12.8 | 13.3 | 13.8 | 14.3 | 14.8 | 15.2 |
|  | Metal | 1.2 | 1.3 | 2.1 | 2.6 | 3.1 | 3.5 | 3.9 | 4.2 | 4.5 | 4.7 | 4.9 | 5.1 | 5.3 | 5.4 | 5.6 | 5.7 | 5.8 | 5.9 | 6.0 | 6.1 | 6.2 | 6.3 |
| 5" | Wood | 1.4 | 1.6 | 2.6 | 3.6 | 4.6 | 5.5 | 6.3 | 7.2 | 8 | 8.7 | 9.4 | 10.1 | 10.8 | 11.5 | 12.1 | 12.7 | 13.2 | 13.8 | 14.3 | 14.8 | 15.3 | 15.8 |
|  | Metal | 1.2 | 1.4 | 2.1 | 2.7 | 3.2 | 3.7 | 4.1 | 4.4 | 4.7 | 5.0 | 5.2 | 5.4 | 5.6 | 5.8 | 5.9 | 6.1 | 6.2 | 6.3 | 6.5 | 6.6 | 6.7 | 6.8 |
| 5.5 ${ }^{\prime \prime}$ | Wood | 1.4 | 1.6 | 2.6 | 3.6 | 4.6 | 5.5 | 6.4 | 7.3 | 8.1 | 8.9 | 9.6 | 10.3 | 11.0 | 11.7 | 12.4 | 13.0 | 13.6 | 14.2 | 14.7 | 15.3 | 15.8 | 16.3 |
|  | Metal | 1.3 | 1.4 | 2.1 | 2.8 | 3.3 | 3.8 | 4.2 | 4.6 | 4.9 | 5.2 | 5.4 | 5.7 | 5.9 | 6.1 | 6.3 | 6.4 | 6.6 | 6.7 | 6.8 | 7.0 | 7.1 | 7.2 |

All furring thickness values given are actual dimensions.
All values include $0.5^{\prime \prime}$ gypbd on the inner surface; interior surface resistances not included.
$24^{\prime \prime}$ OC Furring 24 Gage, Z-type Metal Furring Douglas-Fir Larch Wood Furring, density $=34.9 \mathrm{lb} / \mathrm{cu} . \mathrm{ft}$.
Insulation assumed to fill the furring space.
Source: Berkeley Solar Group; Concrete Masonry Association of California and Nevada

### 9.3.1 Metal Framing Factors

| Metal Framing Factors |  |  |  |
| :---: | :---: | :---: | :---: |
| Stud Spacing | Stud Depth | Insulation R-Value | Framing Factor |
| $16^{\prime \prime}$ O.C. | $4 \prime$ | R-7 | 0.522 |
|  |  | R-11 | 0.403 |
|  |  | R-13 | 0.362 |
|  |  | R-15 | 0.328 |
|  | $6^{\prime \prime}$ | R-19 | 0.325 |
|  |  | R-21 | 0.300 |
|  |  | R-22 | 0.287 |
|  |  | R-25 | 0.263 |
| 24" O.C. | $4 \prime$ | R-7 | 0.577 |
|  |  | R-11 | 0.458 |
|  |  | R-13 | 0.415 |
|  |  | R-15 | 0.379 |
|  | $6^{\prime \prime}$ | R-19 | 0.375 |
|  |  | R-21 | 0.348 |
|  |  | R-22 | 0.335 |
|  |  | R-25 | 0.308 |

### 9.3.2 Standard Air Film R-Values

| Air Films ${ }^{[1]}$ |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Wall | Roof |  | Floor |
|  |  | Flat ${ }^{[2]}$ | $45^{\circ}$ Angle ${ }^{[3]}$ |  |
| Inside | 0.68 | 0.61 | 0.62 | 0.92 |
| Outside | 0.17 | 0.17 | 0.17 | 0.17 |
| Air Spaces ${ }^{[4]}$ |  |  |  |  |
| 0.5 inch | 0.77 | 0.73 | 0.86 | 0.77 |
| 0.75 inch | 0.84 | 0.75 | 0.81 | 0.85 |
| 1.5 inch | 0.87 | 0.77 | 0.80 | 0.94 |
| 3.5 inch $^{[5]}$ | 0.85 | 0.80 | 0.82 | 1.00 |

NOTE: Values from ASHRAE Handbook of Fundamentals, 1993 edition, Chapter 22, Tables $1 \& 2$.
${ }^{[1]}$ Assumes a nonreflective surface emittance of 0.90 and winter heat flow direction.
${ }^{[2]}$ Use the "Flat" roof $R$-values for roof angles between horizontal and 22 degrees.
${ }^{[3]}$ Use the 45 degree roof $R$-values for roof angles between 23 and 60 degrees.
${ }^{[4]}$ Assumes mean temperature of 90 degrees Fahrenheit, temperature difference of 10 degrees Fahrenheit, surface emittance of 0.82 , and winter heat flow direction.
${ }^{[5]}$ Use these $R$-values for air spaces greater than or equal to 3.5 inches, such as attics.
Source: Berkeley Solar Group, Concrete Masonry Association of California and Nevada.

### 9.4.0 Framed Wall Assemblies—U-factors for Size/Spacing of Wood-Metal Studs

| Framing Type and Spacing | Framing Cavity R-Value | Insulated Sheathing R-Value | Wood Wall U-Factor | Metal Wall U-Factor |
| :---: | :---: | :---: | :---: | :---: |
| $2 \times 4$ @ 16" O.C | 11 (compressed) | 0 | 0.098 | 0.202 |
|  |  | 4 | 0.068 | 0.112 |
|  |  | 5 | 0.064 | 0.101 |
|  |  | 7 | 0.056 | 0.084 |
|  |  | 8.7 | 0.051 | 0.073 |
|  | 13 | 0 | 0.088 | 0.195 |
|  |  | 4 | 0.063 | 0.109 |
|  |  | 5 | 0.059 | 0.099 |
|  |  | 7 | 0.052 | 0.082 |
|  |  | 8.7 | 0.048 | 0.072 |
|  | 15 | 0 | 0.081 | 0.189 |
|  |  | 4 | 0.059 | 0.108 |
|  |  | 5 | 0.055 | 0.097 |
|  |  | 7 | 0.049 | 0.077 |
|  |  | 8.7 | 0.045 | 0.071 |
| $2 \times 4$ @ 24" O.C | 11 | 0 | 0.094 | 0.173 |
|  |  | 4 | 0.066 | 0.102 |
|  |  | 5 | 0.062 | 0.093 |
|  |  | 7 | 0.055 | 0.078 |
|  |  | 8.7 | 0.050 | 0.069 |
|  | 13 | 0 | 0.085 | 0.165 |
|  |  | 4 | 0.061 | 0.099 |
|  |  | 5 | 0.057 | 0.090 |
|  |  | 7 | 0.051 | 0.077 |
|  |  | 8.7 | 0.047 | 0.068 |
|  | 15 | 0 | 0.077 | 0.158 |
|  |  | 4 | 0.056 | 0.097 |
|  |  | 5 | 0.053 | 0.088 |
|  |  | 7 | 0.047 | 0.071 |
|  |  | 8.7 | 0.044 | 0.067 |
| $2 \times 6$ @ 16" O.C. | 19 (compressed) | 0 | 0.065 | 0.158 |
|  |  | 4 | 0.058 | 0.098 |
|  |  | 5 | 0.048 | 0.089 |
|  |  | 7 | 0.043 | 0.075 |
|  |  | 8.7 | 0.040 | 0.067 |
|  | 21 | 0 | 0.059 | 0.157 |
|  |  | 4 | 0.046 | 0.096 |
|  |  | 5 | 0.044 | 0.088 |
|  |  | 7 | 0.041 | 0.075 |
|  |  | 8.7 | 0.037 | 0.066 |
|  | 22 (compressed) | 0 | 0.062 | 0.158 |
|  |  | 4 | 0.048 | 0.097 |
|  |  | 5 | 0.045 | 0.088 |
|  |  | 7 | 0.041 | 0.075 |
|  |  | 8.7 | 0.038 | 0.067 |


| Framing Type and Spacing | Framing Cavity R-Value | Insulated Sheathing R-Value | Wood Wall U-Factor | Metal Wall U-Factor |
| :---: | :---: | :---: | :---: | :---: |
| $2 \times 6$ @ 24" O.C. | 19 (compressed) | 0 | 0.062 | 0.135 |
|  |  | 4 | 0.048 | 0.088 |
|  |  | 5 | 0.045 | 0.081 |
|  |  | 7 | 0.042 | 0.070 |
|  |  | 8.7 | 0.039 | 0.062 |
|  | 21 | 0 | 0.056 | 0.130 |
|  |  | 4 | 0.044 | 0.086 |
|  |  | 5 | 0.042 | 0.079 |
|  |  | 7 | 0.039 | 0.068 |
|  |  | 8.7 | 0.036 | 0.061 |
|  | 22 (compressed) | 0 | 0.058 | 0.132 |
|  |  | 4 | 0.046 | 0.086 |
|  |  | 5 | 0.043 | 0.079 |
|  |  | 7 | 0.040 | 0.068 |
|  |  | 8.7 | 0.037 | 0.061 |
| $2 \times 8$ @ 16" O.C. | 19 | 0 | 0.059 | 0.145 |
|  |  | 4 | 0.047 | 0.092 |
|  |  | 5 | 0.044 | 0.084 |
|  |  | 7 | 0.041 | 0.072 |
|  |  | 8.7 | 0.038 | 0.064 |
|  | 22 | 0 | 0.054 | 0.140 |
|  |  | 4 | 0.043 | 0.090 |
|  |  | 5 | 0.041 | 0.082 |
|  |  | 7 | 0.038 | 0.071 |
|  |  | 8.7 | 0.035 | 0.063 |
|  | 25 | 0 | 0.050 | 0.136 |
|  |  | 4 | 0.040 | 0.088 |
|  |  | 5 | 0.038 | 0.081 |
|  |  | 7 | 0.035 | 0.070 |
|  |  | 8.7 | 0.033 | 0.062 |
|  | 30 (compressed) | 0 | 0.048 | 0.135 |
|  |  | 4 | 0.039 | 0.088 |
|  |  | 5 | 0.037 | 0.081 |
|  |  | 7 | 0.035 | 0.070 |
|  |  | 8.7 | 0.032 | 0.062 |
| $2 \times 8$ @ $24^{\prime \prime}$ O.C. | 19 | 0 | 0.056 | 0.122 |
|  |  | 4 | 0.045 | 0.082 |
|  |  | 5 | 0.043 | 0.076 |
|  |  | 7 | 0.040 | 0.066 |
|  |  | 8.7 | 0.037 | 0.059 |
|  | 22 | 0 | 0.051 | 0.117 |
|  |  | 4 | 0.041 | 0.080 |
|  |  | 5 | 0.040 | 0.074 |
|  |  | 7 | 0.036 | 0.064 |
|  |  | 8.7 | 0.034 | 0.058 |


| Framing Type and Spacing | Framing Cavity R-Value | Insulated Sheathing R-Value | Wood Wall U-Factor | Metal Wall <br> U-Factor |
| :---: | :---: | :---: | :---: | :---: |
|  | 25 | 0 | 0.047 | 0.113 |
|  |  | 4 | 0.038 | 0.078 |
|  |  | 5 | 0.037 | 0.072 |
|  |  | 7 | 0.034 | 0.063 |
|  |  | 8.7 | 0.032 | 0.057 |
|  | 30 (compressed) | 0 | 0.046 | 0.112 |
|  |  | 4 | 0.037 | 0.077 |
|  |  | 5 | 0.036 | 0.072 |
|  |  | 7 | 0.034 | 0.063 |
|  |  | 8.7 | 0.031 | 0.057 |
| $2 \times 10$ @ 16" O.C. | 30 | 0 | 0.041 | 0.120 |
|  |  | 4 | 0.035 | 0.081 |
|  |  | 5 | 0.033 | 0.075 |
|  |  | 7 | 0.031 | 0.065 |
|  |  | 8.7 | 0.029 | 0.059 |
|  | 38 (compressed) | 0 | 0.040 | 0.199 |
|  |  | 4 | 0.033 | 0.080 |
|  |  | 5 | 0.032 | 0.074 |
|  |  | 7 | 0.030 | 0.065 |
|  |  | 8.7 | 0.028 | 0.058 |
| $2 \times 10$ @ $24^{\prime \prime}$ O.C | 30 (compressed) | 0 | 0.039 | 0.099 |
|  |  | 4 | 0.033 | 0.071 |
|  |  | 5 | 0.032 | 0.066 |
|  |  | 7 | 0.038 | 0.058 |
|  |  | 8.7 | 0.028 | 0.053 |
|  | 38 | 0 | 0.038 | 0.097 |
|  |  | 4 | 0.032 | 0.070 |
|  |  | 5 | 0.031 | 0.066 |
|  |  | 7 | 0.029 | 0.058 |
|  |  | 8.7 | 0.027 | 0.053 |

Source: State of California.

| Framing Type and Spacing | Framing Cavity R-Value | Insulated Sheathing R-Value | Wood Wall U-Factor | Metal Wall U-Factor |
| :---: | :---: | :---: | :---: | :---: |
| $2 \times 4$ @ 16" O.C. | 11 (compressed) | 0 | 0.098 | 0.202 |
|  |  | 4 | 0.068 | 0.122 |
|  |  | 5 | 0.064 | 0.101 |
|  |  | 7 | 0.056 | 0.084 |
|  |  | 8.7 | 0.051 | 0.073 |
|  | 13 | 0 | 0.088 | 0.195 |
|  |  | 4 | 0.063 | 0.109 |
|  |  | 5 | 0.059 | 0.099 |
|  |  | 7 | 0.052 | 0.082 |
|  |  | 8.7 | 0.048 | 0.072 |


| Framing Type and Spacing | Framing Cavity R-Value | Insulated Sheathing R-Value | Wood Wall U-Factor | Metal Wall U-Factor |
| :---: | :---: | :---: | :---: | :---: |
|  | 15 | 0 | 0.081 | 0.189 |
|  |  | 4 | 0.059 | 0.108 |
|  |  | 5 | 0.055 | 0.097 |
|  |  | 7 | 0.049 | 0.077 |
|  |  | 8.7 | 0.045 | 0.071 |
| $2 \times 4$ @ $24{ }^{\prime \prime}$ O.C. | 11 | 0 | 0.094 | 0.173 |
|  |  | 4 | 0.066 | 0.102 |
|  |  | 5 | 0.062 | 0.081 |
|  |  | 7 | 0.055 | 0.078 |
|  |  | 8.7 | 0.050 | 0.069 |
|  | 13 | 0 | 0.085 | 0.165 |
|  |  | 4 | 0.061 | 0.099 |
|  |  | 5 | 0.057 | 0.090 |
|  |  | 7 | 0.051 | 0.077 |
|  |  | 8.7 | 0.047 | 0.068 |
|  | 15 | 0 | 0.077 | 0.158 |
|  |  | 4 | 0.056 | 0.097 |
|  |  | 5 | 0.053 | 0.088 |
|  |  | 7 | 0.047 | 0.071 |
|  |  | 8.7 | 0.044 | 0.067 |
| $2 \times 6$ @ 16" O.C. | 19 (compressed) | 0 | 0.065 | 0.158 |
|  |  | 4 | 0.058 | 0.098 |
|  |  | 5 | 0.048 | 0.089 |
|  |  | 7 | 0.043 | 0.075 |
|  |  | 8.7 | 0.040 | 0.067 |
|  | 21 | 0 | 0.059 | 0.157 |
|  |  | 4 | 0.046 | 0.096 |
|  |  | 5 | 0.044 | 0.088 |
|  |  | 7 | 0.041 | 0.075 |
|  |  | 8.7 | 0.037 | 0.066 |
|  | 22 (compressed) | 0 | 0.062 | 0.158 |
|  |  | 4 | 0.048 | 0.097 |
|  |  | 5 | 0.045 | 0.088 |
|  |  | 7 | 0.041 | 0.075 |
|  |  | 8.7 | 0.038 | 0.067 |
| $2 \times 6$ @ $24{ }^{\prime \prime}$ O.C. | 19 (compressed) | 0 | 0.062 | 0.135 |
|  |  | 4 | 0.048 | 0.088 |
|  |  | 5 | 0.045 | 0.081 |
|  |  | 7 | 0.042 | 0.070 |
|  |  | 8.7 | 0.039 | 0.062 |
|  | 21 | 0 | 0.056 | 0.130 |
|  |  | 4 | 0.044 | 0.086 |
|  |  | 5 | 0.042 | 0.079 |
|  |  | 7 | 0.039 | 0.068 |
|  |  | 8.7 | 0.036 | 0.061 |
|  | 22 (compressed) | 0 | 0.058 | 0.132 |
|  |  | 4 | 0.046 | 0.086 |
|  |  | 5 | 0.043 | 0.079 |
|  |  | 7 | 0.040 | 0.068 |
|  |  | 8.7 | 0.037 | 0.061 |

Source: State of California.

# 9.5.0 Acoustics 101-Reflection, Absorption, Isolation-the Methods by Which Sound Can be Identified, Measured, and Controlled 

## Reflections



Reflected sound strikes a surface or several surfaces before reaching the receiver. These reflections can have unwanted or even disastrous consequences. Although reverberation is due to continued multiple reflections, controlling the reverberation time in a space does not ensure that the space will be free from problems from reflections.

Reflective corners or peaked ceilings can create a "megaphone" effect, potentially causing annoying reflections and loud spaces. Reflective parallel surfaces lend themselves to a unique acoustical problem called standing waves, creating a "fluttering" of sound between the two surfaces.

Reflections can be attributed to the shape of the space as well as the material on the surfaces. Domes and concave surfaces cause reflections to be focused rather than dispersed, which can cause annoying sound reflections. Absorptive surface treatments can help to eliminate both reverberation and reflection problems.

## Noise Reduction Coefficient (NRC)

The Noise Reduction Coefficient (NRC) is a single-number index for rating how absorptive a particular material is. Although the standard is often abused, it is simply the average of the midfrequency sound absorption coefficients ( $250,500,1000$, and 2000 Hertz rounded to the nearest $5 \%$ ). The NRC gives no information as to how absorptive a material is in the low and high frequencies, nor does it have anything to do with the material's barrier effect (STC).

## Sound Transmission Class (STC):



The Sound Transmission Class (STC) is a single-number rating of a material's or assembly's barrier effect. Higher STC values are more efficient in reducing sound transmission. For example, loud speech can be understood fairly well through an STC 30 wall but should not be audible through an STC 60 wall. The rating assesses the airborne sound transmission performance at a surface before reaching the receiver. These reflections can
have unwanted or even disastrous consequences. Although reverberation is due to continued multiple reflections, controlling the reverberation time in a space does not ensure the space will be free from problems from reflections.

Reflective corners or peaked ceilings can create a "megaphone" effect, potentially causing annoying reflections and loud spaces. Reflective parallel surfaces lend themselves to a unique acoustical problem called standing waves, creating a "fluttering" of sound between the two surfaces.

Reflections can be attributed to the shape of the space as well as the material on the surfaces. Domes and concave surfaces cause reflections to be focused rather than dispersed, which can cause annoying sound reflections. Absorptive surface treatments can help to eliminate both reverberation and reflection problems.

### 9.5.1 Reverberation Time Creating a Buildup of Noise



In an enclosed space, sound source stops emitting energy, and it takes some time for the sound to become inaudible. This prolongation of the sound in the room caused by continued multiple reflections is called reverberation.

Reverberation time plays a crucial role in the quality of music and the ability to understand speech in a given space. When room surfaces are highly reflective, sound continues to reflect or reverberate. The effect of this condition is described as a live space with a long reverberation time. A high reverberation time will cause a buildup of the noise level in a space. The effects of reverberation time on a given space are crucial to musical conditions and understanding speech. It is difficult to choose an optimum reverberation time in a multifunction space, as different uses require different reverberation times. A reverberation time that is optimum for a music program could be disastrous to the intelligibility of the spoken word. Conversely, a reverberation time that is excellent for speech can cause music to sound dry; flat assemblies should provide specific sound transmission class (STC) ratings when separating a core learning space from an adjacent space:

- STC-45 if the adjacent space is a corridor, staircase, office or conference room.
- STC-50 if the adjacent space is another core learning space, speech clinic, health care room or outdoors.
- STC-53 if the adjacent space is a restroom.
- STC-60 if the adjacent space is a music room, mechanical equipment room, cafeteria, gymnasium, or indoor swimming pool.
- Classroom doors should be rated as STC-30 or more, and music room doors as STC-40 or more. Entry doors across a corridor should be staggered to minimize noise transmission.
- STC ratings ranging from 45-60 are outlined for assemblies separating ancillary spaces from adjacent spaces.
- (Note: Open-plan classroom designs will not meet the requirements of this standard.)

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Achieving a specific STC rating depends highly on the materials and the installation methods used. Wall and ceiling assemblies can be specified and detailed to meet a required STC rating. This is the architect or designer's responsibility. However, specifying an STC level is not all that will be required. It is important to note that sound transmission can be strongly affected by sound leakage through penetrations, joints, and over or around the structure.

The number and location of penetrations through the wall, as well as the number and location of electrical outlet, should be considered in the design. In order to meet a specified STC, installation methods become crucial. Placement and installation instructions for the electrical system are given within Annex B in order to limit sound transfer between rooms. For single stud walls, electrical boxes should not be located within the same stud space. For staggered or dual-stud walls, boxes should be separated by at least $24^{\prime \prime}$. If back-to-back electrical boxes cannot be avoided, they should be enclosed in full gypsum board enclosures that do not contact the framing of the other row of studs. In addition, all joints and air gaps should be sealed air tight with caulking or acoustical sealant.

As mentioned previously, background noise is a major concern in learning facilities. STC ratings will help to limit the background

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### 9.5.2 Isolation—Measured by Sound Transmission Class (STC)

The amount of airborne sound blocked from transmitting through a partition is measured in a sound transmission class (STC) rating. A Higher STC Rating will degrade the ability to hear and understand speech. Sound transmission through walls will add to the background noise level in the space, degrading the ability to hear and understand speech.

Single or composite walls, floor-ceiling and roof-ceiling noise levels within a space (depending on the effect of sound transmission on the background noise level). It may be necessary to increase a required STC rating in order to meet a specified background noise level requirement.

Sound transmission problems can be avoided or lessened by good $I$ site selection and good space planning.
Typical, single-stud construction will not meet the required STC ratings. The walls will most likely require staggered or dual-stud construction and/or multiple layers of drywall. (There are also specialty products that can help ensure compliance.) It is also important to note that acoustical ceiling tiles will not prevent sound transmission over the wall. Walls surrounding core learning spaces should extend to the deck of the building structure in order to adequately control sound transmission.

Carefully consider the placement of electrical outlets. Do not place them back-to-back. Again it will be important to work with your electrical engineer in order to specify installation instructions that will limit sound transmission. Specify on your drawings for contractors to seal all joints and penetrations with an acoustical sealant.

Most importantly, do not locate mechanical equipment rooms, restrooms, music rooms, gymnasiums, cafeterias, or any other noisy space adjacent to a class room or core learning space.

### 9.5.3 Impact Insulation Class—IIC—Blocking Noise from Being Transmitted Floor-to-Floor

Impact Insulation Class (IIC) is a rating for the ability of a floor-ceiling assembly to block impact/structureborne noise from transmitting to the space below. A floor-ceiling assembly with a low IIC rating will potentially cause distracting noise in the room below, leading to possible annoyance and problems with communication.

- IIC ratings for floor-ceiling assemblies above core learning spaces should be at least IIC-45 and preferably IIC-50 (measured without carpeting on the floor).
- In new construction, gymnasia, dance studios, or other high floor impact activities shall not be located above core learning spaces.
- In existing facilities IIC-65-70 (depending on the volume of 9.5.3-p. 2 the space below) is recommended if gymnasia, dance studios, or other high floor impact activities are located above core learning spaces.

IIC is a major concern for multistory educational facilities. The floor-ceiling system should be specified and constructed in order to meet the specified IIC rating. Installing carpet on the floor above will help reduce the impact of sounds. It may be necessary to isolate the finished floor from the structural floor or to isolate the ceiling from the floor above. For any vibrating machinery located on the floor above or on the roof structure, rubber pads or spring systems should be installed. As with all requirements in the standard, it is the architect's or designer's responsibility to take the necessary steps in specification and design, but careful construction and installation will be necessary to ensure compliance.

This is only a concern for multistory schools. In most cases, installing carpet on the floor above will dramatically improve the IIC rating. In order to achieve the specified levels, a separate hard lid ceiling assembly could be required. Ideally, this would be completely isolated from the floor structure above. The classroom below may still need an acoustically absorptive ceiling treatment in order to meet the required reverberation time. Working with your mechanical engineer, be sure to specify appropriate vibration dampening measures for mechanical equipment.

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### 9.5.4 More Sound Absorption Factors for Building Materials and Finishes

| Floor Materials | $\mathbf{1 2 5 ~ H z}$ | $\mathbf{2 5 0 ~ H z}$ | $\mathbf{5 0 0} \mathbf{~ H z}$ | $\mathbf{1 0 0 0} \mathbf{~ H z}$ | $\mathbf{2 0 0 0} \mathbf{~ H z}$ | $\mathbf{4 0 0 0} \mathbf{~ H z}$ |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| Concrete or tile | 0.01 | 0.01 | 0.15 | 0.02 | 0.02 | 0.02 |
| Linoleum/vinyl tile on concrete | 0.02 | 0.03 | 0.03 | 0.03 | 0.03 | 0.03 |
| Wood on joists | 0.05 | 0.11 | 0.10 | 0.07 | 0.06 | 0.07 |
| Parquet on concrete | 0.04 | 0.04 | 0.07 | 0.06 | 0.06 | 0.07 |
| Carpet on concrete | 0.02 | 0.06 | 0.14 | 0.37 | 0.60 | 0.65 |
| Carpet on foam | 0.08 | 0.24 | 0.57 | 0.69 | 0.71 | 0.73 |


| Seating Materials | $\mathbf{1 2 5 ~ H z}$ | $\mathbf{2 5 0 ~ H z}$ | $\mathbf{5 0 0} \mathbf{~ H z}$ | $\mathbf{1 0 0 0} \mathbf{~ H z}$ | $\mathbf{2 0 0 0} \mathbf{~ H z}$ | $\mathbf{4 0 0 0} \mathbf{~ H z}$ |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| Fully occupied—fabric upholstered | 0.60 | 0.74 | 0.88 | 0.96 | 0.93 | 0.85 |
| Occupied wooden pews | 0.57 | 0.61 | 0.75 | 0.86 | 0.91 | 0.86 |
| Empty—fabric upholstered | 0.49 | 0.66 | 0.80 | 0.88 | 0.82 | 0.70 |
| Empty metal/wood seats | 0.15 | 0.19 | 0.22 | 0.39 | 0.38 | 0.30 |


| Wail Materials | $\mathbf{1 2 5 ~ H z}$ | $\mathbf{2 5 0 ~ H z}$ | $\mathbf{5 0 0} \mathbf{~ H z}$ | $\mathbf{1 0 0 0} \mathbf{~ H z}$ | $\mathbf{2 0 0 0} \mathbf{~ H z}$ | $\mathbf{4 0 0 0} \mathbf{~ H z}$ |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| Brick: unglazed | 0.03 | 0.03 | 0.03 | 0.04 | 0.05 | 0.07 |
| Brick: unglazed \& painted | 0.01 | 0.01 | 0.02 | 0.02 | 0.02 | 0.03 |
| Concrete block— coarse | 0.36 | 0.44 | 0.31 | 0.29 | 0.39 | 0.25 |
| Concrete block—painted | 0.10 | 0.05 | 0.06 | 0.07 | 0.09 | 0.08 |
| Curtain: 10 oz/sq yd fabric molleton | 0.03 | 0.04 | 0.11 | 0.17 | 0.24 | 0.35 |
| Curtain: 14 oz/sq yd fabric molleton | 0.07 | 0.31 | 0.49 | 0.75 | 0.70 | 0.60 |


| Wail Materials | $\mathbf{1 2 5} \mathbf{~ H z}$ | $\mathbf{2 5 0} \mathbf{~ H z}$ | $\mathbf{5 0 0} \mathbf{~ H z}$ | $\mathbf{1 0 0 0} \mathbf{~ H z}$ | $\mathbf{2 0 0 0} \mathbf{~ H z}$ | $\mathbf{4 0 0 0} \mathbf{~ H z}$ |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| Curtain: 18 oz/sq yd fabric molleton | 0.14 | 0.35 | 0.55 | 0.72 | 0.70 | 0.65 |
| Fiberglass: 2" 703 no airspace | 0.22 | 0.82 | 0.99 | 0.99 | 0.99 | 0.99 |
| Fiberglass: spray 5" | 0.05 | 0.15 | 0.45 | 0.70 | 0.80 | 0.80 |
| Fiberglass: spray 1" | 0.16 | 0.45 | 0.70 | 0.90 | 0.90 | 0.85 |
| Fiberglass: 2" rolls | 0.17 | 0.55 | 0.80 | 0.90 | 0.85 | 0.80 |
| Foam: Sonex 2" | 0.06 | 0.25 | 0.56 | 0.81 | 0.90 | 0.91 |
| Foam: SDG $3^{\prime \prime}$ | 0.24 | 0.58 | 0.67 | 0.91 | 0.96 | 0.99 |
| Foam: SDG 4" | 0.33 | 0.90 | 0.84 | 0.99 | 0.98 | 0.99 |
| Foam: polyur. $1^{\prime \prime}$ | 0.13 | 0.22 | 0.68 | 1.00 | 0.92 | 0.97 |
| Foam: polyur. 1/2" | 0.09 | 0.11 | 0.22 | 0.60 | 0.88 | 0.94 |
| Glass: $1 / 4^{\prime \prime}$ plate large | 0.18 | 0.06 | 0.04 | 0.03 | 0.02 | 0.02 |
| Glass: window | 0.35 | 0.25 | 0.18 | 0.12 | 0.07 | 0.04 |
| Plaster: smooth on tile/brick | 0.013 | 0.015 | 0.02 | 0.03 | 0.04 | 0.05 |
| Plaster: rough on lath | 0.02 | 0.03 | 0.04 | 0.05 | 0.04 | 0.03 |
| Marble/Tile | 0.01 | 0.01 | 0.01 | 0.01 | 0.02 | 0.02 |
| Sheetrock $1 / 2^{\prime \prime} 16^{\prime \prime}$ on center | 0.29 | 0.10 | 0.05 | 0.04 | 0.07 | 0.09 |
| Wood: $3 / 8^{\prime \prime}$ plywood panel | 0.28 | 0.22 | 0.17 | 0.09 | 0.10 | 0.11 |
|  |  |  |  |  |  |  |


| Ceiling Materials | $\mathbf{1 2 5 ~ H z}$ | $\mathbf{2 5 0} \mathbf{~ H z}$ | $\mathbf{5 0 0} \mathbf{~ H z}$ | $\mathbf{1 0 0 0} \mathbf{~ H z}$ | $\mathbf{2 0 0 0} \mathbf{~ H z}$ | $\mathbf{4 0 0 0} \mathbf{~ H z}$ |
| :--- | :--- | :--- | :---: | :---: | :---: | :---: |
| Acoustic Tiles | 0.05 | 0.22 | 0.52 | 0.56 | 0.45 | 0.32 |
| Acoustic Ceiling Tiles | 0.70 | 0.66 | 0.72 | 0.92 | 0.88 | 0.75 |
| Fiberglass: $\mathbf{2}^{\prime \prime} 703$ no airspace | 0.22 | 0.82 | 0.99 | 0.99 | 0.99 | 0.99 |
| Fiberglass: spray 5" | 0.05 | 0.15 | 0.45 | 0.70 | 0.80 | 0.80 |
| Fiberglass: spray 1" | 0.16 | 0.45 | 0.70 | 0.90 | 0.90 | 0.85 |
| Fiberglass: $2^{\prime \prime}$ rolls | 0.17 | 0.55 | 0.80 | 0.90 | 0.85 | 0.80 |
| Wood | 0.15 | 0.11 | 0.10 | 0.07 | 0.06 | 0.07 |
| Foam: Sonex 2" | 0.06 | 0.25 | 0.56 | 0.81 | 0.90 | 0.91 |
| Foam: SDG $3^{\prime \prime}$ | 0.24 | 0.58 | 0.67 | 0.91 | 0.96 | 0.99 |
| Foam: SDG $4^{\prime \prime}$ | 0.33 | 0.90 | 0.84 | 0.99 | 0.98 | 0.99 |
| Foam: polyur. $1^{\prime \prime}$ | 0.13 | 0.22 | 0.68 | 1.00 | 0.92 | 0.97 |
| Foam: polyur. $1 / 2^{\prime \prime}$ | 0.09 | 0.11 | 0.22 | 0.60 | 0.88 | 0.94 |
| Plaster: smooth on tile/brick | 0.013 | 0.015 | 0.02 | 0.03 | 0.04 | 0.05 |
| Plaster: rough on lath | 0.02 | 0.03 | 0.04 | 0.05 | 0.04 | 0.03 |
| Sheetrock $1 / 2^{\prime \prime} 16^{\prime \prime}$ on center | 0.29 | 0.10 | 0.05 | 0.04 | 0.07 | 0.09 |
| Wood: $3 / 8^{\prime \prime}$ plywood panel | 0.28 | 0.22 | 0.17 | 0.09 | 0.10 | 0.11 |
|  |  |  |  |  |  |  |


| Miscellaneous Materials | $\mathbf{1 2 5 ~ H z}$ | $\mathbf{2 5 0 ~ H z}$ | $\mathbf{5 0 0} \mathbf{~ H z}$ | $\mathbf{1 0 0 0} \mathbf{~ H z}$ | $\mathbf{2 0 0 0} \mathbf{~ H z}$ | $\mathbf{4 0 0 0} \mathbf{~ H z}$ |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Water | 0.008 | 0.008 | 0.013 | 0.015 | 0.020 | 0.025 |
| People (adults) | 0.25 | 0.35 | 0.42 | 0.46 | 0.5 | 0.5 |

[^29]
### 9.5.5 Absorption Coefficients for Various Wall and Floor Coverings

| Material | $\mathbf{1 2 8 ~ H z}$ | $\mathbf{2 5 6 ~ H z}$ | $\mathbf{5 1 2 ~ H z}$ | $\mathbf{1 , 0 2 4 ~ H z}$ | $\mathbf{2 , 0 4 8} \mathbf{~ H z}$ | $\mathbf{4 , 0 9 6} \mathbf{~ H z}$ |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Draperies hung straight, in contact with wall, cotton <br> fabric, 10 oz. per square yard | 0.04 | 0.05 | 0.11 | 0.18 | 0.30 | 0.44 |
| The same, velour, 18 oz. per square yard | 0.05 | 0.12 | 0.35 | 0.45 | 0.40 | 0.44 |
| Same as above, hung 4 inches from wall | 0.09 | 0.33 | 0.45 | 0.52 | 0.50 | 0.44 |
| Felt, all hair, contact with wall | 0.13 | 0.41 | 0.56 | 0.69 | 0.65 | 0.49 |
| Rock wool (1 inch) | 0.35 | 0.49 | 0.63 | 0.80 | 0.83 | - |
| Carpet on concrete (0.4 inch) | 0.09 | 0.08 | 0.21 | 0.26 | 0.27 | 0.37 |
| Carpet, on 1/8 inch felt, on concrete (0.4 inch) | 0.11 | 0.14 | 0.37 | 0.43 | 0.27 | 0.27 |
| Concrete, unpainted | 0.010 | 0.012 | 0.016 | 0.019 | 0.023 | 0.035 |
| Wood sheeting, pine (0.8 inch) | 0.10 | 0.11 | 0.10 | 0.08 | 0.08 | 0.11 |
| Brick wall, painted | 0.012 | 0.013 | 0.017 | 0.020 | 0.023 | 0.025 |
| Plaster, lime on wood studs, rough finish (1/2 inch) | 0.039 | 0.056 | 0.061 | 0.089 | 0.054 | 0.070 |

Source: Simon Fraser University, Vancouver, British Columbia, Canada.

### 9.6.0 Checklist for Masking Open Space Systems

## How Masking Systems Work

Masking systems provide ambient background sound that reduces exposure to distracting office noises by emitting a discreet, electronically generated sound through specially installed, unobtrusive speakers. When installed properly, employees won't be aware of the pink noise being generated around them, but they will be able to focus on their work without unwanted sound distractions. Of course, carefully choosing office furniture, wall treatments, and flooring systems will also contribute to a productive work area.

## Checklist of Masking Systems for Open Plans

- Ideally, speakers should be in enclosures located just above suspended ceilings, aimed upward toward hard plenum surfaces. If sound-absorbing insulation is applied to the underside of the structural deck, speakers should be aimed downward (or possibly sideways), or speaker enclosures that reflect sound downward should be used.
- Plenums should have uncomplicated air duct layouts and smooth sound-reflecting structural surfaces to allow wider spacings between loudspeakers.
- Coverage should include adjacent areas (or zones) so that occupants moving about the building will not notice the masking system.
- Masking should not exceed a sound level of 45 to 50 dBA because occupants tend to raise voices to compensate, thus defeating the intended masking effects. Occupants may also begin to complain about the sound level when it exceeds 50 dBA .
- To reduce the likelihood that occupants will notice background masking, consider installation procedures that initially operate the system at low sound levels. Then gradually increase the level by about 1 dB each day until the desired masking level is achieved in a week to 10 days or longer.
- A well-designed masking system deliberately garbles the sound it produces and therefore should not be used for paging and routine office functions.
- Provisions can be made to reduce masking noise levels during off-hours to enhance the ability of security personnel to hear unusual sounds.
- Be sure to consider the consequences of background masking on the usability of open plans by hearingimpaired persons. For example, when background noise levels exceed 30 dBA , hearing-impaired persons (even when using hearing aids) have far more difficulty understanding speech than do normal-hearing persons.

In addition to avoiding excessive noise levels, background noise from electronic sound masking systems in open-office plans should have a neutral tonal quality. This may be facilitated by designing the system to simulate familiar building sounds, such as the air flow at diffusers and registers of HVAC systems. The electronically produced sound levels in the finished room should be no higher than necessary to mask unwanted intruding speech and so that pronounced hisses are avoided. The sound level of the masking system should be neither too high nor too low, and the snectrum should roll off at the high end of the frequency range.

### 9.6.1 Use of Loudspeakers and Formula for Installation Spacing

In open plans, loudspeakers can usually be hidden in plenums above suspended ceilings. This strategy can achieve uniform masking sound throughout the room. Be careful when designing this kind of installation because openings for return or supply air in ceilings and luminaries can be noticeable sound leaks, which make it difficult to achieve uniform masking sound.

For preliminary planning, loudspeaker spacing, S, can be found by:

$$
\mathrm{S}=1.4(2 \mathrm{D}+\mathrm{H}-4)
$$

where $\mathrm{S}=$ spacing between loudspeakers ( ft )
$\mathrm{D}=$ plenum depths (ft)
$\mathrm{H}=$ floor-to-ceiling height (ft)
Closer spacings may be required when spray-on, sound-absorbing fire protection, or insulation is applied to the underside of structural decks, or when complicated air duct layouts or deep structural members obstruct plenums.

By permission: Acoustics.com

### 9.7.0 Decibel Levels of Some Common Sounds

|  |  |  |  |
| :--- | ---: | :--- | ---: |
| Threshold of hearing | 0 dB | Motorcycle (30 feet) | 88 dB |
| Rustling leaves | 20 dB | Food blender (3 feet) | 90 dB |
| Quiet whisper (3 feet) | 30 dB | Subway (inside) | 94 dB |
| Quiet home | 40 dB | Diesel truck (30 feet) | 100 dB |
| Quiet street | 50 dB | Power mower (3 feet) | 107 dB |
| Normal conversation | 60 dB | Pneumatic riveter (3 feet) | 115 dB |
| Inside car | 70 dB | Chainsaw (3 feet) | 117 dB |
| Loud singing (3 feet) | 75 dB | Amplified Rock and Roll (6 feet) | 120 dB |
| Automobile (25 feet) | 80 dB | Jet plane (100 feet) | 130 dB |


| Typical Average Decibel Levels (dBA) of Some Common Sounds |  |
| :--- | :--- |
| STC | What Can Be Heard |
| 25 | Normal speech can be understood quite easily and distinctly through wall |
| 30 | Loud speech can be understood fairly well, normal speech heard but not understood |
| 35 | Loud speech audible but not intelligible |
| 40 | Onset of "privacy" |
| 42 | Loud speech audible as a murmur |
| 45 | Loud speech not audible; 90\% of statistical population not annoyed |
| 50 | Very loud sounds such as musical instruments or a stereo can be faintly heard; 99\% of |
| $60+\quad$ population not annoyed. |  |

Source: U.S. Department of Energy.

### 9.7.1 ANSI Recommended Levels for Various Types of Occupancy

| Occupancy | NCB Curve |
| :--- | :---: |
| Broadcast Studios (distant microphone pickup used) | 10 |
| Concert halls, opera houses, and recital halls (listening to faint musical sounds) | $10-15$ |
| Large auditoriums, large drama theatres, and large churches (for very good <br> speech articulation) | $15-20$ |
| TV and broadcast studios (close microphone pickup only) | $15-25$ |
| Private Residences: | $25-30$ |
| Bedrooms  <br> Apartments  <br> Family rooms and living rooms $28-38$ <br> Schools: $28-38$ <br> Lecture and classrooms $25-30$ <br> Open-plan classrooms $33-37$ |  |


| Occupancy | NCB Curve |
| :---: | :---: |
| Hotels/Motels: |  |
| Individual rooms or suites | 28-33 |
| Meeting/banquet rooms | 25-35 |
| Halls, corridors, lobbies | 38-43 |
| Service support areas | 38-48 |
| Office Buildings: |  |
| Executive offices | 25-30 |
| Conference rooms (large) | 25-30 |
| Conference rooms (small) and private offices | 30-35 |
| General secretarial areas | 38-43 |
| Open-plan areas | 35-40 |
| Business machines/computers | 38-43 |
| Public circulation | 38-48 |
| Hospitals and clinics: |  |
| Private rooms | 25-30 |
| Wards | 30-35 |
| Operating rooms | 25-30 |
| Laboratories | 33-43 |
| Corridors | 33-43 |
| Public areas | 38-43 |
| Small auditoriums | 25-30 |
| Movie theatres | 27-37 |
| Churches | 30-35 |
| Courtrooms | 33-37 |
| Libraries | 33-37 |
| Restaurants | 38-43 |
| Light maintenance shops, industrial plant control rooms, kitchens, and laundries | 43-53 |
| Shops, garages | 50-60 |

Source- ANSI/Public documents.

### 9.7.2 Decibel Comparison Chart of Environmental Noises

Here are some interesting numbers, collected from a variety of sources, that help one to understand the volume levels of various sources and how they can affect our hearing.

## Environmental Noise

| Weakest sound heard | 0 dB |
| :--- | ---: |
| Whisper quiet library | 30 dB |
| Normal conversation $\left(3-5^{\prime}\right)$ | $60-70 \mathrm{~dB}$ |
| Telephone dial tone | 80 dB |
| City traffic (inside car) | 85 dB |


| Train whistle at 500', Truck Traffic | 90 dB |
| :--- | ---: |
| Subway train at 200' | 95 dB |
| Level at which sustained exposure may result in hearing loss | $90-95 \mathrm{~dB}$ |
| Power mower at 3' | 107 dB |
| Snowmobile, motorcycle | 100 dB |
| Power saw at 3' | 110 dB |
| Sandblasting, loud rock concert | 115 dB |
| Pain begins | 125 dB |
| Pneumatic riveter at 4' | 125 dB |
| Even short-term exposure can cause permanent damage- | 140 dB |
| Loudest recommended exposure WITH hearing protection | 140 dB |
| Jet engine at 100', Gun Blast | 180 dB |
| Death of hearing tissue | 194 dB |
| Loudest sound possible |  |

### 9.7.3 OSHA Daily Permissible Noise-Level Exposure

OSHA Daily Permissible Noise Level Exposure

| Hours per day | Sound level |
| :--- | ---: |
| 8 | 90 dB |
| 6 | 92 dB |
| 4 | 95 dB |
| 3 | 97 dB |
| 2 | 100 dB |
| 1.5 | 102 dB |
| 1 | 105 dB |
| .5 | 110 dB |
| .25 or less | 115 dB |

By permission: Galen Carol Audio, San Antonio, TX

### 9.7.4 Perceptions of Increases in Noise Levels

## Perceptions of Increases in Decibel Level

Imperceptible change 1 dB
Barely perceptible change 3 dB

Clearly noticeable change 5 dB
About twice as loud 10 dB
About four times as loud 20 dB

### 9.7.5 Sound Levels of Music

## Sound Levels of Music

| Normal piano practice | $60-70 \mathrm{~dB}$ |
| :--- | :---: |
| Fortissimo Singer, $3^{\prime}$ | 70 dB |
| Chamber music, small auditorium | $75-85 \mathrm{~dB}$ |
| Piano Fortissimo | $84-103 \mathrm{~dB}$ |
| Violin | $82-92 \mathrm{~dB}$ |
| Cello | $85-111 \mathrm{~dB}$ |
| Oboe | $95-112 \mathrm{~dB}$ |
| Flute | $92-103 \mathrm{~dB}$ |
| Piccolo | $90-106 \mathrm{~dB}$ |
| Clarinet | $85-114 \mathrm{~dB}$ |
| French horn | $90-106 \mathrm{~dB}$ |
| Trombone | $85-114 \mathrm{~dB}$ |
| Tympani \& bass drum | 106 dB |
| Walkman on 5/10 | 94 dB |
| Symphonic music peak | $120-137 \mathrm{~dB}$ |
| Amplifier rock, 4-6' | 120 dB |
| Rock music peak | 150 dB |
| NOTES: |  |
| - One-third of the total power of a 75-piece orchestra comes from the bass drum. |  |
| - High-frequency sounds of 2-4000 Hz are the most damaging. The uppermost octave of the piccolo is 2048-4096 Hz. |  |
| - Aging causes gradual hearing loss, mostly in the high frequencies. |  |
| - Speech reception is not seriously impaired until there is about 30 dB loss; by that time damage is severe. |  |


| Sound | Noise <br> Level (dBA) | Effect |
| :--- | :--- | :--- |
| Jet Engines (Near) | 140 |  |
| Shotgun Firing | 130 |  |
| Jet Takeoff (100-200 Ft.) | 130 |  |
| Rock Concert (Varies) | $110-140$ | Threshold of pain (125 dB) |
| Oxygen Torch | 121 |  |
| Discotheque/Boom Box | 120 | Threshold of sensation (120 dB) |
| Thunderclap (Near) | 120 |  |
| Stereo (Over 100 Watts) | $110-125$ |  |
| Symphony Orchestra | 110 | Regular exposure of more than 1 minute risks permanent hearing |
| Power Saw (Chain Saw) | 110 | loss (over 100 dB) |
| Jackhammer | 110 |  |
| Snowmobile | 105 |  |
| Jet Fly-over (1000 Ft.) | 103 |  |


| Sound | Noise <br> Level (dBA) | Effect |
| :---: | :---: | :---: |
| Electric Furnace Area | 100 | No more than 15 minutes of unprotected exposure recommended ( $90-100 \mathrm{~dB}$ ) |
| Garbage Truck/Cement Mixer | 100 |  |
| Farm Tractor | 98 |  |
| Newspaper Press | 97 |  |
| Subway, Motorcycle (25 Ft) | 88 | Very annoying |
| Lawnmower, Food Blender | 85-90 | Level at which hearing damage (8 hrs.) begins (85dB) |
| Recreational Vehicles, TV | 70-90 |  |
| Diesel Truck (40 Mph, 50 Ft .) | 84 |  |
| Average City Traffic Noise | 80 | Annoying; interferes with conversation; constant exposure may cause damage |
| Garbage Disposal | 80 |  |
| Washing Machine | 78 |  |
| Dishwasher | 75 |  |
| Vacuum Cleaner | 70 | Intrusive; interferes with telephone conversation |
| Hair Dryer | 70 |  |
| Normal Conversation | 50-65 |  |
| Quiet Office | 50-60 | Comfortable (under 60 dB ) |
| Refrigerator Humming | 40 |  |
| Whisper | 30 | Very quiet |
| Broadcasting Studio | 30 |  |
| Rustling Leaves | 20 | Just audible |
| Normal Breathing | 10 |  |
|  | 0 | Threshold of normal hearing (1000-4000 Hz) |
|  |  |  |
| $2 \times 4$ studs, $5 / 8^{\prime \prime}$ gyp (2 layers total), Batt insulation | 56-59 |  |

By permission: Galen Carol Audio, San Antonio, TX
3. Metal stud walls perform better than wood stud walls.
(NOTE: This only applies to single-stud assemblies. For double-stud assemblies, there is virtually no difference.)

| Description | Estimated STC Rating | Wall Assembly |
| :---: | :---: | :---: |
| $2 \times 4$ stud, 5/8' ${ }^{\prime \prime}$ gyp (2 layers total), Batt insulation | 34-39 |  |
| $35 / 8^{\prime \prime}$ metal studs, 5/8" gyp (2 layers total), Batt insulation | 43-44 |  |

4. Resilient channel can improve the STC rating of an assembly.
(NOTE: These ratings are based on laboratory tests. Because of the special care required when installing resilient channels, actual results could be substantially lower.)

| Description | Estimated STC Rating | Wall Assembly |
| :--- | :--- | :--- |
| $2 \times 4$ stud, $5 / 8^{\prime \prime}$ gyp (2 layers total), Batt insulation | $34-39$ |  |
|  |  |  |
| insulation |  |  |

5. Adding additional layers of drywall can improve the STC rating of an assembly.

| Description | Estimated STC Rating | Wall Assembly |
| :--- | :---: | :---: |
| $2 \times 4$ stud, $5 / 8^{\prime \prime}$ gyp (2 layers total), Batt insulation | $34-39$ |  |

### 9.8.0. Sound Transmission Coefficient (STC) of Various Types of Insulated Partitions

1. Insulation will noticeably improve the STC rating of an assembly.

| Description | Estimated STC Rating | Wall Assembly |
| :--- | :---: | :---: | :---: |
| $35 / 8^{\prime \prime}$ metal studs, $5 / 8^{\prime \prime}$ gyp (2 layers total), No insulation | $38-40$ |  |
| $35 / 8^{\prime \prime}$ metal studs, $5 / 8^{\prime \prime}$ gyp (2 layers total), Batt insulation | $43-44$ |  |

2. Staggered or double-stud walls are higher rated than single-stud walls.

3. Metal stud walls perform better than wood stud walls.
(NOTE: This only applies to single-stud assemblies. For double-stud assemblies, there is virtually no difference.)

4. Resilient channel can improve the STC rating of an assembly.
(NOTE: These ratings are based on laboratory tests. Because of the special care required when installing resilient channels, actual results could be substantially lower.)

| Description | Estimated STC Rating | Wall Assembly |
| :--- | :--- | :--- |
| $2 \times 4$ stud, $5 / 8^{\prime \prime}$ gyp (2 layers total), Batt insulation | $34-39$ |  |
| $2 \times 4$ stud, $5 / 8^{\prime \prime}$ gyp (2 layers total), Resilient Channel, Batt |  |  |
| insulation |  |  |

5. Adding additional layers of drywall can improve the STC rating of an assembly.

| Description | Estimated STC Rating |
| :--- | :---: |
| $2 \times 4$ stud, $5 / 8^{\prime \prime}$ gyp (2 layers total), Batt insulation | $34-39$ |
| $5 / 8^{\prime \prime}$ metal studs, $5 / 8^{\prime \prime}$ gyp (3 layers total), Batt insulation | Nall Assembly |
| $2 \times 4$ stud, $5 / 8^{\prime \prime}$ gyp (4 layers total), Batt insulation |  |

6. Drywall between double studs can dramatically reduce the STC rating of an assembly.

| Description | Estimated STC Rating |
| :--- | :--- |
| $2 \times 4$ studs, $5 / 8^{\prime \prime}$ gyp (4 layers total), Batt insulation | $44-45$ |

$2 \times 4$ studs, $5 / 8^{\prime \prime}$ gyp (2 layers total), Batt insulation

## STC RATINGS FOR MASONRY WALLS

STC ratings for masonry/CMU walls are based on weight of the block, on whether or not the cells are filled, and on what material it is filled with.

Estimated STC Ratings for CMU Walls

| Wall Thickness, in. | Hollow Unite |  | Grout Filled |  | Sand Filled |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Weight | STC | Weight | STC | Weight | STC |
| 4 | 20 | 44 | 38 | 47 | 32 | 46 |
| 6 | 32 | 46 | 63 | 51 | 50 | 49 |
| 8 | 42 | 48 | 86 | 55 | 68 | 52 |
| 10 | 53 | 50 | 109 | 60 | 86 | 55 |

The STC rating of a CMU wall can be estimated based on its weight using the following formula:
$\mathrm{STC}=0.18 \mathrm{~W}+40$
where $\mathrm{W}=$ pounds per square foot (psf)

### 9.8.1 Testing for STC of Residential Carpet over Joist and Wood Subfloor

## Observations: B-2

1. Impact Noise Ratings were all lower than those found in Test Series B-1. Various cushion/carpet combinations yielded substantially lower INR (Impact Noise Ratings) values for wood joist floors than for concrete floors.
2. Test Series B-1 has already shown that as pile weight increases, the INR increases. The assumption is also true with wood joist construction, but probably with lower relative ratings.

## Testing for Sound Transmission Class (STC) of a Residential Carpet Installed with Cushion over a Joist and Plywood Subfloor

Tested Materials: Carpet and cushion-25 ounces per square yard $100 \%$ nylon cut pile residential carpet installed over bonded polyurethane bonded cushion $1 / 2$ inch thickness with $6.0 \mathrm{lb} / \mathrm{ft}^{3}$ density.

Test Floor: Open Joist 2000 system, 13 inches deep installed, 24 inches on center. Subfloor 5/8 inch thick T\&G plywood. Bridging of continuous $2 \times 4$ inch wood nailed to bottom chord and the sides of the diagonals with 2 -inch long nails. Cellulose insulation with density of $1.6 \mathrm{pcf}, 51 / 2$ inches thick was used. Resilient channels of 24 -gauge galvanized steel placed 16 inches on center and attached to bottom chords with screws. Ceiling of gypsum board of $5 / 8$ inches thick. Sheets fastened to resilient channels by means of $11 / 2$ inch screws, spaced 6 inches on center. Joints taped and finished with two layers of compound.

Procedure: Sound transmission loss was determined per ASTM E90-87, Standard Test Method for Laboratory Measurement of Sound Transmission Loss in Building Partitions, by mounting and perimeter sealing the test specimen (carpet over cushion) as a partition between two reverberation rooms. Sound is introduced in one of the rooms (the source room), and measurements are made of noise reduction between the source room and the receiving room. The rooms are so arranged and constructed that the only significant sound transmission between them is through the test specimen.

Results of Test: Sound Transmission Classification was found to be 49 per ASTM E413-94, Classification for Rating Sound Insulation.

## Overall Conclusions about Carpet and Sound

Carpet is highly effective in controlling noise in buildings by absorbing airborne sound, reducing surface noise generation, and reducing the impact of sound transmission to rooms below. Properly specified carpet/cushion combinations have proven to handle the vast majority of sound absorption requirements in architectural spaces. Specifying for critical areas such as theaters, broadcast studios, and open-plan office areas may require full details of impact insulation properties and noise absorption characteristics.

Source: Carpet \& Rug Institute, Dalton, GA.

### 9.8.2 STC Ratings for Masonry Walls

STC ratings for masonry/CMU walls are based on weight of the block and on whether or not the cells are filled and what material it is filled with.

| Wall Thickness, in. | Hollow Units |  | Grout Filled |  | Sand Filled |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Weight | STC | Weight | STC | Weight | STC |
| 4 | 20 | 44 | 38 | 47 | 32 | 46 |
| 6 | 32 | 46 | 63 | 51 | 50 | 49 |
| 8 | 42 | 48 | 86 | 55 | 68 | 52 |
| 10 | 53 | 50 | 109 | 60 | 86 | 55 |

The STC rating of a CMU wall can be estimated based on its weight using the following formula:
$\mathrm{STC}=0.18 \mathrm{~W}+40$
where $\mathrm{W}=$ pounds per square foot (psf)
By permission: racoustics.com

### 9.8.3 How Insulation, Staggered Studs, Resillient Channels, and Added Layers of Drywall Can Affect STC Ratings

Below are the STC ratings of various wall assemblies, each presented to help illustrate concepts, improvements, and rules of thumb. The estimated ratings are based on laboratory test results from various compendiums of STC ratings. It is recommended that you consult a professional acoustician for more detailed information or to analyze the specifics of your project/assembly.

To view different wall assemblies, click on each point below that may apply to your project.

1. Insulation will noticeably improve the STC rating of an assembly.
2. Staggered or double-stud walls are higher rated than single-stud walls.
3. Metal stud walls perform better than wood stud walls.
4. Resilient channels can improve the STC rating of an assembly.
5. Adding additional layers of drywall can improve the STC rating of an assembly.
6. Drywall between double studs can dramatically reduce the STC rating of an assembly.
7. Insulation will noticeably improve the STC rating of an assembly.

| Description | Estimated STC Rating | Wall Assembly |
| :---: | :---: | :---: |
| $35 / 8^{\prime \prime}$ metal studs, 5/8" ${ }^{\prime \prime}$ gyp (2 layers total), No insulation | 38-40 |  |
| $35 / 8^{\prime \prime}$ metal studs, 5/8" gyp (2 layers total), Batt insulation | 43-44 | $\square \ggg \gg$ |

2. Staggered or double-stud walls are higher rated than single-stud walls.

| Description |
| :--- |
| $2 \times 4$ stud, $5 / 8^{\prime \prime}$ gyp (2 layers total), Batt insulation |
| Staggered studs, $5 / 8^{\prime \prime}$ gup (2 layers total), Batt insulation |
| $3 / 8^{\prime \prime}$ metal studs, $5 / 8^{\prime \prime}$ gyp (3 layers total), Batt insulation |
| 45 stud, $5 / 8^{\prime \prime}$ gyp (4 layers total), Batt insulation |

By permission:acoustics.com
6. Drywall between double studs can dramatically reduce the STC rating of an assembly.

| Description |
| :--- |
| $2 \times 4$ studs, $5 / 8^{\prime \prime}$ <br> Batt insulation |
| $2 \times 4$ studs, $5 / 8^{\prime \prime}$ <br> Batt insulation |
| $2 \times 4$ stapers total), |
| Batt insulation |

### 9.9.0 Impact Insulation Class —IIC—What Is This?

## Impact Insulation Class (IIC)

## IIC-What Is It?



In a multilevel home or business, when a floor covering in one of the upper rooms is impacted, by dropping an object or moving furniture, for example, the impact creates a vibration that travels through the floor, subfloor, and through the ceiling to the room below. These vibrations result in unwanted and annoying sounds in those
rooms. This is called impact sound transmission. Floor coverings with a high IIC rating help to reduce impact sound transmissions to lower levels, thus reducing or eliminating those bothersome noises. The lowest IIC rated floors/ceiling assemblies come in at around 25, and the highest rated systems can come in at 85 and up.

Common Guidelines to Use When Selecting the Proper IIC Rating for Your Space
IIC 50-The least amount of impact sound transmission reduction considered effective. Some occupants would be dissatisfied with this level of sound transmission.
IIC 60-Considered a medium level of impact sound transmission reduction.
IIC 65-Considered a high level of impact sound transmission reduction that would satisfy most occupants.
By permission findanyfloor.com/sound/SoundControl.xhtml

## Interior Finishes

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### 10.1.0 Gypsum Drywall Panels—Types, Thickness, Width, Length

| Panel Sizes |  |  |  |
| :---: | :---: | :---: | :---: |
| Drywall Type | Thickness | Width | Length |
| Standard gypsum drywall panels | $\begin{aligned} & 1 / 4^{\prime \prime} \\ & 5 / 16^{\prime \prime} \\ & 3 / 8^{\prime \prime} \\ & 1 / 2^{\prime \prime} \end{aligned}$ | $\begin{aligned} & 4^{\prime} \\ & 4^{\prime} \\ & 4^{\prime} \\ & 4^{\prime} \end{aligned}$ | $\begin{aligned} & 8^{\prime}, 10^{\prime}, 12^{\prime} \\ & 8^{\prime}, 10^{\prime}, 12^{\prime} \\ & 8^{\prime}, 10^{\prime}, 12^{\prime} \\ & 8^{\prime} \text { to } 16^{\prime} \end{aligned}$ |
| Fire-resistant drywall Type X for commercial use where multiple layers are required for extended fire wall rating durations | 1/2" | $4^{\prime}$ | $8^{\prime}$ to $16^{\prime}$ |
| Fire-resistant drywall Type X | 5/8" | $4^{\prime}$ | $8^{\prime}$ to $16^{\prime}$ |
| Fire-resistant backer board Type X | 1/2" | $4^{\prime}$ | $8^{\prime}$ |
| $\downarrow$ | 5/8" | $4^{\prime}$ | $8^{\prime}$ |
| Fire-resistant shaft liner drywall | $1^{\prime \prime}$ | 23-7/8" | $8^{\prime}$ to $12^{\prime}$ |
| Water-resistant drywall | 1/2" | $4^{\prime}$ | $8^{\prime}, 10^{\prime}, 12^{\prime}$ |
| Fire- and water-resistant drywall | 1/2" | $4^{\prime}$ | $8^{\prime}, 10^{\prime}, 12^{\prime}$ |
| $\downarrow$ | 5/8' | $4^{\prime}$ | $8^{\prime}, 10^{\prime}, 12^{\prime}$ |
| Foil-back panels | 3/8" | $4^{\prime}$ | $6^{\prime}$ to $16^{\prime \prime}$ |
| $\downarrow$ | 1/2" | $4^{\prime}$ | $6^{\prime}$ to $16^{\prime \prime}$ |
| Foil-back Type X | 5/8' | $4^{\prime}$ | $6^{\prime}$ to $16^{\prime \prime}$ |
| $54^{\prime \prime}$ wide panels | 1/2" | $54^{\prime \prime}$ | $8^{\prime}, 10^{\prime}, 12^{\prime}$ |
| $\downarrow$ | 5/8' | $54^{\prime \prime}$ | $8^{\prime}, 10^{\prime}, 12^{\prime}$ |
| Exterior ceiling panels (soffit board) | 1/2" | $4^{\prime}$ | $8^{\prime}, 9^{\prime}, 10^{\prime}$ |
| $\downarrow$ | 5/8' | $4^{\prime}$ | $8^{\prime}, 9^{\prime}, 10^{\prime}$ |
| High-strength ceiling panels | $1 / 2^{\prime \prime}$ | $4^{\prime}$ | $8^{\prime}, 9^{\prime}, 10^{\prime}$ |
| Drywall panels for factory decoration | 5/16" | $4^{\prime}$ | cut to specified size |
| Sound deadening panels | 1/4" | $4^{\prime}$ | $8^{\prime}$ |
| Tile-backing panels | 1/4" | $32^{\prime \prime}, 4^{\prime}$ | $4^{\prime}$ |
| $\downarrow$ | 1/2" | $4^{\prime}$ | $5^{\prime}, 8^{\prime}$ |
| Type X fire-resistant tile-backing panels | 5/8' | $4^{\prime}$ | $8^{\prime}$ |
| Exterior sheathing | 1/2" | $4^{\prime}$ | $8^{\prime}, 9^{\prime}, 10^{\prime}$ |
| $\downarrow$ | 5/8' | $4^{\prime}$ | $8^{\prime}, 9^{\prime}, 10^{\prime}$ |
| Veneer-base drywall (blue board) | $3 / 8^{\prime \prime}$ | $4^{\prime}$ | $8^{\prime}$ to $16^{\prime \prime}$ |
| $\downarrow$ | 1/2" | $4^{\prime}$ | $8^{\prime}$ to $16^{\prime \prime}$ |

### 10.1.1 Wall Framing and Drywall Panel Measurements-U.S. and Metric

Wall Framing Spacing-U.S. and Metric
16 inches $=406 \mathrm{~mm}$
24 inches $=510 \mathrm{~mm}$
Framing Fasteners
2 inch centers $=51 \mathrm{~mm}$
$21 / 2$ inch centers $=64 \mathrm{~mm}$
6 inch centers $=153 \mathrm{~mm}$
7 inch center $=178 \mathrm{~mm}$
8 inch center- 203 mm
12 inch centers $=305 \mathrm{~mm}$
16 inch centers $=406 \mathrm{~mm}$
24 inch centers $=610 \mathrm{~mm}$
Gypsum Wallboard Panel Size and Thickness-U.S. and Metric
Panel width
2 feet $=610 \mathrm{~mm}(61 \mathrm{~cm})$
4 feet $=1219 \mathrm{~mm}(121.9 \mathrm{~cm})$
Panel length
4 feet $=1219 \mathrm{~mm}$ or 1.219 meters
5 feet $=1524 \mathrm{~mm}$ or 1.524 meters
6 feet $=1828 \mathrm{~mm}$ or 1.828 meters
8 feet $=2428 \mathrm{~mm}$ or 2.428 meters
10 feet $=3048 \mathrm{~mm}$ or 3.048 meters
12 feet $=3658 \mathrm{~mm}$ or 3.658 meters
Panel Thickness
$1 / 4$ inch $=6.4 \mathrm{~mm}$
$3 / 8$ inch $=9.54 \mathrm{~mm}$
$1 / 2$ inch $=12.7 \mathrm{~mm}$
$5 / 8$ inch $=15.9 \mathrm{~mm}$
One inch $=25.4 \mathrm{~mm}$

### 10.1.2 Gypsum Wall Panel Coverage Calculator

| Gypsum Panel Coverage Calculator |  |  |  |
| :--- | :--- | :--- | :--- |
| Size of Panels |  |  |  |
| No. of Panels | $\mathbf{4}^{\prime} \times \mathbf{8}^{\prime}$ | $\mathbf{4}^{\prime} \times \mathbf{1 0}^{\prime}$ | $\mathbf{4}^{\prime} \times \mathbf{1 2}^{\mathbf{\prime}}$ |
| $\mathbf{1 0}$ | 320 sq. ft. | 400 sq. ft. | 480 sq. $\mathrm{ft}$. |
| $\mathbf{1 1}$ | 352 | 440 | 528 |
| 12 | 384 | 480 | 576 |
| 13 | 416 | 520 | 624 |
| 14 | 448 | 560 | 672 |
| 15 | 480 | 600 | 720 |
| 16 | 512 | 640 | 768 |
| 17 | 544 | 680 | 816 |
| 18 | 576 | 720 | 864 |
| 19 | 608 | 760 | 912 |


| Gypsum Panel Coverage Calculator-Cont'd |  |  |  |
| :--- | :--- | :---: | :---: |
|  |  |  |  |
| No. of Panels | $\mathbf{4}^{\prime} \times \mathbf{8}^{\prime}$ | $\mathbf{4}^{\prime} \times \mathbf{1 0}^{\prime}$ | $\mathbf{4}^{\prime} \times \mathbf{1 2}^{\mathbf{\prime}}$ |
| 20 | 640 | 800 | 960 |
| 21 | 672 | 840 | 1008 |
| 22 | 704 | 880 | 1056 |
| 23 | 736 | 920 | 1104 |
| 24 | 768 | 960 | 1152 |
| 25 | 800 | 1000 | 1200 |
| 26 | 832 | 1040 | 1248 |
| 27 | 864 | 1080 | 1296 |
| 28 | 896 | 1120 | 1344 |
| 29 | 928 | 1160 | 1392 |
| 30 | 960 | 1200 | 1440 |
| 31 | 992 | 1240 | 1488 |

Use the following table to determine the maximum frame spacing for direct application of gypsum panels to wood framing.

Frame Spacing for Single-Layer Application

| Board Thickness | Location | Application Method $^{(1)}$ | Max. Frame Spacing on center in. |
| :--- | :--- | :--- | :---: |
| $3 / 8^{\prime \prime}$ | ceiling $^{(2)(3)}$ | perpendicular ${ }^{(1)}$ | 16 |
|  | sidewall | parallel or perpendicular | 16 |
| $1 / 2^{\prime \prime}$ | ceiling $^{(4)}$ | parallel $^{(3)}$ |  |
|  | perpendicular |  |  |
| parallel or perpendicular | 16 |  |  |
| $5 / 8^{\prime \prime}$ | ceiling $^{(4)}$ | parallel $^{(3)}$ | $24^{(5)}$ |
|  | sidewall | perpendicular | 24 |

For Sheetrock Brand Interior Ceiling Panels-Sag-Resistant

| Board Thickness | Location | Application Method ${ }^{(1)}$ | Max. Frame Spacing on center in. |
| :--- | :--- | :--- | :---: |
| $1 / 2^{\prime \prime}$ | ceilings | parallel or perpendicular | 24 |

${ }^{(1)}$ Long edge position relative to framing.
${ }^{(2)}$ Not recommended below unheated spaces.
${ }^{(3)}$ Not recommended if water-based texturing material is to be applied.
${ }^{(4)}$ Sheetrock Brand Gypsum Panels—Water Resistant are not recommended for ceiling where framing is greater than $12^{\prime \prime}$ on center for single layer resilient application where file is to be supplied.
${ }^{(5)}$ Max. spacing $16^{\prime \prime}$ on center if water-based texturing material to be applied.

This information is a copyrighted work of USG Corporation.

### 10.1.3 Fastener/Compound/Tape Calculator

## Planning the Job

To estimate the quantity of fasteners, compound, and tape you will need, use the following table.

Fastener/Compound/Tape Calculator

| With this amount of Sheetrock Brand Gypsum Panels sq. ft | Use this amount of wallboard nails ${ }^{(1)}$ lb. | Or this amount of Type W Screws ${ }^{(2)}$ lb. | Use this amount of Sheetrock Brand Joint Tape ft. |
| :---: | :---: | :---: | :---: |
| 100 | 0.6 | 0.3 | 37 |
| 200 | 1.1 | 0.6 | 74 |
| 300 | 1.6 | 0.9 | 111 |
| 400 | 2.1 | 1.2 | 148 |
| 500 | 2.7 | 1.4 | 185 |
| 600 | 3.2 | 1.6 | 222 |
| 700 | 3.7 | 1.9 | 259 |
| 800 | 4.2 | 2.2 | 296 |
| 900 | 4.8 | 2.4 | 333 |
| 1000 | 5.3 | 2.7 | 370 |


| With this amount of Sheetrock Brand Gypsum Panels sq. ft. | Use this amount of Sheetrock Brand All-Purpose Ready Mixed Joint Compound ${ }^{(3)}$ lb. | Use this amount of Sheetrock Brand Lightweight All-Purpose Ready Mixed Joint Compound (Plus 3) ${ }^{(3)}$ gal. | Use this amount of Sheetrock Brand First Coat gal. |
| :---: | :---: | :---: | :---: |
| 100 | 14 | 0.9 | 0.3 |
| 200 | 28 | 1.9 | 0.6 |
| 300 | 41 | 2.8 | 0.9 |
| 400 | 55 | 3.8 | 1.1 |
| 500 | 69 | 4.7 | 1.4 |
| 600 | 83 | 5.6 | 1.7 |
| 700 | 97 | 6.6 | 2.0 |
| 800 | 110 | 7.5 | 2.3 |
| 900 | 124 | 8.5 | 2.6 |
| 1000 | 138 | 9.4 | 2.9 |

[^30]This information is a copyrighted work of USG Corporation.

### 10.1.4 Drywall Finishing Guide

## Scope

The following industry specifications describe various levels of gypsum board finishes prior to the application of decoration. The recommended level of finish for gypsum board wall and ceiling surfaces varies depending on their location in the structure, type of paint to be applied, and final decoration to be applied and can also be dependent on the type and strength of illumination striking the surface. Each recommended level of finish is described with typical applications.

## Terminology

Accessories: Metal or plastic beads, trim or molding used to protect, conceal, or decorate corners, edges, or the abutments of the gypsum board construction.

Back-Roll: Rolling a spray painted surface (while still wet) with a paint roller immediately following spray application.

Critical or Severe Lighting: Strong side lighting from windows or surface-mounted light fixtures. Wall and ceiling areas abutting window mullions or skylights, long hallways, or atriums with large surface areas flooded with artificial and/or natural lighting are a few examples of critical lighting areas. Strong side lighting from windows or surface-mounted light fixtures may reveal even minor surface imperfections. Light striking the surface obliquely, at a very slight angle, greatly exaggerates surface irregularities. If critical lighting cannot be avoided, the effects can be minimized by skim coating the gypsum board surfaces, by decorating the surface with medium to heavy textures, or by the use of draperies and blinds that soften shadows. In general: gloss, semigloss, and enamel paint finishes highlight surface defects; textures hide minor imperfections.

Joint Photographing or Telegraphing: The shadowing of the finished joint areas through the surface decoration.

Source: K. E. McNurney, Inc., Dimi, CA.
Paint: Any pigmented liquid, liquefiable, or mastic composition designed for application to a substrate as a thin layer that is converted to an opaque solid film after application. Used for protection, decoration, or identification or to serve some functional purpose, such as filing or concealing surface irregularities.

Primer/Sealer Drywall: A paint material formulated to fill the pores and equalize the suction difference between gypsum board surface paper and the compound used on finished joints, angles, fastener heads, accessories, and over skim coatings. A good primer / sealer (note: I always recommend a good quality paint as the primer coat because the "PVA" drywall sealers lack enough solids) formulated with higher binder solids, applied undiluted, is typically specified for new gypsum board surfaces prior to the application of texture materials and gloss, semigloss, and flat latex wall paints. An alkali and moisture-resistant primer and a tinted enamel undercoat may be required under enamel paints. Always consult with the finish paint manufacturer for specific recommendations.

Primer/Sealer Wall Coverings: White, self-sizing water base, "universal" (all-purpose) wall covering primers have recently been introduced into the marketplace for use on new gypsum surfaces. It is claimed that these products are drywall strippable, bind poor latex paint, allow hanging over glossary surfaces and existing vinyls, hide wall colors, and are water washable.

Properly Painted Surface: A surface that is uniform in appearance, color, and sheen. It is one that is free of foreign material, lumps, skins, runs, sags, holidays, misses, strike-through, or insufficient coverage. It is a surface that is free of chips, splatters, spills, or over spray that was caused by the contractors' workforce. Compliance with the criteria of a "properly painted surface" should be determined when viewed without magnification at a distance of 5 feet or more, under normal lighting conditions, and from a normal viewing position. (Note: A surface uniform in appearance, color, and sheen may not be sufficiently achieved with a coat of primer or a single coat of topcoat.)

Skim Coat: A thin coat of joint compound over the entire surface to fill imperfections in the joint work, smooth the paper texture, and provide a uniform surface for decorating.

Spotting: Method used to cover fastener heads (nails, screws, staples) with joint compound.
Texture: A decorative treatment of gypsum board surfaces.
Texturing: Regular or irregular patterns typically produced by applying a mixture of joint compound and water, or proprietary texture materials, including latex base texture paint, to a gypsum board surface previously coated with primer/sealer. Texture material is applied by brush, roller, spray, trowel, or a combination of these tools, depending on the desired result. Textured wall surfaces are normally overpainted with the desired finish; overpainting of textured ceiling may not be deemed necessary where an adequate amount of material is applied to provide sufficient hiding properties. A primer/sealer may not be required with certain proprietary texture materials. Always consult with the manufacturer of the texture material for specific recommendations.

Tool Marks and Ridges: A smooth surface may be achieved by lightly sanding or wiping the joint compound down with a dampened sponge. Great care should be exercised to ensure that the nap of the gypsum board facing paper is not raised during sanding operations.

Topcoat: The finish coat(s) of a coating system, formulated for appearance and or environmental resistance.
Wall Covering: Any type of paper, vinyl, fabric, or specialty material that is pasted onto a wall or ceiling in a wide array of colors, patterns, textures, and performance characteristics, such as washability and abrasion resistance.

## Levels of Gypsum Board Finish

LEVEL 0: No taping, finishing, or accessories required. This level of finish may be useful in temporary construction or whenever the final decoration has not been determined.

LEVEL 1: All joints and interior angles shall have tape embedded in joint compound. Surface shall be free of excess joint compound. Tool marks and ridges are acceptable. This level of finish, often referred to as "fire taping," is frequently specified in plenum areas above ceilings, in attics, in areas where the assembly would generally be concealed, or in building service corridors and other areas not normally open to public view. Accessories (cornerbead, base shoe, other trims) are optional at specifier discretion in corridors and other areas with pedestrian traffic.

LEVEL 2: All joints and interior angles shall have tape embedded in joint compound and one separate coat of joint compound applied over all joints, angles, fastener heads, and accessories. Surface shall be free of excess joint compound; tool marks and ridges are acceptable. This level of finish is specified where water-resistant drywall is used as a substrate for tile; it may be specified in garages, warehouse storage, or similar areas where surface appearance is not of primary concern.

LEVEL 3: All joints and interior angles shall be tape embedded in joint compound, and two coats of joint compound applied over all joints, angles, fastener heads, and accessories. All joint compound shall be smooth and free of tool marks and ridges. Note: It is recommended that the prepared surface be coated with a primer/ sealer prior to the application of final finishes. This level of finish is typically specified in appearance areas that are to receive heavy or medium texture (spray or hand applied) finishes before final painting, or where heavygrade wall coverings are to be applied as the final decoration.

LEVEL 4: All joints and interior angles shall be tape embedded in joint compound and three coats of joint compound applied over all joints, angles, fastener heads, and accessories. All joint compound shall be smooth and free of tool marks and ridges. Note: It is recommended that the prepared surface be coated with a primer/ sealer prior to the application of final finishes. This level of finish is typically specified where light textures or wall coverings are to be applied, or economy is of concern.

LEVEL 5: All joints and interior angles shall be tape embedded in joint compound and three separate coats of joint compound applied over all joints, angles, fastener heads and accessories. A thin skim coat of joint compound, or a material manufactured especially for this purpose, shall be applied to the surface. The surface shall be smooth and free of tool marks and ridges. Note: It is recommended that the prepared surface be coated with a primer/sealer prior
to the application of final finishes. This level of finish is recommended where gloss, semigloss, enamel, or nontextured flat paints are specified or where severe side-lighting conditions occur. This highest quality finish is the most effective method to provide a uniform surface and minimize the possibility of joint photographing and of fasteners showing through the final decoration. (Note: Application of primer/paint products over Level 4 and Level 5 smooth finish) Industry experience demonstrates that an effective method for achieving a visually uniform surface for both the primer and topcoat is spray application immediately followed by back rolling or roller application using good roller techniques such as finishing in one direction and using roller types and naps recommended by the paint manufacturer.

## Resources

ASTM C-840 - Standard Specifications for Application and Finishing of Gypsum Board. American Society for Testing and Materials.
GA-505 - Gypsum Board Terminology. Gypsum Association.
GA-214-96 - Recommended Levels of Gypsum Board Finish. Gypsum Association.
FSCT - Coatings Encyclopedia Dictionary. Federation of Societies for Coatings Technology.
DWFC - Recommended Specification for Preparation of Gypsum Board Surfaces Prior to Texture Application. Drywall Finishing Council Inc.
DWFC - Interior Job Condition Specifications for the Application of Drywall Joint Compounds, Drywall Textures, and Paint/Coatings. Drywall Finishing Council Inc.

### 10.2.0 Calculations to Determine Sealer and Filler Yield

| Crack/Joint Width $\times$ Depth (inches) | Feet per Gallon | Crack/Joint Width $\times$ Depth (inches) | Feet per Gallon | Crack/Joint Width $\times$ Depth (inches) | Feet per Gallon | Crack/Joint Width $\times$ Depth (inches) | Feet per Gallor |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $1 / 8 \times 3 / 8$ | $\begin{aligned} & 410.7 \\ & 308.0 \end{aligned}$ | $3 / 8 \times 3 / 8$ | $\begin{aligned} & 136.9 \\ & 102.7 \end{aligned}$ | $5 / 8 \times 3 / 8$ | $\begin{aligned} & 82.1 \\ & 61.6 \end{aligned}$ | $7 / 8 \times 3 / 8$ | $\begin{aligned} & 58.7 \\ & 44.0 \end{aligned}$ |
| $1 / 8 \times 1 / 2$ | $\begin{aligned} & 246.4 \\ & 205.3 \end{aligned}$ | $3 / 8 \times 1 / 2$ | $\begin{aligned} & 82.1 \\ & 68.4 \end{aligned}$ | $5 / 8 \times 1 / 2$ | $\begin{aligned} & 49.3 \\ & 41.1 \end{aligned}$ | $7 / 8 \times 1 / 2$ | $\begin{aligned} & 35.2 \\ & 29.3 \end{aligned}$ |
| $1 / 8 \times 5 / 8$ | $\begin{aligned} & 176.0 \\ & 154.0 \end{aligned}$ | $3 / 8 \times 5 / 8$ | $\begin{aligned} & 58.7 \\ & 51.3 \end{aligned}$ | $5 / 8 \times 5 / 8$ | $\begin{aligned} & 35.2 \\ & 30.8 \end{aligned}$ | $7 / 8 \times 5 / 8$ | $\begin{aligned} & 25.1 \\ & 22.0 \end{aligned}$ |
| $1 / 8 \times 3 / 4$ | 308.0 | $3 / 8 \times 3 / 4$ | 154.0 | $5 / 8 \times 3 / 4$ | 102.7 | $7 / 8 \times 3 / 4$ | 77.0 |
| $1 / 8 \times 7 / 8$ | $\begin{aligned} & 205.0 \\ & 154.0 \end{aligned}$ | $3 / 8 \times 7 / 8$ | $\begin{array}{r} 102.7 \\ 77.0 \end{array}$ | $5 / 8 \times 7 / 8$ | $\begin{aligned} & \hline 68.4 \\ & 51.3 \end{aligned}$ | $7 / 8 \times 7 / 8$ | $\begin{aligned} & 51.3 \\ & 38.5 \end{aligned}$ |
| $1 / 8 \times 1$ | $\begin{aligned} & 123.2 \\ & 102.7 \end{aligned}$ | $3 / 8 \times 1$ | $\begin{aligned} & 61.6 \\ & 51.3 \end{aligned}$ | $5 / 8 \times 1$ | $\begin{aligned} & \hline 41.1 \\ & 34.2 \end{aligned}$ | $7 / 8 \times 1$ | $\begin{aligned} & 30.8 \\ & 25.7 \end{aligned}$ |
| $1 / 4 \times 1 / 4$ | $\begin{aligned} & 88.0 \\ & 77.0 \end{aligned}$ | $1 / 2 \times 1 / 4$ | $\begin{aligned} & 44.0 \\ & 38.5 \end{aligned}$ | $3 / 4 \times 1 / 4$ | $\begin{aligned} & 29.3 \\ & 25.7 \end{aligned}$ | $1 \times 1 / 4$ | $\begin{aligned} & 22.0 \\ & 19.3 \end{aligned}$ |
| 1/4×3/8 |  | $1 / 2 \times 3 / 8$ |  | $3 / 4 \times 3 / 8$ |  | $\begin{aligned} & 1 \times 3 / 8 \\ & 1 \times 1 / 2 \end{aligned}$ |  |
| $1 / 4 \times 1 / 2$ |  | $1 / 2 \times 1 / 2$ |  | $3 / 4 \times 1 / 2$ |  | $\begin{aligned} & 1 \times 5 / 8 \\ & 1 \times 3 / 4 \end{aligned}$ |  |


| Crack/Joint Width $\times$ Depth (inches) | Feet per Gallon | Crack/Joint Width $\times$ Depth (inches) | Feet per Gallon | Crack/Joint Width $\times$ Depth (inches) | Feet per Gallon | Crack/Joint Width $\times$ Depth (inches) | Feet per Gallor |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1/4 $\times 5 / 8$ |  | $1 / 2 \times 5 / 8$ |  | $3 / 4 \times 5 / 8$ |  | $\begin{aligned} & 1 \times 7 / 8 \\ & 1 \times 1 \end{aligned}$ |  |
| $1 / 4 \times 3 / 4$ |  | $1 / 2 \times 3 / 4$ |  | $3 / 4 \times 3 / 4$ |  |  |  |
| $1 / 4 \times 7 / 81 / 4 \times 1$ |  | $\begin{aligned} & 1 / 2 \times 7 / 8 \\ & 1 / 2 \times 1 \end{aligned}$ |  | $\begin{aligned} & 3 / 4 \times 7 / 8 \\ & 3 / 4 \times 1 \end{aligned}$ |  |  |  |


| Band-Aid* Coverage: | Band-Aid <br> Configuration | Feet per <br> Gallon | Band-Aid <br> Configuration | Feet per <br> Gallon |
| :--- | :--- | :--- | :--- | :--- |
| *i.e., material squeegeed on surface. Does not | $1 / 16^{\prime \prime} \times 2^{\prime \prime}$ | 154.0 | $3 / 32^{\prime \prime} \times 2^{\prime \prime}$ | 102.4 |
| include crack volume. | $1 / 16^{\prime \prime} \times 3^{\prime \prime}$ | 102.7 | $3 / 32^{\prime \prime} \times 3^{\prime \prime}$ | 68.4 |
|  | $1 / 16^{\prime \prime} \times 4^{\prime \prime}$ | 77.0 | $3 / 32^{\prime \prime} \times 4^{\prime \prime}$ | 51.4 |
|  |  |  |  |  |

## Wallcovering Basics

## Coversion Table

For the convenience of our clients, OMNOVA Solutions provides the following dimensional conversion table which converts square feet to lineal yards-for our $54^{\prime \prime}$ Commercial Wallcovering. The table should prove helpful when estimating and ordering the amount of material needed for a specific project.

## Helpful Guidelines

- Standard OMNOVA Commercial Wall Covering is $54^{\prime \prime}$ or 4.5 feet wide.
- As a result, there are 13.5 square feet in every lineal yard of our wall coverings: 3 feet ( 1 yard) in height $\times 4.5$ feet in width equals 13.5 square feet.
- To convert lineal yards to square feet, multiply the number of lineal yards by 13.5. (Example: 5 lineal yards multiplied by 13.5 yields 67.5 square feet.)
- To convert square feet to lineal yards, divide the number of square feet by 13.5. (Example: 108 square yards divided by 13.5 yields 8 lineal yards.)
- Our wall covering is stocked and shipped in standard roll sizes of 30 and 60 lineal yards. (A cutting charge applies for yardage orders that are less than standard roll size.)
For $54^{\prime \prime}$ widths


## Conversion Table

| Square <br> Feet | Lineal <br> Yards |
| :--- | ---: |
| 13.5 | 1 |
| 67.5 | 5 |
| 135 | 10 |
| 270 | 20 |
| 405 | 30 |
| 607.5 | 45 |
| 810 | 60 |
| 1620 | 120 |
| 2700 | 200 |
| 3240 | 240 |
| 6750 | 500 |

### 10.3.0 Understanding Wall Coverings-Types and Usage

 OverviewWall coverings can be used in virtually any residential or contract environment. Since there are many different types of wall coverings on the market-some for very specific uses-it is important to understand the qualities of each type and in what type of environment the wall coverings will be used.

## Key Points

- When selecting wall coverings, the first variable to consider is the amount of traffic the area will receive.
- Paper and natural wall coverings are most appropriate where traffic is minimal. They are more delicate than their vinyl counterparts, yet offer ample durability and a special style to a variety of placements.
- Vinyl and synthetic textiles, with their maximum durability and ease of cleaning, are especially appropriate for hospitals, sporting arenas, schools, and other high-traffic situations.
- While wall coverings are categorized by residential and contract segments, it is not uncommon to use residential wall coverings in contract settings, like assisted living facilities for a homey feel, for instance, or to use contract wall coverings in some of today's more avant-garde homes.


## Contract Wall Coverings

Contract wall coverings are produced specifically for use in hotels, apartment buildings, office buildings, retail outlets, schools, and hospitals. They are manufactured to meet or surpass minimum physical and performance characteristics set forth in Federal Specifications CCC-W-408.

The most popular types of wall coverings for contract installations are as follows:

- Vinyl Coated Paper -wall coverings that have a paper substrate on which the decorative surface has been sprayed or coated with an acrylic type vinyl or polyvinyl chloride (PVC).
- Paper-Backed Vinyl/Solid Sheet Vinyl—wall coverings that have a paper (pulp) substrate laminated to a solid decorative surface. These types of wall coverings are very durable since the decorative surface is a solid sheet of vinyl. They are classified as scrubbable and peelable.
- Fabric Backed Vinyl—wall coverings that have a woven substrate of fabric or a nonwoven synthetic substrate. In either case, the substrate is laminated to a solid vinyl decorative surface.

General categories of these types of wall coverings include the following:

- Type I (Light Duty)—for use in office areas, hospital patient rooms, and hotel rooms. Also intended for ceilings and areas of light abrasion.
- Type II (Medium/Heavy Duty)—for use in foyers, lounges, corridors, and classrooms, or areas of average to heavy scuffing. Can also be used as wainscot.


## Specialty Wall Coverings

A special category of wall coverings is used in highly specialized circumstances or for areas of light traffic. Many of these types of wall coverings have been replaced by vinyl wall coverings that simulate the same look with greater durability. Nonetheless, many of the types of wall coverings outlined below have historical importance and can be produced by specialty manufacturers or custom firms. They are highly decorative and appropriate for use in any contract area where a dramatic look is desired.

- String Effects—wall coverings that have very fine vertical threads laminated to a paper-type substrate. Most suitable for offices, boardrooms, and areas of light traffic.
- Natural Textile Wall Coverings-natural textiles usually laminated to a backing to enhance dimensional stability and to prevent the adhesive from coming through to the surface. These backings are usually acrylic or paper. Textiles are manufactured in a variety of widths and are constructed of natural fibers. Natural textiles can be finely designed or coarse in texture depending on the desired look.
- Polyolefin/Synthetic Textile Wall Coverings-woven and nonwoven-looking wall coverings developed to give the aesthetic appearance of a natural textile while adding an increased value in stain and abrasion resistance. These products are generally put up with an acrylic or paper backing. Many of these products are comprised of polyolefin yarns, which are olefin fibers made from polymers or copolymers of propylene. These types of wall coverings are appropriate for higher traffic areas.
- Acoustical Wall Coverings-designed for use on vertical surfaces, panels, operable walls, and anyplace sound reduction is a primary factor, such as meeting rooms, offices, theaters, auditoriums, restaurants as well as corridors and elevator lobbies. These products are predominantly made of man-made polyester and olefin fibers, and are tested for a special sound-absorption rating known as a Noise Reduction Coefficient (NCR) rating. This rating indicates the amount of sound absorbed into the wall. The higher the number, the more noise absorption.


## Types and Usage

- Cork and Cork Veneer—variegated texture with no definite pattern or design. Cork veneer is shaved from cork planks or blocks and laminated to a substrate that may be colored or plain. Offers some degree of sound resistance; can be used as bulletin boards.
- Digital Wall Covering-borders, murals, and wall covering. Unlimited supplies of designs, ideas, and colors. Digital wall coverings allow the person the freedom to express any theme, style, or design on a ground of his or her choice.
- Wood Veneer-wood wall coverings mostly laminated to fabric backing. They are usually made in sheets 18 to 24 inches wide and provided in any length up to 144 inches long. Due to characteristics relative to environmental and grain matching, wood veneers are used mostly in the office or conference room environment along with some other specialty areas, such as large columns.
- Foils -a thin sheet of metallic material with a paper or fabric substrate. Popular in the 1960s and 1970s. Require a very smooth surface and extreme care when installing. Usage is limited; highly decorative.
- Mylar (by DuPont) - wall coverings ground made of vacuum-metallized polyester film laminated to a substrate. Offers a highly reflective surface with an appearance similar to foil with less stiffness.
- Flocks-resemble cut velvet and very popular in the sixteenth and seventeenth centuries. Also popular in the 1970s. Produced by laminated shredded fibers to paper, vinyl, Mylar, or foil. Highly decorative, period wall coverings; limited abrasion resistance. For use in low-traffic areas.
- Underliner-blank stock-type wall coverings. Comes in different weights such as light, medium, and heavy. Can be plain paper stock or a nonwoven-type material. Liner can be used on almost any wall surface, such as plaster, sheetrock (drywall), paneling, and cinder block. Its purpose is to provide a smooth surface for the installation of wall coverings.

Source: Wallcovering Association, Chicago, ILL.

### 10.3.1 Basic Types of Fabric-Backed Vinyl Wall Coverings

Wall Covering Basics: Types of Wall Covering
There are three general categories of fabric-backed vinyl wall covering-Type I, Type II, and Type III, which refer to the weight and performance associated with the wall coverings in these categories.

## Type I

Type I is a light-duty wall covering. The exact weight of the Type I vinyl wall covering to specify depends on the application or the decorative effects to be achieved.

- Type I lightweight vinyl wall covering is considered to have a weight less than 15 oz . per linear yard (based on a $54^{\prime \prime}$ width) and is produced with a scrim backing or nonwoven fabric. It is intended for use on interior commercial walls where a combination of wall protection and design effect is desired.
- A Type I heavyweight vinyl wall covering is considered to have a weight between 15 oz . and 19 oz . per linear yard (based on a $54^{\prime \prime}$ width) and is produced with a scrim backing or nonwoven fabric. It is recommended for use in corridors or offices where moderate traffic is expected.

The 15 oz . per linear yard (based on a $54^{\prime \prime}$ width) wall covering is the most widely specified weight of Type I vinyl wall coverings.

## Type II

Type II is a medium-duty wall covering. It weighs between 20 and 32 oz . per linear yard (based on a $54^{\prime \prime}$ width) and is produced on Osnaburg (poly-cotton or polyester) or nonwoven fabric backing. Type II is considered the "work-horse" among vinyl wall coverings and is typically specified for areas where greater than normal traffic and surface abrasion is evident or expected. It is ideal for offices, hospital wards, public areas and rooms in hotels, lounges, dining rooms, public corridors, and classrooms.

The 20 oz . per linear yard (based on a $54^{\prime \prime}$ width) is the most widely specified weight of Type II vinyl wall coverings.

## Type III

Type III is a heavy-duty wall covering. It generally weighs in excess of 33 oz. per linear yard (based on a $54^{\prime \prime}$ width) and is made with a drill fabric backing. Type III wall covering is recommended as a wall protection for areas exposed to extraordinarily hard use, vehicular traffic, and abrasive conditions.

Source: omnova.com

### 10.3.2 Formulas for Estimating Wall Covering Quantities

## Formulas and Estimating

- Measuring Accurately
- Square Foot Area Method
- Stairways or Cathedral Walls
- Estimating Commercial Square Footage


## Measuring before Estimating Wall Covering Needs

The most important step in estimating wall covering is accurate measurements. Use a yardstick or cloth tape measure. Take measurements in feet rounding off to the next highest half foot or for doors, windows and ceiling height. If a wall is unusually broken up with a fireplace or built-in book shelves, detailed measurements will be beneficial in figuring the square footage of wall covering needed.

Measure wall height from floor to ceiling. Exclude baseboards and moldings. Measure length of windows. Find the total square feet of the wall(s) by multiplying ceiling height by total wall length covered. (Standard doors are about $3 \times 7$ feet or 21 square feet; standard windows about $3 \times 4$

These calculations give the total number of square feet to be covered. Using this the number of wall covering can be determined.

For example:


In the above figure, each wall is $12^{\prime}$ long with an $8^{\prime}$ ceiling. Multiply $12 \times 8=96$ square feet for e (since there are four walls with 96 square feet each $)=384$ total square feet for the room.

## Metric Single Roll

| Repeat Length | Usable Yield |
| :--- | :---: |
| $0^{\prime \prime}$ to $6^{\prime \prime}$ | 25 sq. ft. |
| $7^{\prime \prime}$ to $12^{\prime \prime}$ | 22 sq. ft |
| $13^{\prime \prime}$ to $18^{\prime \prime}$ | 20 sq. ft |
| $19^{\prime \prime}$ to $23^{\prime \prime}$ | 18 sq. ft. |

Estimate Square Footage:
Ceiling height $0 \times$ Total Wall Length 0

- Calculate Square Footage
$=\square$ TOTAL SQUARE FEET


Using the above diagram as an example, figure the amount of wall covering that will be needed for 384 square feet has not taken into account the square footage of the doors and windows. Subtract each opening such as 21 square feet for the door and 12 square feet for each of the windows. 384 sq. $12+12=45$ ) $=339$ square feet of wall space that will be covered with wall covering. If you are using a repeat of 8 inches, figure that each metric single roll will contain 22 square feet of usable walk amount of wall space from above that will be hung) divided by 22 square feet (from Usable Yield round up to 16 metric single rolls (msr) that will be needed to hang the example room ( 8 metric rolls)

The equation would look like this:
384 sq. ft. (room size)
-21 sq. ft. (one standard door)
-12 sq. ft. (one standard window)
-12 sq. ft. (one standard window)
$=339$ sq. ft. of wall space that will be hung
339 sq. ft. / 22 sq. ft. $=15.4$ rolls to hang the room, rounded up to 16 rolls

## Calculate Square Foot Area Method:

Room Size: 0
Number of Standard Doors: 0
Number of Standard Windows: 0
Calculate Square Footage
$=\square$ TOTAL SQUARE FEET
Back to top

## Stairways or Cathedral Walls

When estimating a wall that has a diagonal, remember that there will be extra waste to allow for this pitch. There are two different types of stairways to figure: one with a horizontal ceiling line, and a ceiling line that parallels the fall of the steps.


In both cases, the first step is to divide the wall into either squares or rectangles to determine the above, the upstairs ceiling height is $8^{\prime \prime}$, and the downstairs ceiling height is $8^{\prime \prime}$. Next, measure wall width horizontally from the top of the stairs to an imaginary vertical line which in the example is $15^{\prime \prime}$. Taking the top rectangle, figure $8^{\prime \prime} \times 15^{\prime \prime}=120$ sq. ft. Next, figure this sq. ft., but since a portion of this wall area is under the stairs, multiply the bottom rectangle sq. ft standard. Add the two figures together to arrive at the square feet that needs to be hung with will look as follows:

```
\(8^{\prime \prime} \times 15^{\prime \prime}=120\) sq. ft. (top rectangle)
\(8^{\prime \prime} \times 15^{\prime \prime}=120\) sq. ft. \(\times 65 \%=78\) sq. ft. (bottom rectangle)
120 sq. ft. +78 sq. ft. \(=198\) sq. ft.
```

Once you have the square footage figured, estimate the amount of wall covering just as you would the usable square feet for the particular pattern from the Usable Yield Chart and then dividing square footage of each roll to determine number of rolls required.

## Formulas and Estimating

For example, if one were using a wall covering with a repeat of $14^{\prime \prime}$, each msr would contain 20 square feet. The equation would look as follows: $198 \mathrm{sq} . \mathrm{ft} . / 20 \mathrm{sq}$. ft. $=9.9 \mathrm{msr}$ rounded to 10 msr . If the stair the first example in finding the width and length of the imaginary rectangle or square. The next rectangle/square figures multiplied by $65 \%$ to find the square feet of wall area. The equation will look like this:
$8^{\prime \prime} \times 15^{\prime \prime}=120 \mathrm{sq} . \mathrm{ft}$. (top rectangle)
$8^{\prime \prime} \times 15^{\prime \prime}=120$ sq. ft. (bottom rectangle)
120 sq. ft. +120 sq. ft. $=240$ sq. ft.
240 sq. $\mathrm{ft} . \times 65 \%=156$ sq. ft. of wall area to be covered
Using the same pattern with a repeat of $14^{\prime \prime}$, each msr would contain 20 square feet of usable would look as follows:
$156 \mathrm{sq} . \mathrm{ft} . / 20 \mathrm{sq} . \mathrm{ft} .=7.8 \mathrm{msr}$ rounded to 8 msr
A cathedral ceiling would be estimated the same way, squaring the top rectangle, multiplying then adding that figure to the square feet of the bottom rectangle.

## Calculate Square Footage for Stairways with Straight Ceiling

Square Footage of Top Rectangle: 0
Square Footage of Bottom Rectangle: 0

```
Calculate Square Footage
=0 TOTAL SQUARE FEET
```


## Calculate Square Footage for Cathedral Ceilings

Square Footage of Top Rectangle: 0
Square Footage of Bottom Rectangle: 0
Calculate Square Footage
$=0$ TOTAL SQUARE FEET
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## Estimating Commercial Square Footage

After the wall covering has been determined from the specification, now figure the square footage of the job. Once the width is known, the number of square feet in a lineal yard for that particular is determined. An important formula to remember is as follows:

Width divided by $12=$ number of feet
Number of feet multiplied by 3 (1 yard) = square feet/width (square feet per lineal yard)
Divide square feet of wall space to cover by square feet/width

## For example:

54 -inch-wide material used to cover 1500 square feet is figured
54 divided by $12=4.5$
4.5 multiplied by $3=13.5$ square feet per lineal yard

1500 divided by $13.5=111.11$
1500 square feet of wall space would require 112 yards without waste.
Once the width and the square footage for the width are known, any amount can be determined. If the yardage for a particular width is known, and the width of the material is changed, to convert from one width to another, yardage.

## For example:

150 yards of 54 -inch-wide material
54 inches is 13.5 square feet per yard $(54 / 12=4.5 \times 3=13.5)$
150 yards multiplied by $13.5=2,025$ square feet of wall space to cover
Formulas and Estimating
$36 / 12=3$
$3 \times 3=9$ square feet
2,025 divided by $9=225$ yards of 36 -inch wide wall covering instead of original 1 material.
These are exact yardage amounts and do not allow for waste caused by pattern repeat. However, a matching pattern with a large repeat would require additional material pattern without a match. All contractors should be aware of the pattern, width, and match before ordering.

## Estimate Commercial Square Footage:

Width of Material (in inches): 0
Square Footage to be covered: 0
Calculate Material Needed
$=\square$ YARDS NEEDED (Without Waste)

### 10.3.3 Calculating How Much Wallpaper Is Required

Wallpaper is usually sold in double rolls, each roll of which contains 30 sf.
The following calculations are based on determining wall square footage and assuming various ceiling heights for each size room.

| Room Size | W/8 Foot Ceiling | w/9' <br> Ceiling | w/10' Ceiling | w/12 ${ }^{\prime}$ Ceiling | Single Rolls for Ceiling |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $8 \times 10$ | 10 | 11 | 12 | 14 | 3 |
| $9 \times 12$ | 11 | 13 | 14 | 17 | 4 |
| $10 \times 10$ | 11 | 13 | 15 | 17 | 4 |
| $10 \times 12$ | 12 | 13 | 15 | 18 | 4 |
| $10 \times 14$ | 13 | 14 | 16 | 19 | 5 |
| $12 \times 14$ | 14 | 16 | 18 | 21 | 6 |
| $12 \times 16$ | 15 | 17 | 19 | 22 | 7 |
| $12 \times 18$ | 16 | 18 | 20 | 24 | 8 |
| $12 \times 20$ | 17 | 19 | 21 | 26 | 8 |
| $14 \times 16$ | 16 | 19 | 20 | 24 | 8 |
| $14 \times 18$ | 17 | 19 | 21 | 26 | 8 |
| $14 \times 20$ | 18 | 20 | 22 | 27 | 10 |
| $14 \times 22$ | 19 | 22 | 24 | 29 | 11 |
| $16 \times 18$ | 19 | 20 | 23 | 28 | 10 |
| $16 \times 20$ | 19 | 22 | 24 | 29 | 11 |
| $16 \times 22$ | 21 | 23 | 25 | 30 | 12 |
| $16 \times 24$ | 21 | 24 | 27 | 32 | 13 |
| $18 \times 20$ | 21 | 23 | 25 | 30 | 12 |

As a contingency add $10 \%$ for waste.

### 10.3.4 Wall Covering Adhesives

There are many types of wall covering adhesives, each formulated for various performance characteristics. These characteristics fall into two general categories:

- How they bond the wall covering to the wall surface.
- How they apply to the wall covering (this is a major consideration for the installer).

Wall covering adhesives are formulated for specific applications. Some adhesives are formulated for lightweight and delicate fabrics, whereas others are designed to adhere to heavyweight vinyl and acoustical coverings.

Adhesives vary in level of wet-tack, solids, open-time, strippability, and ease of application. All wall covering adhesives contain a biocide system. These systems are designed to prevent bacterial contamination and mildew/fungal infestation both "in-the-can" and in the dried adhesive. Wall covering adhesives are generally applied on the back of the wall covering either by roller or pasting machine. However, nonwoven acoustical wall coverings require the adhesive be applied to the wall using a brush or roller. Consult the manufacturer's installation instructions.

There are four main categories of adhesives:

- Prepasted Activators-were created to assist in the hanging of prepasted wall coverings by activating the existing adhesive, increasing slip, and extending the open-time available (helpful when matching patterns). Using activators is generally considered less messy, minimizes seam splitting, and reduces seam lifting.
- Clear Adhesives-are either corn or wheat based. They are generally considered to have more open-time and are easier to clean up than clay-based adhesives. Clear adhesives are designed for both the retail customer and commercial installer.
- Clay Adhesives-like clear adhesives are starched based, but clay is added as a filler to increase the wet-tack and level of solids. Clay adhesives are generally not recommended for retail customers. They are considered to have a higher level of wet-tack and are more difficult to clean up versus clear adhesives.
- Vinyl-Over-Vinyl—and border adhesives contain synthetic polymers and are specifically formulated to bond to vinyl. In addition to the synthetic polymers, these adhesives may contain some starch and other ingredients to assist in their application. These adhesives require extra attention during application because once they are dried they are permanent. Vinyl-over-vinyl adhesives are specifically designed to hang new wall coverings to existing wall coverings or borders to wall coverings.
Source: Wallcovering Association, Chicago, ILL.


### 10.4.0 General Information to Calculate Various Types of Floor and Wall Tiles

## Floor and wall tile

The following paragraphs include information pertaining to the various types of tile and their installation procedures. The number of tiles needed is calculated by performing the following procedures:
First, calculate the square feet of the area to be tiled. If you are using 12 -inch-square tiles, the total floor area (in square feet) equals the total number of tiles needed, plus an additional 10 percent waste factor. If another size of tiles is being used, multiply the area by 144 to convert to square inches. Then divide that number by the area (square inches) of the tiles to find the required amount (include a 10 percent waste factor).

Example: You are using tiles $9 \times 9$ inches. To tile a floor 12 feet long and 9 feet wide-
Multiply the room dimensions to find the area: 12 feet $\times 9$ feet $=108$ square feet
Multiply the area by 144 : $108 \times 144=15,552$ square inches
Calculate the area of the tile: 9 inches $\times 9$ inches $=81$ square inches
Divide the room area (square inches) by the tile area (square inches): 15,552 divided by $81=192$ tiles
Add 10 percent waste factor: $192+19=211$ tiles required

## Resilient floor tile

Resilient floor tile is durable, easily maintained, comfortable and attractive, and low cost. It is made of rubber, vinyl, linoleum, and asphalt. Common sizes of this tile are either $9 \times 9$ inches or $12 \times 12$ inches.

A notched trowel (used for spreading adhesive) and a tile cutter are required for installation. To lay out and install resilient floor tile, perform the following procedures:

Locate the center of the end walls of the room. Establish a main centerline by snapping a chalk line between these two points. Lay out another centerline at right angles to the main centerline. This line may be established using a framing square or the triangulation method. With the centerline established, make a trial layout of the tiles along the centerlines. Measure the distance between the wall and the last tile. If this measurement is less than $1 / 2$ tile, move the centerline half the width of the tile, closer to the wall. This adjustment will eliminate the need to install border tiles that are too narrow. Since the original centerline is moved exactly half the tile size, the border tile will remain uniform on opposite sides of the room. Check the layout along the other centerline in the same way.

Spread adhesive over one quarter of the total area, starting with the quarter farthest from the door and working toward the door. Ensure that the floor surface is clean before you spread the adhesive. Spread up to the chalk lines but do not cover them. Be sure to use a notched trowel with the notch depth recommended by the manufacturer of the adhesive. Allow the adhesive to take an initial set before setting the first tile. The time required will vary, depending on the type of adhesive used.

Source: Constructionknowledge.org
Start laying the tiles at the center of the room. Make sure the edges of the tiles are aligned with the chalk line. Lay rows by width, stair-stepping additional rows and ensuring that the tiles are tight against one another in a cross-grained pattern unless otherwise specified. After all of the full tiles have been laid, install the border or edge tiles around the room. To lay out a border tile, place a loose tile over the last tile in the outside row with the grains running in opposite directions (if using a cross-grained pattern). Then, take another tile and place it in position against the wall and mark a pencil line on the first tile. Cut the tile along the marked line.

After all the tiles have been installed, remove any excess adhesive using a cleaner or solvent and procedures approved by the manufacturer.

## Ceramic and other specialty tiles

This tile is used extensively where sanitation, stain resistance, easy cleaning, and low maintenance are desired. Types of tile include ceramic, mosaic, paver, quarry, brick-veneer, cement-bodied, marble, and other stone tiles. These can be used for both interior and exterior flooring. Tile is used on both walls and floors. Field tile is regular tile placed on all courses in the main field of an installation. Trim tile is a specially shaped tile used to border and complete the main field of tile; it is available in a wide variety of shapes, sizes, and colors to match field tile.

Tiles come with two types of finishes-glazed and unglazed. Glazed tiles are coated with a glaze before firing to give the tile color and to preserve its surface. They may be fired to a smooth or textured finish. Glazed tiles are most commonly used for walls but may also be applied to floors and countertops. They are used mainly for interiors. Unglazed tiles are fired without a glaze coating. They derive their color from the clay from which they are made. Adhesives used are Thinset or Organic Mastic. Thinset is a powdered cement-based product that is mixed with either water, a latex or acrylic additive, or epoxy. It is very versatile. Organic Mastic is premixed in a solvent or latex base. It may deteriorate if exposed to heat or water.

Grout is a powder made from sand and cement and is used to seal the cracks between the tiles. It is mixed with either water or, to increase durability, an additive. It is available in a variety of colors.

The following tools and equipment are required for installing ceramic and specialty tiles:

- A striking tool is used to compact the grout into the joints.
- A beating block is a board used to even the tile surface after it has been set.
- A square-notched trowel is used to spread adhesive.
- A pointing trowel is used to spread adhesive in tight spots.
- A tile cutter is used to score the tile surface so that it can be snapped by applying pressure to the score.
- A fine file or tile stone is used to smooth rough edges after cutting tile.
- A time nipper is used to clip tile and cut irregular openings.
- A squeegee or sponge is used to remove excess grout from the tile surface.
- A sponge float or rubber-faced trowel is used to spread grout over the surface.
- An electric tile saw is similar to a mason saw. It is used to make clean, accurate cuts.
- To lay out and install ceramic and specialty tiles, perform the following procedures:
- Check the area to be tiled to determine if it is square. If the area is slightly out of square, minor changes in the layout can accommodate these conditions. If the area is seriously out of square, the process stops for any required structural repairs or surface preparation. If the framing problems are serious, it may not be possible to tile the area.

Draw the layout on paper. Layout depends greatly on the pattern desired and the type, size, and shape of the tile being used. Use as many full tiles and as few cut tiles as possible. Place cut tile away from visual focal points (doorways, thresholds, and so forth); tiles should be set symmetrically for a more attractive finish and appearance.

Place reference lines on the floor or wall. Once the layout has been established on paper, transfer it to the floor or wall. A reference line should be snapped to mark the rows of cut tiles around the perimeter. A grid of reference lines should be snapped to enclose all full tiles in sections no larger than 3 square feet.

To install tiles, first spread adhesive over a small area or section ( $3 \times 3$ feet), making sure to spread it just up to the lines so that the lines will still be visible. Align the first tile against a $90^{\circ}$ intersection in the grid and press it gently into the adhesive. After each course of tile is applied, use a beating block to level the surface. After all the tiles are set, allow the adhesive to set the required time, according to manufacturer's instructions. Prepare the grout and spread it over the tile surface, ensuring that the joints are filled. When the grout begins to dry, clean the tile with a damp sponge. After the grout has dried, wipe off the haze with a clean rag or towel. After the grout has completely dried and hardened (approximately 72 hours), a grout sealer may be applied.

### 10.4.1 Calculating Requirements for Ceiling Tile

Suspended ceilings are primarily designed for acoustical control; however, ceilings are also lowered to save on heating and air conditioning expenses; finish off exposed joints; and cover damaged plaster.

## Acoustical tile

Acoustical tile absorbs sound, reduces noise, reflects light, and resists flame. Its thickness ranges from 3/16 to $3 / 4$ inch; its width from 12 to 30 inches; and its length from 12 to 60 inches. The most common size panels used are $\overline{2 \times 2} 2$ feet and $2 \times 4$ feet.

## Grid-system components

The grid-system components used in suspended ceilings include the following: the main tee (12-foot lengths), the cross tee ( 2 - and 4 -foot lengths), the wall angle ( 10 -foot lengths), the splice plate (available in aluminum only), suspending devices, and suspending wire.

Suspending devices include screw eyelets; suspending hooks and nails; 8d common nails or larger, driven into wood joists and bent into a U-shape; and an approved Hilti fastener for concrete or steel.

Suspending wire includes 16-gauge anneal wire placed at 4-foot intervals and attached to suspending devices at the ceiling and to the main tees in the grid system.

## Installation

First, lay out the grid pattern. This is based on the ceiling's length and width at the new ceiling height. If the ceiling's length or width is not divisible by 2 feet, increase to the next higher dimension divisible by 2 feet. For example, if a ceiling measures 12 feet 7 inches $\times 10$ feet 4 inches, the dimensions should be increased to 14 feet $\times 12$ feet for layout purposes. Draw the layout on paper. Make sure that the main tees run perpendicular to the joists. Position the main tees so that the border panels at the room's edges are equal and as large as possible. Draw in cross tees so that the border panels at the room's ends are equal and as large as possible. Determine the number of pieces of wall angle by dividing the perimeter by 10 and adding 1 additional piece for any fraction. Determine the number of main tees and cross tees by counting them on the grid pattern layout.

Next, establish the ceiling height. Mark a line around the entire room at the desired height to serve as a reference line. There must be a minimum of 2 inches between the new ceiling and the existing ceiling. Ensure that this line is level and marked continuously so that it meets at intersecting corners. Next, install the wall angle. Secure the wall angle along the reference line, ensuring that it is level.

Install the suspension wire. Suspension wires are required every 4 feet along the main tees and on each side of all splices. Attach the wires to the suspending devices. The wires should be cut at least 2 feet longer than the distance between the old and new ceiling. Now, install the main tees. Main tees need to be laid out from the center to ensure that the slots line up with the cross-tee locations. Cut them where appropriate. Tees 12 feet long or less are installed by resting the ends on opposite wall angles and inserting the suspension wire. Tees over 12 feet long must be cut to ensure that the cross tees will not intersect the main tee at a splice joint. Rest the cut end on the wall angle and attach suspension wires along the tee. Make necessary splices and continue attaching suspension wires along the tee until the tee rests on the opposite wall angle. Ensure that the main tees are level and secured before continuing.

Install the cross tees. Cut and install border tees on one side of the room. Install the remaining cross tees according to the grid-pattern layout. At opposite wall angles, install the remaining border tiles. Finally, install the acoustical panels. Install the full-size panels first. Handle panels with care and ensure that the surfaces are kept clean from hand prints and smudges. If you are working on a large project, work from several cartons to avoid a noticeable change of uniformity. Cut and install the border panels.

Source: Constructionknowledge.org

### 10.4.2 Painting Ceilings and Walls

The following tools and equipment are required for painting:

- Paint brushes, wall, 2 to 4 inches wide
- Paint roller with cover
- Paint pan
- Stepladder
- Paddle (stir stick)
- Rags
- Paint, latex, flat
- Bucket of water

Prepare the paint for application. Remove the cover from the paint container. Remove any film layer from the top of the paint. Using the paddle (stir stick), mix the paint thoroughly, in a figure-8 motion.

Scrape off and break up any unsettled matter on the bottom or lower sides of the container. Pour the paint into the paint pan until it is $2 / 3$ full.

## Ceiling

Brush a narrow strip of paint around the perimeter of the ceiling along the inside edges where the wall and the ceiling meet. Using a roller, paint the remaining portion of the ceiling. Cross roll to ensure complete paint coverage without voids.

## Walls

Brush a narrow strip of paint along the inside corners of the wall and corner post. Cut in around all trim and baseboards with a trim brush. Using a roller, paint the remaining portion of the wall and corner post. The corner post may be painted with a brush. When the first coat has completely dried, apply a second coat in the same way. Ensure that the entire surface is covered and without voids.

### 10.5.0 Types of Carpeting

Having a basic understanding of the various types of carpet construction will help you find the best carpet for your specific situation. For example, some types of carpeting do better in high-traffic areas than others. Some carpets have a more casual appearance, while others look more formal.

Carpeting is basically constructed in two types of piles: loop pile or cut pile. Patterned carpet is a combination of both loop and cut yarns or variations of loops set to different pile heights.

Today's carpet manufacturers and fiber producers are combining softer fibers and better built-in stain resistance with special backings that block odors, spills, and more! These types of features are built-in to many of the better quality carpets offered by the leading carpet manufacturers. For example: see our page on Mohawk SmartStrand Carpet or Shaw Carpets with SoftBac Platinum for some major advancements in carpet construction technology.

Most carpeting today is tufted rather than woven. The tufting process is similar to using a large sewing machine that is about 13 feet wide with 100 of needles that sew the yarn into a synthetic backing. The majority of carpet sold today are made from nylon fibers (such as Solutia's Wear-Dated ${ }^{\circledR}$ carpet fibers) and twisted into yarns and then tufted into carpet.

How long a carpet will retain its like-new texture and appearance is based on the type of carpet fiber, how tight the yarns are twisted and heatset, and the pile density. Also, how tightly the tufts are packed together will also affect the long-term durability of a carpet. Obviously, not cleaning your carpets regularly, or using improper cleaning methods will affect the appearance and life of your carpet as well.

See Berber carpets for information about berber carpet styles.
Below are listed some of the common types of carpet constructions and their main features:
By permission: floorfacts.com
Types of carpeting I carpet construction

## Cut Pile Saxony Carpets

- Generally made in solid colors
- Surface has a smooth appearance
- Generally made with nylon, wool, or polyester fibers
- Good performance and appearance
- Works well with traditional or formal room settings


## Textured Cut Pile Saxony Carpet

- Surface appearance is textured
- Stylish, casual appearance
- Won't show vacuum cleaner marks or footprints
- Very popular carpet style today
- Good choice for active areas of the home


## Frieze Carpet

- Very textured, knobby surface appearance
- Extremely durable and excellent wearing
- Yarns very tightly twisted
- Will cost more than textured cut pile carpets
- Great for active areas of the home


## Cut and Uncut Patterned Carpet

- Intermixed loops and cut pile, creates a patterned design
- Loops are shorter than the cut pile, creating a carved appearance
- Usually constructed in multicolor designs
- Helps hide footprints and traffic patterns
- Great choice for a variety of room settings


## Multilevel Loop Carpets

- Have several different heights of loops
- Generally multicolored
- Very durable, casual appearance
- Offered in many unique-looking designs and patterns
- Helps hides traffic patterns
- Great for family rooms, basements, etc.


## Level Loop Carpets

- Loops are same height and generally multicolored
- Usually made from polypropylene (olefin) carpet fibers
- Often called Indoor-Outdoor or Commercial Carpet
- Casual appearance but extremely durable
- Great for family rooms or basements


### 10.5.1 Calculating the Amount of Carpet Required-Rooms 8-35 Feet in Length and 13-20 Feet in Width

| Your Room Size | Width | Carpets Are 12 Feet Wide | 13 ft | 14 ft | 15 ft | 16 ft | 17 ft | 18 ft | 19 ft | 20 ft |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Length |  |  |  |  |  |  |  |  |  |  |
| 8 ft |  | 10.66 yd | 11.55 | 12.44 | 13.33 | 14.22 | 15.11 | 16 | 16.88 | 17.77 |
| 9 ft |  | 12 yd | 13 yd | 14 yd | 15 yd | 16 yd | 17 | 18 | 19 | 20 |
| 10 ft |  | 13.33 | 14.44 | 15.55 | 16.66 | 17.77 | 18.88 | 20 | 21.11 | 22.22 |
| 11 ft |  | 14.66 | 15.88 | 17.11 | 18.33 | 19.55 | 20.77 | 22 | 23.22 | 24.44 |
| 12 ft |  | 16 | 17.33 | 18.66 | 20 | 21.33 | 22.66 | 24 | 25.33 | 26.66 |
| 13 ft |  | 17.33 | 18.77 | 20.22 | 21.66 | 23.11 | 24.55 | 26 | 27.44 | 28.88 |
| 14 ft |  | 18.66 | 20.22 | 21.77 | 23.33 | 24.88 | 26.44 | 28 | 29.55 | 31.11 |
| 15 ft |  | 20 | 21.66 | 23.33 | 25 | 26.66 | 28.33 | 30 | 31.66 | 33.33 |
| 16 ft |  | 21.33 | 23.11 | 24.88 | 26.66 | 28.44 | 30.22 | 32 | 33.77 | 35.55 |
| 17 ft |  | 22.66 | 24.55 | 26.44 | 28.33 | 30.22 | 32.11 | 34 | 35.88 | 37.77 |
| 18 ft |  | 24 | 26 | 28 | 30 | 32 | 34 | 36 | 38 | 40 |
| 19 ft |  | 25.33 | 27.44 | 29.55 | 31.66 | 33.77 | 35.88 | 38 | 40.11 | 42.22 |
| 20 ft |  | 26.66 | 28.88 | 31.11 | 33.33 | 35.35 | 37.77 | 40 | 42.22 | 44.44 |
| 21 ft |  | 28 | 30.33 | 32.66 | 35 | 37.33 | 39.66 | 42 | 44.33 | 46.66 |
| 22 ft |  | 29.33 | 31.77 | 34.22 | 36.66 | 39.11 | 41.55 | 44 | 46.44 | 48.88 |
| 23 ft |  | 30.66 | 33.22 | 35.77 | 38.33 | 40.88 | 43.44 | 46 | 48.55 | 51.11 |
| 24 ft |  | 32 | 34.66 | 37.33 | 40 | 42.66 | 45.33 | 48 | 50.66 | 53.33 |
| 25 ft |  | 33.33 | 36.11 | 38.88 | 41.66 | 44.44 | 47.22 | 50 | 52.77 | 55.55 |
| 26 ft |  | 34.66 | 37.55 | 40.44 | 43.33 | 46.22 | 49.11 | 52 | 54.88 | 57.77 |
| 27 ft |  | 36 | 39 | 42 | 45 | 48 | 51 | 54 | 57 | 60 |
| 28 ft |  | 37.33 | 40.44 | 43.55 | 45.66 | 49.77 | 52.88 | 56 | 59.11 | 62.22 |
| 29 ft |  | 38.66 | 41.88 | 45.11 | 48.33 | 51.55 | 54.77 | 58 | 61.22 | 64.44 |
| 30 ft |  | 40 | 43.33 | 46.66 | 50 | 53.33 | 56.66 | 60 | 63.33 | 66.66 |
| 31 ft |  | 41.33 | 44.77 | 48.22 | 51.66 | 55.11 | 58.55 | 62 | 65.44 | 68.88 |
| 32 ft |  | 42.66 | 46.22 | 49.77 | 53.33 | 56.88 | 60.44 | 64 | 67.55 | 71.11 |
| 33 ft |  | 44 | 47.66 | 51.33 | 55 | 58.66 | 62.33 | 66 | 69.66 | 73.33 |
| 34 ft |  | 45.33 | 49.11 | 52.88 | 56.33 | 60.44 | 64.22 | 68 | 71.77 | 75.55 |
| 35 ft |  | 46.66 | 50.55 | 54.44 | 58.66 | 62.22 | 66.11 | 70 | 73.88 | 77.77 |

By permission: www.abccarpets.com

### 10.6.0 Solid Hardwood Flooring

Solid wood floors have been used for centuries and never seem to lose their charm and warmth. We generally think of solid hardwood floors as a $3 / 4^{\prime \prime}$ thick plank that comes in a narrow $21 / 4^{\prime \prime}$ strip and has to be finished on the job site. This is the classic hardwood strip floor.

Today, manufacturers offer solid hardwood floors in a variety of widths, thicknesses, finishes, and wood species. The most common North American hardwood species used for solid wood flooring are red oak, white oak, ash, and maple, but you can also get solid hardwood flooring in many exotic wood species, such as Brazilian cherry, tiger wood, Australian cypress, and many others from around the world. Red oak is still the most popular and commonly used hardwood floor.

When we talk about unfinished wood flooring, we generally think of solid wood floors. Unfinished solid oak floors come in several different qualities. These qualities are clear, select and better, \#1 common, and \#2 common. The clear has no visual blemishes or knots and is extremely expensive. The select and better quality has some small knots and very little dark graining, while the \#1 common and \#2 common have more knots and more dark graining. When buying an unfinished solid wood floor, make sure you know which quality you are buying.

Solid wood planks are cut out as a solid block right from the tree. The wood blocks are then sawn into solid flooring planks with tongue and grooves edges. The planks are than either prefinished at the factory or placed into unfinished bundles of varying lengths.

All solid wood floors will react to the presence of moisture. In the winter heating months the lack of humidity can cause solid wood floors to contract, which leaves unsightly gaps between each plank. In the summer months when the humidity is higher the wood planks will expand and the gapping will disappear. If there is too much moisture present, the wood planks may cup or buckle. This is why it is so important to leave the proper expansion gap along all vertical walls and to acclimate the solid wood planks prior to installation. (Engineered wood planks are not nearly as affected by humidity as solid wood floors.

By permission: floorfacts.com

### 10.6.1 Janka Wood Hardness Scale for Wood Flooring Species

Thus a common use of the Janka Hardness Scale is to determine a woods suitability as a wood for hardwood flooring. The higher the number, the greater its resistance to denting as it lives life.

For the woodworker it will be a good indicator of the difficulty of sawing and nailing-or maybe just moving the wood around your shop...

| California Redwood | 420 |
| :--- | ---: |
| Douglas Fir | 660 |
| Southern Yellow Pine (loblolly \& short leaf) | 690 |
| Honduran Mahogany | 800 |
| African Mahogany | 830 |
| South American Lacewood | 840 |
| Southern Yellow Pine (longleaf) | 870 |
| Black Cherry | 950 |
| American Black Walnut | 1010 |
| American Black Walnut Hardwood Flooring |  |
| Peruvian Walnut | 1080 |
| Brazilian Eucalyptus | 1125 |


| Teak | 1155 |
| :---: | :---: |
| Bamboo (carbonized) | 1180 |
| Larch | 1200 |
| Heart Pine | 1225 |
| Caribbean Heart pine | 1240 |
| Yellow Birch | 1260 |
| Red Oak (Northern) | 1290 |
| American Beech | 1300 |
| Ash | 1320 |
| White Oak | 1360 |
| Australian Cypress | 1375 |
| Bamboo (natural) | 1380 |
| Royal Mahogany | 1400 |
| Hard Maple | 1450 |
| African Walnut/Sappelle | 1500 |
| Brazilian Maple | 1500 |
| Zebrawood | 1575 |
| Wenge | 1630 |
| Brazilian Oak | 1650 |
| Bamboo | 1650 |
| Patens | 1691 |
| Peruvian Maple | 1700 |
| Kempas | 1710 |
| African Pedauk (Padeuk/African Cherry) | 1725 |
| Bolivian Rosewood/Morado | 1780 |
| Hickory/Pecan | 1820 |
| Kempas | 1854 |
| Purpleheart | 1860 |
| Jarrah | 1910 |
| Amendoim | 1912 |
| Merbau | 1925 |
| African Rosewood (Bubinga) | 1980 |
| Grapia | 2053 |
| Jarrah | 2082 |
| Purple Heart | 2090 |
| Tigerwood | 2160 |
| Burma Mahogany | 2170 |
| Amberwood | 2200 |
| Cabreuva (Santos Mahogany) | 2200 |
| Caribbean Rosewood | 2300 |
| Mesquite | 2345 |
| Brazilian Cherry (Jatoba) | 2350 |
| Peruvian Cherry | 2350 |
| Red Walnut | 2450 |
| African Cedar / Bosse | 2600 |
| Patagonian Rosewood | 2800 |
| Bloodwood | 2900 |
| Brazilian Rosewood (Tamarindo) | 3000 |
| Brazilian Redwood | 3190 |
| Tiete Rosewood | 3280 |
| Cumaru (Brazilian Teak) | 3540 |
| Southern Chestnut, Tiete Chestnut | 3540 |
| Lapacho (usually grouped with IPE Wood) Ipe Lumber | 3640 |
| Bolivian Cherry | 3650 |


| African Pearwood/Moabi | 3680 |
| :--- | :--- |
| Brazilian Walnut / Ipe Ipe Decking | 3680 |
| Brazilian Ebony | 3692 |
| Patagonian Rosewood | 3840 |
| Brazilian Tiger Mahogany | 3840 |
| Curupy | 3880 |

## Source: woodsthe best.com

### 10.6.2 Laminate Flooring

## Laminate Flooring Construction Review

Laminate flooring comes in both planks and square tiles. It is constructed with several different layers of various materials that are thermofused together to form the laminated flooring planks and tiles.

The four basic laminate flooring construction layers are as follows:

- Wear Layer-This is the transparent top surface that protects the floor from scratching, staining, and scuffing and also protects the printed design layer below. The wear layer is a combination of melamine with aluminum oxide particles which makes it extremely durable.
- In-Register Embossing-Many manufacturers have developed specialized methods of texturizing the top layer (called in-register embossing) to add more authentic realism to the flooring. Many also offer beveled plank edges to give the floors even more of a realistic appearance.
- AC Ratings-Laminate flooring manufacturers have also adopted a method of scoring the durability of the top layer to help consumers with choosing the right laminate floor for their situation. This is called the AC Ratings. AC stands for Abrasion Coefficient. The AC Ratings go from AC1 to AC5, with AC5 being the best. Both the in-store samples and laminate flooring cartons should have their AC Rating marked for consumers to see. For very active areas and kids' playrooms, it's best to choose a laminate floor with an AC Rating of AC3 or greater.
- Photographic Image Layer-This is the photographic image layer of either a real hardwood plank, ceramic tile, stone, or some other material. The photographic images are extremely clear, vibrant, and realistic. Combined with texturizing the top layer, this creates a true, authentic looking, natural floor appearance. For example, some laminate designs are actually photographic images of old historical floors.
- Inner Core Layer-The inner core is generally made from high-density fiberboard and is also used to form the tongue and groove edges for locking laminated planks together. The core is also the base for the photographic image and wear layer. Most manufacturers also saturate the inner core with melamine resins or a water-resistant sealer to help protect the inner core from moisture.
- Backing Layer-This layer is fused to the inner core to add stability and create a barrier that helps protect the planks from moisture and warping. Like the inner core the backing is also treated with some sort of waterresistant sealer.

Note: The inner core combined with the backing layer is what really makes up the overall thickness of each plank. Planks generally range from 8 mm to 12 mm in thickness. The thicker planks are more rigid and help overcome minor irregularities in the subflooring.

By permission: floorfacts.com

## Types of Laminate Floors

Today, there are several different types of laminate floors to choose from, as well as different shapes, thicknesses, and installation locking systems. What they all have in common is they must be floated over the subfloor and have a print layer to give a realistic appearance of a real natural floor material. Laminate floors can be installed on all grade levels and over fully cured concrete slabs, wood subfloors, and some types of existing hard surfaces floors.

## Basic Types of Laminate Floor Construction

High-Pressure Laminate (HPL) - planks are usually fused together in either a one- or two-step process. Several layers are first glued together, and then these layers are combined with the remaining materials and than glued and fused into a plank. This gives a harder finish, more durable plank than DPL.
Direct Pressure Laminate (DPL)—all materials are fused together in one step, which reduces the costs of manufacturing.
Laminate Floor Thickness-laminate floors come in various thicknesses from around 7-8 mm to 12 mm thick. The thicker planks will be slightly more costly but should be sturdier and more durable, especially if the subfloor is not perfectly level.

## Laminate Floor Locking Systems

Depending on the manufacturer, you will find several different types of locking systems for installing the planks together. There are mechanical locking systems (reinforced from underneath by an aluminum, mechanical locking system), specially designed tongue and groove fiber core locking systems, and a few tongue and groove pre-glued systems as well. By far the most popular are the specially designed tongue and groove locking systems that are part of the middle fiber core of each plank. These floors are often referred to as: glueless laminate floors. Most of these glueless floors are snapped together by holding the plank at a 45 degree angle and pressing the tongue into the groove of the other plank.

## Laminate Floor Styles

Laminate floor styles have improved dramatically over the past few years. The print layers have become much better and more realistic looking. Also, some manufacturers have added what is called "embossed in register." This means the plank's surface has the realistic graining and textures found in natural flooring products. Some laminate wood planks now have micro-beveled edges giving the look of many hardwood floors. Obviously, the more realism and more rich design styles will cost more than the lesser grades.

Laminate floor designs are offered in wood plank designs, ceramic tile designs, and natural stone and slate patterns. The tile patterns are usually in squares, although some are offered in long rectangular planks. The patterns will repeat every 3-4 planks in the box. So be sure to lay the planks out and do some dry laying to view the design before actual installation.

## Laminate Construction

The North American Laminate Flooring Association (NALFA) has a Certification Seal for laminate flooring manufacturers. The Seal certifies that the laminate floor has passed a rigorous and demanding series of ANSI tests designed to evaluate the performance, durability, strength, and overall quality of the laminate flooring. Look for it on the manufacturer's sample boards.

The laminated planks are usually fused together in either a one- or two-step process. In the two-step process several layers are first glued together, and then these layers are combined with the remaining materials and then
glued and fused into a plank. This method is called High Pressure Laminate (HPL). The other method is that in which all materials are fused together in one step, and this is called Direct Pressure Laminate (DPL).

## AC Ratings Overview

- AC1—floors with this rating are suitable for low-traffic areas, such as bedrooms
- AC2-floors suitable to low to medium traffic, such as living rooms or dining rooms
- AC3-floors suitable for most areas in homes, including hallways and light commercial
- AC4-anywhere in the home as well as commercial buildings. For example: an office or store
- AC5-can be used in heavy-traffic commercial areas.


## Laminate Flooring Installation Systems

The planks have tongue and grooved edges on all four sides to secure the planks together. Today, most laminate floors use some sort of glueless locking system, often referred to as "clic" floors. Glueless laminate floors can go almost anywhere in the home and are ideal for do-it-yourself projects.

The two main glueless locking systems either involve a tongue and groove that is reinforced from underneath by an aluminum, mechanical locking system or a tongue-and-groove glueless locking system built right into the middle core that allows the planks to snap or click together during installation.

Some other laminate floors have a tongue that was pre-glued at the factory with a specially formulated, wa-ter-resistant glue. Once the tongue is moistened with a wet sponge, it activates the glue and locks the planks together. Laminate floors are also offered that require specially formulated glue to be applied to the tongue and groove at the time of the installation to secure the planks to one another.

## Laminate Flooring Definitions for Some Commonly Used Terms

- Backing-is usually a melamine plastic layer used to give additional structural stability and more moisture protection to the planks.
- Core—generally made from high-density fiber board (HDF), particleboard, or plastic, the core adds impact resistance, and forms the tongue and groove locking system. Melamine plastic resins are also impregnated in the core by some of the manufacturers to improve the moisture resistance of the core.
- Melamine-is a plastic-type resin used throughout the construction process to add durability and stability to the laminated planks.
- Print Film-is also called the decorative layer and gives the floor the appearance of a real hardwood or tile. Some manufacturers have been able to replicate the old wood floors found only in some old historical buildings.
- Wear layer-is a tough clear melamine layer with aluminum oxide particles. Using heat and pressure, the wear layer becomes an incredibly hard and durable finish. The resin-filled wear layer is so dense it becomes extremely difficult to stain, scratch, or burn.
- Underlayment-is a clear thin plastic sheet that is installed over the substrate before the laminate floor is floated. The plastic sheet helps the laminate floor to float freely above the substrate.


### 10.7.0 Finishing of Interior Wood

Interior finishing differs from exterior finishing primarily in that interior woodwork usually requires much less protection against moisture but more exacting standards of appearance and cleanability. A much wider range of finishes and finish methods are possible indoors because weathering does not occur. Good finishes used indoors should last much longer than paint or other coatings on exterior surfaces. The finishing of veneered panels and plywood may still require extra care because these wood composites tend to surface check.

Much of the variation in finishing methods for wood used indoors is caused by the wide latitude in the uses of wood- from wood floors to cutting boards. There is a large range of finishing methods for just furniture. Factory finishing of furniture is often proprietary and may involve more than a dozen steps. Methods for furniture finishing will not be included in this publication; however, most public libraries contain books on furniture finishing. In addition, product literature often contains recommendations for application methods. This section will include general information on wood properties, some products for use in interior finishing, and brief subsections on finishing of wood floors and kitchen utensils.

Color change of wood can sometimes cause concern when using wood in interiors, particularly if the wood is finished to enhance its natural appearance. This color change is a natural aging of the newly cut wood, and nothing can be done to prevent it, except, of course, to keep the wood in the dark. The color change is caused by visible light, not the UV radiation associated with weathering. It is best to keep all paintings and other wall coverings off paneling until most of the color change has occurred. Most of this change occurs within two to three months, depending on the light intensity. If a picture is removed from paneling and there is a color difference caused by shadowing by the picture, it can be corrected by leaving the wood exposed to light. The color will even out within several months.

To avoid knots, the use of finger-jointed lumber has become common for interior trim. As with exterior wood, the quality of the lumber is determined by the poorest board. Pieces of wood for finger-jointed lumber often come from many different trees that have different amounts of extractives and resins. These extractives and resins can discolor the finish, particularly in humid environments such as bathrooms and kitchens. When finishing fingerjointed lumber, it is prudent to use a high-quality stain-blocking primer to minimize discoloration.

### 10.7.1 Opaque, Transparent Finishes, Stains

## Opaque Finishes

The procedures used to paint interior wood surfaces are similar to those used for exterior surfaces. However, interior woodwork, especially wood trim, requires smoother surfaces, better color, and a more lasting sheen. Therefore, enamels or semigloss enamels are preferable to flat paints. Imperfections such as planer marks, hammer marks, and raised grain are accentuated by high-gloss finishes. Raised grain is especially troublesome on flat-grain surfaces of the denser softwoods because the hard bands of latewood are sometimes crushed into the soft earlywood in planning, and later expand when the wood moisture content changes. To obtain the smoothest wood surface, it is helpful to sponge it with water, allow it to dry thoroughly, and sand before finishing. Remove surface dust with a tack cloth. In new buildings, allow woodwork adequate time to come to equilibrium moisture content in the completed building before finishing the woodwork.

To effectively paint hardwoods with large pores, such as oak and ash, the pores must be filled with wood filler (see subsection on wood fillers). The pores are first filled and sanded; then interior primer/sealer, undercoat, and top coat are applied. Knots, particularly in the pines, should be sealed with shellac or a special knotsealer before priming to retard discoloration of light-colored finishes by colored resins in the heartwood of these species. One or two coats of undercoat are next applied, which should completely hide the wood and also provide a surface that can be easily sanded smooth. For best results, the surface should be sanded just before applying the coats of finish. After the final coat has been applied, the finish may be left as is, with its natural gloss, or rubbed to a soft sheen.

## Transparent Finishes

Transparent finishes are often used on hardwoods and some softwood trim and paneling. Most finish processes consist of some combination of the fundamental operations of sanding, staining, filling, sealing, surface coating, and sometimes waxing. Before finishing, planer marks and other blemishes on the wood surface that would be accentuated by the finish must be removed.

## Stains

Some softwoods and hardwoods are often finished without staining, especially if the wood has an attractive color. When stain is used, however, it often accentuates color differences in the wood surface because of unequal absorption into different parts of the grain pattern. With hardwoods, such emphasis of the grain is usually desirable; the best stains for this purpose are dyes dissolved in either water or solvent. The water-soluble stains give the most pleasing results, but they raise the grain of the wood and require extra sanding after they dry.

The most commonly used stains are those that do not raise grain and are dissolved in solvents that dry quickly. These stains often approach the water-soluble stains in clearness and uniformity of color. Stains on softwoods color the earlywood more strongly than the latewood, reversing the natural gradation in color unless the wood has been initially sealed. To give more nearly uniform color, softwoods may be coated with penetrating clear sealer before applying any type of stain. This sealer is often called a "wash coat."

If stain absorbs into wood unevenly causing a blotchy appearance, the tree was probably infected with bacteria and/or blue-stain fungi prior to being cut for lumber. Once the log is cut into lumber, the infection occurs across grain boundaries and makes infected areas more porous than normal wood. When such areas are stained, they absorb excessive amounts of stain very quickly, giving the wood an uneven blotchy appearance. Although this problem is not very common, should it occur it can be difficult to fix. Blue stain on lumber can easily be seen; the infected pieces can either be discarded or sealed before staining. However, bacteria-infected areas cannot be detected prior to staining. If the wood is to be used for furniture or fine woodwork, it might be a good idea to check the lumber, before planing, by applying a stain. Pieces on which the stain appears blotchy should not be used. Sealing the lumber with varnish diluted 50/50 with mineral spirits prior to staining may help; commercial sealers are also available. Bacteria or blue-stain infection may occur in the sapwood of any species, but it seems to be more problematic with the hardwoods because these species tend to be used for furniture, cabinets, and fine woodwork.

Source: U.S. Department of Agriculture.

### 10.7.2 Fillers and Sealers

In hardwoods with large pores, the pores must be filled, usually after staining and before varnish or lacquer is applied, if a smooth coating is desired. The filler may be transparent and not affect the color of the finish, or it may be colored to either match or contrast with the surrounding wood. For finishing purposes, hardwoods may be classified as shown in Table 15-6. Hardwoods with small pores may be finished with paints, enamels, and varnishes in exactly the same manner as softwoods. A filler may be a paste or liquid, natural or colored. Apply the filler by brushing it first across and then with the grain. Remove surplus filler immediately after the glossy wet appearance disappears. First, wipe across the grain of the wood to pack the filler into the pores; then, wipe with a few light strokes along the grain. Allow the filler to dry thoroughly and lightly sand it before finishing the wood.

## Sealers

Sealers are thinned varnish, shellac, or lacquer that are used to prevent absorption of surface coatings and to prevent the bleeding of some stains and fillers into surface coatings, especially lacquer coatings. Lacquer and shellac sealers have the advantage of drying very quickly.

## Surface Coats

Transparent surface coatings over the sealer may be gloss varnish, semigloss varnish, shellac, nitrocellulose lacquer, or wax. Wax provides protection without forming a thick coating and without greatly enhancing the natural luster of the wood. Other coatings are more resinous, especially lacquer and varnish; they accentuate

TABLE 15-6 Classification of Hardwoods by Size of Pores ${ }^{\text {a }}$

| Large Pores | Small Pores |
| :--- | :--- |
| Ash | Aspen |
| Butternut | Basswood |
| Chestnut | Beech |
| Elm | Cherry |
| Hackberry | Cottonwood |
| Hickory | Gum |
| Lauan | Magnolia |
| Mahogany | Maple |
| Mahogany, African | Red alder |
| Oak | Sycamore |
| Sugarberry | Yellow-poplar |
| Walnut |  |

${ }^{a}$ Birch has pores large enough to take wood filler effectively, but small enough to be finished satisfactorily without filling.
the natural luster of some hardwoods and seem to give the surface more "depth." Shellac applied by the laborious process of French polishing probably achieves this impression of depth most fully, but the coating is expensive and easily marred by water. Rubbing varnishes made with resins of high refractive index for light (ability to bend light rays) are nearly as effective as shellac. Lacquers have the advantages of drying rapidly and forming a hard surface, but more applications of lacquer than varnish are required to build up a lustrous coating. If sufficient film buildup is not obtained and the surface is cleaned often, such as the surface of kitchen cabinets, these thin films can fail.

Varnish and lacquer usually dry to a high gloss. To decrease the gloss, surfaces may be rubbed with pumice stone and water or polishing oil. Waterproof sandpaper and water may be used instead of pumice stone. The final sheen varies with the fineness of the powdered pumice stone; coarse powders make a dull surface and fine powders produce a bright sheen. For very smooth surfaces with high polish, the final rubbing is done with rottenstone and oil. Varnish and lacquer made to produce a semigloss or satin finish are also available.

Flat oil finishes commonly called Danish oils are also very popular. This type of finish penetrates the wood and does not form a noticeable film on the surface. Two or more coats of oil are usually applied; the oil may be followed by a paste wax. Such finishes are easily applied and maintained but they are more subject to soiling than is a film-forming type of finish. Simple boiled linseed oil or tung oil are also used extensively as wood finishes.

### 10.7.3 Finishes for Floors

Wood possesses a variety of properties that make it a highly desirable flooring material for homes, factories, and public buildings. A variety of wood flooring products are available, both unfinished and prefinished, in many wood species, grain characteristics, flooring types, and flooring patterns.

The natural color and grain of wood floors accentuate many architectural styles. Floor finishes enhance the natural beauty of wood, protect it from excessive wear and abrasion, and make the floor easier to clean. The finishing process consists of four steps: sanding the surface, applying a filler (for open-grain woods), staining to achieve a desired color effect, and finishing. Detailed procedures and specified materials depend to a great extent on the species of wood used and finish preference.

Careful sanding to provide a smooth surface is essential for a good finish because any irregularities or roughness in the surface will be accentuated by the finish. Development of a top-quality surface requires sanding in
several steps with progressively finer sandpaper, usually with a machine unless the area is small. When sanding is complete, all dust must be removed with a vacuum cleaner and then a tack cloth. Steel wool should not be used on floors unprotected by finish because minute steel particles left in the wood later cause iron stains. A filler is required for wood with large pores, such as oak and walnut, if a smooth, glossy varnish finish is desired (Table 15-6).

Stains are sometimes used to obtain a more nearly uniform color when individual boards vary too much in their natural color. However, stains may also be used to accent the grain pattern. The stain should be an oil-based or non-grain-raising type. Stains penetrate wood only slightly; therefore, the finish should be carefully maintained to prevent wearing through to the wood surface; the clear top-coats must be replaced as they wear. It is difficult to renew the stain at worn spots in a way that will match the color of the surrounding area.

Finishes commonly used for wood floors are classified as sealers or varnishes. Sealers, which are usually thinned varnishes, are widely used for residential flooring. They penetrate the wood just enough to avoid formation of a surface coating of appreciable thickness. Wax is usually applied over the sealer; however, if greater gloss is desired, the sealed floor makes an excellent base for varnish. The thin surface coat of sealer and wax needs more frequent attention than do varnished surfaces. However, re-waxing or resealing and waxing of high-traffic areas is a relatively simple maintenance procedure, as long as the stained surface of the wood hasn't been worn.

Varnish may be based on phenolic, alkyd, epoxy, or polyurethane resins. Varnish forms a distinct coating over the wood and gives a lustrous finish. The kind of service expected usually determines the type of varnish. Varnishes especially designed for homes, schools, gymnasiums, or other public buildings are available. Information on types of floor finishes can be obtained from flooring associations or individual flooring manufacturers.

The durability of floor finishes can be improved by keeping them waxed. Paste waxes generally provide the best appearance and durability. Two coats are recommended, and if a liquid wax is used, additional coats may be necessary to get an adequate film for good performance.

Source: U.S. Department of Agriculture.

### 10.7.4 Finishes for Items Used for Food

The durability and beauty of wood make it an attractive material for bowls, butcher blocks, and other items used to serve or prepare food. A finish also helps keep the wood dry, which makes it less prone to harbor bacteria and less likely to crack. When wood soaks up water, it swells; when it dries out, it shrinks. If the wood dries out rapidly, its surface dries faster than the inside, resulting in cracks and checks. Finishes that repel water will decrease the effects of brief periods of moisture (washing), making the wood easier to clean.

Finishes that form a film on wood, such as varnish or lacquer, may be used but they may eventually chip, crack, and peel. Penetrating finishes, either drying or nondrying, are often a better choice for some products.

## Types of Finish

## Sealers and Drying Oils

Sealers and drying oils penetrate the wood surface, then solidify to form a barrier to liquid water. Many commercial sealers are similar to thinned varnish. These finishes can include a wide range of formulations including polyurethane, alkyds, and modified oils. Unmodified oils such as rung, linseed, and walnut oil can also be used as sealers if they are thinned to penetrate the wood.

## Nondrying Oils

Nondrying oils simply penetrate the wood. They include both vegetable and mineral oils. Vegetable oils (such as olive, com, peanut, and safflower) are edible and are sometimes used to finish wood utensils. Mineral (or paraffin) oil is a nondrying oil from petroleum. Since it is not a natural product, it is not prone to mildew or to harboring bacteria.

## Paraffin Wax

Paraffin wax is similar to paraffin oil but is solid at room temperature. Paraffin wax is one of the simplest ways to finish wood utensils, especially countertops, butcher blocks, and cutting boards.

## Eating Utensils

Wood salad bowls, spoons, and forks used for food service need a finish that is resistant to abrasion, water, acids, and stains and a surface that is easy to clean when soiled.

Appropriate finishes are varnishes and lacquers, penetrating wood sealers and drying oils, and nondrying vegetable oils.

Many varnishes and lacquers are available, and some of these are specifically formulated for use on wood utensils, bowls, and/or cutting boards. These film-forming finishes resist staining and provide a surface that is easy to keep clean; however, they may eventually chip, peel, alligator, or crack. These film-forming finishes should perform well if care is taken to minimize their exposure to water. Utensils finished with such finishes should never be placed in a dishwasher.

Penetrating wood sealers and drying oils may also be used for eating utensils. Some of these may be formulated for use on utensils. Wood sealers and oils absorb into the pores of the wood and fill the cavities of the wood cells. This decreases the absorption of water and makes the surface easy to clean and more resistant to scratching compared with unfinished wood. Penetrating wood sealers are easy to apply and dry quickly. Worn places in the finish may be easily refinished. Some of these finishes, particularly drying oils, should be allowed to dry thoroughly for several weeks before use.

Nondrying vegetable oils are edible and are sometimes used to finish wood utensils. They penetrate the wood surface, improve its resistance to water, and can be refurbished easily. However, such finishes can become rancid and can sometimes impart undesirable odors and/or flavors to food.

Of these finish types, the impermeable varnishes and lacquers may be the best option for bowls and eating utensils; this kind of finish is easiest to keep clean and most resistant to absorption of stains.

Note: Whatever finish is chosen for wood utensils used to store, handle, or eat food, it is important to be sure that the finish is safe and not toxic (poisonous). Also be sure that the finish you select is recommended for use with food or is described as food grade. For information on the safety and toxicity of any finish, check the label, contact the manufacturer and/or the Food and Drug Administration, or consult your local extension home economics expert or county agent.

### 10.7.5 Finishes for Butcher Blocks and Cutting Boards

One of the simplest treatments for wood butcher blocks and cutting boards is the application of melted paraffin wax (the type used for home canning). The wax is melted in a double-boiler over hot water and liberally brushed on the wood surface. Excess wax, which has solidified on the surface, can be melted with an iron to absorb it into the wood, or it may be scraped off. Refinishing is simple and easy. Other penetrating finishes (sealers, drying and nondrying oils) may also be used for butcher blocks and cutting boards. As mentioned in the subsection on eating utensils, vegetable oils may become rancid. If a nondrying oil is desired, mineral oil may be used. Film-forming finishes are not recommended for butcher blocks or cutting boards.

### 10.7.6 Wood Cleaners and Brighteners

The popularity of wood decks and the desire to keep them looking bright and new has led to a proliferation of commercial cleaners and brighteners. Mildew growth on unpainted and painted wood continues to be the primary cause of discoloration. Although it can be removed with a dilute solution of household bleach and detergent, many commercial products are available that can both remove mildew and brighten the wood surface.

The active ingredient in many of these products is sodium percarbonate (disodium peroxypercarbonate). This chemical is an oxidizing agent, as is bleach, and it is an effective mildew cleaner. It also helps brighten the wood surface. Some cleaners and brighteners are reported to restore color to wood. It is not possible to add color to wood by cleaning it. Removing the discoloration reveals the original color. Brightening the wood may make it appear as if it has more color. Once all the colored components of the wood surface have been removed through the weathering process, the surface will be a silvery gray. If color is desired after weathering occurs, it must be added to the wood by staining.

In addition to sodium percarbonate, other oxidizing products may contain hydrogen peroxide by itself or in combination with sodium hydroxide. If sodium hydroxide is used without a brightener, it will darken the wood. Commercial products are also formulated with sodium hypochlorite and/or calcium hypochlorite (household bleach is a solution of sodium hypochlorite). These products usually contain a surfactant or detergent to enhance the cleansing action of the oxidizing agent. Other types of brighteners contain oxalic acid. This chemical removes stains caused by extractives bleed and iron stains and also brightens the wood, but it is not very effective for removing mildew.

### 10.7.7 Paint Strippers

Removing paint and other film-forming finishes from wood is a time-consuming and often difficult process. It is generally not done unless absolutely necessary to refinish the wood. Removing the finish is necessary if the old finish has extensive cross-grain cracking caused by buildup of many layers of paint, particularly oil-based paint. If cracking and peeling are extensive, it is usually best to remove all the paint from the affected area. Total removal of paint is also necessary if the paint has failed by intercoat peeling. It may be necessary to remove paint containing lead; however, if the paint is still sound and it is not illegal to leave it on the structure, it is best to repaint the surface without removing the old paint.

This discussion of paint strippers is limited to film-forming finishes on wood used in structures. Removing paint from furniture can be done using the same methods as described here. Companies that specialize in stripping furniture usually immerse the furniture in a vat of paint stripper and then clean and brighten the wood. This procedure removes the paint very efficiently.

Some of the same methods can be used for the removal of interior and exterior paint. Because of the dust caused by mechanical methods or the fumes given off by chemical strippers, it is extremely important to use effective safety equipment, particularly when working indoors. A good respirator is essential, even if the paint does not contain lead (see Lead-Based Paint).

Note: The dust masks sold in hardware stores do not block chemical fumes and are not very effective against dust.

Two general types of stripping methods are discussed here: mechanical and chemical. The processes are discussed in general terms primarily in regard to their effect on wood; some attention is given to their ease of use and safety requirements. Consult product literature for additional information on appropriate uses and safety precautions.

Source: U.S. Department of Agriculture.

## Mechanical Methods

Finishes can be removed by scraping, sanding, wet or dry sandblasting, spraying with pressurized water (power washing), and using electrically heated pads, hot air guns, and blow torches. Scraping is effective only in removing loosely bonded paint or paint that has already partially peeled from the wood. This method is generally used when paint needs to be removed only from small areas of the structure, and it is usually combined with sanding to feather the edge of the paint still bonded to the wood (see Lead-Based Paint).

When the paint is peeling and partially debonded on large areas of a structure, the finish is usually removed by power washing or wet sandblasting. These methods work well for paint that is loosely bonded to the wood. If the paint is well bonded, complete removal can be difficult without severely damaging the wood surface. The pressure necessary to debond paint from the wood can easily cause deep erosion of the wood. The less dense earlywood erodes more than the dense latewood, leaving behind a surface consisting of latewood, which is more difficult to repaint. Power washing is less damaging to the wood than is wet or dry sandblasting, particularly if low pressure is used. If high pressure is necessary to remove the paint, it is probably bonded well enough that it does not need to be removed for normal refinishing. If more aggressive mechanical methods are required, wet sandblasting can remove even well-bonded paint, but it causes more damage to the wood than does water blasting. Dry sandblasting is not very suitable for removing paint from wood because it can quickly erode the wood surface along with the paint, and it tends to glaze the surface.

A number of power sanders and similar devices are available for complete paint removal. Many of these devices are suitable for removing paint that contains lead; they have attachments for containing the dust. Equipment that has a series of blades similar to a power hand-planer is less likely to "gum up" with paint than equipment that merely sands the surface. Some of this equipment is advertised in the Old House Journal and the Journal of Light Construction. Please consult the manufacturers' technical data sheets for detailed information to determine the suitability of their equipment for your needs and to meet government regulations on lead-containing paint.

Paint can be removed by heating and then scraping it from the wood, but this method must not be used for paint that contains lead. Paint can be softened by using electrically heated pads, hot air guns, or blow torches. Heated pads and hot air guns are slow methods, but they cause little damage to the wood. Sanding is still necessary, but the wood should be sound after the paint is removed. Blow torches have been used to remove paint and, if carefully used, do not damage the wood. Blow torches are extremely hazardous; the flames can easily ignite flammable materials beneath the siding through gaps in the siding. These materials may smolder, undetected, for hours before bursting into flame and causing loss of the structure.

Note: Removing paint with a blow torch is not recommended.
Source:U.S. Department of Agriculture.

## Chemical Methods

If all the paint needs to be removed, then mechanical methods should be used in concert with other methods, such as chemical paint strippers. For all chemical paint strippers, the process involves applying paint stripper, waiting, scraping off the softened paint, washing the wood (and possibly neutralizing the stripper), and sanding the surface to remove the wood damaged by the stripper and/or the raised grain caused by washing. Chemical paint strippers, though tedious to use, are sometimes the most reasonable choice. A range of paint strippers are available. Some are extremely strong chemicals that quickly remove paint but are dangerous to use. Others remove the paint slowly but are safer. With the exception of alkali paint stripper (discussed below), there appears to be an inverse correlation between how safe a product is and how fast it removes the paint.

## Solvent-Based Strippers

Fast-working paint strippers usually contain methylene chloride, a possible carcinogen that can burn eyes and skin. Eye and skin protection and a supplied-air respirator are essential when using this paint stripper. Paint strippers that have methylene chloride can remove paint in as little as 10 minutes. Because of concerns with
methylene chloride, some paint strippers are being formulated using other strong solvents; the same safety precautions should be used with these formulations as with those containing methylene chloride. To remain effective in removing paint, a paint stripper must remain liquid or semiliquid; slow-acting paint stripers are often covered to keep them active. Solvent-type strippers contain a wax that floats to the surface to slow the evaporation of the solvent. Covering the paint stripper with plastic wrap also helps to contain the solvent.

### 10.8.0 Characteristics of Selected Woods for Painting

| Wood Species | Specific <br> Gravity ${ }^{\text {a }}$ <br> Green/ <br> Dry | Shrinkage (\%) ${ }^{\text {b }}$ |  | Paint-holding <br> Characteristic <br> (I, best; V, worst) ${ }^{\text {c }}$ |  | Weathering |  | Color of Heartwood |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Flat <br> Grain | Vertical <br> Grain | Oil-based <br> Paint | Latex <br> Paint | Resistance <br> to Cupping <br> (1, most; 4, <br> least) | Conspicuousness of Checking (1, least; 2, most) |  |
| Softwoods |  |  |  |  |  |  |  |  |
| Baldcypress | 0.42/0.46 | 6.2 | 3.8 | 1 | I | 1 | 1 | Light brown |
| Cedars |  |  |  |  |  |  |  |  |
| Incense | 0.35/0.37 | 5.2 | 3.3 | I | 1 | - | - | Brown |
| Northern white | 0.29/0.31 | 4.9 | 2.2 | I | I | - | - | Light Brown |
| Port-Orford | 0.39/0.43 | 6.9 | 4.6 | I | I | - | 1 | Cream |
| Western red | 0.31/0.32 | 5 | 2.4 | I | I | 1 | 1 | Brown |
| Yellow | 0.42/0.44 | 6 | 2.8 | I | I | 1 | 1 | Yellow |
| Douglas-fir ${ }^{\text {d }}$ | 0.45/0.48 ${ }^{\text {e }}$ | 7.6 | 4.8 | IV | II | 2 | 2 | Pale red |
| Larch, western | 0.48/0.52 | 9.1 | 4.5 | IV | II | 2 | 2 | Brown |
| Pine |  |  |  |  |  |  |  |  |
| Eastern white | 0.34/0.35 | 6.1 | 2.1 | II | II | 2 | 2 | Cream |
| Ponderosa | 0.38/0.42 | 6.2 | 3.9 | III | II | 2 | 2 | Cream |
| Southern ${ }^{\text {d }}$ | 0.47/0.51 ${ }^{\text {f }}$ | 8 | 5 | IV | III | 2 | 2 | Light brown |
| Sugar | 0.34/0.36 | 5.6 | 2.9 | II | II | 2 | 2 | Cream |
| Western white | 0.36/0.38 | 7.4 | 4.1 | II | II | 2 | 2 | Cream |
| Redwood, old growth | 0.38/0.40 | 4.4 | 2.6 | 1 | 1 | 1 | 1 | Dark brown |
| Spruce, Engelmann | 0.33/0.35 | 7.1 | 3.8 | III | II | 2 | 2 | White |
| Tamarack | 0.49/0.53 | 7.4 | 3.7 | IV | - | 2 | 2 | Brown |
| White fir | 0.37/0.39 | 7.0 | 3.3 | III | - | 2 | 2 | White |
| Western hemlock | 0.42/0.45 | 7.8 | 4.2 | III | II | 2 | 2 | Pale brown |
| Hardwoods |  |  |  |  |  |  |  |  |
| Alder | 0.37/0.41 | 7.3 | 4.4 | III | - | - | - | Pale brown |
| Ash, white | 0.55/0.60 | 8 | 5 | V or III | - | 4 | 2 | Light brown |
| Aspen, bigtooth | 0.36/0.39 | 7 | 3.5 | III | II | 2 | 1 | Pale brown |
| Basswood | 0.32/0.37 | 9.3 | 6.6 | III | - | 2 | 2 | Cream |
| Beech | 0.56/0.64 | 11.9 | 5.5 | IV | - | 4 | 2 | Pale brown |
| Birch, yellow | 0.55/0.62 | 9.5 | 7.3 | IV | - | 4 | 2 | Light brown |
| Butternut | 0.36/0.38 | 6.4 | 3.4 | V or III | - | - | - | Light brown |
| Cherry | 0.47/0.50 | 7.1 | 3.7 | IV | - | - | - | Brown |
| Chestnut | 0.40/0.43 | 6.7 | 3.4 | V or III | - | 3 | 2 | Light brown |
| Cottonwood, eastern | 0.37/0.40 | 9.2 | 3.9 | III | II | 4 | 2 | White |


| Wood Species | Specific <br> Gravity <br> Green/ <br> Dry | Shrinkage (\%) |  | Paint-holding Characteristic (I, best; V, worst) |  | Weathering |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Flat <br> Grain | Vertical Grain | Oil-based <br> Paint | Latex <br> Paint | Resistance to Cupping (1, most; 4, least) | Conspicuousness of Checking (1, least; 2, most) | Color of Heartwood |
| Elm, American | 0.46/0.50 | 9.5 | 4.2 | V or III | - | 4 | 2 | Brown |
| Hickory, shagbark | 0.64/0.72 | 11 | 7 | V or IV | - | 4 | 2 | Light brown |
| Lauan plywood | _g | 8 | 4 | IV | - | 2 | 2 | Brown |
| Magnolia, southern | 0.46/0.50 | 6.6 | 5.4 | III | - | 2 | - | Pale brown |
| Maple, sugar | 0.56/0.63 | 9.9 | 4.8 | IV | - | 4 | 2 | Light brown |
| Oak |  |  |  |  |  |  |  |  |
| White | 0.60/0.68 | 8.8 | 4.4 | V or IV | - | 4 | 2 | Brown |
| Northern red | 0.56/0.63 | 8.6 | 4.0 | V or IV | - | 4 | 2 | Brown |
| Sweetgum | 0.46/0.52 | 10.2 | 5.3 | IV | III | 4 | 2 | Brown |
| Sycamore | 0.46/0.49 | 8.4 | 5 | IV | - | - | - | Pale brown |
| Walnut | 0.51/0.55 | 7.8 | 5.5 | V or III | - | 3 | 2 | Dark brown |
| Yellow-poplar | 0.40/0.42 | 8.2 | 4.6 | III | II | 2 | 1 | Pale brown |

${ }^{\text {a }}$ Specific gravity based on weight oven dry and volume at green or $12 \%$ moisture content.
${ }^{b}$ Value obtained by drying from green to oven dry.
${ }^{c}$ Woods ranked in Group V have large pores that require wood filler for durable painting. When pores are properly filled before painting, Group II applies. Vertical-grain lumber was used for cedars and redwood. Other species were primarily flat-grain. Decrease in paintability is caused by a combination of species characteristics, grain orientation, and greater dimensional change of flat-grain lumber. Flat-grain lumber causes at least a 1-unit decrease in paintability.
${ }^{d}$ Lumber and plywood.
${ }^{e}$ Coastal Douglas-fir.
${ }^{\text {f }}$ Loblolly, shortleaf, specific gravity of 0.54/0.59 for longleaf and slash.
${ }^{g}$ Specific gravity of different species varies from 0.33 to 0.55 .

### 10.9.0 When Calculating and Measuring for Interior Trim and Millwork—Learn Tool Basics

## Marking and Measuring Tool Basics

If you're like most woodworkers, you've spent a lot of time picking out the best woodworking machinery, hand planes, chisels, scrapers, rasps, clamps, and all manner of specialized jigs, tools, and accessories that help make your work more accurate and go more smoothly. In the process, have you given much thought to the most fundamental tools in your shop-your marking and measuring tools?

It's worth taking stock of your marking and measuring tool kit. Most of the common problems in woodworking-joints that fit badly, out of square frames and casework, and so on-can be traced back to marking and measuring errors. And the majority of marking and measuring errors are rooted in a simple matter of using the wrong marking and measuring tools. In short, a tape measure just wasn't designed for making the close-tolerance measurements that many woodworking projects require. In this article we'll take a look at a few of the most common marking and measuring tasks in woodworking and at some of the tools that make the process easy, intuitive, and accurate. Then, to help you get set up with a basic kit, we'll pick out a few essentials from Rockler's broad selection of marking and measuring tools.

## Measuring and Marking Linear Dimensions

In most woodworking projects, measuring and marking linear dimensions is the first crucial step, and depending on the project, it can make for some exacting work. In projects that involve intricate joinery and small, closefitting parts, measuring and marking errors as small as a few 100 this of an inch can turn up later as gaps in joints, misaligned parts, and a host of other less-than-appealing results.

Measuring from point A to point B is a simple process, but your results still depend on how accurately you are able to translate a measurement into a physical mark on a piece of wood. If you've ever tried to hold a tape measure flat on a board while you accurately mark off a measurement, you know that just getting a clearly defined mark in exactly the right spot can be a surprising challenge. For precise measuring and marking, the tool you use needs to be readable and, of course, accurately calibrated. Going beyond that, the best distance measuring tools offer a little help in getting the mark in the right place.

Incra Precision Marking Rules are famous for their accuracy, lay flat, and have an easy-to-read scale. But what makes them the tool of choice for linear measuring is that they make it virtually impossible to put a mark in the wrong place. Incra rules are made with micro fine guide holes positioned at $1 / 32^{\prime \prime}$ increments so that, used in conjunction with a mechanical pencil or a metal scribe, you'd really have to try hard to put a measurement mark anywhere other than exactly where it's supposed to be.


Special features of some tools further simplify one of the most common measuring tasks in woodworkingmeasuring and marking a distance from an edge. Tools like the Incra Precision T-Rule and Precision Bend Rule take care of positioning "point A " in the "point A to B " measuring formula, while still offering the famous Incra accuracy and easy-to-use design. All that's left for the woodworker to make a perfectly positioned mark at a precise distance from ah edge is to get the scribe or mechanical pencil in the right guide hole and make a mark.

By permission: rockler.com

## Measuring Squareness

When you put a square on the end of a board to check a cut, you're trusting the "known" 90 degree angle of the square to tell you something about the piece of wood. But how square is a square? Some manufacturers tell you. Crown Hand Tools' rosewood and brass Try-Miter Square is manufactured in accordance with British Standard 3322 , which means that it is accurate within a tolerance range of $.01 \mathrm{~mm} / \mathrm{cm}$, or a little over 2 thousandths of an inch over its $6^{\prime \prime}$ blade. Part of the benefit of knowing the tight tolerance range of the square you are using is knowing that it is manufactured in accordance with a standard. A good many squares on the market don't boast a manufacturing standard at all.


Do fractions of a degree really matter? Often, inaccuracies in angle measurements that you are not even aware of multiply in accordance with the number of slightly off-square marks and cuts you make over the course of a project. When you are joining a large number of parts a tenth of a degree here and a tenth of a degree there, it really starts to add up. Remember, too, that you'll use a square to check the angle of your table saw blade and miter saw fence alignment. Slightly off-square angle settings on these tools are multiplied by two every time you make a joint or join two boards and can noticeably affect the flatness of edge glue-ups and miter joints.

The Crown try-miter square is especially handy because it also allows you to check and mark the second most common angle in woodworking- 45 degrees. A combination square takes that versatility and adds to it with a graduated scale and a blade that can be adjusted and locked into position to gauge depth or distance relative to the edge of a material. The combination square was borrowed from the machinist's tool chest years ago, and because of its all-around usefulness, it remains the "workhorse" square in most woodworking shops.


## Angles Other Than 90 and 45 Degrees

The 90 and 45 degree angles may be the most common in woodworking, but they're not the only ones that come up. For marking angles other than 90 and 45 degrees, most woodworkers use a sliding bevel, "T-bevel." The T-bevel's sliding blade is infinitely positionable and has the added benefit of giving you four possible handle-to-blade angle orientations when the tool is set up in the " T " shape (with some of the blade on either side of the handle).


If you're shopping for a T-bevel, it's important to look for one that has a good lock-down mechanism so that you don't run the risk of accidentally moving the angle setting while you're using the tool. This classic example by Crown with a rosewood body and steel blade cinches down more than well enough to hold a setting for as long as you need it to.

## Measuring Angles

A T-bevel is strictly an angle marking tool; it doesn't tell you anything about the measurement of the angle. For that you need an angle measuring tool, and there you have a few choices. But to simplify matters, we can divide angle measurement into two basic tasks. In general, you'll either want to set a tool or make a mark at a known angle, or you'll want to know the angle measurement of an existing angle, like the angle of a corner where two walls meet.


For cases where you need to set up a marking tool-like a T-bevel—with a known angle, the Mastergaqe Universal Angle Guide is about as good as it gets. The guide is laid out with a computer-guided laser etched angle scale in $1 / 2$ degree increments on heavy gauge aluminum and makes it easy to transfer angle measurements to a T-bevel with dead-on accuracy. A tool that gives you accurate angle settings quickly and easily, like the Universal Angle Guide, is indispensable in working with the odd angles that turn up in complicated joinery projects or in any project where corners meet at other than the "usual" angles.

On the other hand, if you need an accurate measurement of an existing angle, you won't do better than the Starrett Protractor/Angle Finder. The tool is calibrated to read both inside and outside corners and quickly not only gives you the angle of the corner, but also offers the correct miter setting for your saw. Starrett is one of the most trusted names in calibration and measurement tools, so you can be confident that the Protractor/Angle Finder's precision matches its speed. A tool like the Starrett angle finder is an essential angle measurement tool for fitting your work into the real world, which, as anyone who's ever installed cabinets, crown molding, or any kind of trim will tell you, isn't always laid out in perfect 90 degree angles.


## Measuring Depth, Gap, and Thickness

Often, woodworking projects require that you measure a short distance with extreme accuracy. Fine tuning the depth of a rabbet, checking the width of a dado, and measuring the thickness of veneer or stock all call for a tool that will give you extremely precise short distance readings. For these necessarily finicky measuring tasks, you really can't go wrong with a digital caliper.


The digital caliper is the latest advancement in a precision measuring tool that migrated from the machinist's tool kit into the wood shop years ago. They're equipped with sets of jaws that measure inside and outside dimensions with accuracy in the 1000ths of an inch range, and a probe that slides down from the bottom of the tool to gauge depth with equal precision. Calipers are also available in models that have a dial readout and a standard calibrated scale, but the modern digital variety is so easy to use and read that most woodworkers find the slight upcharge for the feature well worth the price.

## Putting Together a Basic Measuring and Marking Kit

At this point, we've just scratched the surface of marking and measuring. There are many other truly useful marking and measuring tools, many of which are extremely helpful in specialized measuring tasks. Wood turners will want to take a look at the J-Square Center Finder and a Wood Turner's Caliper Set. If your projects have you marking off a lot of curved shapes, then you might want to add a set of French Curves or a Flexible Curve to your marking tool collection. And we'd also like to point out that tools that help you get the most accurate results from your woodworking machinery, like calibration tools and precision fences and miter gauges, are in essence measuring tools.


But our purpose is to help you get set up with the marking and measuring tools that we think are "standard equipment" for any woodworking operation. Here are the ten tools that will cover the most common and important marking and measuring tasks in woodworking:


1. Tape Measure. Did we give you the impression that we don't like tape measures? A tape measure will always be an irreplaceable tool.
2. Incra Precision Marking Rule. As we've pointed out, you can't beat this tool for precision measuring and marking.
3. Incra Precision T-Rule. Measuring and marking a distance from an edge is one of the most common tasks in woodworking. The Incra T-Rule is the tool for measurements up to twelve inches.
4. Cabinetmaker's Pencil Set. A quality graphite pencil that sharpens to a micro-fine point for accurate marking.
5. Try -Miter Square or Engineer Square. Judging squareness is so central to woodworking that we think every shop should have a tool that does it accurately.
6. Combination Square. One of the most versatile marking and measuring tools ever introduced into woodworking.
7. T-Bevel. For years and years the T-bevel has been the tool for marking angles other than 90 degrees.
8. Mastergage Universal Angle Guide or Incra Precision Protractor. Either tool will give you the angle measurement precision you need for complex projects.
9. Starrett Protractor/Angle Finder. The best tool for dead-on accurate angle readings.
10. Digital Caliper. There really isn't any other way to get precise measurements of the depth of your rabbets, the width of your dadoes, or the thickness of your stock.

## Plumbing and HVAC Calculations

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### 11.0.0 Water Supply Force Units (WSFUs) Established by the Uniform Plumbing Code Determines the Water Supply Required for Proper Functioning of Plumbing Fixtures. Developing Plumbing Fixtures that Conserve Water, at an Economic Cost, is the Biggest Challenge Facing the Construction Industry Today

The Water Supply Fixture Units (WSFUs) are defined by the Uniform Plumbing Code (UPC) and can be used to determine the water supply to fixtures and their service systems.

| Individual Fixtures | Minimum Fixture Branch Pipe Size (inch) | Water Supply Fixture Units |  |
| :---: | :---: | :---: | :---: |
|  |  | Private Installations | Public Installations |
| Bathtub | 1/2 | 4 | 4 |
| Bathtub with $3 / 4^{\prime \prime}$ fill valve | 3/4 | 10 | 10 |
| Bidet | 1/2 | 1 |  |
| Dishwasher, domestic | 1/2 | 1.5 | 1.5 |
| Drinking fountain | 1/2 | 0.5 | 0.5 |
| Hose Bibb | 1/2 | 2.5 | 2.5 |
| Lavatory | 1/2 | 1 | 1 |
| Bar sink | 1/2 | 1 | 2 |
| Clinic faucet sink | 1/2 | 3 |  |
| Kitchen sink, domestic | 1/2 | 1.5 | 1.5 |
| Laundry sink | 1/2 | 1.5 | 1.5 |
| Service or mop basin | 1/2 | 1.5 | 3 |
| Washup basin | 1/2 | 2 |  |
| Shower head | 1/2 | 2 | 2 |
| Urinal with flush tank | 1/2 | 2 | 2 |
| Wash fountain | 3/4 | 4 |  |
| Water closet with gravity tank | 1/2 | 2.5 | 2.5 |
| Water closet with flushometer tank | 1/2 | 2.5 | 2.5 |
| Water cooler | 1/2 | 0.5 | 0.5 |

### 11.0.1 Mean Daily Residential Water Use as Determined at 12 Study Sites



FIGURE 1 Mean Daily Residential Water Use at 12 Study Sites. Source: Residential End Uses of Water, American Water Works Association Research Foundation (1999), p. xxv.

### 11.0.2 Calculate Usage Based upon Maximum Allowable Water Efficiency Standards for Toilets, Kitchen and Lavatory Faucets, and Shower Heads

| Fixture Type | Maximum Allowable Water Use |
| :---: | :---: |
| Toilets, including gravity tank-type toilets, ${ }^{\text {a }}$ flushometer tank toilets, ${ }^{\text {b }}$ and electromechanical hydraulic toilets ${ }^{\text {c }}$ | 1.6 gallons per flush |
| Kitchen and lavatory faucets (or replacement aerators ${ }^{\text {d }}$ ) | 2.5 gallons per minute, when measured at a flowing water pressure of 80 pounds per square inch |
| Showerheads | 2.5 gallons per minute, when measured at a flowing water pressure of 80 pounds per square inch |
| Urinals | 1.0 gallon per flush |
| ${ }^{a}$ A gravity tank-type toilet is designed to flush by gravity only with wa ${ }^{b}$ A flushometer tank toilet is designed to flush using a flushometer valve, opens the line for direct water flow into the bowl at a rate and predet cAn electromechanical hydraulic toilet is designed to flush using electron or macerators in place of or as an aid to gravity in flushing the toilet bous) ${ }^{d}$ An aerator is an apparatus for controlling water flow (e.g., from faucets) | polied to the bowl. <br> is attached to a pressurized water supply pipe and, when actuated, <br> ed quantity needed to properly operate the toilet. <br> ally controlled devices, such as air compressors, pumps, motors, |

Under the Department of Energy's regulations, water-efficient plumbing fixtures must meet the standards for maximum water consumption. For each model of a regulated plumbing fixture, manufacturers and private labelers must submit a compliance statement to the Department to certify that the model complies with the applicable water conservation standard and that all required testing has been conducted according to the test requirements prescribed in the regulations. In addition, the Department's regulations prohibit manufacturers
and private labelers from distributing in commerce any fixture that does not meet the water conservation standard prescribed under the Energy Policy Act of 1992, and provide for the assessment of a civil penalty of not more than $\$ 110$ per violation.

Source:U.S General Accounting Office.

### 11.0.3 Calculate Water Usage of Various Types of Low- and High-Volume Toilets

## Water Use by Type of Toilet

|  |  | Average water use |  |  |
| :--- | :--- | :---: | :---: | :---: |
| Household Toilet Types | Average Gallons <br> per Flush | Number of <br> Households | Gallons per Toilet <br> per Day | Gallons per Capita <br> per Day |
| Low-flow only | $<2.0$ | 101 | 24.2 | 9.6 |
| Mix of low-flow and higher-volume | $2.0-3.5$ | 311 | 45.4 | 17.6 |
| Higher-volume only | $>4.0$ | 776 | 47.9 | 20.1 |
| All households |  | 1188 | 45.2 | 18.5 |

## Legend

$<$ means less than
$>$ means greater than
Source: Residential End Uses of Water, American Water Works Association Research Foundation, pp. 131-132.

In addition to the comprehensive study by the American Water Works Association's Research Foundation, a number of studies have used similarly sophisticated equipment to measure water flow to individual appliances at a small number of households. The purpose of these studies was to estimate whether water-efficient fixtures reduce water consumption in residences and if so, by how much. Toilets consume the most water in residences, and, as such, they have been the focus of the greatest attention, but showerheads, faucets, and clothes washers have also been considered in these studies, although clothes washers will not be subject to national standards until 2004. The studies all agree that compared with older toilets, ultra-low-flow toilets save significant amounts of water, easily overwhelming any changes in user practices (such as the frequency of flushing).

Source:U.S. General Accounting Office.

### 11.0.4 Reported Savings Due to Use of Low-Flow Toilets in Four Studies

While it is widely believed that the installation of water-efficient showerheads and sink faucets also results in significant savings, the studies we reviewed are not in complete agreement on this point. The East Bay, California, study reported savings of 1.7 gallons per capita per day with low-flow showerheads-about one-third of the savings resulting from toilet replacement. The Tampa, Florida, study reported savings of 3.6 gallons per capita per day-more than half of the savings from toilet replacement.

Reported Savings Attributable to Low-Flow Toilets in Studies Using Precise Measurements

| Location | Date <br> Published | Number of Households | Water Use in Toilets (gal. per capita per day) |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Before <br> Retrofit | After <br> Retrofit | Amount Saved | Percentage Saved |
| Boulder, $\mathrm{CO}^{\text {a }}$ | May 1996 | 14 | 15.9 | 7.6 | 8.3 | 52 |
| East Bay Municipal Utility District, CA | Oct. 1991 | 25 | $12.8{ }^{\text {c }}$ | $6.7^{\text {c }}$ | $5.3{ }^{\text {c }}$ | $41^{\text {c }}$ |
| Seattle, WA ${ }^{\text {b }}$ | July 2000 (draft) | 37 | 18.8 | 8.1 | 10.6 | 57 |
| Tampa, FL | Feb. 1993 | 25 | 13.3 | 7.2 | 6.1 | 46 |

${ }^{a}$ In this study, half of the toilets were replaced with low-flow toilets and half were not; the reported savings were obtained by averaging the results for all toilets-higher-volume and low-flow. For the purpose of this table, we computed the water use and the amount of savings on the basis of the results for the replaced toilets.
${ }^{b}$ We obtained a copy of the draft report on this study. Because the authors are still finalizing the report, we did not have all of the information that would be useful in evaluating the results of this study.
${ }^{c}$ Because the study did not explicitly report the average water use before and after retrofit, we estimated these values by multiplying the average volume per flush by the number of flushes per person. The difference between these values does not equal the amount of savings reported in the study, which was measured separately for each toilet before averaging and, thus, is more accurate.

## Sources

Boulder: Project Report: Measuring Actual Retrofit Savings and Conservation Effectiveness Using Flow Trace Analysis. Prepared for: City of Boulder, Colorado, Utilities Division, Office of Water Conservation, by Aquacraft Water Engineering \& Management (May 16, 1996).
East Bay: East Bay Municipal Utility District Water Conservation Study. Prepared for: East Bay MUD, Oakland, California, A. Aher et al., Stevens Institute of Technology, Building Technology Laboratory, T. P. Konen, Director, Report No. R 219 (Oct. 1991).
Seattle: Draft report prepared for EPA and Seattle Public Utilities, by P. Mayer and W. DeOreo, Aquacraft, Inc., private communication from P. Mayer (July 2000).
Tampa: The Impact of Water Conserving Plumbing Fixtures on Residential Water Use Characteristics: A Case Study in Tampa, Florida. Prepared for: City of Tampa Water Department, Water Conservation Section, by Stevens Institute of Technology and Ayres Associates, T. P. Konen and D. L. Anderson, Principal Investigators (Feb. 1993).

### 11.1.0 Evolution of Low-Flow Toilet Testing Procedures

## Evolution of Industry Testing Requirements for Low-Flow Toilets

| Test Name | Test Description | 1990 <br> Edition | 1995 <br> Edition | Pending Revision, <br> $\mathbf{2 0 0 0}^{\text {a }}$ |
| :--- | :--- | :--- | :--- | :--- |
| Water consumption <br> per flush | To determine average water consumption: average <br> consumption shall not exceed 1.6 gallons. | New | Same | Same |
| Maximum water <br> consumption per flush | To determine maximum water consumption after <br> adjusting trim components for maximum water <br> use: average water consumption shall not exceed <br> 2.4 gallons. | $\mathrm{N} / \mathrm{A}$ | $\mathrm{N} / \mathrm{A}$ | New |
| Ball test | To determine solids removal: 100 polypropylene <br> balls are placed in toilet bowl; 75 must be removed <br> in initial flush. | Same | Same | Deleted; combined <br> with granule test |
| Granule test | To determine solids removal: 2500 polyethylene <br> disc-shaped pellets are placed in toilet bowl; not <br> more than 125 may remain after initial flush. | Same | Same | Adds 100 nylon balls; <br> not more than 3 are <br> allowed to remain after <br> initial flush |


| Test Name | Test Description | 1990 <br> Edition | 1995 <br> Edition | Pending Revision, $2000^{\text {a }}$ |
| :---: | :---: | :---: | :---: | :---: |
| Ink line test | To determine rim washing: a water soluble ink is marked on a bowl's surface; after initial flush, no line segment can exceed $1 / 2$ inch, and aggregate of all segments may not exceed 2 inches. | Same | Same | A second line is added 2 inches below rim jets; this line is completely washed away |
| Dye test | To determine water exchange: a dye is added to bowl; 100\% dilution must occur after initial flush. | Same | Same | Deleted |
| Trap seal test | To determine if trap seal works properly: fixture must return to full trap seal after each flush. | Same | Same | Same |
| Mixed media test | To determine solids removal: 12 sponges and 10 paper balls are used; not more than 4 sponges or balls may remain after initial flush. | N/A | N/A | New |
| Drain line carry test | To determine length of transport of solid wastes: fixture must carry waste a minimum of 40 feet in the drain line. | New | Same | Same |
| Overflow test | To determine leakage of gravity tank-type toilets: tank fill valve is opened to maximum flow for 5 minutes; fixture shall not leak. | N/A | N/A | New |
| Water rise test | To determine wetting of person sitting on seat during flush: a vertically positioned rod is placed 3 inches under the bowl rim; during flush, water should not touch rod. | New | Same | Deleted |
| Rim top and seat fouling test | To determine soiling of rim top and seat: a plate is placed over toilet bowl; no water shall splash on plate during flushing. | N/A | New | Deleted |

Source: U.S. General Accounting Office.

### 11.1.1 Three Common Types of Toilet Construction and Related Efficiencies

There are three common varieties of toilets: gravity flow, (siphon-jet) flush valve, and pressurized tank systems. Similarly, there are four common varieties of urinals: the siphonic jet urinal, washout/wash-down urinals, blowout urinals, and waterless urinals. All of these must meet federal water efficiency standards, though waterless urinals go far beyond the conservation minimums. Composting toilets also use no water, but potential applications are generally limited to national park facilities and small highway rest stops.

## Opportunities

The vast majority of toilets and urinals in federal facilities were installed at a time when there was little or no regard for using water efficiently. Consequently, there are ample opportunities to make significant savings in water usage. Complete replacement is the desired option. Retrofit of existing toilets and urinals is a second choice that may be more attractive if there are budget constraints. While retrofits reduce the amount of water
used per flush, most fixtures were not designed to use reduced amounts of water and their performance may suffer. Only complete replacement of porcelain fixtures ensures that, even with less water, they can still perform efficiently and effectively.

## Technical Information

Toilets account for almost half of a typical building's water consumption. Americans flush about 4.8 billion gallons ( 18.2 billion liters) of water down toilets each day, according to the U.S. Environmental Protection Agency. According to the Plumbing Foundation, replacing all existing toilets with 1.6 gallons ( 6 liters) per flush, ultra-low-flow (ULF) models would save almost 5500 gallons ( 25,000 liters) of water per person each year. A widespread toilet replacement program in New York City apartment buildings found an average 29\% reduction in total water use for the buildings studied. The entire program, in which 1.3 million toilets were replaced, is estimated to be saving 60-80 million gallons (230-300 million liters) per day.

There is a common perception that ULF toilets do not perform adequately. A number of early 1.6 -gallons-per-flush (gpf) (6-liter) gravity-flush toilets that were simply adapted from 3.5- gpf (16-liter) models-rather than being designed from the ground up to operate effectively with the ULF volume-performed very poorly, and some low-cost toilets today still suffer from that problem. But studies show that most 1.6-gpf (6-liter) toilets work very well. Where flush performance is a particular concern, or water conservation beyond that of a $1.6-\mathrm{gpf}$ ( 6 -liter) model is required, pressurized-tank toilets, vacuum toilets, and dual-flush toilets should be considered. Carefully choose toilet models based on recommendations from industry surveys or experienced plumbers and facility managers. You may also want to contact some managers of facilities that have already installed the toilets under consideration.

While some retrofit options for toilets reduce water use (see next page), none of these modifications will perform as effectively or use as little water as quality toilets manufactured after January 1, 1994. These retrofits will merely allow the fixture to operate using less water until it is replaced.

Even greater water conservation can be achieved in certain (limited) applications with composting toilets. Because of the size of composting tanks, lack of knowledge about performance, local regulatory restrictions, and higher first-costs, composting toilets are rarely an option except in certain unique applications, such as national park facilities. Composting toilets are being used very successfully, for example, at Grand Canyon National Park.

With urinals, water conservation well beyond the standard 1.0 -gpf ( 4.5 -liter) performance for new products can be obtained using waterless urinals. These products, available from the Waterless Company, use a special trap with a lightweight biodegradable oil that lets urine and water pass through but prevents odors from escaping into the restroom; there are no valves to fail, and clogging does not cause flooding.

Projected Water Savings from Installing Waterless Urinals

| Building | No. <br> Males | No. <br> Urinals | Uses/ <br> Day | Gal/ <br> Flush | Days/ <br> Year | Ann. Water <br> Gallons | Savings/Urinal <br> Liters |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Small Office | 25 | 1 | 3 | 3.0 | 260 | 58,500 | 220,000 |
| New const. | 25 | 1 | 3 | 1.0 | 260 | 19,500 | 73,800 |
| Restaurant | 150 | 3 | 1 | 3.0 | 360 | 54,000 | 204,000 |
| New const. | 150 | 3 | 1 | 1.0 | 360 | 18,000 | 68,100 |
| School | 300 | 10 | 2 | 3.0 | 185 | 33,300 | 126,000 |
| New const. | 300 | 10 | 2 | 1.0 | 185 | 11,100 | 42,000 |

Source: Environmental Building News, February 1998.
11.2.0 The National Efficiency Standards and Specifications for Residential and Commercial Water Using Fixtures Enacted in 1992 and updated in 2005-Relating to Residential Fixtures


### 11.2.1 Current and Proposed Residential Dishwasher Standards

| Fixtures and Appliances | EPAct 1992, EPAct 2005 (or backlog NAECA updates) |  | WaterSense ${ }^{\circledR}$ or ENERGY STAR ${ }^{\circledR}$ |  | Consortium for Energy Efficiency |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Current Standard | Proposed/Future Standard | Current Specification | Proposed/Future Specification | Current Specification | Proposed/ <br> Future <br> Specification |
| Residential Dishwashers ${ }^{5}$ | Standard models: <br> $\mathrm{EF} \geq 0.46$ cycles/kWh <br> Compact models: <br> EF $\geq 0.62$ cycles/kWh <br> *No specified water use factor <br> (Energy Independence \& Security Act of 2007: <br> As of January 1, 2010 <br> Standard models: <br> 355 kWh/year WF <br> $\leq 6.5$ gallons/cycle <br> Compact models: <br> 260 kWh/year WF <br> $\leq 4.5$ gallons/cycle) | New standards under development: <br> DOE scheduled final action: March 2009; Stakeholder meeting held 4/27/2006 <br> Proposed to DOE Asst. Sec. jointly by AHAM and efficiency advocates to be effective in 2010 Standard models: 355 kWh/year (. 62 EF +1 watt standby) WF $\leq 6.5$ gallons/cycle <br> Compact models: 260 kWh/year WF $\leq 4.5$ gallons/cycle | ENERGY STAR <br> (DOE) <br> Standard models. <br> $\mathrm{EF} \geq 0.65$ <br> cycles/kWh <br> Compact models: <br> $\mathrm{EF} \geq 0.88$ cycles/ <br> kWh <br> *No specified water use factor | Proposed to DOE Asst. Sec. jointly by AHAM \& efficiency advocates to be effective in 2009 Standard models: 324 kWh/year (0.68 EF +1 watt standby) WF $\leq 5.8$ gallons/cycle Compact models: 234 kWh/year WF $\leq 4.0$ gallons/cycle <br> Phase Two (Proposed by DOE): July 1, 2011 Standard models: 307 kWh/yr 5.0 gallons/cycle Compact models: 222 kWh/yr 3.5 gallons/cycle | Standard models: <br> Tier 1: EF $\geq 0.65$ cycles/kWh; maximum $339 \mathrm{kWh} /$ year <br> Tier 2: EF $\geq 0.68$ cycles/kWh; maximum $325 \mathrm{kWh} /$ year Compact models: Tier 1: EF $\geq 0.88$ cycles/kWh; maximum 252 kWh/year *No specified water use factor | In December 2006, CEE announced they will consider adding a water factor in future dishwasher specifications |
| ${ }^{5}$ Standard models <br> DOE: Departmen EPA: Environmen EPAct 1992: Ener EPAct 2005: Ener | capacity is greater than or eq | to eight place settings and six | serving pieces; Compa <br> ns per flush watt hour dified energy factor imum performance | models: capacity is less than <br> NAECA: National Appliance psi: pounds per square inch WF: water factor | t place settings and six <br> gy Conservation Act | erving pieces |

### 11.2.2 Current and Proposed Commercial Plumbing Fixture Water Usage Rates



### 11.2.3 Current and Proposed Commercial Clothes Washers Water/Energy Usage Rates

| Fixtures and Appliances | EPAct 1992, EPAct 2005 (or backlog NAECA updates) |  | WaterSense ${ }^{\circledR}$ or ENERGY STAR ${ }^{\circledR}$ |  | Consortium for Energy Efficiency |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Current <br> Standard | Proposed/Future Standard | Current Specification | Proposed/Future Specification | Current Specification | Proposed/Future Specification |
| Commercial <br> Clothes <br> Washers (Family- <br> sized) | MEF $\geq 1.26 \mathrm{ft}^{3} /$ kWh/cycle; WF $\leq 9.5 \mathrm{gal} /$ cycle/ft ${ }^{3}$ | New standards under development: <br> DOE scheduled final action: January 2010; <br> Stakeholder meeting held 4/27/2006 | ENERGY STAR (DOE) MEF $\geq 1.72 \mathrm{ft}^{3} /$ kWh/cycle; WF $\leq 8.0 \mathrm{gal} /$ cycle/ft ${ }^{3}$ | Effective July 1, 2009 MEF $\geq 1.8 \mathrm{ft}^{3} / \mathrm{kWh} /$ cycle <br> Effective January 1, 2011 <br> MEF $\geq 2.0 \mathrm{ft}^{3} / \mathrm{kWh} /$ cycle $W F \leq 6.0 \mathrm{gal} /$ cycle/ft ${ }^{3}$ | Tier 1: <br> MEF $\geq 1.80 \mathrm{ft}^{3} /$ kWh/cycle; WF $\leq 7.5 \mathrm{gal} /$ cycle/ft ${ }^{3}$ <br> Tier 2: <br> MEF $\geq 2.00 \mathrm{ft}^{3} /$ <br> kWh/cycle; <br> WF $\leq 6.0$ gal/ cycle/ft ${ }^{3}$ <br> Tier 3: <br> MEF $\geq 2.20 \mathrm{ft}^{3} /$ <br> kWh/cycle; <br> WF $\leq 4.5 \mathrm{gal} /$ cycle/ft ${ }^{3}$ |  |
| DOE: Department of EPA: Environmental EPAct 1992: Energy EPAct 2005: Energy <br> Source: U.S. Environ | Energy rotection Agency olicy Act of 1992 olicy Act of 2005 mental Protection Ag | EF: energy factor gpf: gallons per flush <br> $\mathrm{ft}^{3}:$ cubic feet KWh: kilowatt hour <br> gal: gallons MEF: modified energy factor <br> gpm: gallons per minute MaP: maximum performance |  | NAECA: National Appliance Energy Conservation Act psi: pounds per square inch WF: water factor |  |  |

### 11.2.4 Commercial Dishwashers—Only Current Energy Star, Water Sense Specifications Prevail


11.2.5 Automatic Commercial Ice Makers—No Current Standards—Proposed Only for 2010

| Fixtures <br> and <br> Appliances | EPAct 1992, EPAct 2005 (or backlog NAECA updates) |  | WaterSense ${ }^{\text {® }}$ or ENERGY STAR ${ }^{\text {® }}$ |  | Consortium for Energy Efficiency |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Current <br> Standard | Proposed/Future Standard | Current Specification | Proposed/Future Specification | Current Specification | Proposed/ <br> Future <br> Specification |
| Automatic Commercial Ice Makers ${ }^{10}$ | No standard | Effective 1/1/2010: Energy and condenser water efficiency standards vary by equipment type on a sliding scale depending on harvest rate and type of cooling (see link to additional information at end of this table) | No specification | ENERGY STAR (EPA) <br> Effective 1/1/08: Energy <br> and water efficiency <br> standards vary by equipment type on a sliding scale depending on harvest rate. Water cooled machines excluded from Energy Star (see link to additional information at end of this table) | Energy and water (potable and condenser) standards are tiered and vary by equipment type on a sliding scale depending on harvest rate and type of cooling (see link to additional information at end of this table) |  |
| Pre-rinse Spray Valves | Flow rate $\leq$ 1.6 gpm (no pressure specified; no performance requirement) |  | No specification | Proposed ENERGY STAR specification abandoned after standard established in EPAct 2005 <br> WaterSense specification under consideration | No specification (program guidance recommends 1.6 gpm at 60 psi and a cleanability requirement) |  |

${ }^{10}$ Optional standards for other types of automatic ice makers are also authorized under EPAct 2005.

| DOE: Department of Energy | EF: energy factor | gpf: gallons per flush | NAECA: National Appliance Energy Conservation Act |
| :--- | :--- | :--- | :--- |
| EPA: Environmental Protection Agency | $\mathrm{ft}^{3}$ : cubic feet | kWh: kilowatt hour | psi: pounds per square inch |
| EPAct 1992: Energy Policy Act of 1992 | gal: gallons | MEF: modified energy factor | WF: water factor |
| EPAct 2005: Energy Policy Act of 2005 | gpm: gallons per minute | MaP: maximum performance |  |

Source: U.S. Environmental Protection Agency.

### 11.3.0 U.S. Green Building Council LEED (R)—Plumbing Fixture Water Efficiency Goals

The Water Efficiency category of the LEED ${ }^{\circledR}$ rating system is the least emphasized, with a potential of three LEED ${ }^{\circledR}$ points obtainable through Innovative Waste Water Technologies and Water Use Reduction. The LEED ${ }^{\circledR}$ rating system for Water Use Reduction is based on the U.S. Energy Policy Act of 1992. This Act set maximum plumbing fixture flow rates.

Energy Policy Act of 1992

| Fixture: | Maximum Flow Rate: |
| :--- | :--- |
| Water Closet | 1.6 Gallons Per Flush (GPF) |
| Urinals | 1.0 Gallons Per Flush (GPF) |
| Faucets | 2.5 Gallons Per Minute (GPM) |
| Shower Heads | 2.5 Gallons Per Minute (GPM) |

We now have a basis to evaluate sustainable features in a nonresidential building. The purpose of LEED ${ }^{\circledR}$ is to make buildings more efficient and sustainable than the maximum required levels. I will not differentiate between new or existing construction, it should be clear which features will be easiest to implement in the type of building you are evaluating or designing.

## Innovative Waste Water Technologies: WE Credit 2, 1-Point

(Based on LEED ${ }^{\circledR}$-NC Version 2.2 Reference Guide)

## Intent:

To reduce generation of wastewater and potable water demand, while increasing the local aquifer recharge.

## Requirements:

## Option 1

Reduce potable water use for building sewage conveyance by $50 \%$ through the use of water-conserving fixtures (water closets and urinals) or nonpotable water (captured rainwater, recycled graywater, and on-site or municipally treated wastewater).

OR

## Option 2

Treat $50 \%$ of wastewater on-site to tertiary standards. Treated water must be filtered or used on-site.
My experience is that the collection of rainwater is a fairly straightforward method of obtaining this credit point. I recommend the collection of rainwater from the roof rather than parking lots, which contain oils and other hazardous waste contaminants that are more difficult to filter and handle for disposal to be used in the plumbing system. The collection of rainwater requires that the rainwater drainage system be collected and piped to a collection $\operatorname{tank}(s)$ (underground or above ground). The rainwater is in most cases pumped from
the collection tank through a series of filters ( 5 micron to collect the large particles and 50 micron to collect any other solids) and then passed through a UV sterilizer to kill any bacteria. The collected rainwater is now ready to be used to flush the water closets and urinals. Keep in mind that this is nonpotable and should be treated as such. In my designs I provide a backup connection to the potable water system in the building in the event no rainwater is available. The backup connection of potable water is protected by a reducedpressure backflow device to protect the building potable water system from the cross-connected reclaimed rainwater. This system is most likely to be designed in new construction, since an existing building retrofit would be cost prohibitive.

## Example Case:

In this example, we will show potable water calculations for sewage flows for a nonresidential building with an occupant capacity of 100 ( 50 males and 50 females). The calculation is based on a typical 8 -hour workday. Male occupants are assumed to use water closets once and urinals twice a day. Female occupants are assumed to use water closets three times.

## Baseline Case

| Fixture Type | Daily Uses | Flow Rate (GPF) | Occupants | Sewage Generation (GAL) |
| :--- | :---: | :---: | :---: | :---: |
| Water Closet (Male) | 1 | 1.6 | 50 | 80 |
| Water Closet (Female) | 3 | 1.6 | 50 | 240 |
| Urinal (Male) | 2 | 1.0 | 50 | 100 |

Total Daily Volume (GAL) 420
Annual Work Days 260
TOTAL ANNUAL VOLUME (GAL) 109,200

| Design Case |  |  |  |  |
| :--- | :---: | :---: | :---: | :---: |
| Fixture Type | Daily Uses | Flow Rate (GPF) | Occupants | Sewage Generation (GAL) |
| Water Closet (Male) | 1 | 1.2 | 50 | 60 |
| Water Closet (Female) | 3 | 1.2 | 50 | 180 |
| Urinal (Male) | 2 | 0.5 | 50 | 50 |

Total Daily Volume (GAL) 290
Annual Work Days 260
Annual Volume (GAL) 75,400
Rainwater Volume (GAL) $(25,000)$
TOTAL ANNUAL VOLUME (GAL) 50,400
The baseline case flow rates use the maximum flow rates based on the U.S. Energy Policy Act of 1992. Using a combination of water-conserving fixtures and rainwater collection, the design case building indicates a $54 \%$ reduction in potable water volume used for sewage conveyance; this therefore qualifies for the one point credit.

| Baseline Case |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Fixture Type | Daily <br> Uses | Flow Rate (GPF) | Duration (flush) | Auto Controls (N/A) | Occupants | Water Use (GAL) |
| Water Closet (Male) | 1 | 1.6 | 1 |  | 50 | 80 |
| Water Closet (Female) | 3 | 1.6 | 1 |  | 50 | 240 |
| Urinal (Male) | 2 | 1.0 | 1 |  | 50 | 100 |
| Fixture Type | Daily <br> Uses | Flow Rate (GPM) | Duration (Sec) | Auto Controls | Occupants | Water Use (GAL) |
| Lavatory | 3 | 2.5 | 15 | No | 100 | 188 |

Total Daily Volume (GAL) 608
Annual Work Days 260
TOTAL ANNUAL VOLUME (GAL) 158,080

| Design Case |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Fixture Type | Daily Uses | Flow Rate (GPF) | Duration (flush) | Auto Controls (N/A) | Occupants | Water Use (GAL) |
| Water Closet (Male) | 1 | 1.2 | 1 |  | 50 | 60 |
| Water Closet (Female) | 3 | 1.2 | 1 |  | 50 | 180 |
| Urinal (Male) | 2 | 0.5 | 1 |  | 50 | 50 |
| Fixture Type | Daily <br> Uses | Flow Rate (GPM) | Duration (Sec) | Auto Controls | Occupants | Water Use (GAL) |
| Lavatory | 3 | 0.5 | 10 | Yes | 100 | 25 |

Total Daily Volume (GAL) 315
Annual Work Days 260
TOTAL ANNUAL VOLUME (GAL) 81,900
The baseline case flow rates use the maximum flow rates based on the U.S. Energy Policy Act of 1992. Using a combination of water conserving fixtures the design case building indicates a $48 \%$ reduction in potable water volume; this therefore qualifies for the one-point credit for the $20 \%$ reduction plus an additional one point for exceeding the $30 \%$ water-use reduction for a total of two LEED $^{\circledR}$ points. What makes this so simple is the fact that standard fixtures are available from all the major fixture manufacturers to meet these criteria. If we only used a 0.5 GPF urinal and water-saving metering faucets on the lavatories, we would still realize a $35 \%$ water reduction without changing the water closets.

The LEED ${ }^{\circledR}$ point system also allows for Innovative \& Design Process points (maximum of 4). These allow the designer to submit to the USGBC an innovative design concept that might not be covered within the existing point structure, such as Press-Fit copper piping or CSST gas piping. These systems may qualify for an innovative credit point because they use recyclable materials and are solder-less and oil-less, which is environmental friendly.

### 11.3.1 Preexisting State and Local Standards for Water-Efficient Plumbing Fixtures

Preexisting State and Local Standards for Water-Efficient Plumbing Fixtures and Their Status If National Standards Were Repealed

Sixteen states and six localities had water efficiency standards for at least two of the plumbing fixtures regulated under the Energy Policy Act before the national standards took effect in 1994. Table shown below compares the standards adopted by each jurisdiction with the national standards.

State and Local Standards for Water-Efficient Plumbing Fixtures

| State/Locality | Water Efficiency Standard |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Effective Date ${ }^{\text {a }}$ | Ultra-LowFlow Toilets (gal. per flush) | Low-Flow Showerhead (gal. per minute) | Kitchen <br> Faucets (gal. per min.) | Lavatory <br> Faucets (gal. per min.) | Urinals <br> (gal. per flush) |
| National Standard | Jan. 1, 1994 | 1.6 | 2.50 | 2.5 | 2.5 | 1.0 |
| States |  |  |  |  |  |  |
| Arizona | Jan. 1, 1993 | 1.6 | 2.50 | 2.5 | 2.0 | None |
| California | Jan. 1, 1992 | 1.6 | 2.50 | 2.5 | None | 1.0 |
| Connecticut ${ }^{\text {b }}$ | Jan. 1, 1990 | 1.6 | 2.50 | 2.5 | 2.5 | 1.0 |
| Delaware | Apr. 1, 1992 | 1.6 | 2.50 | 2.5 | 2.0 | 1.0 |
| Georgia | Apr. 1, 1992 | 1.6 | 2.50 | 2.5 | 2.0 | 1.0 |
| Maryland | Apr. 1, 1992 | 1.6 | 2.50 | 2.5 | 2.0 | 1.0 |
| Massachusetts | Mar. 2, 1989 | 1.6 | 3.00 | None | None | 1.0 |
| Nevada | Mar. 1, 1993 | 1.6 | 2.50 | 2.5 | 2.5 | 1.0 |
| New Jersey ${ }^{\text {b }}$ | July 1, 1991 | 1.6 | 3.00 | 3.0 | 3.0 | 1.5 |
| New York ${ }^{\text {b }}$ | Jan. 1, 1992 | 1.6 | 3.00 | None | 2.0 | 1.0 |
| North Carolina ${ }^{\text {b }}$ | Jan. 1, 1993 | 1.6 | 3.00 | 3.0 | 3.0 | 1.5 |
| Oregon | July 1, 1993 | 1.6 | 2.50 | 2.5 | 2.5 | 1.0 |
| Rhode Island ${ }^{\text {b }}$ | Mar. 1, 1991 | 1.6 | 2.50 | 2.0 | 2.0 | 1.0 |
| Texas | Jan. 1, 1992 | 1.6 | 2.75 | 2.2 | 2.2 | 1.0 |
| Utah | July 1, 1992 | 1.6 | 2.50 | None | None | None |
| Washington | July 1, 1993 | 1.6 | 2.50 | 2.5 | 2.5 | 1.0 |
| Localities |  |  |  |  |  |  |
| Dade County, FL | Jan. 1, 1992 | 1.6 | 2.50 | 2.5 | 2.0 | 1.0 |
| Denver, CO | Mar. 1, 1992 | 1.6 | 2.50 | 2.2 | 2.2 | 1.0 |
| District of Columbia | Jan. 1, 1992 | 1.6 | 2.50 | 2.5 | 2.0 | 1.0 |
| Hillsborough County, FL | Mar. 26, 1992 | 1.6 | 2.50 | 2.2 | 2.2 | 1.0 |
| Palm Beach, $\mathrm{FL}^{\text {b }}$ | Apr. 1, 1991 | 1.6 | 3.00 | None | None | 1.5 |
| Tampa, FL ${ }^{\text {b }}$ | June 1, 1990 | 2.0 | 2.50 | 2.0 | 2.0 | 1.0 |

### 11.3.2 Projected Reduction in Walter Consumption 2010-2020—With and Without Daily Savings

Projected Reduction in Water Consumption by 2010 and 2020, by Location

| Projected Water Use and Savings in Millions of Gallons per Day |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Average Daily Water Use |  |  | Projected Daily Water Savings |  |
| Location | Population | Year | Without Water Efficiency Standards | With Water Efficiency Standards | Amount | Percentage |
| Austin, TX, City of Austin Water \& Wastewater Utility | 650,000 | 2010 | 167.5 | 160.8 | 6.7 | 4.0 |
|  |  | 2020 | 230.9 | 215.2 | 15.7 | 6.8 |
| Boston, MA, Boston Water \& Sewer Commission | 650,000 | 2010 | 84.2 | 81.0 | 3.2 | 3.8 |
|  |  | 2020 | 85.1 | 79.4 | 5.7 | 6.7 |
| Cary, NC, Town of Cary | 84,779 | 2010 | 16.1 | 15.1 | 1.0 | 6.2 |
|  |  | 2020 | 23.1 | 21.0 | 2.1 | 9.1 |
| Clarksburg, WVa, Clarksburg Water Board | 19,000 | 2010 | 2.9 | 2.8 | 0.1 | 3.4 |
|  |  | 2020 | 3.6 | 3.4 | 0.2 | 5.5 |
| Fort Worth, TX, Fort Worth Water Department | 753,116 | 2010 | 170.0 | 166.6 | 3.4 | 2.0 |
|  |  | 2020 | 178.8 | 172.9 | 5.9 | 3.3 |
| Laurel, MD, Washington Suburban Sanitary Commission | 1,700,000 | 2010 | 206.3 | 199.7 | 6.6 | 3.2 |
|  |  | 2020 | 224.1 | 212.0 | 12.1 | 5.4 |
| Los Angeles, CA, Los Angeles Department of Water \& Power | 3,800,000 | 2010 | 560.6 | 542.1 | 18.5 | 3.3 |
|  |  | 2020 | 560.3 | 527.8 | 32.5 | 5.8 |
| Michigan City, IN, Michigan City Department of Water | 41,000 | 2010 | 12.1 | 11.7 | 0.4 | 3.3 |
|  |  | 2020 | 12.7 | 12.0 | 0.7 | 5.5 |
| Oceanside, CA, City of Oceanside Water | 157,869 | 2010 | 37.5 | 36.6 | 0.9 | 2.4 |
|  |  | 2020 | 46.2 | 44.4 | 1.8 | 3.9 |
| Phoenix, AZ, Phoenix Water Services | 1,252,425 | 2010 | 341.4 | 331.5 | 9.9 | 2.9 |
|  |  | 2020 | 393.6 | 375.1 | 18.5 | 4.7 |
| Pinellas County, FL, Pinellas County Utilities | 643,191 | 2010 | 84.4 | 80.6 | 3.8 | 4.5 |

### 11.4.0 Where Does Our Water Come From? Volume of Earth's Oceans

| Ice | 29,492 | 2.2 | Much of this ice is in the Antarctic |
| :--- | :--- | :--- | :--- |
| Groundwater | 6,733 | 0.5 | Underground aquifers, deep wells |
| Lakes | 242 | 0.02 | Provide drinking water, irrigation water, fish and recreation |
| Soil Moisture | 74 | 0.005 | This is being used by our crops, trees, and surface vegetation |
| Water Vapor in the Atmosphere | 14 | 0.001 | Clouds, fog, and dew |
| Rivers | 1.3 | 0.0001 | Provide water for drinking, irrigation, and recreation |

Adapted from: Environment Canada.

One Estimate of Global Water Distribution

| Water Source | Water Volume, <br> in cubic miles | Water Volume, <br> in cubic kilometers | Percent of <br> Fresh Water | Percent of <br> Total Water |
| :--- | ---: | :---: | :---: | :---: |
| Oceans, Seas, \& Bays | $321,000,000$ | $1,338,000,000$ | - | 96.5 |
| Ice caps, Glaciers, \& Permanent Snow | $5,773,000$ | $24,064,000$ | 68.7 | 1.74 |
| Groundwater | $5,614,000$ | $23,400,000$ | - | 1.7 |
| Fresh | $2,526,000$ | $10,530,000$ | 30.1 | 0.76 |
| Saline | $3,088,000$ | $12,870,000$ | - | 0.94 |
| Soil Moisture | 3,959 | 16,500 | 0.05 | 0.001 |
| Ground Ice \& Permafrost | 71,970 | 300,000 | 0.86 | 0.022 |
| Lakes | 42,320 | 176,400 | - | 0.013 |
| Fresh | 21,830 | 91,000 | 0.26 | 0.007 |
| Saline | 20,490 | 85,400 | - | 0.006 |

### 11.5.0 How Much Water Do We Use on Average?

## Managing Water Resources

Having a reliable and safe supply of fresh water is very important for us to stay alive. The way that many of us live our lives means that we use a lot more fresh water than we need just to stay alive. If the number of people in the world increases and they all use more fresh water, we will not be able to supply all the fresh water that is wanted. One way of helping to solve this problem is for each of us to use fresh water more carefully.

## Activity 1: Making an estimate

Here's how to estimate how much water you use each day:
Fill in the first column of the table below for the volume of water that you use, on average, each day. Then calculate how much water you use in a year.

In some cases you will need to work out your share of the water used in your home. These cases are marked with a * in the table. For example, if your family uses a washing machine to wash clothes three times a week this uses $3 \times 120=360$ liters per week. On average, this is $360 \div 7=51$ liters per day. If there are five members of your family, this is just over 10 liters per day for each member of the family.

You will probably find the following average values helpful:

- Brushing teeth: 0.01-1 liter
- Cooking a meal: 1-5 liters
- Flushing toilet: 5-10 liters
- Washing hands: 1-3 liters
- Dish washer: 30-50 liters
- Washing machine: 30-100 liters
- Shower: 1-40 liters
- Cleaning car: 5-200 liters
- Drinking: 1-2 liters
- Watering garden: $1-17$ liters per $\mathrm{m}^{2}$
- Bath: $50-150$ liters
(Washing clothes or dishes by hand only uses a quarter as much water.)
To convert these UNESCO litres to gallons
1 liter $=0.264$ gallons
3 liters $=0.79$ gallons
5 liters $=1.3$ galloons
10 liters $=2.64$ gallons
17 liters $=4.49$ gallons
30 liters $=7.925$ gallons
40 liters $=10.57$ gallons
50 liters $=13.21$ gallons
150 liters $=39.6$ galloons
200 liters $=52.8$ gallons


### 11.5.1 Create a Personal Water Usage Chart

## Activity Handout How Much Water do You Use?

Survey: How Much Water Do You Use?
Directions This is a survey to find how much water you use in your home during one full week. Place a tally mark in the Times per Day column every time someone in your family does the activity.

| Activity | Times per Day |  |  |  |  |  |  | Weekly Total | Water per Activity* | Total Water Used |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Sun | Mon | Tues | Wed | Thurs | Fri | Sat |  |  |  |
| Toilet Flushing |  |  |  |  | $+$ |  | $+$ |  | $\times 5$ gallons $=$ |  |
| Short Shower (5-10 minutes) |  | $+$ |  |  | $+$ |  | $+\quad=$ |  | $\times 25$ gallons $=$ |  |
| Long Shower ( $>10$ minutes) |  |  |  |  | $+$ |  | $+\quad=$ |  | $\times 35$ gallons $=$ |  |
| Tub Bath |  | $+$ |  | $\stackrel{+}{+}$ | $+$ |  | $+\quad=$ |  | $\times 35$ gallons $=$ |  |
| Teeth Brushing |  |  | $\stackrel{+}{+}$ | $\stackrel{+}{+}$ | $+$ |  | $+$ |  | $\times 2$ gallons $=$ |  |
| Washing Dishes with Running Water |  |  |  |  | $+$ |  | $+$ |  | $\times 30$ gallons $=$ |  |
| Washing Dishes Filling a Basin |  |  |  |  | $+$ |  | $+$ |  | $\times 10$ gallons $=$ |  |
| Using Dishwasher |  | $+$ | $\stackrel{+}{+}$ | + | $+$ |  | $+\quad=$ |  | $\times 20$ gallons $=$ |  |
| Washing Clothes |  |  |  | $\stackrel{+}{+}$ | $+$ |  | $+\quad=$ | GRAND | $\begin{aligned} \times 40 \text { gallons } & = \\ \text { TOTAL } & = \end{aligned}$ |  |

NOTE: Another significant seasonal water use is lawn and garden watering. This survey deals with daily water use in the home, but most of us use additional amounts of water at school, at work, and other places throughout the day.
*These are estimated values.
Source: U.S. Environmental Protection Agency.

### 11.6.0 Calculating the Size of Storage and Heat Pump Water Heaters

## U.S. Department of Energy-Energy Efficiency and Renewable Energy Energy Savers

## Sizing Storage and Heat Pump (with Tank) Water Heaters

To properly size a storage water heater-including a heat pump water heater with a tank- for your home, use the water heater's first hour rating (FHR). The FHR is the amount of hot water in gallons the heater can supply per hour (starting with a tank full of hot water). It depends on the tank capacity, source of heat (burner or element), and the size of the burner or element.

The EnergyGuide Label lists the FHR in the top left corner as "Capacity (first hour rating)." The Federal Trade Commission requires an EnergyGuide Label on all new conventional storage water heaters but not on heat pump water heaters. Product literature from a manufacturer may also provide the FHR. Look for water heater models with a FHR that matches within 1 or 2 gallons of your peak hour demand-the daily peak 1-hour hot water demand for your home.

Do the following to estimate your peak hour demand:

- Determine what time of day (morning, noon, evening) you use the most hot water in your home. Keep in mind the number of people living in your home.
- Use the worksheet below to estimate your maximum usage of hot water during this one hour of the day-this is your peak hour demand. Note: The worksheet does not estimate total daily hot water usage.

The worksheet example shows a total peak hour demand of 46 gallons. Therefore, this household would need a water heater model with a first hour rating of 44 to 48 gallons.

Worksheet for Estimating Peak Hour Demand/First Hour Rating

| Use | Average Gallons of Hot <br> Water per Usage | Times Used <br> during 1 Hour | Gallons Used <br> in 1 Hour |  |
| :--- | :---: | :---: | :--- | :--- |
| Shower | 12 | $\times$ | $=$ |  |
| Bath | 9 | $\times$ | $=$ |  |
| Shaving | 2 | $\times$ | $=$ |  |
| Hands and face washing | 4 | $\times$ | $=$ |  |
| Hair shampoo | 4 | $\times$ | $=$ |  |
| Hand dishwashing | 4 | $\times$ | $=$ |  |
| Automatic dishwasher | 14 | $\times$ | $=$ |  |
| Food preparation | 5 | $\times$ | $=$ |  |
| Wringer clothes washer | 26 | $\times$ |  | $=$ |
| Automatic clothes washer | 32 | $\times$ |  | $=$ |
|  |  |  | Total Peak | $=$ |
|  |  |  | Hour Demand |  |


| Example |  |  |  |  |  |
| :--- | ---: | :--- | :--- | :--- | ---: |
| 3 showers | 12 | $\times$ | 3 | $=$ | 36 |
| 1 shave | 2 | $\times$ | 1 | $=$ | 2 |
| 1 shampoo | 4 | $\times$ | 1 | $=$ | 4 |
| 1 hand dishwashing | 4 | $\times$ | 1 | $=$ | 4 |
| Peak Hour Demand |  |  | $=$ | 46 |  |
|  |  |  |  |  |  |

Before selecting a storage water heater, you also want to consider the following:

- Fuel type and availability
- Energy efficiency
- Cost.

If you haven't yet considered what type of water heater might be best for your home, here are your options:

- Conventional storage water heater
- Demand (tankless or instantaneous) water heater
- Heat pump water heaters
- Solar water heater
- Tankless coil and indirect water heaters


### 11.6.1 Calculating the Cost of a Demand, Storage, or Heat Pump Water Heater

## U.S. Department of Energy—Energy Efficiency and Renewable Energy Energy Savers

## Estimating a Storage, Demand, or Heat Pump Water Heater's Costs

When considering a water heater model for your home, estimate its annual operating cost. Then compare costs with other more and/or less energy-efficient models. This will help you determine the energy savings and payback period of investing in a more energy-efficient model, which will probably have a higher purchase price.

Before you can choose and compare the costs of various models, you need to determine the correct size water heater for your home. If you haven't done this already, see the following:

- Sizing a Demand (Tankless or Instantaneous) Water Heater
- Sizing Storage and Heat Pump (with Tank) Water Heaters


## Calculating Annual Operating Cost

To estimate the annual operating cost of a storage, demand (tankless or instaneous), or heat pump (not geothermal heat pump) water heater, you need to know the following about the model:

- Energy factor (EF)
- Fuel type and cost (your local utility can provide current rates)

Then, use the following calculations:

## For gas and oil water heaters

You need to know the unit cost of fuel by Btu (British thermal unit) or therm. (1 therm $=100,000 \mathrm{Btu}$ )
$365 \times 41045 / \mathrm{EF} \times$ Fuel Cost $(\mathrm{Btu})=$ estimated annual cost of operation
OR
$365 \times 0.4105 / \mathrm{EF} \times$ Fuel Cost $($ therm $)=$ estimated annual cost of operation
Example: A natural gas water heater with an EF of .57 and a fuel cost of $\$ 0.00000619 / \mathrm{Btu}$
$365 \times 41045 / .57 \times \$ 0.00000619=\$ 163$
For electric water heaters, including heat pump units
You need to know or convert the unit cost of electricity by kilowatt-hour (kWh).
$365 \times 12.03 / \mathrm{EF} \times$ Electricity Cost by $\mathrm{kWh}=$ estimated annual cost of operation
Example: A heat pump water heater with an EF of 2.0 and a electricity cost of $\$ 0.0842 / \mathrm{kWh}$
$365 \times 12.03 / 2.0 \times \$ 0.0842=\$ 185$

## Comparing Costs and Determining Payback

Once you know the purchase and annual operating costs of the water heater models you want to compare, you can use the following table to determine the cost savings and payback of the more energy-efficient model(s).

| Models | Price of Water EF <br> Heater | Estimated Annual Operating Cost |
| :--- | :--- | :--- |
| Model A |  |  |
| Model B (higher EF) | Price of Model B - Price of Model A $=$ \$Additional <br> Cost of Model B |  |
| Additional cost of more efficient <br> model (Model B) | Model B Annual Operating Cost - Model A Annual <br> Operating Cost $=\$$ Model B's Cost Savings per Year |  |
| Estimated annual operating cost <br> savings (Model B) | \$Additional Cost of Model B/\$Model B's Cost Savings per <br> Year = Payback period/years |  |
| Payback period for Model B |  |  |

## Example

Comparison of two gas water heaters with a local fuel cost of $\$ 0.60$ per therm.

| Models | Price of Water <br> Heater | EF | Estimated Annual <br> Operating Cost |
| :--- | :--- | :--- | :--- |
| Model A | $\$ 165$ | 0.54 | $\$ 166$ |
| Model B | $\$ 210$ | 0.58 | $\$ 155$ |
| Additional cost of more efficient model <br> (Model B) | $\$ 210-\$ 165=\$ 45$ |  |  |
| Estimated annual operating cost savings <br> (Model B) | $\$ 166-\$ 155=\$ 11$ per year |  |  |
| Payback period for Model B | $\$ 45 / \$ 11$ per year $=4.1$ years |  |  |

## Other Costs

If you want to include installation and maintenance costs, consult the manufacturer(s) and a qualified contractor to help estimate these costs. These costs will vary among system types and sometimes even from water heater model to model.

### 11.7.0 HVAC—Understanding and Calculating Relative Humidity

## Relative Humidity

The amount of water vapor in the air at any given time is usually less than that required to saturate the air. The relative humidity is the percentage of saturation humidity, generally calculated in relation to saturated vapor density.

$$
\text { Relative Humidity }=\frac{\text { actual vapor density }}{\text { saturation vapor density }} \times 100 \%
$$

The most common units for vapor density are $\mathrm{gm} / \mathrm{m}^{3}$. For example, if the actual vapor density is $10 \mathrm{~g} / \mathrm{m}^{3}$ at $20^{\circ} \mathrm{C}$ compared to the saturation vapor density at that temperature of $17.3 \mathrm{~g} / \mathrm{m}^{3}$, then the relative humidity is

$$
R . H .=\frac{10 \mathrm{~g} / \mathrm{m}^{3}}{17.3 \mathrm{~g} / \mathrm{m}^{3}} \times 100 \%=57.8 \% \quad \text { Calculation }
$$

Careful! There are dangers and possible misconceptions in these common statements about relative humidity.
Relative humidity is the amount of moisture in the air compared to what the air can "hold" at that temperature. When the air can't "hold" all the moisture, then it condenses as dew.

## What's the Problem?

Saturation vapor pressure Dewpoint Relative humidity calculation

### 11.7.1 HVAC—Understanding and Calculating Dewpoint

## Dewpoint

If the air is gradually cooled while maintaining the moisture content constant, the relative humidity will rise until it reaches $100 \%$. This temperature, at which the moisture content in the air will saturate the air, is called the dew point. If the air is cooled further, some of the moisture will condense.


### 11.7.2 Methods of Calculating Heating Efficiency-Combined Heat and Power (CHP) Methods for Calculating Efficiency

CHP is an efficient and clean approach to generating power and thermal energy from a single fuel source. It is used either to replace or supplement conventional separate heat and power (SHP) (i.e., central station electricity available via the grid and an onsite boiler or heater).

- CHP System Efficiency Defined
- Key Terms Used in Calculating CHP Efficiency
- Calculating Total System Efficiency
- Calculating Effective Electric Efficiency
- Which CHP Efficiency Metric Should You Select?


## Basic Information

- Catalog of CHP Technologies
- Biomass CHP
- Efficiency Benefits
- Reliability Benefits
- Environmental Benefits
- Economic Benefits


## CHP System Efficiency Defined

Every CHP application involves the recovery of otherwise wasted thermal energy to produce additional power or useful thermal energy. Because CHP is highly efficient, it reduces emissions of traditional air pollutants and carbon dioxide, the leading greenhouse gas associated with global climate change.

Efficiency is a prominent metric used to evaluate CHP performance and compare it to SHP.
The illustration below illustrates the potential efficiency gains of CHP when compared to SHP.

## Conventional Generation versus CHP: Overall Efficiency



In this example of a typical CHP system, to produce 75 units of useful energy, the conventional generation or separate heat and power systems use 154 units of energy- 98 for electricity production and 56 to produce heatresulting in an overall efficiency of $49 \%$. However, the CHP system needs only 100 units of energy to produce the 75 units of useful energy from a single fuel source, resulting in a total system efficiency of $75 \%$.

Source: U.S. Environmental Protection Agency.

## Key Terms Used in Calculating CHP Efficiency

Calculating a CHP system's efficiency requires an understanding of several key terms, described as follows:

- CHP system. The CHP system includes the unit in which fuel is consumed (e.g., turbine, boiler, engine), the electric generator, and the heat recovery unit that transforms otherwise wasted heat to usable thermal energy.
- Total fuel energy input ( $\mathbf{Q}_{\text {FUEL }}$ ). The thermal energy associated with the total fuel input. Total fuel input is the sum of all the fuel used by the CHP system. The total fuel energy input is often determined by multiplying the quantity of fuel consumed by the heating value of the fuel.

Commonly accepted heating values for natural gas, coal, and diesel fuel are:

- 1020 Btu per cubic foot of natural gas
- 10,157 Btu per pound of coal
- 138,000 Btu per gallon of diesel fuel
- Net useful power output $\left(\mathbf{W}_{\mathbf{E}}\right)$. Net useful power output is the gross power produced by the electric generator minus any parasitic electric losses-in other words, the electrical power used to support the CHP system. (An example of a parasitic electric loss is the electricity that may be used to compress the natural gas before the gas can be fired in a turbine.)
- Net useful thermal output $\left(\Sigma Q_{\mathbf{T H}}\right)$. Net useful thermal output is equal to the gross useful thermal output of the CHP system minus the thermal input. An example of thermal input is the energy of the condensate return and makeup water fed to a heat recovery steam generator (HRSG). Net useful thermal output represents the otherwise wasted thermal energy that was recovered by the CHP system.

Gross useful thermal output is the thermal output of a CHP system utilized by the host facility. The term utilized is important here. Any thermal output that is not used should not be considered. Consider, for example, a CHP system that produces 10,000 pounds of steam per hour, with $90 \%$ of the steam used for space heating and the remaining $10 \%$ exhausted in a cooling tower. The energy content of 9,000 pounds of steam per hour is the gross useful thermal output.

## Calculating Total System Efficiency

The most commonly used approach to determining a CHP system's efficiency is to calculate total system efficiency. Also known as thermal efficiency, the total system efficiency ( $\eta_{\mathrm{o}}$ ) of a CHP system is the sum of the net useful power output $\left(\mathrm{W}_{\mathrm{E}}\right)$ and net useful thermal outputs $\left(\Sigma \mathrm{Q}_{\mathrm{TH}}\right)$ divided by the total fuel input $\left(\mathrm{Q}_{\mathrm{FUEL}}\right)$, as shown below:

$$
\eta_{\mathrm{o}}=\frac{\mathrm{W}_{\mathrm{E}}+\Sigma \mathrm{Q}_{\mathrm{TH}}}{\mathrm{Q}_{\mathrm{FUEL}}}
$$

The calculation of total system efficiency is a simple and useful method that evaluates what is produced (i.e., power and thermal output) compared to what is consumed (i.e., fuel). CHP systems with a relatively high net useful thermal output typically correspond to total system efficiencies in the range of 60 to $85 \%$.

Note that this metric does not differentiate between the value of the power output and the thermal output; instead, it treats power output and thermal output as additive properties with the same relative value. In reality and in practice, thermal output and power output are not interchangeable because they cannot be converted easily from one to another. However, typical CHP applications have coincident power and thermal demands that must be met. It is reasonable, therefore, to consider the values of power and thermal output from a CHP system to be equal in many situations.

## Calculating Effective Electric Efficiency

Effective electric efficiency calculations allow for a direct comparison of CHP to conventional power generation system performance (e.g., electricity produced from central stations, which is how the majority of electricity is produced in the United States). Effective electric efficiency ( $\xi \mathrm{EE}$ ) can be calculated using the equation below, where $\left(\mathrm{W}_{\mathrm{E}}\right)$ is the net useful power output, $\left(\Sigma \mathrm{Q}_{\mathrm{TH}}\right)$ is the sum of the net useful thermal outputs, $\left(\mathrm{Q}_{\mathrm{FUEL}}\right)$ is the
total fuel input, and $\eta$ equals the efficiency of the conventional technology that otherwise would be used to produce the useful thermal energy output if the CHP system did not exist:

$$
\varepsilon_{\mathrm{EE}}=\frac{\mathrm{W}_{\mathrm{E}}}{\mathrm{Q}_{\mathrm{FUEL}}-\sum\left(\mathrm{Q}_{\mathrm{TH}} / \alpha\right)}
$$

For example, if a CHP system is fired by natural gas and produces steam, then $\alpha$ represents the efficiency of a conventional natural gas-fired boiler. Typical $\alpha$ values for boilers are 0.8 for natural gas-fired boiler, 0.75 for a biomass-fired boiler, and 0.83 for a coal-fired boiler.

The calculation of effective electric efficiency is essentially the CHP net electric output divided by the additional fuel the CHP system consumes over and above what conventional systems would have used to produce the thermal output for the site. In other words, this metric measures how effectively the CHP system generates power once the thermal demand of a site has been met.

Typical effective electrical efficiencies for combustion turbine-based CHP systems are in the range of 51 to $69 \%$. Typical effective electrical efficiencies for reciprocating engine-based CHP systems are in the range of 69 to $84 \%$.

## Which CHP Efficiency Metric Should You Select?

The selection of an efficiency metric depends on the purpose of calculating CHP efficiency.

- If the objective is to compare CHP system energy efficiency to the efficiency of a site's SHP options, then the total system efficiency metric may be the right choice. Calculation of SHP efficiency is a weighted average (based on a CHP system's net useful power output and net useful thermal output) of the efficiencies of the SHP production components. The separate power production component is typically $33 \%$ efficient grid power. The separate heat production component is typically a 75 to $85 \%$ efficient boiler.
- If CHP electrical efficiency is needed for a comparison of CHP to conventional electricity production (i.e., the grid), then the effective electric efficiency metric may be the right choice. Effective electric efficiency accounts for the multiple outputs of CHP and allows for a direct comparison of CHP and conventional electricity production by crediting that portion of the CHP system's fuel input allocated to thermal output.

Both the total system and effective electric efficiencies are valid metrics for evaluating CHP system efficiency. They both consider all the outputs of CHP systems and, when used properly, reflect the inherent advantages of CHP. However, since each metric measures a different performance characteristic, use of the two different metrics for a given CHP system produces different values.

For example, consider a gas turbine CHP system that produces steam for space heating with the following characteristics:

| Fuel Input (MMBtu/hr) | 41 |
| :--- | ---: |
| Electric Output (MW) | 3.0 |
| Thermal Output (MMBtu/hr) | 17.7 |

Using the total system efficiency metric, the CHP system efficiency is $68 \%(3.0 * 3.413+17.7) / 41)$.
Using the effective electric efficiency metric, the CHP system efficiency is 54\% (3.0*3.413)/(41 - (17.7/0.8).
This is not a unique example; a CHP system's total system efficiency and effective electric efficiency often differ by 5 to $15 \%$.

NOTE: Many CHP systems are designed to meet a host site's unique power and thermal demand characteristics. As a result, a truly accurate measure of a CHP system's efficiency may require additional information and broader examination beyond what is described in this document.

### 11.7.3 How Much Moisture Can the Air "Hold"?

Careful! There are dangers and possible misconceptions in these common statements about relative humidity.
Relative humidity is the amount of moisture in the air compared to what the air can "hold" at that temperature. When the air can't "hold" all the moisture, then it condenses as dew.

Of all the statements about relative humidity that I have heard in everyday conversation, the above is probably the most common. It may represent understanding of the phenomenon and has some common-sense utility, but it may represent a complete misunderstanding of what is going on physically. The air doesn't "hold" water vapor in the sense of having some attractive force or capturing influence. Water molecules are actually lighter and higher speed than the nitrogen and oxygen molecules that make up the bulk of the air, and they certainly don't stick to them and are not in any sense held by them. If you examine the thermal energy of molecules in the air at a room temperature of $20^{\circ} \mathrm{C}$, you find that the average speed of a water molecule in the air is over $600 \mathrm{~m} / \mathrm{s}$ or over 1400 miles/hr! You are not going to "hold" that molecule!

Another possibly helpful perspective would be to consider the space between air molecules under normal atmospheric conditions. From knowledge of atomic masses and gas densities and the modeling of the mean free path of gas molecules, we can conclude that the separation between air molecules at atmospheric pressure and $20^{\circ} \mathrm{C}$ is about 10 times their diameter. They will typically travel on the order of 30 times that separation between collisions. So water molecules in the air have a lot of room to move about and are not "held" by the air molecules.

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When one says that the air can "hold" a certain amount of water vapor, the fact that is being addressed is that a certain amount of water vapor can be resident in the air as a constituent of the air. The high-speed water molecules act, to a good approximation, as particles of an ideal gas. At an atmospheric pressure of 760 mm Hg , you can express the amount of water in the air in terms of a partial pressure in mm Hg that represents the vapor pressure contributed by the water molecules. For example, at $20^{\circ} \mathrm{C}$, the saturation vapor pressure for water vapor is 17.54 mm Hg , so if the air is saturated with water vapor, the dominant atmospheric constituents nitrogen and oxygen are contributing most of the other 742 mm Hg of the atmospheric pressure.

But water vapor is a very different type of air constituent than oxygen and nitrogen. Oxygen and nitrogen are always gases at Earth temperatures, having boiling points of 90 K and 77 K , respectively. Practically, they always act as ideal gases. But extraordinary water has a boiling point of $100^{\circ} \mathrm{C}=373.15 \mathrm{~K}$ and can exist in solid, liquid, and gaseous phases on the Earth. It is essentially always in a process of dynamic exchange of molecules between these phases. In air at $20^{\circ} \mathrm{C}$, if the vapor pressure has reached 17.54 mm Hg , then as many water molecules are entering the liquid phase as are escaping to the gas phase, so we say that the vapor is "saturated." It has nothing to do with the air "holding" the molecules, but common usage often suggests that. As the air approaches saturation, we say that we are approaching the "dewpoint." The water molecules are polar and will exhibit some net attractive force on each other and therefore begin to depart from ideal gas behavior. By collecting together and entering the liquid state, they can form droplets in the atmosphere to make clouds, or near the surface to form fog, or on surfaces to form dew.

Another approach that might help clarify the point that air does not actually "hold" water is to note that the relative humidity really has nothing to do with the air molecules (i.e., $\mathrm{N}_{2}$ and $\mathrm{O}_{2}$ ). If a closed flask at $20^{\circ} \mathrm{C}$ had liquid water in it but no air at all, it would reach equilibrium at the saturated vapor pressure 17.54 mm Hg . At that point it would have a vapor density of $17.3 \mathrm{gm} / \mathrm{m}^{3}$ of pure water vapor in the gas phase above the water surface. But if you had just removed the air and sealed the container with liquid water in it, you might have a situation where there was only $8.65 \mathrm{gm} / \mathrm{m}^{3}$ resident in the gas phase at that particular moment. We would say that the relative humidity in the flask is $50 \%$ at that point because the resident water vapor density is half its saturation density. That is exactly the same thing we would say if the air were present $-8.65 \mathrm{gm} / \mathrm{m}^{3}$ of water vapor in the air at $20^{\circ} \mathrm{C}$ represents $50 \%$ relative humidity. Under these conditions, water molecules would be evaporating from the surface into the gas phase faster than they would be entering the water surface, so the vapor pressure of the water vapor above the surface would be rising toward the saturation vapor pressure.

### 11.7.4 General Heating Formulas—Energy Required to Heat, Offset Losses

## General heating formulas

Energy required for heat up:

$$
\frac{M \times c \times \text { Delta } T}{3214}=\mathrm{kW} \text { hours/hours for heat up }=\mathrm{kW}
$$

$\mathrm{M}=$ Weight of Material in lbs.
$\mathrm{c}=$ Specific Heat of Material
Delta $\mathrm{T}=$ Desired temperature - starting temperature
hrs = heat up time expressed in hours
Energy required to offset losses:

$$
\frac{\operatorname{Losses}(\text { Watts }) / \text { square } \mathrm{ft} \times \text { area }(\text { square } \mathrm{ft})}{1000}=\mathrm{kW} \text { required to offset losses }
$$

Energy required to change state:

$$
\frac{\text { Weight }(\mathrm{lbs} .) \times \text { latent heat of fusion }}{3412 \times \text { heat up time }(\text { in hrs } .)}=\mathrm{kW}
$$

### 11.7.5 General Heating Formulas—Energy Required to Heat Air Flow

## Air heating formulas

Energy required to heat air flow: (approx.)

$$
\mathrm{kW}=\frac{\mathrm{SCFM} \times(\mathrm{T} 2-\mathrm{T} 1)^{\circ} \mathrm{F}}{2500}
$$

$\mathrm{kW}=$ Energy (Kilowatts) required to heat flow
SCFM = Air flow rate in standard cubic feet per minute
$\mathrm{T}_{2}=$ Temperature at heater exit
$\mathrm{T}_{1}=$ Temperature at heater inlet
$1.2=$ Efficiency factor (assumes 20\% energy is lost)

### 11.7.6 Formula to Convert Actual CFM (ACFM) to Standard Cubic Feet per Minute (SCFM)

Converting actual CFM (ACFM) to standard cubic feet per minute (SCFM):

$$
\mathrm{SCFM}=\mathrm{ACFM} \times((\operatorname{Pg}+\mathrm{Patm}) / \text { Patm }) \times((\text { Tref }+460) /(\text { Tact }+460))
$$

SCFM = Air flow in standard cubic feet per minute (corrected for temp. and pressure)
ACFM = Air flow in actual cubic feet per minute (uncorrected)
$\mathrm{Pg}=$ Gage pressure (psig)
Patm $=$ Atmospheric pressure ( 14.7 psia )
Tact $=$ Actual air temperature $\left({ }^{\circ} \mathrm{F}\right)$
Tref $=$ Reference air temperature $\left(70^{\circ} \mathrm{F}\right)$
Constants and conversion formulas:
Density of Air at $32^{\circ} \mathrm{F}$ and $14.7 \mathrm{psia}=1.293 \mathrm{~kg} / \mathrm{m}^{3}\left(.081 \mathrm{lbs} / \mathrm{ft}^{3}\right)$
Kilowatts $\times 3412=\mathrm{BTU} / \mathrm{hr}$

Meters ${ }^{3} /$ hour $\times 35.3=\mathrm{ft}^{3} / \mathrm{hr}$
Liters $/ \mathrm{min} \times 2.12=\mathrm{ft}^{3} / \mathrm{hr}$
CFH/60 $=$ CFM
${ }^{\circ} \mathrm{C}=5 / 9\left({ }^{\circ} \mathrm{F}-32\right)$
${ }^{\circ} \mathrm{F}=9 / 5^{\circ} \mathrm{C}+32$

### 11.8.0 Estimated Average Fuel Conversion Efficiency of Common Heating Appliances

Estimated Average Fuel Conversion Efficiency of Common Heating Appliances

| Fuel Type-Heating Equipment | Efficiency (\%) |
| :--- | :---: |
| Coal (bituminous) |  |
| Central heating, hand-fired | 45.0 |
| Central heating, stoker-fired | 60.0 |
| Water heating, pot stove (50 gal.) | 14.5 |
| Oil |  |
| High-efficiency central heating | 89.0 |
| Typical central heating | 80.0 |
| Water heater (50 gal.) | 59.5 |
| Gas |  |
| High-efficiency central furnace | 97.0 |
| Typical central boiler | 85.0 |
| Minimum efficiency central furnace | 78.0 |
| Room heater, unvented | 99.0 |
| Room heater, vented | 65.0 |
| Water heater (50 gal.) | 62.0 |
| Electricity | 99.0 |
| Baseboard, resistance | 97.0 |
| Central heating, forced air | $200+$ |
| Central heating, heat pump | $300+$ |
| Ground source heat pump | 97.0 |
| Water heaters (50 gal.) |  |
| Wood and Pellets | $30.0-40.0$ |
| Franklin stoves | $40.0-70.0$ |
| Stoves with circulating fans | $65.0-75.0$ |
| Catalytic stoves | $85.0-90.0$ |
| Pellet stoves |  |

Armed with these numbers, several "Evaluation Tools" above will allow you to calculate the cost of energy delivered to your home for different fuels and systems. The actual cost of fuels is best determined by examining your utility bills or contacting your utility and by contacting local suppliers of other fuels. The DOE's Energy Information Administration also tracks some fuel prices.

### 11.8.1 Reading Those Gas Meters

## Calculating Fuel Consumption and Carbon Footprint of the Glass Studio

Calculating approximate fuel consumption and carbon output involves several things. With gas, it is necessary to isolate each piece of equipment, and, using a gas meter, clock the actual consumption of the unit, both in various phases of firing and as a total for the day or week. Those using natural gas are likely to have a gas meter for the studio. The older meters are of the multiple spinning dial variety where the consumption can be measured quickly on the dial clocking half a cubic foot, or perhaps 2 cubic feet.


Newer models look more like an odometer, and the numbers turn only after 100 cubic feet have passed through. This is cheaper for the gas company and easy for the meter reader, but it makes clocking the consumption of individual units tedious.

| 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 1 | 2 | 3 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |

### 11.9.0 Calculating Home Heating Energy—Gas versus Electric Resistance Heating Calculating Home Heating Energy

Heat transfer from your home can occur by conduction, convection, and radiation. It is typically modeled in terms of conduction, although infiltration through walls and around windows can contribute a significant additional loss if they are not well sealed. Radiation loss can be minimized by using foil-backed insulation as a radiation barrier.

The U.S. heating and air conditioning industry uses almost entirely the old British and U.S. common units for their calculations. For compatibility with the commonly encountered quantities, this example will be expressed in those units.


Heat loss rate $=\frac{\mathrm{Q}}{\mathrm{t}}=\frac{(\text { Area }) \times\left(\mathrm{T}_{\text {inside }}-\mathrm{T}_{\text {outside }}\right)}{\text { Thermal resistance of wall }}$
If $\mathrm{Q} / \mathrm{t}$ is in $\mathrm{BTU} / \mathrm{hr}$
Area in $\mathrm{ft}^{2}$
$\mathrm{T}_{\text {in }}-\mathrm{T}_{\text {out }}$ in ${ }^{\circ} \mathrm{F}$
then the thermal resistance is the "R-factor" quoted by insulation manufacturers. The units of the R -factor are

$$
\frac{\mathrm{ft}^{2} \times{ }^{\circ} \mathrm{F}}{\mathrm{BTU} / \mathrm{hr}}
$$

For standard R11 wall insulation, you lose $1 / 11 \mathrm{BTU} / \mathrm{hr}$ per square foot of wall space, per degree Fahrenheit temperature difference.
I. Calculate wall-loss rate in BTUs per hour.

For a 10 ft by 10 ft room with an 8 ft ceiling, with all surfaces insulated to R 19 as recommended by the U.S. Department of Energy, with inside temperature $68^{\circ} \mathrm{F}$ and outside temperature $28^{\circ} \mathrm{F}$ :

$$
\text { Heat loss rate }=\frac{\left(320 \mathrm{ft}^{2}\right) \times\left(68^{\circ} \mathrm{F}-28^{\circ} \mathrm{F}\right)}{19 \frac{\mathrm{ft}^{2} \times{ }^{\circ} \mathrm{F}}{\mathrm{BTU} / \mathrm{hr}}}=674 \mathrm{BTU} / \mathrm{hr}
$$

II. Calculate loss per day at these temperatures.

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$$
\text { Heat loss per day }=(674 \mathrm{BTU} / \mathrm{hr})(24 \mathrm{hr})=16168 \mathrm{BTU}
$$

Note that this is just the loss through the walls. The loss through the floor and ceiling is a separate calculation, and usually involves different R -values.
III. Calculate loss per "degree day."

This is the loss per day with a one degree difference between inside and outside temperature.

$$
\text { Loss per degree day }=\mathrm{Q}=\frac{\left(320 \mathrm{ft}^{2}\right) \times\left(1^{\circ} \mathrm{F}\right)}{19 \frac{\mathrm{ft}^{2} \times{ }^{\circ} \mathrm{F}}{\mathrm{BTU} / \mathrm{hr}}} \times 24 \mathrm{hr} / \text { day }=404 \frac{\mathrm{BTU}}{\text { degree day }}
$$

If the conditions of case II prevailed all day, you would require 40 degree-days of heating, and therefore require 40 degree-days $\times 404$ BTU/degree-day $=16168$ BTU to keep the inside temperature constant.
IV. Calculate heat loss for entire heating season.

The typical heating requirement for an Atlanta heating season, September to May, is 2980 degree-days (a long-term average).

$$
\text { Heat loss }=\mathrm{Q}=404 \frac{\mathrm{BTU}}{\text { degree day }} \times 2980 \text { degree days }=1.20 \text { million } \mathrm{BTU}
$$

The typical number of degree-days of heating or cooling for a given geographical location can usually be obtained from the weather service.
V. Calculate heat loss per heating season for a typical uninsulated southern house in Atlanta.

The range of loss rates given by DOE for uninsulated typical dwellings is 15,000 to $30,000 \mathrm{BTU} /$ degree-day. Choosing 25,000 BTU/degree-day:

$$
\text { Heat loss }=\mathrm{Q}=25,000 \frac{\mathrm{BTU}}{\text { degree day }} \times 2980 \text { degree days }=74.5 \text { million } \mathrm{BTU}
$$

VI. Calculate annual heating cost.

Assume natural gas cost of $\$ 12$ per million BTU in a furnace operating at $70 \%$ efficiency.

$$
\left[\frac{74.5 \text { million BTU }}{0.70}\right]\left[\frac{\$ 12}{\text { million BTU }}\right]=\$ 1277
$$

Assume electric resistance heating at $100 \%$ efficiency*, $9 \not \subset / \mathrm{kWh}$.

$$
\left(7.45 \times 10^{7} \mathrm{BTU}\right)\left(2.93 \times 10^{-4} \mathrm{kWh} / \mathrm{BTU}\right)(\$ .09 / \mathrm{kWh})=\$ 1965
$$

Assume an electric heat pump with coefficient of performance $=3$

$$
\frac{\$ 1965}{3}=\$ 655
$$

When you are heating with natural gas, you are using the primary fuel at your house, and this is clearly preferable to using electric resistance heating, which is wasteful of the high-quality delivered electric energy. By using an electric heat pump, at least in the southern United States, you can get a coefficient of performance of about 3 and just about balance off that $3: 1$ loss in the generation process. In the above example, the electric heat pump is considerably cheaper, but that may be an artifact of the currently high natural gas prices. Over the last 25 years or so, natural gas and electric heat pump heating have stayed comparable in cost.

### 11.10.0 Comparing Fuel Costs of Heating and Cooling Systems-Gas, Electric, Kerosene, Wood, Pellets



[^31]
## Comparing Fuel Costs of Heating and Cooling Systems

June 2003

## Introduction

One of the most common questions posed to energy specialists at Engineering Extension asks for a comparison between costs to operate different heating and cooling systems. It might be a comparison of a furnace to a heat pump, a regular furnace to a high-efficiency furnace, or a wood burning stove to a pellet stove.

There are two components to cost: the initial cost to purchase and install the system, and the ongoing fuel cost. In general, higher efficiency equipment costs more initially but saves operating costs. To determine the purchase price, get bids from one or more contractors. Be certain bids include all costs to make the system fully functional, including duct work, thermostats, and chimneys. The accompanying fact sheets will help you compare the cost of fuel for several types of heating and cooling systems.

Annual cost of delivering heating and cooling to a home depends on cost of the fuel, the efficiency with which the system converts the fuel source into heating or cooling energy, and the quantity of heating and cooling required. The following section, "Estimating the Cost of Heating or Cooling," allows you to estimate the cost of one million Btus for several fuels and system types. However, if you want to compare annual estimated costs for two or more fuels, then you will also need to estimate the heating load of your home.

## What Is the Price of Fuel?

Fuel prices vary between suppliers, may change seasonally, and are affected by world events. To estimate fuel costs, you can either contact your local utility or supplier or you can use past billings.

To estimate natural gas costs from your utility bill, divide the monthly charge by the consumption, usually measured in MCF (MCF $=1000$ Cubic feet). The cost should be between $\$ 3$ and $\$ 12$ per MCF. Use a winter bill so that meter charges are spread out over several units of gas. If your bill shows gas consumption in CCF $(C C F=100$ Cubic feet $)$, you will need to multiply the gas cost by 10 to get it in dollars per MCF.

To obtain an average $\$$ per kilowatt hour ( kWh ), divide the total monthly cost by the consumption in kWh . Use a midwinter bill if you want to estimate heating costs and a midsummer bill if you want to estimate cooling costs. The cost for electricity in Kansas varies from $\$ 0.04$ to $\$ 0.15$ per kWh.

Propane, fuel oil, wood, and pellets are sold in simple units and should be easy to determine.

## Estimating the Cost of Heating or Cooling

If you just want to compare operating costs of different systems, you can use Tables 1 through 7 to directly determine the cost of delivering one million Btus (MBTUs) of heating and Table 8 for one MBTU of cooling. For example, you could compare the cost of delivering one MBTU to your home from a high-efficiency natural gas furnace to the cost of delivering one MBTU from wood in a modern wood stove. There are several measures of system efficiency. A brief explanation is provided in the description of the tables.

Table 1 is for natural gas furnaces and boilers. There are three efficiency levels, and gas prices range from $\$ 5$ to $\$ 15$ per thousand cubic feet (MCF). If your furnace was installed before about 1985, use the "older equipment" column. If you have a modern but normal-efficiency unit, use the $78 \%$ column. The last column is for high-efficiency (condensing) equipment.

Tables 2 and 3 are similar to Table 1, but are for propane and fuel oil, respectively.
Modern natural gas, propane, and fuel oil furnaces and boilers receive an annual fuel-utilization efficiency (AFUE) rating. Older units were not rated but an assumed performance of $65 \%$ is reasonable.

TABLE 1 Natural Gas Heating Costs-\$ per MBTU Delivered for Three Appliance Efficiencies

|  |  | Furnace or Boiler efficiency |  |
| :--- | :---: | :---: | :---: |
| Gas Price | 65\% (low) Older <br> Equipment | AFUE $=\mathbf{7 8 \%}$ (average) <br> Current Minimum | AFUE $=\mathbf{9 5 \%}$ High <br> Efficiency |
| $\$ 5.00$ | $\$ 7.69$ | $\$ 6.41$ | $\$ 5.26$ |
| $\$ 5.50$ | $\$ 8.46$ | $\$ 7.05$ | $\$ 5.79$ |
| $\$ 6.00$ | $\$ 9.23$ | $\$ 7.69$ | $\$ 6.32$ |
| $\$ 6.50$ | $\$ 10.00$ | $\$ 8.33$ | $\$ 6.84$ |
| $\$ 7.00$ | $\$ 10.77$ | $\$ 8.97$ | $\$ 7.37$ |
| $\$ 7.50$ | $\$ 11.54$ | $\$ 9.62$ | $\$ 7.89$ |
| $\$ 8.00$ | $\$ 12.31$ | $\$ 10.26$ | $\$ 8.42$ |
| $\$ 8.50$ | $\$ 13.08$ | $\$ 10.90$ | $\$ 8.95$ |
| $\$ 9.00$ | $\$ 13.85$ | $\$ 11.54$ | $\$ 9.47$ |
| $\$ 9.50$ | $\$ 14.62$ | $\$ 12.18$ | $\$ 10.00$ |
| $\$ 10.00$ | $\$ 15.38$ | $\$ 12.82$ | $\$ 10.53$ |
| $\$ 10.50$ | $\$ 16.15$ | $\$ 13.46$ | $\$ 11.05$ |
| $\$ 11.00$ | $\$ 16.92$ | $\$ 14.10$ | $\$ 11.58$ |
| $\$ 11.50$ | $\$ 17.69$ | $\$ 14.74$ | $\$ 12.11$ |
| $\$ 12.00$ | $\$ 18.46$ | $\$ 15.38$ | $\$ 12.63$ |
| $\$ 12.50$ | $\$ 19.23$ | $\$ 16.03$ | $\$ 13.16$ |
| $\$ 13.00$ | $\$ 20.00$ | $\$ 16.67$ | $\$ 13.68$ |
| $\$ 13.50$ | $\$ 20.77$ | $\$ 17.31$ | $\$ 14.21$ |
| $\$ 14.00$ | $\$ 21.54$ | $\$ 17.95$ | $\$ 14.74$ |
| $\$ 14.50$ | $\$ 22.31$ | $\$ 18.59$ | $\$ 15.26$ |
| $\$ 15.00$ | $\$ 23.08$ | $\$ 19.23$ | $\$ 15.79$ |

TABLE 2 Propane Heating Costs - \$ per MBTU Delivered for Three Appliance Efficiencies

|  | Furnace or Boiler Efficiency |  |  |
| :--- | :---: | :---: | :---: |
| Propane | 65\% (low) Older | AFUE $=\mathbf{7 8} \%$ (average) | AFUE $=\mathbf{9 5 \%}$ High |
| Price \$/gal. | Equipment | Current Minimum | Efficiency |
| $\$ 0.60$ | $\$ 10.14$ | $\$ 8.24$ | $\$ 6.94$ |
| $\$ 0.65$ | $\$ 10.99$ | $\$ 8.93$ | $\$ 7.52$ |
| $\$ 0.70$ | $\$ 1.83$ | $\$ 9.62$ | $\$ 8.10$ |
| $\$ 0.75$ | $\$ 12.68$ | $\$ 10.30$ | $\$ 8.68$ |
| $\$ 0.80$ | $\$ 13.52$ | $\$ 10.99$ | $\$ 9.25$ |
| $\$ 0.85$ | $\$ 14.37$ | $\$ 11.68$ | $\$ 9.83$ |
| $\$ 0.90$ | $\$ 15.22$ | $\$ 12.36$ | $\$ 10.41$ |
| $\$ 0.95$ | $\$ 16.06$ | $\$ 13.05$ | $\$ 10.99$ |
| $\$ 1.00$ | $\$ 16.91$ | $\$ 13.74$ | $\$ 11.57$ |
| $\$ 1.05$ | $\$ 17.75$ | $\$ 14.42$ | $\$ 12.15$ |
| $\$ 1.10$ | $\$ 18.60$ | $\$ 15.11$ | $\$ 12.72$ |
| $\$ 1.15$ | $\$ 19.44$ | $\$ 15.80$ | $\$ 13.30$ |
| $\$ 1.20$ | $\$ 20.29$ | $\$ 16.48$ | $\$ 13.88$ |
| $\$ 1.25$ | $\$ 21.13$ | $\$ 17.17$ | $\$ 14.46$ |
| $\$ 1.30$ | $\$ 21.98$ | $\$ 17.86$ | $\$ 15.04$ |


| Propane <br> Price $\$ /$ gal. | Furnace or Boiler Efficiency |  |  |
| :---: | :---: | :---: | :---: |
|  | 65\% (low) Older <br> Equipment | AFUE $=78 \%$ (average) Current Minimum | AFUE $=95 \%$ High Efficiency |
| \$1.35 | \$22.82 | \$18.54 | \$15.62 |
| \$1.40 | \$23.67 | \$19.23 | \$16.19 |
| \$1.45 | \$24.51 | \$19.92 | \$16.77 |
| \$1.50 | \$25.36 | \$20.60 | \$17.35 |
| \$1.55 | \$26.20 | \$21.29 | \$17.93 |
| \$1.60 | \$27.05 | \$21.98 | \$18.51 |

TABLE 3 Fuel Oil Heating Costs - \$ per MBTU Delivered for Three Appliance efficiencies

| Oil Price <br> \$/gallon | Furnace or Boiler Efficiency |  |  |
| :---: | :---: | :---: | :---: |
|  | 65\% (low) Older Equipment | AFUE $=78 \%$ (average) Current Minimum | AFUE $=86 \%$ High Efficiency |
| \$0.70 | \$7.76 | \$6.47 | \$5.87 |
| \$0.75 | \$8.32 | \$6.93 | \$6.29 |
| \$0.80 | \$8.87 | \$7.39 | \$6.71 |
| \$0.85 | \$9.43 | \$7.86 | \$7.13 |
| \$0.90 | \$9.98 | \$8.32 | \$7.55 |
| \$0.95 | \$10.54 | \$8.78 | \$7.96 |
| \$1.00 | \$11.09 | \$9.24 | \$8.38 |
| \$1.05 | \$11.65 | \$9.71 | \$8.80 |
| \$1.10 | \$12.20 | \$10.17 | \$9.22 |
| \$1.15 | \$12.76 | \$10.63 | \$9.64 |
| \$1.20 | \$13.31 | \$11.09 | \$10.06 |
| \$1.25 | \$13.87 | \$11.55 | \$10.48 |
| \$1.30 | \$14.42 | \$12.02 | \$10.90 |
| \$1.35 | \$14.97 | \$12.48 | \$11.32 |
| \$1.40 | \$15.53 | \$12.94 | \$11.74 |
| \$1.45 | \$16.08 | \$13.40 | \$12.16 |
| \$1.50 | \$16.64 | \$13.87 | \$12.58 |
| \$1.55 | \$17.19 | \$14.33 | \$12.99 |
| \$1.60 | \$17.75 | \$14.79 | \$13.41 |
| \$1.65 | \$18.30 | \$15.25 | \$13.83 |
| \$1.70 | \$18.86 | \$15.71 | \$14.25 |

Table 4 is for electric heat. The price per MBTU for electric resistance heat includes both baseboard and central resistance heating systems. Sections for air-source heat pumps, groundwater heat pumps, and ground-loop heat pumps are provided, and each contains three performance levels.

Air-source heat pumps are the most common heat pump. They have an inside blower and coil with an outside compressor and coil, and look like a conventional air conditioner. Use an air-source heat pump heating seasonal performance factor (HSPF) of 5 for older heat pumps, 6.8 for an average-performance unit, and 9.4 if you have or plan to buy a superior-performance unit.

Ground-loop and groundwater are both geothermal heat pump systems. A ground-loop heat pump, Figure 1, circulates water through buried piping loop. Coefficient of performance (COP) is the measure of performance for geothermal heat pumps. A COP of 3.1 would be appropriate for an older or low-performance system; a COP of 3.5 is representative of average equipment sold today; and a system with a COP of 4.2 would represent superior performance.

Unlike a ground-loop system that circulates water in a piping system, a groundwater heat pump, Figure 2, draws water from a well, extracts heat from the water in the winter or rejects heat to it in the summer, and then discharges the water, typically to another well. The heat pump is normally located inside, but there will be one or two wells associated with its operation. Groundwater heat pumps also use COP as a measure of performance, with a COP of 3.2 for an older or low-performance system, 4.1 for average performance, and 4.7 for superior performance. In many cases, the same equipment is used for both ground-loop and groundwater systems. They are rated with different COPs because of the differences between ground-loop and groundwater temperatures.

TABLE 4 Electric Heating Costs - \$ per MBTU Delivered for Several Appliances and Performance Levels

| Electricity \$/kWh | Electric <br> Resistance | Air-source <br> Heat Pump <br> Performance |  |  | Ground-loop <br> Heat Pump <br> Performance |  |  | Groundwater <br> Heat Pump <br> Performance |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | $\mathrm{HSPF}=5.0$ <br> (low) <br> Older <br> Equipment | HSPF = <br> 6.8 <br> (average) <br> Current <br> Minimum | $\begin{aligned} & \mathrm{HSPF}= \\ & 9.4 \\ & \text { (superior) } \end{aligned}$ | $\begin{aligned} & \text { COP } \\ & =3.1 \\ & \text { (low) } \end{aligned}$ | $\begin{aligned} & \mathrm{COP}= \\ & 3.5 \\ & \text { (average) } \end{aligned}$ | $\begin{aligned} & \mathrm{COP}= \\ & 4.2 \\ & \text { (superior) } \end{aligned}$ | $\begin{aligned} & \text { COP } \\ & =3.6 \\ & \text { (low) } \end{aligned}$ | $\begin{aligned} & \mathrm{COP}= \\ & 4.1 \\ & \text { (average) } \end{aligned}$ | $\begin{aligned} & \mathrm{COP}= \\ & 4.7 \\ & \text { (superior) } \end{aligned}$ |
| \$0.040 | \$11.73 | \$8.00 | \$5.88 | \$4.26 | \$4.21 | \$3.74 | \$3.11 | \$3.54 | \$3.10 | \$2.70 |
| \$0.045 | \$13.20 | \$9.00 | \$6.62 | \$4.79 | \$4.74 | \$4.21 | \$3.49 | \$3.98 | \$3.49 | \$3.04 |
| \$0.050 | \$14.66 | \$10.00 | \$7.35 | \$5.32 | \$5.26 | \$4.67 | \$3.88 | \$4.42 | \$3.88 | \$3.38 |
| \$0.055 | \$16.13 | \$11.00 | \$8.09 | \$5.85 | \$5.79 | \$5.14 | \$4.27 | \$4.87 | \$4.26 | \$3.72 |
| \$0.060 | \$17.60 | \$12.00 | \$8.82 | \$6.38 | \$6.32 | \$5.61 | \$4.66 | \$5.31 | \$4.65 | \$4.05 |
| \$0.065 | \$19.06 | \$13.00 | \$9.56 | \$6.91 | \$6.84 | \$6.07 | \$5.05 | \$5.75 | \$5.04 | \$4.39 |
| \$0.070 | \$20.53 | \$14.00 | \$10.29 | \$7.45 | \$7.37 | \$6.54 | \$5.43 | \$6.19 | \$5.43 | \$4.73 |
| \$0.075 | \$21.99 | \$15.00 | \$11.03 | \$7.98 | \$7.89 | \$7.01 | \$5.82 | \$6.64 | \$5.81 | \$5.07 |
| \$0.080 | \$23.46 | \$16.00 | \$11.76 | \$8.51 | \$8.42 | \$7.48 | \$6.21 | \$7.08 | \$6.20 | \$5.41 |
| \$0.085 | \$24.93 | \$17.00 | \$12.50 | \$9.04 | \$8.95 | \$7.94 | \$6.60 | \$7.52 | \$6.59 | \$5.74 |
| \$0.090 | \$26.39 | \$18.00 | \$13.24 | \$9.57 | \$9.47 | \$8.41 | \$6.99 | \$7.96 | \$6.98 | \$6.08 |
| \$0.095 | \$27.86 | \$19.00 | \$13.97 | \$10.11 | \$10.00 | \$8.88 | \$7.38 | \$8.41 | \$7.36 | \$6.42 |
| \$0.100 | \$29.33 | \$20.00 | \$14.71 | \$10.64 | \$10.53 | \$9.35 | \$7.76 | \$8.85 | \$7.75 | \$6.76 |
| \$0.105 | \$30.79 | \$21.00 | \$15.44 | \$11.17 | \$11.05 | \$9.81 | \$8.15 | \$9.29 | \$8.14 | \$7.09 |
| \$0.110 | \$32.26 | \$22.00 | \$16.18 | \$11.70 | \$11.58 | \$10.28 | \$8.54 | \$9.73 | \$8.53 | \$7.43 |
| \$0.115 | \$33.72 | \$23.00 | \$16.91 | \$12.23 | \$12.11 | \$10.75 | \$8.93 | \$10.18 | \$8.91 | \$7.77 |
| \$0.120 | \$35.19 | \$24.00 | \$17.65 | \$12.77 | \$12.63 | \$11.21 | \$9.32 | \$10.62 | \$9.30 | \$8.11 |
| \$0.125 | \$36.66 | \$25.00 | \$18.38 | \$13.30 | \$13.16 | \$11.68 | \$9.70 | \$11.06 | \$9.69 | \$8.45 |
| \$0.130 | \$38.12 | \$26.00 | \$19.12 | \$13.83 | \$13.68 | \$12.15 | \$10.09 | \$11.50 | \$10.08 | \$8.78 |
| \$0.135 | \$39.59 | \$27.00 | \$19.85 | \$14.36 | \$14.21 | \$12.62 | \$10.48 | \$11.95 | \$10.47 | \$9.12 |
| \$0.140 | \$41.06 | \$28.00 | \$20.59 | \$14.89 | \$14.74 | \$13.08 | \$10.87 | \$12.39 | \$10.85 | \$9.46 |



FIGURE 1 Groundwater heat pump.


FIGURE 2 Groundwater heat pump.

Table 5 is used to estimate the cost per MBTU for unvented kerosene heaters. They are $100 \%$ efficient because all of the heat is delivered to the home. If you are using a vented kerosene appliance, use Table 3.

TABLE 5 Kerosene Heating Costs-\$ per Million MBTU Delivered

| Unvented Kerosene Heater |  |
| :--- | :---: |
| Kerosene Price \$/Gallon | Unvented Equipment |
| $\$ 1.00$ | $\$ 7.56$ |
| $\$ 1.10$ | $\$ 8.31$ |
| $\$ 1.20$ | $\$ 9.07$ |
| $\$ 1.30$ | $\$ 9.83$ |
| $\$ 1.40$ | $\$ 10.58$ |
| $\$ 1.50$ | $\$ 11.34$ |


|  | Unvented Kerosene Heater |
| :--- | ---: |
| Kerosene Price \$/Gallon | Unvented Equipment |
| $\$ 1.60$ | $\$ 12.09$ |
| $\$ 1.70$ | $\$ 12.85$ |
| $\$ 1.80$ | $\$ 13.61$ |
| $\$ 1.90$ | $\$ 14.36$ |
| $\$ 2.00$ | $\$ 15.12$ |
| $\$ 2.10$ | $\$ 15.87$ |
| $\$ 2.20$ | $\$ 16.63$ |
| $\$ 2.30$ | $\$ 17.38$ |
| $\$ 2.40$ | $\$ 18.14$ |
| $\$ 2.50$ | $\$ 18.90$ |
| $\$ 2.60$ | $\$ 19.65$ |
| $\$ 2.70$ | $\$ 20.41$ |
| $\$ 2.80$ | $\$ 21.16$ |
| $\$ 2.90$ | $\$ 21.92$ |
| $\$ 3.00$ | $\$ 22.68$ |

## Estimate the cost of delivered heating energy

> Example: Compare the cost of heat from a propane furnace to the cost of heat from an air-source heat pump.
> First, you will need to know the cost of both fuels and efficiencies of the systems. Follow this example to learn how to use Tables 1 through 8 .
> Table 2 is for propane appliances. Assuming you have an old propane furnace, the efficiency will be about $65 \%$. If you pay $\$ 0.90$ per gallon for propane, the cost per million Btus (MBTUs) will be $\$ 15.22$.
> Table 4 is for electric appliances. Compare this to the cost of heating with an average-efficiency, air-source heat pump with electricity costing $\$ 0.07$ per kilowatt hour ( kWh ). The cost per MBTU will be about $\$ 10.29$.
> Delivered heat from the heat pump costs about two-thirds that of propane.

Table 6 will allow you to estimate the cost per MBTU for several wood heating appliances. The species of wood, cost per cord, and appliance efficiency are all important to getting an accurate estimate. The efficiency ratings provided are typical but may vary between manufacturers. Several common wood species are listed, with cord costs ranging from $\$ 80$ to $\$ 140$. There are sections of the table for open fireplaces; pre-1980 wood stoves; masonry heaters; and post-1980, EPA-certified wood stoves. For more details on solid-fuel heating appliances, obtain a copy of Solid-Fuel Heating Appliances online at www.engext.ksu.edul. Look under publications.

Table 7 provides heating cost estimates for pellet- and corn-burning appliances.
Table 8 will estimate the cost of providing one MBTU of cooling for air conditioners and heat pumps. A seasonal energy efficiency rating (SEER) is the performance measure for modern air conditioners and air-source heat pumps. Older units may not be rated, and a SEER of 7 is reasonable for estimating operating costs.

## Estimating Annual Costs

Once you have determined the cost per MBTU for any fuel, you can estimate annual heating or cooling costs. It is important to remember these are estimates only; lifestyle, actual housing conditions, house configuration, and other factors can greatly influence heating and cooling costs.

TABLE 6 Wood Heating Costs - \$per Million BTU for Several

|  | Wood Heating Appliance Efficiency |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 10\%-typical open fireplace |  |  |  | 50\% - typical central boiler, furnace, or pre-1980 wood stove |  |  |  |
| Price per cord | \$80 | \$100 | \$120 | \$140 | \$80 | \$100 | \$120 | \$140 |
| Species |  |  |  |  |  |  |  |  |
| Cottonwood | \$50.63 | \$63.29 | \$75.95 | \$88.61 | \$10.13 | \$12.66 | \$15.19 | \$17.72 |
| Elm, American | \$40.00 | \$50.00 | \$60.00 | \$70.00 | \$8.00 | \$10.00 | \$12.00 | \$14.00 |
| Hackberry | \$37.74 | \$47.17 | \$56.60 | \$66.04 | \$7.55 | \$9.43 | \$11.32 | \$13.21 |
| Honeylocust | \$29.96 | \$37.45 | \$44.94 | \$52.43 | \$5.99 | \$7.49 | \$8.99 | \$10.49 |
| Maple, Silver | \$42.11 | \$52.63 | \$63.16 | \$73.68 | \$8.42 | \$10.53 | \$12.63 | \$14.74 |
| Oak, Red | \$32.52 | \$40.65 | \$48.78 | \$56.91 | \$6.50 | \$8.13 | \$9.76 | \$11.38 |
| Osage Orange | \$24.32 | \$30.40 | \$36.47 | \$42.55 | \$4.86 | \$6.08 | \$7.29 | \$8.51 |
|  | 60\% - typical masonry heater |  |  |  | 70\% - typical EPA-certified wood stoves and inserts |  |  |  |
| Price per cord | \$80 | \$100 | \$120 | \$140 | \$80 | \$100 | \$120 | \$140 |
| Species |  |  |  |  |  |  |  |  |
| Cottonwood | \$8.44 | \$10.55 | \$12.66 | \$14.77 | \$7.23 | \$9.04 | \$10.85 | \$12.66 |
| Elm, American | \$6.67 | \$8.33 | \$10.00 | \$11.67 | \$5.71 | \$7.14 | \$8.57 | \$10.00 |
| Hackberry | \$6.29 | \$7.86 | \$9.43 | \$11.01 | \$5.39 | \$6.74 | \$8.09 | \$9.43 |
| Honeylocust | \$4.99 | \$6.24 | \$7.49 | \$8.74 | \$4.28 | \$5.35 | \$6.42 | \$7.49 |
| Maple, Silver | \$7.02 | \$8.77 | \$10.53 | \$12.28 | \$6.02 | \$7.52 | \$9.02 | \$10.53 |
| Oak, Red | \$5.42 | \$6.78 | \$8.13 | \$9.49 | \$4.65 | \$5.81 | \$6.97 | \$8.13 |
| Osage Orange | \$4.05 | \$5.07 | \$6.08 | \$7.09 | \$3.47 | \$4.34 | \$5.21 | \$6.08 |

Wood Species, Heating Appliance Efficiencies, and Cord Wood Costs

TABLE 7 Pellet and Corn Hating Costs - \$ per MBTU

| Pellet price |  | Typical Pellet Stove | Corn Price | Typical Corn Stove |
| :--- | :--- | :---: | :--- | ---: |
| Price per 40-pound Bag | Price per Ton |  | Price per Bushel |  |
| $\$ 2.50$ | $\$ 125$ | $\$ 9.77$ | $\$ 1.50$ | $\$ 5.05$ |
| $\$ 3.00$ | $\$ 150$ | $\$ 11.73$ | $\$ 2.00$ | $\$ 8.42$ |
| $\$ 3.50$ | $\$ 175$ | $\$ 13.68$ | $\$ 2.50$ | $\$ 11.78$ |
| $\$ 4.00$ | $\$ 200$ | $\$ 15.63$ | $\$ 3.00$ | $\$ 15.15$ |
|  |  |  |  |  |

Table $9^{1}$ provides estimates of heating and cooling requirements of homes in Kansas. Three levels of home efficiency are listed. Standard practice represents homes as they have generally been constructed in Kansas, energy code-compliant applies to a home that would meet modern energy codes, and energy efficient represents homes where high performance was a major design goal. There are also three climate areas listed.

Based on the type of home and location, choose the appropriate index. Multiply it by the size of your home (square feet of living space) and the cost of your fuel in \$ per MBTU, then divide by 1000 to estimate annual costs. If you live in an older, poorly insulated and weatherized home, your heating costs will be higher than those estimated by this method. To estimate savings for using higher performance equipment or other fuels, calculate the costs for each and compare.

[^32]TABLE 8 Electric Cooling Costs-\$ per MBTU Cooling for Several Appliances and Performance Levels

| Electricity \$/kWh | Air Conditioner or Air-source Heat Pump Performance |  |  | Groundwater Heat Pump Performance |  |  | Ground-loop Heat Pump Performance |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\text { SEER }=7$ <br> (low) Older <br> Equipment | $\operatorname{SEER}=12$ <br> (average) | $S E E R=15$ <br> (superior) | $\begin{aligned} & \mathrm{EER}=16 \\ & \text { (low) } \end{aligned}$ | $\mathrm{EER}=19$ <br> (average) | $\mathrm{EER}=24$ <br> (superior) | $\begin{aligned} & \mathrm{EER}=13 \\ & \text { (low) } \end{aligned}$ | $\mathrm{EER}=16$ <br> (average) | $\begin{aligned} & \mathrm{EER}=20 \\ & \text { (superior) } \end{aligned}$ |
| \$0.040 | \$5.71 | \$4.00 | \$2.67 | \$2.61 | \$2.22 | \$1.78 | \$3.18 | \$2.62 | \$2.13 |
| \$0.045 | \$6.43 | \$4.50 | \$3.00 | \$2.93 | \$2.50 | \$2.00 | \$3.58 | \$2.95 | \$2.39 |
| \$0.050 | \$7.14 | \$5.00 | \$3.33 | \$3.26 | \$2.78 | \$2.22 | \$3.98 | \$3.28 | \$2.66 |
| \$0.055 | \$7.86 | \$5.50 | \$3.67 | \$3.58 | \$3.06 | \$2.44 | \$4.38 | \$3.61 | \$2.92 |
| \$0.060 | \$8.57 | \$6.00 | \$4.00 | \$3.91 | \$3.33 | \$2.67 | \$4.78 | \$3.94 | \$3.19 |
| \$0.065 | \$9.29 | \$6.50 | \$4.33 | \$4.23 | \$3.61 | \$2.89 | \$5.18 | \$4.27 | \$3.46 |
| \$0.070 | \$10.00 | \$7.00 | \$4.67 | \$4.56 | \$3.89 | \$3.11 | \$5.57 | \$4.59 | \$3.72 |
| \$0.075 | \$10.71 | \$7.50 | \$5.00 | \$4.89 | \$4.17 | \$3.33 | \$5.97 | \$4.92 | \$3.99 |
| \$0.080 | \$11.43 | \$8.00 | \$5.33 | \$5.21 | \$4.44 | \$3.56 | \$6.37 | \$5.25 | \$4.25 |
| \$0.085 | \$12.14 | \$8.50 | \$5.67 | \$5.54 | \$4.72 | \$3.78 | \$6.77 | \$5.58 | \$4.52 |
| \$0.090 | \$12.86 | \$9.00 | \$6.00 | \$5.86 | \$5.00 | \$4.00 | \$7.17 | \$5.91 | \$4.78 |
| \$0.095 | \$13.57 | \$9.50 | \$6.33 | \$6.19 | \$5.28 | \$4.22 | \$7.56 | \$6.23 | \$5.05 |
| \$0.100 | \$14.29 | \$10.00 | \$6.67 | \$6.51 | \$5.56 | \$4.44 | \$7.96 | \$6.56 | \$5.32 |
| \$0.105 | \$15.00 | \$10.50 | \$7.00 | \$6.84 | \$5.83 | \$4.67 | \$8.36 | \$6.89 | \$5.58 |
| \$0.110 | \$15.71 | \$11.00 | \$7.33 | \$7.17 | \$6.11 | \$4.89 | \$8.76 | \$7.22 | \$5.85 |
| \$0.115 | \$16.43 | \$11.50 | \$7.67 | \$7.49 | \$6.39 | \$5.11 | \$9.16 | \$7.55 | \$6.11 |
| \$0.120 | \$17.14 | \$12.00 | \$8.00 | \$7.82 | \$6.67 | \$5.33 | \$9.55 | \$7.87 | \$6.38 |
| \$0.125 | \$17.86 | \$12.50 | \$8.33 | \$8.14 | \$6.94 | \$5.56 | \$9.95 | \$8.20 | \$6.65 |
| \$0.130 | \$18.57 | \$13.00 | \$8.67 | \$8.47 | \$7.22 | \$5.78 | \$10.35 | \$8.53 | \$6.91 |
| \$0.135 | \$19.29 | \$13.50 | \$9.00 | \$8.79 | \$7.50 | \$6.00 | \$10.75 | \$8.86 | \$7.18 |
| \$0.140 | \$20.00 | \$14.00 | \$9.33 | \$9.12 | \$7.78 | \$6.22 | \$11.15 | \$9.19 | \$7.44 |

TABLE 9 Annual Heat and Cooling Indices-1000 Btus/square foot

|  | Heating |  |  |  | Cooling |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Northwest | Central | Southeast |  | Northwest | Central | Southeast |
| Current practice | 50 | 45 | 40 |  | 11 | 13 | 14 |
| Energy code compliant | 36 | 32 | 29 |  | 10 | 11 | 12 |
| Energy efficient | 28 | 25 | 23 |  | 9 | 10 | 11 |

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## Estimating Annual Costs

Example: Estimate the annual cost of heating a 2000-square-foot home in rural Sedgwick County. The home was built in the 1960s. The home owner is considering both propane and an air-source heat pump. Fuel costs were determined in the previous example to be $\$ 15.22 / \mathrm{MBTU}$ for propane and $\$ 10.29 / \mathrm{MBTU}$ for a heat pump.

The heating index for the home would be 45 . Annual heating costs would be

$$
\begin{aligned}
& \frac{2,000 \times 45 \times 15.22}{1,000}=\$ 1,370 \text { for propane, and } \\
& \frac{2,000 \times 45 \times 10.29}{1,000}=\$ 925 \text { for the heat pump. }
\end{aligned}
$$

### 11.11.0 Residential Ground Source Heat Pump (GSHP) Savings versus Electric, Gas, and Fuel Oil

## Residential GSHP Annual Savings

|  | Mean Annual Savings (\%) |  |  |  |
| :--- | :---: | :---: | :---: | :---: |
| Conventional System | Number | Energy | Number | Dollars |
| Electric Resistance Heat/AC |  |  |  |  |
| Air-Source Heat Pump | 21 | $57 \%$ | 18 | $54 \%$ |
| Natural Gas Furnace/AC | 33 | $31 \%$ | 21 | $31 \%$ |
| Oil Furnace/AC | 17 | $67 \%$ | 21 | $18 \%$ |
| Other (propane, unspecified) | 6 | $71 \%$ | 9 | $33 \%$ |

${ }^{\text {a. AC means with electric air conditioning. }}$
${ }^{b}$. Natural gas or oil furnaces with electric air conditioning had annual operating costs less than GSHP systems for $23 \%$ of the case studies.
The mean annual dollar savings of GSHPs shown above may appear attractive; however, due to the relatively low-annual operating costs of conventional energy systems, it is difficult in many cases to recover the additional incremental cost (ground loop) of GSHP systems.
Residential GSHP system peak demand reduction compared to single-zone electric resistance heating for 13 case studies ranged from 5.3 kW to 10.4 kW with a mean of 7.2 kW .

## School Ground-Source Heat Pumps

The potential for savings of GSHP systems in schools is documented in 26 case studies, which include 54\% vertical ground-coupled, $19 \%$ groundwater, $12 \%$ horizontal ground-coupled, and $15 \%$ other types of systems.

## School GSHP Annual Savings

|  | Mean Annual Savings (\%) |  |  |  |
| :--- | :---: | :---: | :---: | :---: |
|  | Number | Energy | Number | Dollars |
| Conventional System | 2 | $51 \%$ | 3 | $45 \%$ |
| Electric Resistance Heat | 3 | $61 \%$ | 1 | $13 \%$ |
| Natural Gas | 1 | $76 \%$ | 1 | $58 \%$ |
| Fuel Oil |  |  |  |  |

Source: Study conducted by U.S. Department of Energy via Grant C92-12025-001.

### 11.11.1 Paybacks for Residential GSHP Economics

| Residential GSHP Economics |  |  |  |
| :--- | :---: | :--- | ---: |
|  |  | Simple Payback (yrs) |  |
| Conventional System | Number | Range | Mean |
| Electric Resistance Heat/AC | 4 | 2.7 to 6.8 | 4.4 |
| Air-Source Heat Pump | 3 | 2.0 to 9.5 | 5.9 |
| Natural Gas/AC | 9 | 4.2 to 24.1 | 11.6 |
| Fuel Oil/AC | 6 | 1.4 to 7.1 | 4.4 |
| Other | 5 | 2.0 to 6.8 | 4.3 |

The biggest barrier to faster paybacks of GSHP systems is the incremental cost of the ground loop.
Since residential GSHP systems are usually included in the mortgage, a break-even value of electric rates is a more meaningful value than simple payback.

In January 1995, data became available on the cost of purchasing and installing residential GSHP systems. Based on this data, an earlier analysis of a new well-insulated home, and a 30-year fixed rate mortgage at $8 \%$, the electrical break-even rates were calculated for two different climate zones. In the colder zone, the break-even rates were $\$ 0.061 / \mathrm{kWh}$ for vertical and $\$ 0.058 / \mathrm{kWh}$ for horizontal ground-coupled systems. In the warmer zones, they were $\$ 0.097$ and $\$ 0.084$, respectively. Electric rates in excess of these break-even values would result in the GSHP system having a positive cash flow to the homeowner.

Simple paybacks for school systems were reported in only 5 out of 23 case studies. These simple paybacks ranged from 5 to 14 years for electric resistance heating, 3.5 years for natural gas systems, and 5 to 7 years for others.

Case studies for commercial buildings reported simple paybacks for 17 out of 46 GSHP systems. The range of simple paybacks was 1.3 to 4.7 years, with a mean of 2.8 years.

For commercial buildings, all but four of the simple paybacks represent buildings located in northern climates.

Caution should be used in arriving at economic conclusions for any of the three groups presented in this summary. This is due to variables of climate, ground characteristics, GSHP system type, equipment efficiency, sizing, complex utility rate structures, and a variety of economic analysis methods used in the case studies.

Benefits reported for using GSHP systems in schools are: addition of mechanical cooling, improved control, and simplicity of maintenance and repair. In southern climates, the benefits include: elimination of cooling towers, outdoor equipment, mechanical rooms, and ductwork.

### 11.11.2 Commercial Ground Source Heat Pump (GSHP) Savings versus Electric, Gas, Fuel Oil

Case studies (46) documented for commercial GSHP systems ranged in capacity from 30 to 4700 tons. These systems employed vertical ground-coupled (43\%), groundwater (35\%), horizontal ground-coupled ( $11 \%$ ), and other ( $11 \%$ ) types of ground systems. Commercial GSHP systems were monitored in $84 \%$ of the case studies, and conventional energy systems represent only $20 \%$ of the comparisons.

The average annual energy savings of GSHP systems ranged from 40 to $72 \%$, and dollar savings ranged from 31 to $56 \%$.

### 11.11.3 Paybacks for Commercial GSHP Economics

|  | Mean Annual Savings (\%) |  |  |  |
| :--- | :---: | :---: | :---: | :---: |
| Conventional System | Number | Energy | Number | Dollars |
| Electric Resistance Heat/AC | 6 | $59 \%$ | 5 | $56 \%$ |
| Air-Source Heat Pump | 3 | $40 \%$ | 3 | $37 \%$ |
| Natural Gas | 4 | $69 \%$ | 4 | $49 \%$ |
| Fuel Oil | 6 | $72 \%$ | 7 | $31 \%$ |

The savings attributable to the use of GSHP systems in commercial buildings vary over a wide range. In addition to parameters common to all GSHP applications, unique to commercial buildings are building use, internal heat gains, and more complex rate structures.

Predictions of savings to be achieved with a GSHP system are a very site-specific endeavor for commercial buildings.

## Economics

The economics of residential GSHP were reported as simple paybacks in only $15 \%$ of the 184 case studies. A favorable simple payback is considered to be less than 5 years.

Residential GSHP system simple paybacks ranged from 1.4 to 24.1 years, and the mean was 7.0 years.

| Commercial GSHP Economics |  |  |  |
| :--- | :---: | :---: | :---: |
| Conventional System | Number | Range Years | Mean Years |
| Electric Resistance Heat | 5 | $1.3-3.0$ | 2.3 |
| Natural Gas | 3 | $1.9-4.7$ | 3.4 |
| Fuel Oil | 7 | $2.2-4.5$ |  |
| Other | 2 | $2.5-2.7$ | 2.5 |

When considering a GSHP system for either new or retrofit situations, it is imperative that a deliberate economic analysis be performed.

## Utility Programs

Ground-source heat pumps (GSHPs) are one of many technologies utilities are considering or implementing for demand-side management (DSM), especially aimed at improving the efficiency with which customers use electricity. Information was developed on the status of DSM programs for GSHPs including: utility/contacts, marketing, barriers to market entry, incentives, number of units installed in service areas, and benefits. A total of 57 utilities and rural electric cooperatives out of 178 investigated were reported to have DSM programs involving GSHPs.

Marketing techniques employed by utilities included utility publications and seminars ( $36 \%$ ), newspaper and radio/TV advertising ( $16 \%$ ), test/demonstrations ( $10 \%$ ), education ( $6 \%$ ), home shows ( $6 \%$ ), and other ( $26 \%$ ).

The primary market penetration barrier cited by utilities was the first cost of installation of GSHP systems, especially the incremental cost (median cost of $\$ 700$ to $\$ 900 /$ ton of the ground loop). Other barriers include
low-annual cost of natural gas, lack of manufacturers, dealers and loop installers, customer resistance to heat pump technology, and regulatory problems.

Utilities have designed a number of incentive packages to encourage the installation of GSHPs. The most common are cash rebates to customers (mean values of $\$ 208 /$ ton and $\$ 382 /$ unit) and trade ally ( $\$ 200 /$ ton). In many cases, the utility specifies a minimum Seasonal Energy Efficiency Ratio (SEER), usually 10 or greater, energy audits, or minimum insulation standards. Other types of incentives include special financing, discounted energy rates, and free ground loop installation.

More than 18,800 GSHP systems (3-ton equivalent) were reported by 35 utilities out of the 57 contacted. The types of systems installed include: horizontal ground-coupled (46\%), undefined systems ( $40 \%$ ), groundwater systems (7\%), and vertical ground-coupled (7\%).

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### 12.0.0 Converting Watts to Volts

## Source: www.bugclub.org

1. Convert Watts to Volts:

Voltage $=$ Watts/AMPS
$\mathrm{E}=\mathrm{P} \div \mathrm{I}$

### 12.0.1 Converting Watts to Amps

## Source: www.bugclub.org

2. Convert Watts to AMPS:

AMPS $=$ Watts/Voltage
$\mathrm{I}=\mathrm{P} \div \mathrm{E}$
Example:
2300 WATTS $=2300 \mathrm{w}$ divided by $120 \mathrm{v}=$ 19.1 AMPS
(for 3 Phase divide by 1.73)

### 12.0.2 Converting Amps to Watts

Source: www.bugclub.org
3. Convert AMPS to Watts:

Watts $=$ Voltage $\times$ Amps
$\mathrm{P}=\mathrm{E} \times \mathrm{I}$
Example: 19.1 AMPS multiplied by $120 \mathrm{v}=2300$ Watts (for 3 phase multiply by 1.73 )

### 12.0.3 Converting Horsepower to Amps

## Source: www.bugclub.org

4. Convert Horse Power to AMPS:

HORSEPOWER $=(\mathrm{V} \times \mathrm{A} \times \mathrm{EFF}) \div 746$
EFFICIENCY $=(746 \times \mathrm{HP}) \div(\mathrm{V} \times \mathrm{A})$
Multiply Horse Power by 746 w ( $1 \mathrm{HP}=746$ Watts)
Find Circuit Voltage and Phase

## Example:

30 HP at 480 (3 Phase) - 746 multiplied by $30=22380$
22380 divided by $480(3$ Phase) $=46.5$
46.5 divided by $1.73=29.5$ AMPS

Multiply all the motor loads by $1.50 \%$ and go to the next circuit size.

### 12.0.4 Converting KVA to Amps

## Source: www.bugclub.org

5. Convert KVA to AMPS:

Multiply KVA by 1000/voltage

## Example:

30 KVA multiplied by $1000 \mathrm{v}=30,000$ Watts
30,000 Watts divided by $480=62.5$ AMPS
(for 3 phase divide by 1.73)

### 12.0.5 Converting Kw to Amps

## Source: www.bugclub.org

6. Convert KW to AMPS:

Multiply KW by 1000/voltage and then by power factor

## Example:

30 KW multiplied by $1000 \mathrm{v}=30,000$
30,000 divided by $480=62.5 \times .90=56.25$ AMPS
(for 3 phase divide by 1.73)

### 12.0.6 Symbols for Electrical Terms

## Source: www.bugclub.org

$$
\begin{aligned}
\mathrm{E} & =\text { VOLTS or }(\mathrm{V}=\mathrm{VOLTS}) \\
\mathrm{P} & =\text { WATTS or }(\mathrm{W}=\mathrm{WATTS}) \\
\mathrm{R} & =\text { OHMS or }(\mathrm{R}=\text { RESISTANCE }) \\
\mathrm{I} & =\text { AMPERES or }(\mathrm{A}=\mathrm{AMPERES}) \\
\mathrm{HP} & =\text { HORSEPOWER } \\
\mathrm{PF} & =\text { POWER FACTOR } \\
\mathrm{kW} & =\text { KILOWATTS } \\
\mathrm{kWh} & =\text { KILOWATT HOUR } \\
\mathrm{VA} & =\text { VOLT-AMPERES } \\
\mathrm{kVA} & =\text { KILOVOLT }-\mathrm{AMPERES} \\
\mathrm{C} & =\text { CAPACITANCE } \\
\mathrm{EFF} & =\text { EFFICIENCY }(\text { expressed as a decimal })
\end{aligned}
$$

### 12.1.0 Ohm's Law

## Source: www.ziplink.net

- $\mathrm{E}=$ Voltage - measured in Volts
- $\mathrm{I}=$ Current - measured in Amperes
- $\mathrm{R}=$ Resistance - measured in Ohms
- $\mathrm{E}=\mathrm{I} * \mathrm{R}$, answer in Volts
- $\mathrm{I}=\mathrm{E} / \mathrm{R}$, answer in Amperes
- $\mathrm{R}=\mathrm{E} / \mathrm{I}$, answer in Ohms


### 12.1.1 Resistors Networks—Terminology and How Identified

## Resistor Networks

- Resistors are labeled by a \# following the R : R1...R2. . .Rn
- $\mathrm{Rn}=$ Continues for all the resistors you have
- $\mathrm{Rt}=$ Resistance total
- Resistors in SERIES add : Rt $=\mathrm{R} 1+\mathrm{R} 2+\mathrm{R} 3 \ldots+\mathrm{Rn}$
- Two (2) Resistors in PARALLEL : $\mathrm{Rt}=(\mathrm{R} 1 * \mathrm{R} 2) /(\mathrm{R} 1+\mathrm{R} 2)$ This the Product divided by the Sum
- More than Two Resistors in PARALLEL: $\mathrm{Rt}=1 /(1 / \mathrm{R} 1+1 / \mathrm{R} 2 \ldots+1 / \mathrm{Rn})$. This is called the reciprocal formula.
- Another form of this formula is : $1 / \mathrm{Rt}=1 / \mathrm{R} 1+1 / \mathrm{R} 2 \ldots+1 / \mathrm{Rn}$

Source: www.ziplink.net

### 12.1.2 Inductor Networks-Terminology and How Identified

## Inductor Networks

- The rules for Inductors are exactly like those for Resistors.
- Inductors are labeled by a \# following the L : L1...L2. . .Ln.
- $\mathrm{Ln}=$ Continues for all the Inductors you have.
- $\mathrm{Lt}=$ Inductance total.
- Inductors in SERIES add : $\mathrm{Lt}=\mathrm{L} 1+\mathrm{L} 2+\mathrm{L} 3 \ldots+\mathrm{Ln}$.
- Two (2) Inductors in PARALLEL : $\mathrm{Lt}=(\mathrm{L} 1 * \mathrm{~L} 2) /(\mathrm{L} 1+\mathrm{L} 2)$ This the Product divided by the Sum.
- More than Two Inductors in PARALLEL: $\mathrm{Lt}=1 /(1 / \mathrm{L} 1+1 / \mathrm{L} 2 \ldots+1 / \mathrm{Ln})$ This is called the reciprocal formula.
- Another form of this formula is : $1 / \mathrm{Lt}=1 / \mathrm{L} 1+1 / \mathrm{L} 2 \ldots+1 / \mathrm{Ln}$.

Source: www.ziplink.net

### 12.1.3 Capacitor Networks—Terminology and How Identified

## Capacitor Networks

- The rules for Capacitor Networks are exactly opposite those of Resistors and Inductors.
- Capacitors are labeled by a \# following the C : C1...C2. . .Cn.
- $\mathrm{Cn}=$ Continues for all the Capacitors you have.
- $\mathrm{Ct}=$ Capacitance total.
- Capacitors in PARALLEL add : $\mathrm{Ct}=\mathrm{C} 1+\mathrm{C} 2+\mathrm{C} 3 \ldots+\mathrm{Cn}$.
- Two (2) Capacitors in SERIES : $\mathrm{Ct}=(\mathrm{C} 1 * \mathrm{C} 2) /(\mathrm{C} 1+\mathrm{C} 2)$ This is the Product divided by the Sum.
- More than Two Capacitors in SERIES : $\mathrm{Ct}=1 /(1 / \mathrm{C} 1+1 / \mathrm{C} 2 \ldots+1 / \mathrm{Cn})$. This is called the reciprocal formula.
- Another form of this formula is : $1 / \mathrm{Ct}=1 / \mathrm{C} 1+1 / \mathrm{C} 2 \ldots .+1 / \mathrm{Cn}$.

Source: www.ziplink.net

### 12.1.4 Watt's Law

- $\mathrm{P}=$ Power - measured in Watts
- $\mathrm{I}=$ Current - measured in Amperes
- $\mathrm{E}=$ Voltage - measured in Volts
- $\mathrm{R}=$ Resistance - measured in Ohms
- $\mathrm{P}=\mathrm{I} * \mathrm{E}$, answer in Watts (easy as "pie")
- $\mathrm{P}=\mathrm{I}^{*} \mathrm{I}^{*} \mathrm{R}$, answer in Watts (read as I squared R)
- $P=E^{*} E / R$, answer in Watts (read as E squared, divided by R)
- Note: Use Ohm's Law to derive other formulas for I, R, and E, using the formulas above.

Source: ziplink.net

### 12.1.5 Calculating Reactance

- Inductive Reactance
- $\mathrm{Pi}=\pi=3.14$
- $\mathrm{f}=$ Frequency, in Hertz
- $\mathrm{L}=$ Inductance of coil, in Henries
- $\mathrm{Xl}=$ Inductive Reactance, in Ohms
- $\mathrm{Xl}=2 \pi \mathrm{fL}$, answer in Ohms
- Note: XI is a linear function; it increases as frequency or inductor value increases.
- Capacitive Reactance
- $\mathrm{Pi}=\pi=3.14$
- $\mathrm{f}=$ Frequency, in Hertz
- $\mathrm{C}=$ Capacitance in Farads
- Xc = Capacitive Reactance, in Ohms
- $\mathbf{X c}=\frac{\mathbf{1}}{\mathbf{2} \boldsymbol{\pi} \mathrm{fC}}$, answer in Ohms
- Note: Xc is inversely proportional to frequency and capacitance. If either frequency or capacitance increases, Xc decreases.

Source: ziplink.net

### 12.1.6 Resonance in RLC Series Circuit—Explanation and Formula

## Resonance in RLC Series Circuit

- Resonance occurs when Xc and XL are equal to each other.
- $\mathrm{Fo}=$ Frequency of resonance, in Hertz
- $\mathrm{Pi}=\pi=3.14$
- $\mathrm{L}=$ Inductance in Henries
- $\mathrm{C}=$ Capacitance in Farads
- $\mathbf{F o}=\frac{1}{2 \pi \sqrt{\mathrm{LC}}}$


### 12.1.7 Bandwidth and Quality Factor

- $\mathrm{Q}=$ Quality Factor of the circuit
- $\mathrm{BW}=$ Bandwidth in Hertz
- $\mathrm{XL}=$ Inductive Reactance of the Inductor
- $\mathrm{R}=$ Resistance of Inductor plus any external Resistance
- $\mathrm{Fo}=$ Frequency of Resonance
- $\mathbf{Q}=\mathbf{X I} / \mathbf{R}$, Quality Factor
- $\mathbf{B W}=\mathbf{F o} / \mathbf{Q}$, measured in Hertz
- Note: As Quality Factor increases the Bandwidth decreases, but has more gain. And the opposite is true as well. When the Quality Factor decreases, the Bandwidth increases, but has less gain.

Source: ziplink.net

### 12.1.8 Wavelength-Explained

## Wavelength

- Wavelength is how long a wave is for a given Frequency.
- The speed of light is $\mathbf{1 8 6 , 0 0 0}$ miles per second or about $\mathbf{3 0 0 , 0 0 0 , 0 0 0}$ meters per second.
- f is the Frequency in Hertz/Second (or Cycles/Second).
- Wavelength $=(300,000,000$ meters $/$ second $) /(\mathrm{f}$ in Cycles/Second $)$.
- The answer is in meters/cycle.

Source: ziplink.net

### 12.1.9 Frequency and Time-Explained

## Frequency and Time

- f is the Frequency in Hertz/Second or Cycles/Second
- t is time in Seconds
- $\mathrm{f}=1 / \mathrm{t}$, answer in Hertz/Second or Cycles/Second.
- $\mathrm{t}=1 / \mathrm{f}$, answer in Seconds

Source: ziplink.net

### 12.1.10 Impedance of a Circuit—Formula and Rules for Circuits with Both Capacitive and Inductive Reactances

## Impedance of a Circuit

- $\mathrm{Xc}=$ Capacitive Reactance, in Ohms
- $\mathrm{Xl}=$ Inductive Reactance, in Ohms
- $\mathrm{R}=$ Resistance, in Ohms
- $\mathrm{Z}=$ Impedance, in Ohms
- For a Capacitive circuit
- $\mathbf{z}=\sqrt{\mathbf{R}^{2}+\mathbf{X c}}{ }^{2}$
- For an Inductive Circuit
- $\mathbf{z}=\sqrt{\mathbf{R}^{2}+\mathbf{X I} \mathbf{l}^{2}}$
- Rules for when you have a circuit with both Capacitive and Inductive Reactances
- When Xl is larger than Xc, use the formula in figure (a). If Xc is Larger than Xl, use the formula in Figure (b).
- a. $\mathbf{z}=\sqrt{\mathbf{R}^{2}+\left(\mathbf{X I}-\mathbf{X c} \mathbf{c}^{2}\right)}$
b. $\mathbf{z}=\sqrt{\mathbf{R}^{2}+\left(\mathbf{X c}-\mathbf{X l}^{2}\right)}$


### 12.2.0 Rules for Amp Draw per Horsepower at Voltage Ranging from 115 V to 575 V

- At 575 volts, a 3-phase motor draws 1 amp per horsepower.
- At 460 volts, a 3-phase motor draws 1.27 amps per horsepower.
- At 230 volts, a 3-phase motor draws 2.5 amps per horsepower.
- At 230 volts, a single-phase motor draws 5 amps per horsepower.
- At 115 volts, a single phase motor draws 10 amps per horsepower.
*These above are approximations.
- 746 watts $=1 \mathrm{HP}$


### 12.2.1 How to Figure Out the Phase for a Certain Circuit Number

An easy way to figure the phase of a circuit number is to divide it by 6 . If it divides evenly, it is always "C" phase.
Let's say you have circuit number 27. Divide it by 6 . Six will go into 27 four times with a reaminder of 3 . Normally, panels are labeled with the odds on the left and the evens on the right (see Panel layout example below). So 3 is the second one down from the top on the odd side; therefore it will be " B " phase. Let's try another one. Say your circuit number is 50 . Divide it by 6 . Six will go into 50 eight times with a remainder of 2 . So the correct phase for circuit 50 will be "A" phase. This is taken in consideration if the three-phase system is phased $\mathrm{A}, \mathrm{B}, \mathrm{C}$ left to right, top to bottom. This is the normal phasing of a system.


### 12.3.0 Typical Wattage of Various Appliances

Here are some examples of the range of nameplate wattages for various household appliances:

- Aquarium $=50-1210$ Watts
- Clock radio $=10$
- Coffee maker $=900-1200$
- Clothes washer $=350-500$
- Clothes dryer $=1800-5000$
- Dishwasher $=1200-2400$ (using the drying feature greatly increases energy consumption)
- Dehumidifier $=785$
- Electric blanket- Single/Double $=60 / 100$
- Fans
- Ceiling $=65-175$
- Window $=55-250$
- Furnace $=750$
- Whole house $=240-750$
- Hair dryer $=1200-1875$
- Heater $($ portable $)=750-1500$
- Clothes iron $=1000-1800$
- Microwave oven $=750-1100$
- Personal computer
- CPU - awake/asleep $=120 / 30$ or less
- Monitor - awake/asleep $=150 / 30$ or less
- Laptop $=50$
- Radio (stereo) $=70-400$
- Refrigerator (frost-free, 16 cubic feet) $=725$
- Televisions (color)
- $19^{\prime \prime}=65-110$
- $27^{\prime \prime}=113$
- $36^{\prime \prime}=133$
- $53^{\prime \prime}-61^{\prime \prime}$ Projection $=170$
- Flat screen $=120$
- Toaster $=800-1400$
- Toaster oven $=1225$
- $\mathrm{VCR} / \mathrm{DVD}=17-21 / 20-25$
- Vacuum cleaner $=1000-1440$
- Water heater $(40$ gallon $)=4500-5500$
- Water pump $($ deep well $)=250-1100$
- Water bed (with heater, no cover) $=120-380$


### 12.3.1 Typical Start-up and Running Wattage for Tools and Equipment

| Tool | Start-up Wattage | Running Wattage |
| :---: | :---: | :---: |
| Air compressor-1/2 HP | 2000 | 1000 |
| Air compressor-1 HP | 4500 | 1500 |
| Air compressor-2 HP | 7700 | 2800 |
| Battery charger | 0 | 3000 |
| Chainsaw, electric-1/2 HP | 0 | 900 |
| Drill, 1/4" -3 amps | 400 | 300 |
| Drill, 3/8" - 4 amps | 600 | 440 |
| Drill, 1/2 "-5.4 amps | 900 | 600 |
| Floor polisher-16", 3/4 HP | 3100 | 1400 |
| Grinder, Bench-6" | 1000 | 720 |
| Grinder, Bench-10" | 3600 | 1600 |
| Hammer, demolition | 3300 | 1900 |
| Hedge trimmer-18 ${ }^{\prime \prime}$ | 0 | 400 |
| Impact wrench-1/2" | 1200 | 750 |
| Impact wrench-1" | 2400 | 1400 |
| Mixer, 55 gallons-1/4 HP | 1200 | 700 |
| Pump, submersible-400 GPH | 400 | 200 |
| Pump, submersible-900 GPH | 400 | 500 |
| Saw, band-14" | 1400 | 1100 |
| Saw, circular-8 1/4" heavy duty | 3000 | 1800 |
| Saw, mitre-10 amp | 2000 | 1100 |
| Saw, mitre-15 amp | 3000 | 1650 |
| Saw, Table-9" radial | 3000 | 1500 |
| Saw, Table-10" radial | 4500 | 1800 |



The above start-up and running wattage will vary according to model, make, and efficiency of the appliance and tool, and these calculations are based on an average and are to be viewed as "approximations" and not as a way to determine wiring or circuitry sizes.

Wattage Requirement-Typical Both for Start-up and for Running Appliances and Residential Equipment

| Equipment | Start-Up Wattage | Running Wattage |
| :--- | :---: | :---: |
| Blanket, electric | 0 | 400 |
| Blender | 0 | 200 |
| Bread maker | 2300 | 600 |
| Broiler, electric | 0 | 1350 |
| Broom, electric | 0 | 500 |
| CD Player/speaker | 0 | 100 |
| Clothes dryer-gas | 720 | 650 |
| Clothes dryer-electric | 1800 | 750 |
| Coffee maker | 000 | 550 |
| Computer | 0 | $720-900$ |
| Computer printer | 0 | $350-720$ |
| Copy machine | 800 | 1600 |
| Dehumidifier | 1400 | 650 |
| Dishwasher, cool dry | 1400 | 700 |
| Dishwasher, hot dry | 2200 | 1450 |
| Freezer | 700 |  |


| Equipment | Start-Up Wattage | Running Wattage |
| :---: | :---: | :---: |
| Fry pan, electric | 0 | 1300 |
| Furnace | 500-2350 | 300-875 |
| Garage door opener | 1100-1400 | 550-725 |
| Hair dryer | 0 | 900 |
| Hot tub heater | 1700 | 1700 |
| Hot tub pump | 950 | 800 |
| Iron | 0 | 1000 |
| Light, incandescent | 0 | 100 |
| Light, fluorescent | 125 | 90 |
| Light, flood | 0 | 500 |
| Microwave oven | 800 | 625 |
| Range, electric-6" ${ }^{\prime \prime}$ elements | 0 | 1500 |
| Range, electric- $8^{\prime \prime}$ elements | 0 | 1200 |
| Refrigerator | 2200 | 700 |
| Toaster-2 slice | 0 | 1050 |
| Toaster- 4 slice | 0 | 1650 |
| Pump, sump- $1 / 2 \mathrm{hp}$ | 2150 | 1050 |
| Pump, sump-1/3 hp | 1300 | 800 |
| Radio | 0 | 200 |
| Security alarm panel | 0 | 200 |
| Space heater (average) | 0 | 800 |
| Television | 0 | 300 |
| Vacuum cleaner | 0 | 1100 |
| Washer-Dryer | 3000 | 2000 |

### 12.3.2 Estimating Appliance and Home Electronic Energy Use

## Source: U.S. Department of Energy

## Energy Savers

## Estimating Appliance and Home Electronic Energy Use

If you're trying to decide whether to invest in a more energy-efficient appliance or you'd like to determine your electricity loads, you may want to estimate appliance energy consumption.

## Formula for Estimating Energy Consumption

You can use this formula to estimate an appliance's energy use:
(Wattage $\times$ Hours Used Per Day) $\div 1000=$ Daily Kilowatt-hour $(k W h)$ consumption
( 1 kilowatt $(\mathrm{kW})=1,000$ Watts)
Multiply this by the number of days you use the appliance during the year for the annual consumption. You can then calculate the annual cost to run an appliance by multiplying the kWh per year by your local utility's rate per kWh consumed.

Note: To estimate the number of hours that a refrigerator actually operates at its maximum wattage, divide the total time the refrigerator is plugged in by three. Refrigerators, though turned "on" all the time, actually cycle on and off as needed to maintain interior temperatures.

## Examples

Window fan:
(200 Watts $\times 4$ hours/day $\times 120$ days/year) $\div 1000$
$=96 \mathrm{kWh} \times 8.5$ cents $/ \mathrm{kWh}$
$=\$ 8.16 /$ year
Personal Computer and Monitor:
$(120+150$ Watts $\times 4$ hours/day $\times 365$ days/year $) \div 1000$
$=394 \mathrm{kWh} \times 8.5$ cents $/ \mathrm{kWh}$
$=\$ 33.51 / \mathrm{year}$

## Wattage

You can usually find the wattage of most appliances stamped on the bottom or back of the appliance, or on its nameplate. The wattage listed is the maximum power drawn by the appliance. Since many appliances have a range of settings (for example, the volume on a radio), the actual amount of power consumed depends on the setting used at any one time.

If the wattage is not listed on the appliance, you can still estimate it by finding the current draw (in amperes) and multiplying that by the voltage used by the appliance. Most appliances in the United States use 120 volts. Larger appliances, such as clothes dryers and electric cooktops, use 240 volts. The amperes might be stamped on the unit in place of the wattage. If not, find a clamp-on ammeter-an electrician's tool that clamps around one of the two wires on the appliance-to measure the current flowing through it. You can obtain this type of ammeter in stores that sell electrical and electronic equipment. Take a reading while the device is running; this is the actual amount of current being used at that instant.

When measuring the current drawn by a motor, note that the meter will show about three times more current in the first second that the motor starts than when it is running smoothly.

### 12.3.3 Appliance Energy Use Chart Based on an Operating Cost of $\mathbf{\$ 0 . 0 9 5}$ per KWh Appliance Energy Use Chart

The Appliance Energy Use Chart presented below is designed to give you an idea of how much electricity is consumed by many of the most common household appliances. Except where noted, the figures used in the chart have been based on the typical efficiency levels of appliances found in Springfield homes audited by the CWLP Energy Experts and on the price per kilowatt-hour paid by the "average" CWLP residential customer. Appliances with efficiency levels much lower or higher than the norm might consume significantly more or less energy than indicated on this table.

To translate the usages given in this chart into energy dollars, simply multiply the appliance's kilowatt-hour ( kWh ) usage by your average price per kWh (see the following Note for more about this) and the amount or number of times you use the appliances over a specific period.

NOTE: Based on current electric rates and the State Utility Tax, plus the average fuel adjustment charge for the previous year, the average annual cost per kWh of electricity paid by CWLP's regular (not all-electric) residential electric customers is approximately $9.5 \notin$. For all-electric residential customers, the average annual cost is about $8.9 \notin$ per kWh. (Cost-per-kWh estimates were last updated September 30, 2008.)

For instance, using the average cost-per-kWh provided in the Note above and the energy consumption information provided in the Appliance Energy Use Chart, we can calculate that it will cost a regular (Rate 30) CWLP residential electric customer about $\$ 2.57$ a month to watch a 21 -inch color television for an average of three hours a day (approximately 90 hours each month).

$$
0.3 \mathrm{kwh} / \mathrm{hr} \times \$ 0.095 \text { per } \mathrm{kwh} \times 90 \mathrm{hrs} / \mathrm{mo} .=\$ 2.57 \text { per mo. }
$$

Source: City water Light and Power, Springfield, Illinois
An addition to helping you determine the approximate cost of operating your various appliances over time, the Appliance Energy Use Chart can help you realize how changes in your energy use habits-such as using appropriately sized stove burners, substituting a microwave oven for a conventional oven, or turning off lights, TVs, and other appliances when they aren't needed-can help you control your monthly energy costs.

Appliance Energy Use Chart
Appliance $\quad$ kWh Usage $\quad$ Operating Cost (@ 9.5 $/ \mathrm{kWh}$ )

Kitchen

| Toaster | $0.04 \mathrm{kWh} /$ serving | less than $1 \not \subset /$ serving |
| :--- | :--- | :--- |
| Microwave oven | $0.75 \mathrm{kWh} / \mathrm{hr}$ | $7 \notin / \mathrm{hr}$ |
| Electric frying pan | $1.2 \mathrm{kWh} / \mathrm{hr}$ | $11 \not \subset / \mathrm{hr}$ |
| Coffee maker | $0.2 \mathrm{kWh} / \mathrm{pot}$ | $2 \notin / \mathrm{pot}$ |
| Range burner (large) | $2.4 \mathrm{kWh} / \mathrm{hr}$ | $23 \not \subset / \mathrm{hr}$ |
| Range burner (small) | $1.2 \mathrm{kWh} / \mathrm{hr}$ | $11 \not \subset / \mathrm{hr}$ |
| Oven (baking or roasting) | $3.2 \mathrm{kWh} / \mathrm{hr}$ | $30 \not \subset / \mathrm{hr}$ |

## Appliance Energy Use Chart-Cont'd

| Appliance | kWh Usage | Operating Cost (@9.5¢/kWh) |
| :---: | :---: | :---: |
| Oven (broiling) | 3.6 kWh / hr | $34 ¢ / \mathrm{hr}$ |
| Oven (self-cleaning cycle) | $10 \mathrm{kWh} /$ clean | 95¢/ / clean |
| Refrigerator (pre-2002, manual defrost) | $63 \mathrm{kWh} /$ month | \$5.99 / month |
| Refrigerator (pre-2002, frost-free) | 168 kWh / month | \$15.96 / month |
| Refrigerator (2002 or newer) | $82 \mathrm{kWh} /$ month | \$7,79 / month |
| Deep freezer (frost free) | 1835 kWh / month | \$17.39 / month |
| Deep freezer (manual defrost) | $135 \mathrm{kWh} /$ month | \$12.83 / month |
| Dishwasher | $1 \mathrm{kWh} / \mathrm{load}$ | $9.5 ¢ /$ load |
| Living Room/Office/Family Room |  |  |
| Television (21-inch color) | 0.3 kWh / hr | $3 ¢ / \mathrm{hr}$ |
| Stereo | 0.15 kWh / hr | $1 ¢ / \mathrm{hr}$ |
| Computer with monitor (average) | $0.09 \mathrm{kWh} / \mathrm{hr}$ | $1 \phi / \mathrm{hr}$ |
| Computer with monitor (sleep mode) | $0.02 \mathrm{kWh} / \mathrm{hr}$ | less than $1 ¢ / \mathrm{hr}$ |
| Fan | 0.2 kWh / hr | $2 ¢ / \mathrm{hr}$ |
| Room space heater (1500 watt) | 1.5 kWh / hr | $14 \not \subset / \mathrm{hr}$ |
| Bedroom |  |  |
| Waterbed heater | $120 \mathrm{kWh} /$ month | \$11.40 / month |
| Electric blanket | $1 \mathrm{kWh} / \mathrm{night}$ | $9.5 \not \subset /$ night |
| Basement/Utility Room |  |  |
| Washing machine (excluding water) | $0.25 \mathrm{kWh} / \mathrm{load}$ | $2 \nless /$ load |
| Clothes dryer (electric) | 2.7 kWh / load | $35 ¢ /$ load |
| Water heater (for average family of 4) | $400 \mathrm{kWh} / \mathrm{month}$ | \$38.00 / month |
| Dehumidifier | 0.76 kWh / hr | $7 \phi / \mathrm{hr}$ |
| Air conditioner (central, 10 SEER) | $1.2 \mathrm{kWh} / \mathrm{hr} / \mathrm{ton}$ | $11 \not \subset / \mathrm{hr} /$ ton |
| Air conditioner (central, 14 SEER) | $0.85 \mathrm{kWh} / \mathrm{hr} / \mathrm{ton}$ MISCELLANEOUS | $8 ¢ / \mathrm{hr} /$ ton |
| Light bulb (100-watt incandescent) | 0.1 kWh / hr | $4 ¢ / 4 \mathrm{hrs}$ |
| Light bulb (25-watt CFL, 100-watt equiv.) | 0.25 kWh / hr | $1 ¢ / 4 \mathrm{hrs}$ |

12.3.4 Recommended Product Specifications Proposed by the State of Florida

## Appendix A

TABLE A-1 Summary of Recommended Product Standards for Florida

| Product | Annual <br> Average <br> Baseline <br> Energy <br> per <br> Product | Baseline <br> Energy <br> Units | Annual Operating Cost | Recommended Standard | Basis for Standard | Annual <br> Average <br> Energy <br> Savings <br> per <br> Product | Savings <br> Energy <br> Units |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Bottle-type water dispensers | 854 | kWh/yr | \$97.36 | Max. $1.2 \mathrm{kWh} /$ day standby energy | Energy Star and CEC Title 20 | 266 | kWh/yr |
| Commercial boilers | 9246 | therms/yr | \$19,877.83 | Min 0.81 thermal efficiency | Proposal to ASHRAE | 481 | therms/yr |
| Commercial hot food holding cabinets | 2402 | kWh/yr | \$273.83 | Max. idle energy rate 40 W/ $\mathrm{ft}^{3}$ | Energy Star and CEC Title 20 | 1815 | kWh/yr |
| Compact audio products | 64 | kWh/yr | \$7.34 | Max. 2.0 W standby energy | Energy Star and CEC Title 20 | 53 | kWh/yr |
| DVD players and recorders | 26 | kWh/yr | \$3.02 | Max 3.0 W standby energy | Energy Star and CEC Title 20 | 11 | kWh/yr |
| Televisions (added by FSEC)*** | 442 | kWh/yr | \$50.39 | Max 3.0 W standby, Calif. prop. Title 20 | Energy Star and CEC Title 20 | 215 | kWh/yr |
| Metal halide lamp fixtures | 2015 | kWh/yr | \$229.69 | Pulse-start ballast | Pulse-start ballast | 307 | kWh/yr |
| Residential pool heaters* | 1125 | therms/yr | \$2,418.75 | Min. 80\% thermal effic. \& electric ignition | DOE 2004 | 213.75 | therms/yr |
| Portable electric spas (hot | 2500 | kWh/yr | \$285.00 | Max. $5 \mathrm{~V}^{(2 / 3)}$ standby energy | CEC Title 20 | 250 | kWh/yr |
|  | 110 | fan kWh | \$12.54 | 2\% electricity ratio | GAMA/CEE specification | 28 | fan kWh |
| Residential furnaces and residential boilers* (baseline AFUE=80) | 165 | therms/yr | \$354.75 | Boilers min. 84 AFUE; Furnaces/nat. gas. Tier 1 min. 80 AFUE, Tier 2 min. 90 AFUE/oil min. 83 AFUE | For boilers, significant current sales; for Tier 1 nat. gas. furnaces, non-condensing max; for Tier 2 nat. gas. furnaces, condensing; for oil furnaces, significant current sales. | 1.98 | therms/yr |


| Residential pool pumping* | 4200 | kWh/yr | \$478.80 | No split-phase or capacitor start-induction run types; 2speeds; $2^{\prime \prime}$ pipe and motor sizing | 2-speed pump; friction losses | 2688 | kWh/yr |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Singe-voltage external AC to DC power supplies | 38 | kWh/yr | \$4.36 | Varies with size | CEC Title 20 (Tier 1) and other states' standards | 4.1 | kWh/yr |
| State-regulated incandescent reflector lamps | 209 | kWh/yr | \$23.88 | Varies with size | EPAct 1992 standard with MA exemptions | 61 | kWh/yr |
| Walk-in refrigerators and freezers | 18,859 | kWh/yr | \$2,149.94 | Typical installation from CEC case study | CEC Title 20 with a few modifications | 8,220 | kWh/yr |
| Ceiling fans (added by FSEC) | 317 | kWh/yr | \$36.14 | Airflow and lighting efficiency minimums | Energy Star | 142 | kWh/yr |
| Incandescent lighting (added by FSEC) | 55 | kWh/yr | \$6.27 | Phase-out of incandescent lighting | Three states have similar plan/ time line | 37 | kWh/yr |
| Res. water heaters (added by FSEC) | 2300 | kWh/yr | \$262.20 | Electric EF > 2 (phased in over time) | Technology is available | 1,150 | kWh/yr |
|  | 133 | therms/yr | \$285.95 | $\begin{aligned} & \text { Gas EF minimum EF } 0.80 \\ & (2012) \end{aligned}$ |  | 46 | therms/yr |

Note: All data supplied by ACEEE (American Council for an Energy Efficient Economy) except televisions, part of residential pool pumps, ceiling fans, incandescent lighting and electric water heaters supplied by FSEC (Florida Solar Energy Center).
*Original ACEEE data modified based on FSEC experience and judgment. **For televisions, average energy savings/unit includes $15 \%$ energy savings from interaction with Source: State of Florida

### 12.3.5 Appliance and Equipment Efficiency Ratings Explained-EER, SEER, COP, HSPF, AFUE Appliance and Equipment Efficiency Ratings

When referring to the efficiency of an appliance or energy system, we are actually talking about how much energy that system must use to perform a certain amount of work. The higher its energy consumption per unit of output, the less efficient the system is. For example, an air conditioner that requires 750 watts of electricity to provide 6000 Btu of cooling will be less efficient than one that can provide the same amount of cooling for only 500 watts. The most common ratings applied to energy systems are EER and SEER for most central cooling systems; COP for some heat pumps and chillers; HSPF for all-electric heat pumps in their heating modes; and AFUE for gas furnaces and boilers.

| Efficiency <br> Rating | Definition | Explanation |
| :---: | :---: | :---: |
| EER | $\ldots$... (Energy Efficiency Ratio) is the measure of how efficiently a cooling system will operate when the outdoor temperature is at a specific level (usually $95^{\circ} \mathrm{F}$ ). A higher EER means the system is more efficient. The term EER is most commonly used when referring to window and unitary air conditioners and heat pumps, as well as water-source and geothermal heat pumps. | The formula for calculating EER is: $\mathrm{EER}=\frac{\mathrm{Btu} / \mathrm{hr} \text { of cooling at } 95^{\circ}}{\text { watts used at } 95^{\circ}}$ <br> For instance, if you have a window air conditioner that draws 1500 watts of electricity to produce 12,000 Btu per hour of cooling when the outdoor temperature is $95^{\circ}$, it would have an EER of 8 (12,000 divided by 1500). A unit drawing 1200 watts to produce the same amount of cooling would have an EER of 10 and would be more energy efficient. <br> Using this same example, you can see how efficiency can affect a system's operating economy. First, you'll need to determine the total amount of electricity-measured in kilowatt-hours (kWh) -the unit will consume over a period of time. (A kilowatthour is defined as 1000 watts used for one hour. This is the measure by which your monthly utility bills are calculated.) <br> To do this, let's assume you operate your 8 EER window air conditioner-drawing 1500 watts at any given moment-for an average of 12 hours every day during the summer ( 1500 watts $\times$ 12 hours). At this rate, it will use 18,000 watt-hours or 18 kWh each day, leading to a total consumption of 540 kWh over the course of a 30 -day month ( $18 \mathrm{kWh} \times 30$ days). At a summer electric rate of $8.51 \notin$ per kWh , it would cost you about $\$ 46$ to operate that window air conditioner each month ( $540 \mathrm{kWh} \times$ $\$ 0.0851$ )—not including fuel adjustment and state utility tax. At the same time, the 1200-watt, 10 EER system, consuming 14.4 kWh per day and 432 kWh per month, would cost you about $\$ 37$, a $20 \%$ savings over the less efficient model. |
| SEER | ... (Seasonal Energy Efficiency Ratio) measures how efficiently a residential central cooling system (air conditioner or heat pump) will operate over an entire cooling season, as opposed to at a single outdoor temperature. SEER is calculated based on the total amount of cooling (in Btu) the system will provide over the entire season divided by the total number of watt-hours it will consume. | The formula for calculating SEER is: $\text { SEER }=\frac{\mathrm{hr} \text { of cooling at }}{\text { seasonal watt }- \text { hours used }}$ <br> As with EER, a higher SEER reflects a more efficient cooling system. <br> By federal law, every split cooling system manufactured in or imported into the United States today must have a seasonal energy efficiency ratio of at least 13.0. |
| COP | ... (Coefficient of Performance) is the measurement of how efficiently a heating or | COP can be calculated by two different methods. In the first, you divide the Btu of heat produced by the heat pump by the |

cooling system (particularly a heat pump in its heating mode and a chiller for cooling) will operate at a single outdoor temperature condition. When applied to the heating modes of heat pumps, the temperature condition is usually $47^{\circ} \mathrm{F}$.

## Explanation

Btu equivalent of electricity that is required to produce the heat. This formula is stated:
$\mathrm{COP}=\frac{\mathrm{Btu} \text { of heat produced at } 47^{\circ} \mathrm{F}}{\text { Btu worth of electricity used at } 47^{\circ} \mathrm{F}}$
For instance, let's assume a heat pump uses 4000 watts of electricity to produce $42,000 \mathrm{Btu}$ per hour ( $\mathrm{Btu} / \mathrm{hr}$ ) of heat when it is $47^{\circ}$ outside. To determine its COP, you would first convert the 4000 watts of electrical consumption into its Btu/hr equivalent by multiplying 4000 times 3.413 (the number of Btu in one watt-hour of electricity). Then, you would divide your answer- $13,648 \mathrm{Btu} / \mathrm{hr}$-into the $42,000 \mathrm{Btu} / \mathrm{hr}$ heat output. This would show your heat pump to have a $47^{\circ} \mathrm{F}$ COP of 3.08 .

This means that for every Btu of electricity the system uses, it will produce a little more than three Btu of heat when the outdoor temperature is $47^{\circ} \mathrm{F}$.

The second formula is most frequently used to determine chiller efficiency. Using this calculation method, you would divide 3.516 by the number of kilowatts (kW) per ton used by the system. This formula is stated:
$\mathrm{COP}=\frac{3.516}{\mathrm{~kW} / \mathrm{ton}}$
For example, a chiller that consumes 0.8 kW per ton of capacity would have a COP of 4.4 ( 3.516 divided by 0.8 ). On the other hand, a chiller that uses 0.5 kW per ton, would have a COP of 7 (3.516 divided by 0.5).

The higher the COP, the more efficient the system.
... (Heating Seasonal Performance Factor) is the measurement of how efficiently all residential and some commercial all-electric heat pumps will operate in their heating mode over an entire normal heating season. HSPF is determined by dividing the total number of Btu of heat produced over the heating season by the total number of watt-hours of electricity that is required to produce that heat.

The formula for calculating HSPF is:
HSPF $=\frac{\text { Btu of heat produced over the heating season }}{\text { watts }- \text { hours of electricity used over the season }}$
The higher the HSPF, the more efficient the system.
Most all-electric heat pumps installed in Springfield today probably have HSPFs in the 7.0 to 8.0 range, meaning they operate with seasonal efficiencies of anywhere from 205 to $234 \%$. (To convert the HSPF number into a percentage, divide the HSPF by 3.413 , the number of Btu in one watt-hour of electricity.) That means that, for every Btu worth of energy they use over the entire heating season, these systems will put out anywhere from 2.05 to 2.34 Btu of heat. Compare this to electric furnaces, which have nominal efficiencies of $100 \%$ (for each Btu worth of electricity used, they put out 1.0 Btu of heat), or new gas furnaces, which have efficiency ratings of about 80 to $97 \%$ (for each Btu worth of gas used, they put out 0.8 to 0.97 Btu of heat).
(NOTE: When comparing energy systems that use different primary fuel sources with different costs per Btu, it is important that you understand that higher operating efficiency is not necessarily equivalent to better operating economy. Although an electric furnace might work with greater efficiency than a gas furnace, it might or might not be more economical to operate. That will depend on the prices of electricity and natural gas.)

| Efficiency <br> Rating | Definition | Explanation |
| :---: | :---: | :---: |
| AFUE | ... (Annual Fuel Utilization Efficiency) is the measurement of how efficiently a gas furnace or boiler will operate over an entire heating season. The AFUE is expressed as a percentage of the amount of energy consumed by the system that is actually converted to useful heat. For instance, a $90 \%$ AFUE means that for every Btu worth of gas used over the heating season, the system will provide 0.9 Btu of heat. The higher the AFUE, the more efficient the system. | When comparing efficiencies of various gas furnaces, it is important to consider the AFUE, not the steady-state efficiency. Steady state refers to the efficiency of the unit when the system is running continuously, without cycling on and off. Since cycling is natural for the system over the course of the heating season, steady state doesn't give a true measurement of the system's seasonal efficiency. For instance, gas furnaces with pilot lights have steady-state efficiencies of 78 to $80 \%$, but seasonal efficiencies-AFUEs-closer to $65 \%$. <br> Virtually all gas forced-air furnaces installed in this area from the 1950s through the early 1980s had AFUEs of around 65\%. <br> Today, federal law requires most gas furnaces manufactured and sold in the United States to have minimum AFUEs of $78 \%$. (Mobile home furnaces and units with capacities under 45,000 Btu are permitted somewhat lower AFUEs.) Gas furnaces and boilers now on the market have AFUEs as high as $97 \%$. |

Source: City Water Light and Power, Springfield, ILL.

### 12.4.0 Electric Generators-Understanding Your Power Needs

In order to choose the right emergency power source and to size it properly, you need to understand something about the power requirements of the devices you plan to operate.

The basic unit of power measurement is the watt, and with an emergency power source there are two wattage ratings that are important: steady-state wattage and surge wattage. A normal 60 -watt incandescent light bulb requires, as you would expect, 60 watts, and it requires that wattage both when you turn it on and while it is running. A ceiling fan motor, on the other hand, might require 150 watts to get it started and 75 watts while it is running. That extra wattage to start the motor is called the surge wattage and is typical of anything that contains an electric motor. Here are the usual wattages of some of the devices found in a typical household:

| Device | Typical Wattage | Surge Wattage |
| :--- | ---: | ---: |
| Light bulb | 60 watts | 60 watts surge |
| Fan | 75 watts | 150 watts surge |
| Small black/white television | 100 watts | 150 watts surge |
| Color television | 300 watts | 400 watts surge |
| Home computer and monitor | 400 watts | 600 watts surge |
| Electric blanket | 400 watts | 400 watts surge |
| Microwave oven | 750 watts | 1,000 watts surge |
| Furnace fan | 750 watts | 1,500 watts surge |
| Refrigerator | 1,200 watts | 2,400 watts surge |
| Well pump | 2,400 watts | 3,600 watts surge |
| Electric water heater | 4,500 watts | 4,500 watts surge |
| Whole-house A/C or heat pump | 15,000 watts | 30,000 watts surge |

[^33]
### 12.4.1 Electric Generator and Power Generator Safety

When buying a diesel generator, make a list of the lights and equipment that will be running off the generator. Total the wattage requirements to determine the capacity of the generator. Compare wattage requirement and the price of the generator. If the generator is to be connected directly to the electrical system, then it is advisable to hire a qualified technician to install the transfer switch. Ensure that the generator has adequate storage capacity, longer usage time at a stretch, overload protection, and auto shut-off facility.

Portable generators are critical to have in emergencies or for use in areas where there is no traditional electricity. Always keep in mind that there are safety issues related to the proper use of the generators. Portable generators can cause electrocution if they are left in the rain or sitting in water. Keep the generator under a canopy where it is protected, but not totally enclosed. It must still have adequate ventilation. Never touch the generator when you are standing in water or your hands are wet. Never run extension cords through water of any kind. Keep all unauthorized people away from the unit while it is operating.

Make sure all extension cords are of high enough rating for whatever load they are to carry. Also check for fraying, exposed wires, or areas where the cord may be underneath something else and is hard to see. Many people will put cords under rugs or furniture, but this can harm the cord and hide any defects in the cord.

Check cords when the portable generator is operating to make sure they are not overheating. Overheating is an indication of either too much load for the cord or damaged wires inside the cord. Even though the cords may be rated for the load you have, if compressors are turning on and off, the load may be temporarily increasing beyond the rating.

Even with the best alternative system such as solar energy or wind energy, sometimes the weather may not cooperate and will land us in darkness when the main power source fails. To keep our power system running, a backup engine generator may be just the thing. For businesses with sensitive computer networks, or homes with critical medical equipment, a backup power system such as a diesel generator may be necessary even when grid power is available.

When buying a diesel generator, make a list of the lights and equipment that will be running off the generator. Total the wattage requirements to determine the capacity of the generator. Compare wattage requirement and the price of the generator. If the generator is to be connected directly to the electrical system, then it is advisable to hire a qualified technician to install the transfer switch. Ensure that the generator has adequate storage capacity, longer usage time at a stretch, overload protection, and auto shut-off facility.

Source: xgpower, Whitehouse, NJ.

### 12.4.2 Typical Specifications for a Residential Emergency Generator

Our generator packs plenty of power to run heavy-load equipment such as heaters or air compressors. Auto decompression ensures easy starting; dependable, maintenance-free ignition and brushless alternator; large, superquiet muffler; black, tubular steel frame and easy-to-read fuel gauge. This super-fuel-efficient generator gets up to 11 hours of continuous operation on a tank of gas.

Generator Specifications:

- Type: Brushless AVR—Single Phase
- AC Frequency : 60 HZ
- Maximum AC Output : 6700 Watts
- Rated AC Output : 6000 Watts
- Rated / Maximum AC Current : 50.0/55.0 Amps @ 120 Volts
- Driving Method : Direct
- Two 110 -volt and two 220 -volt outlets

Engine Specifications:

- Type : 4-Stroke, OHV, Air Cooled
- Rated Horsepower : 12HP @ 3600 rpm
- Bore $\times$ Stroke : $66 \mathrm{~mm} \times 50 \mathrm{~mm}$
- Displacement : 357cc/12HP
- Cooling : Forced Air
- Lubrication : Wet Sump
- Oil Capacity : 0.63 qt
- Starting Method : Recoil
- Ignition System : Transistor Controlled
- Fuel Tank Capacity : 5.5 gal (20.8 Liters)
- Recommended Fuel : Unleaded Regular
- Continuous Operating Hours : 11.3
- Dry Weight : $185 \mathrm{lb}(84 \mathrm{~kg})$
- Noise Level @ 25 ft : 73.5 dBA
- Package size: 27 in. $\mathrm{L} \times 21 \mathrm{in} . \mathrm{W} \times 22.5 \mathrm{in} . \mathrm{H}$

Source: xgpower, Whitehouse, NJ.

### 12.4.3 The Left-Hand Generator Rule

The left-hand generator rule can be used to determine the relationship of the motion of the conductor in a magnetic field to the direction of the induced current. To use the left-hand rule, place the thumb, forefinger, and center finger at right angles to each other. The forefinger points in the direction of the field flux, assuming that magnetic lines of force are in a direction of north and south. The thumb points in the direction of thrust, or movement of the conductor, and the center finger shows the direction of the current induced into the armature.

Here is an easier way to remember this:
Thumb $=$ Thrust
Forefinger $=$ Flux
Center finger $=$ Current
By permission: www.elec-toolbox.com

### 12.4.4 The Right-Hand Generator Rule

## Right-Hand Motor Rule

The right-hand generator rule is used to determine the rotation of the armature when the magnetic field polarity of the pole pieces and the direction of current flow through the armature are known. The thumb indicates the direction of thrust or movement of the armature. The forefinger indicates the direction of the field flux assuming that flux lines are in a direction of north to south, and the center finger indicates the direction of current flow through the armature.

Here is an easier way to remember this:
Thumb $=$ Thrust (direction of armature rotation)
Forefinger $=$ Field (direction of magnetic field)
Center finger $=$ Current (direction of armature current)
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### 12.5.0 Dielectrics and Dielectric Constants of Various Materials

Dielectrics are materials that do not conduct electricity well, if at all. Materials have different dielectric constants at room temperatures. A low dielectric constant of a material means that the material has a low ability to polarize and hold a charge. A high dielectric material is good at holding a charge and is therefore the preferred dielectric for electronic capacitors.

This chart lists the minimum and maximum dielectric constant of various materials, and these values change as temperatures change.

| Material | Min. | Max. |
| :---: | :---: | :---: |
| Air | 1 | 1 |
| Amber | 2.6 | 2.7 |
| Asbestos fiber | 3.1 | 4.8 |
| Bakelite | 5 | 22 |
| Barium Titanate | 100 | 1250 |
| Beeswax | 2.4 | 2.8 |
| Cambric | 4 | 4 |
| Carbon Tetrachloride | 2.17 | 2.17 |
| Celluloid | 4 | 4 |
| Cellulose Acetate | 2.9 | 4.5 |
| Durite | 4.7 | 5.1 |
| Ebonite | 2.7 | 2.7 |
| Epoxy Resin | 3.4 | 3.7 |
| Ethyl Alcohol | 6.5 | 25 |
| Fiber | 5 | 5 |
| Formica | 3.6 | 6 |
| Glass | 3.8 | 14.5 |
| Glass Pyrex | 4.6 | 5 |
| Gutta Percha | 2.4 | 2.6 |
| Isolantite | 6.1 | 6.1 |
| Kevlar | 3.5 | 4.5 |
| Lucite | 2.5 | 2.5 |
| Mica | 4 | 9 |
| Micarta | 3.2 | 5.5 |
| Mycalex | 7.3 | 9.3 |
| Neoprene | 4 | 6.7 |
| Nylon | 3.4 | 22.4 |
| Paper | 1.5 | 3 |
| Paraffin | 2 | 3 |


| Material | Min. | Max. |
| :---: | :---: | :---: |
| Plexiglass | 2.6 | 3.5 |
| Polycarbonate | 2.9 | 3.2 |
| Polyethylene | 2.5 | 2.5 |
| Polyimide | 3.4 | 3.5 |
| Polystyrene | 2.4 | 3 |
| Porcelain | 5 | 6.5 |
| Quartz | 5 | 5 |
| Rubber | 2 | 4 |
| Ruby Mica | 5.4 | 5.4 |
| Selenium | 6 | 6 |
| Shellac | 2.9 | 3.9 |
| Silicone | 3.2 | 4.7 |
| Slate | 7 | 7 |
| Soil dry | 2.4 | 2.9 |
| Steatite | 5.2 | 6.3 |
| Styrofoam | 1.03 | 1.03 |
| Teflon | 2.1 | 2.1 |
| Titanium Dioxide | 100 | 100 |
| Vaseline | 2.16 | 2.16 |
| Vinylite | 2.7 | 7.5 |
| Water distilled | 34 | 78 |
| Waxes, Mineral | 2.2 | 2.3 |
| Wood dry | 1.4 | 2.9 |

Version 1.1.2

### 12.5.1 Wire Gauges Table—AWG Gauge—Ft/Ohm Calculations

Wire Gauges Information

| Gauge AWG | Ft. / Ohm <br> @ $77{ }^{\circ} \mathrm{F}$ | Ohms/1000 ft @ $77{ }^{\circ} \mathrm{F}$ | Ft/Ohm <br> @ $149^{\circ} \mathrm{F}$ | Ohms/1000 ft @ $149^{\circ}$ F | AMP <br> @ $140^{\circ} \mathrm{F}$ | Dia. in mils <br> (1000th in) | Dia. in mm | Wt. in Lbs. per 1000 ft |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0000 | 20000 | 0.050 | 17544 | 0.057 | 195 | 460.0 | 11.684 | 641.0 |
| 000 | 15873 | 0.063 | 13699 | 0.073 | 165 | 410.0 | 10.414 | 508.0 |
| 00 | 12658 | 0.079 | 10870 | 0.092 | 145 | 365.0 | 9.271 | 403.0 |
| 0 | 10000 | 0.100 | 8621 | 0.116 | 125 | 325.0 | 8.255 | 319.0 |
| 1 | 7936 | 0.126 | 6849 | 0.146 | 110 | 289.0 | 7.348 | 253.0 |

Wire Gauges Information-Cont'd

| Gauge AWG | Ft. / Ohm <br> @ $77{ }^{\circ} \mathrm{F}$ | Ohms/1000 ft @ $77{ }^{\circ} \mathrm{F}$ | $\begin{aligned} & \mathrm{Ft} / \mathrm{Ohm} \\ & @ 149^{\circ} \mathrm{F} \end{aligned}$ | Ohms/1000 ft @ $149^{\circ} \mathrm{F}$ | AMP <br> @ $140^{\circ} \mathrm{F}$ | Dia. in mils (1000th in) | Dia. in mm | Wt. in Lbs. per 1000 ft |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 2 | 6289 | 0.159 | 5435 | 0.184 | 95 | 258.0 | 6.544 | 201.0 |
| 3 | 4975 | 0.201 | 4310 | 0.232 | 85 | 229.0 | 5.827 | 159.0 |
| 4 | 3953 | 0.253 | 3425 | 0.292 | 70 | 204.0 | 5.189 | 126.0 |
| 5 | 3135 | 0.319 | 2710 | 0.369 |  | 182.0 | 4.621 | 100.0 |
| 6 | 24811 | 0.403 | 2151 | 0.465 | 55 | 162.0 | 4.115 | 79.5 |
| 7 | 968 | 0.508 | 1706 | 0.586 |  | 144.0 | 3.665 | 63.0 |
| 8 | 1560 | 0.641 | 1353 | 0.739 |  | 128.0 | 3.264 | 50.0 |
| 9 | 1238 | 0.808 | 1073 | 0.932 | 40 | 114.0 | 2.906 | 39.6 |
| 10 | 980.4 | 1.02 | 847.5 | 1.18 | 30 | 102.0 | 2.588 | 31.4 |
| 11 | 781.3 | 1.28 | 675.7 | 1.48 |  | 91.0 | 2.305 | 24.9 |
| 12 | 617.3 | 1.62 | 534.8 | 1.87 | 25 | 81.0 | 2.053 | 19.8 |
| 13 | 490.2 | 2.04 | 423.7 | 2.36 |  | 72.0 | 1.828 | 15.7 |
| 14 | 387.6 | 2.58 | 336.7 | 2.97 | 20 | 64.0 | 1.628 | 12.4 |
| 15 | 307.7 | 3.25 | 266.7 | 3.75 |  | 57.0 | 1.450 | 9.86 |
| 16 | 244.5 | 4.09 | 211.4 | 4.73 |  | 51.0 | 1.291 | 7.82 |
| 17 | 193.8 | 5.16 | 167.8 | 5.96 |  | 45.0 | 1.150 | 6.2 |
| 18 | 153.6 | 6.51 | 133.2 | 7.51 | - | 40.0 | 1.024 | 4.92 |
| 19 | 121.8 | 8.21 | 105.5 | 9.48 |  | 36.0 | 0.912 | 3.90 |
| 20 | 96.2 | 10.4 | 84.0 | 11.9 |  | 32.0 | 0.812 | 3.09 |
| 21 | 76.3 | 13.1 | 66.2 | 15.1 |  | 28.5 | 0.723 | 2.45 |
| 22 | 60.6 | 16.5 | 52.6 | 19.0 |  | 25.3 | 0.644 | 1.94 |
| 23 | 48.1 | 20.8 | 41.7 | 24.0 | - | 22.6 | 0.573 | 1.54 |
| 24 | 38.2 | 26.2 | 33.1 | 30.2 |  | 20.1 | 0.511 | 1.22 |
| 25 | 30.3 | 33.0 | 26.2 | 38.1 |  | 17.9 | 0.455 | 0.970 |
| 26 | 24.0 | 41.6 | 20.8 | 48.0 |  | 15.9 | 0.405 | 0.769 |
| 27 | 19.0 | 52.5 | 16.5 | 60.6 |  | 14.2 | 0.361 | 0.610 |
| 28 | 15.1 | 66.2 | 13.1 | 76.4 | - | 12.6 | 0.321 | 0.484 |
| 29 | 12.0 | 83.4 | 10.4 | 96.3 |  | 11.3 | 0.286 | 0.384 |
| 30 | 9.5 | 105 | 8.3 | 121 |  | 10.0 | 0.255 | 0.304 |
| 31 | 7.5 | 133 | 6.5 | 153 |  | 8.9 | 0.227 | 0.241 |
| 32 | 6.0 | 167 | 5.2 | 193 |  | 8.0 | 0.202 | 0.191 |
| 33 | 4.7 | 211 | 4.1 | 243 | - | 7.1 | 0.180 | 0.152 |
| 34 | 3.8 | 266 | 3.3 | 307 |  | 6.3 | 0.160 | 0.120 |
| 35 | 3.0 | 335 | 2.6 | 387 |  | 5.6 | 0.143 | 0.095 |
| 36 | 2.4 | 423 | 2.0 | 488 |  | 5.0 | 0.127 | 0.0757 |
| 37 | 1.9 | 533 | 1.6 | 616 |  | 4.5 | 0.113 | 0.0600 |
| 38 | 1.5 | 673 | 1.3 | 776 |  | 4.0 | 0.101 | 0.0476 |
| 39 | 1.2 | 848 | 1.0 | 979 | - | 3.5 | 0.090 | 0.0377 |
| 40 | 0.93 | 1070 | 0.81 | 1230 |  | 3.1 | 0.080 | 0.0200 |

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## Version 3.0.0

### 12.5.2 Wire Gauge Comparison Chart—AWG—Strandings/Wire Diameter, Overall Diameter

This table is the comparison chart for both AWG and metric wire sizes, including the makeup and construction of electrical wire. You can compare AWG to SWG and metric square millimeter cable sizes with the equivalent circular MIL conductor sizes.

Electrical Wire Gauge Comparison Table

| Circ. Mils | Equivalent Circ. Mils | Awg. Size | Metric Wire Size mm ${ }^{2}$ | Stranding/Wire Dia. per Strand |  | Approximate Overall Diameter |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | in | mm | in | mm |
| - | 987 | - | 0.50 | 1/.032 | 1/.813 | . 032 | 0.81 |
| 1020 | - | 20 | - | 7/.0121 | 7/.307 | . 036 | 0.91 |
| - | 1480 | - | 0.75 | 1/.039 | 1/.991 | . 039 | 0.99 |
| 1620 | - | 18 | - | 1/.0403 | 1/1.02 | . 040 | 1.02 |
| 1620 | - | 18 | - | $7 / .0152$ | $7 / .386$ | . 046 | 1.16 |
| - | 1974 | - | 1.0 | 1/.045 | 1/1.14 | . 045 | 1.14 |
| - | 1974 | - | 1.0 | 7/.017 | 7/.432 | . 051 | 1.30 |
| 2580 | - | 16 | - | 1/. 0508 | 1/1.29 | . 051 | 1.29 |
| 2580 | - | 16 | - | 7/.0192 | 7/.488 | . 058 | 1.46 |
| - | 2960 | - | 1.5 | 1/.055 | 1/1.40 | . 055 | 1.40 |
| - | 2960 | - | 1.5 | 7/.021 | 7/5.33 | . 063 | 1.60 |
| 4110 | - | 14 | - | 1/.0641 | 1/1.63 | . 064 | 1.63 |
| 4110 | - | 14 | - | 7/.0242 | 7/.615 | . 073 | 1.84 |
| - | 4934 | - | 2.5 | 1/.071 | 1/1.80 | . 071 | 1.80 |
| - | 4934 | - | 2.5 | 7/.027 | 7/.686 | . 081 | 2.06 |
| 6530 | - | 12 | - | 1/.0808 | 1/2.05 | . 081 | 2.05 |
| 6530 | - | 12 | - | 7/.0305 | 7/.775 | . 092 | 2.32 |
| - | 7894 | - | 4 | 1/.089 | 1/2.26 | . 089 | 2.26 |
| - | 7894 | - | 4 | 7/.034 | 7/.864 | . 102 | 2.59 |
| 10380 | - | 10 | - | 1/. 1019 | 1/2.59 | . 102 | 2.59 |
| 10380 | - | 10 | - | 7/.0385 | $7 / .978$ | . 116 | 2.93 |
| - | 11840 | - | 6 | 1/.109 | 1/2.77 | . 109 | 2.77 |
| - | 11840 | - | 6 | 7/.042 | 7/1.07 | . 126 | 3.21 |
| 13090 | - | 9 | - | 1/.1144 | 1/2.91 | . 1144 | 2.91 |
| 13090 | - | 9 | - | 7/.0432 | 7/1.10 | . 130 | 3.30 |
| 16510 | - | 8 | - | 1/.1285 | 1/3.26 | . 128 | 3.26 |
| 16510 | - | 8 | - | 7/.0486 | 7/1.23 | . 146 | 3.70 |
| - | 19740 | - | 10 | 1/.141 | 1/3.58 | . 141 | 3.58 |
| - | 19740 | - | 10 | 7/.054 | 7/1.37 | . 162 | 4.12 |
| 20820 | - | 7 | - | 1/.1443 | 1/3.67 | . 144 | 3.67 |
| 20820 | - | 7 | - | 7/.0545 | 7/1.38 | . 164 | 4.15 |
| 26240 | - | 6 | - | 1/.162 | 1/4.11 | . 162 | 4.11 |
| 26240 | - | 6 | - | $7 / .0612$ | 7/1.55 | . 184 | 4.66 |
| - | 31580 | - | 16 | 7/.068 | 7/1.73 | . 204 | 5.18 |
| 33092 | - | 5 | - | 7/.0688 | 7/1.75 | . 206 | 5.24 |
| 41740 | - | 4 | - | $7 / .0772$ | 7/1.96 | . 232 | 5.88 |
| - | 49340 | - | 25 | 7/.085 | 7/2.16 | . 255 | 6.48 |
| - | 49340 | - | 25 | 19/.052 | 19/1.32 | . 260 | 6.60 |
| 52620 | - | 3 | - | 7/.0867 | 7/2.20 | . 260 | 6.61 |

Electrical Wire Gauge Comparison Table-Cont'd

| Circ. Mils | Equivalent Circ. Mils | Awg. Size | Metric Wire Size mm ${ }^{2}$ | Stranding/Wire Dia. per Strand |  | Approximate Overall Diameter |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | in | mm | in | mm |
| 66360 | - | 2 | - | 7/.0974 | 7/2.47 | . 292 | 7.42 |
| - | 69070 | - | 35 | 7/.100 | 7/2.54 | . 300 | 7.62 |
| - | 69070 | - | 35 | 19/.061 | 19/1.55 | . 305 | 7.75 |
| 83690 | - | 1 | - | 19/.0664 | 19/1.69 | . 332 | 9.43 |
| - | 98680 | - | 50 | 19/.073 | 19/1.85 | . 365 | 9.27 |
| 105600 | - | 1/0 | - | 19/.0745 | 19/1.89 | . 373 | 9.46 |
| 133100 | - | 2/0 | - | 19/.0837 | 19/2.13 | . 419 | 10.6 |
| - | 138100 | - | 70 | 19/.086 | 19/2.18 | . 430 | 10.9 |
| 167800 | - | 3/0 | - | 19/.094 | 19/2.39 | . 470 | 11.9 |
| 167800 | - | 3/0 | - | 37/.0673 | 37/1.71 | . 471 | 12.0 |
| - | 187500 | - | 95 | 19/. 101 | 19/2.57 | . 505 | 12.8 |
| - | 187500 | - | 95 | 37/.072 | 37/1.83 | . 504 | 12.8 |
| 211600 | - | 4/0 | - | 19/.1055 | 19/2.68 | . 528 | 13.4 |
| - | 237.8 mcm | - | 120 | 37/.081 | 37/2.06 | . 567 | 14.4 |
| 250 mcm | - | - | - | 37/.0822 | 37/2.09 | . 575 | 14.6 |
| 300 mcm | - | - | 150 | $37 / .090$ | 37/2.29 | . 630 | 16.0 |
| 350 mcm | - | - | - | 37/.0973 | 37/2.47 | . 681 | 17.3 |
| - | 365.1 mcm | - | 185 | 37/.100 | 37/2.54 | . 700 | 17.8 |
| 400 mcm | - | - | - | 37/.104 | 37/2.64 | . 728 | 18.5 |
| - | 473.6 mcm | - | 240 | $37 / .114$ | 37/2.90 | . 798 | 20.3 |
| - | 473.6 mcm | - | 240 | 61/.089 | 61/2.26 | . 801 | 20.3 |
| 500 mcm | - | - | - | 37/. 1162 | 37/2.95 | . 813 | 20.7 |
| 500 mcm | - | - | - | 61/.0905 | 61/2.30 | . 814 | 20.7 |
| - | 592.1 mcm |  | 300 | 61/.099 | 61/2.51 | . 891 | 22.6 |
| 600 mcm | - | - | - | 61/.0992 | 61/2.52 | . 893 | 22.7 |
| 700 mcm | - | - | - | 61/. 1071 | 61/2.72 | . 964 | 24.5 |
| 750 mcm | - | - | - | 61/.1109 | 61/2.82 | . 998 | 25.4 |
| 750 mcm | - | - | - | 91/.0908 | 91/2.31 | . 999 | 25.4 |
| - | 789.4 mcm |  | 400 | 61/. 114 | 61/2.90 | 1.026 | 26.1 |
| 800 mcm | - | - | - | 61/.1145 | 61/2.91 | 1.031 | 26.2 |
| 800 mcm | - | - | - | 61/.0938 | 91/2.38 | 1.032 | 26.2 |
| 1000 mcm | 986.8 mcm | - | 500 | 61/. 1280 | 61/3.25 | 1.152 | 29.3 |
| 1000 mcm | - | - | - | 91/. 1048 | 91/2.66 | 1.153 | 29.3 |
| - | 1233.7 mcm | - | 625 | 91/.117 | 91/2.97 | 1.287 | 32.7 |
| 1250 mcm | - | - | - | 91/.1172 | 91/2.98 | 1.289 | 32.7 |
| 1250 mcm | - | - | - | 127/.0992 | 127/2.52 | 1.290 | 32.8 |
| 1500 mcm | - | - | - | 91/.1284 | 91/.1284 | 1.412 | 35.9 |
| 1500 mcm | - | - | - | 127/.1087 | 127/.1087 | 1.413 | 35.9 |
| - | 1578.8 mcm | - | 800 | 91/.132 | 91/.132 | 1.452 | 36.9 |
| - | 1973.5 mcm | - | 1000 | 91/. 147 | 91/3.73 | 1.617 | 41.1 |
| 2000 mcm | - | - | - | 127/.1255 | 127/3.19 | 1.632 | 41.5 |
| 2000 mcm | - | - | - | 169/.1088 | 169/2.76 | 1.632 | 41.5 |

Version 1.0.1

### 12.5.3 Resistance in Ohms per 1000 Feet of Conductor-Aluminium and Copper

Resistance in Ohms per 1000 feet per conductor at $20^{\circ} \mathrm{C}$ and $25^{\circ} \mathrm{C}$ of solid wire and class B concentric strands copper and aluminum conductor

| Conductor Size, AWG or kcmil | Annealed Uncoated Copper Annealed Aluminum |  |  |  |  |  |  | Annealed Coated Copper |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Solid |  |  |  | Stranded Class B |  |  |  | Solid |  | Stranded Class B |  |
|  | $20^{\circ} \mathrm{C}$ |  | $25^{\circ} \mathrm{C}^{*}$ |  | $20^{\circ} \mathrm{C}$ |  | $25^{\circ} \mathrm{C}^{*}$ |  | $20^{\circ} \mathrm{C}$ | $25^{\circ}{ }^{*}$ | $20^{\circ} \mathrm{C}$ | $25^{\circ} C^{*}$ |
|  | CU | AL | CU | AL | CU | AL | CU | AL | CU | CU | $C U$ | $C U$ |
| 24 | 25.7000 | -... | 26.2000 | -... | -... | -... | -... | -... | 26.8000 | 27.3000 |  |  |
| 22 | 16.2000 | -... | 16.5000 | -... | -... | -... | -... | -... | 16.9000 | 17.2000 | -... |  |
| 20 | 10.1000 | -... | 10.3000 | -... | 10.30000 | - | 10.50000 | -... | 10.5000 | 10.7000 | 11.00000 | 11.20000 |
| 19 | 8.0500 | -... | 8.2100 |  | - | -... | -. | -... | 8.3700 | 8.5300 |  |  |
| 18 | 6.3900 | -... | 6.5100 | -... | 6.51000 | -... | 6.64000 | -... | 6.6400 | 6.7700 | 6.92000 | 7.05000 |
| 16 | 4.0200 | -... | 4.1000 | -... | 4.10000 | -... | 4.18000 | -... | 4.1800 | 4.2600 | 4.35000 | 4.44000 |
| 14 | 2.5200 | 4.1400 | 2.5700 | 4.220 | 2.57000 | -. | 2.62000 | - | 2.6200 | 2.6800 | 2.68000 | 2.73000 |
| 12 | 1.5900 | 2.6000 | 1.6200 | 2.660 | 1.62000 | 2.65000 | 1.65000 | 2.70000 | 1.6200 | 1.6800 | 1.68000 | 1.72000 |
| 10 | 0.9990 | 1.6400 | 1.0200 | 1.670 | 1.02000 | 1.67000 | 1.04000 | 1.70000 | 1.0400 | 1.0600 | 1.06000 | 1.08000 |
| 9 | 0.7920 | 1.3000 | 0.8080 | 1.320 | 0.80800 | 1.33000 | 0.82400 | 1.35000 | 0.8160 | 0.8310 | 0.84000 | 0.85700 |
| 8 | 0.6280 | 1.0300 | 0.6410 | 1.050 | 0.64100 | 1.05000 | 0.65400 | 1.07000 | 0.6460 | 0.6590 | 0.66600 | 0.67900 |
| 7 | 0.4980 | . 8170 | 0.5080 | . 833 | 0.51800 | . 83300 | 0.51800 | 0.85000 | 0.5130 | 0.5230 | 0.52800 | 0.53900 |
| 6 | 0.3950 | . 6480 | 0.4030 | . 661 | 0.40300 | . 66100 | 0.41000 | 0.67400 | 0.4070 | 0.4150 | 0.41900 | 0.42700 |
| 5 | 0.3130 | . 5140 | 0.3190 | . 524 | 0.32000 | . 52400 | 0.32600 | 0.53500 | 0.3230 | 0.3290 | 0.33300 | 0.33900 |
| 4 | 0.2480 | . 4070 | 0.2530 | . 415 | 0.25300 | . 41600 | 0.25900 | 0.42400 | 0.2560 | 0.2610 | 0.26400 | 0.26900 |
| 3 | 0.1970 | . 3230 | 0.2010 | . 330 | 0.20500 | . 33000 | 0.20500 | 0.33600 | 0.2030 | 0.2070 | 0.20900 | 0.21300 |
| 2 | 0.1560 | . 2560 | 0.1590 | . 261 | 0.15900 | . 26200 | 0.16200 | 0.26700 | 0.1610 | 0.1640 | 0.16600 | 0.16900 |
| 1 | 0.1240 | . 2030 | 0.1260 | . 207 | 0.12600 | . 20600 | 0.12900 | 0.21100 | 0.1280 | 0.1300 | 0.13100 | 0.13400 |
| 1/0 | 0.0982 | . 1610 | 0.1000 | . 164 | 0.10000 | . 16500 | 0.10200 | 0.16800 | 0.1010 | 0.1030 | 0.10400 | 0.10600 |
| 2/0 | 0.0779 | . 1280 | 0.0795 | . 130 | 0.07950 | . 13100 | 0.08110 | 0.13300 | 0.0798 | 0.0814 | 0.08270 | 0.08430 |
| 3/0 | 0.0618 | . 1010 | 0.0630 | . 103 | 0.06300 | . 10300 | 0.06420 | 0.10500 | 0.0633 | 0.0645 | 0.06560 | 0.06680 |
| 4/0 | 0.0490 | . 0803 | 0.0500 | . 082 | 0.05000 | . 08210 | 0.05090 | 0.08360 | 0.0502 | 0.0512 | 0.05150 | 0.05250 |
| 250 | -... | -... | -... | -... | 0.04230 | . 06950 | 0.04310 | 0.07080 | .. | -... | 0.04400 | 0.04490 |
| 300 | -.... | -... | -.... | -... | 0.03530 | . 05790 | 0.03600 | 0.05900 | -... | -... | 0.03670 | 0.03740 |
| 350 | -.... | -... | - | -... | 0.03020 | . 04960 | 0.03080 | 0.05050 | -.... | -... | 0.03140 | 0.03200 |
| 400 | . | -... | -... | -... | 0.02640 | . 04340 | 0.02700 | 0.04420 | -... | -.... | 0.02720 | 0.02780 |
| 500 | -.... | -... | -... | -... | 0.02120 | . 03480 | 0.02160 | 0.03540 | . | -... | 0.02180 | 0.02220 |
| 600 | -.... | -... | -.... | -... | 0.01760 | . 02900 | 0.01800 | 0.02950 | -.... | -... | 0.01840 | 0.01870 |
| 750 | . | - | - | -... | 0.01410 | . 02320 | 0.01440 | 0.02360 |  | -... | 0.01450 | 0.01480 |
| 1000 | -.... | -... | - | -... | 0.01060 | . 01740 | 0.01080 | 0.01770 | -.... | -.... | 0.01090 | 0.01110 |
| 1250 | $\ldots$ | .. | ... | -... | 0.00846 | . 01390 | 0.00863 | 0.01420 | $\ldots$ | -... | 0.00871 | 0.00888 |
| 1500 | .. | -... | -... | -... | 0.00705 | . 01160 | 0.00719 | 0.01180 | -.. | -.. | 0.00726 | 0.00740 |
| 1750 | -.... | -... | -.... | - | 0.00604 | . 00992 | 0.00616 | 0.01010 | -.... | - | 0.00622 | 0.00634 |
| 2000 |  | -... |  | -... | 0.00529 | . 00869 | 0.00539 | 0.00885 |  |  | 0.00544 | 0.00555 |
| 2500 | -.... | -... | -.... | -... | 0.00427 | . 00702 | 0.00436 | 0.00715 | -.... | -.... | 0.00440 | 0.00448 |

[^34]
### 12.5.3.1 Solid and Concentric Stranding of Class B and Class C Strandings

Compact and Compressed Diameters

| Conductor Size AWG or kcmil | Number of Wires | Compact Diameter (inches) | Compress Diameter (inches) |
| :---: | :---: | :---: | :---: |
| 8..... | 7..... | 0.134... | $0.141 .$. |
| 6.... | 7.... | 0.169... | 0.178.. |
| 4.... | 7.... | 0.213.. | 0.225.. |
| 2.... | 7.... | 0.268... | 0.283.. |
| 1..... | 19.... | 0.299... | 0.322.. |
| 1/0.... | 19.... | 0.336.. | 0.361.. |
| 2/0..... | 19.... | 0.376.. | 0.406.. |
| 3/0..... | 19.... | 0.423.. | 0.456.. |
| 4/0. | 19.... | 0.475.. | 0.512.. |
| 250... | $37 . \ldots$ | 0.520.. | 0.558.. |
| 300..... | 37. | 0.570.. | 0.611.. |
| 350.... | $37 . \ldots$ | 0.616.. | 0.661.. |
| 400..... | $37 \ldots$. | 0.659.. | 0.706.. |
| 500... | 37. | 0.736.. | 0.789.. |
| 600..... | $61 \ldots$. | 0.813.. | 0.866.. |
| 750.... | $61 \ldots$. | 0.908.. | 0.968.. |
| 800..... | $61 \ldots$. | 0.938.. | 1.000.. |
| 900.... | $61 \ldots$ | 0.999.. | 1.060.. |
| 1000.... | $61 \ldots$. | 1.060.. | 1.117.. |

Solid and Concentric Stranding

| Conductor <br> Size AWG <br> or <br> kcmil | Circular <br> Mil <br> Cross- <br> Sec- <br> tional <br> Area | Sq. MM | Solid |  |  | Class "B" Stranding |  |  | Class "C" Stranding |  |  | Conductor wgt. |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Conductor <br> Diameter <br> Mils | Conductor <br> Weight <br> (lb./M ft.) |  | Number of Wires | Diameter <br> of Each <br> Wire <br> Mils | Conductor Diameter (in.) | Number of Wires | Diameter of Each Wire Mils | Conductor Diameter (in.) | Class "B" \& "C" Strandings (lb./M ft.) |  |
|  |  |  |  | Aluminum | Copper |  |  |  |  |  |  | Aluminum | Copper |
| 24. | 404 | 0.205 | 20.1... | - | 1.22 | 7... | 7.6... | 0.023 . | -. | -.... |  | - | 1.24. |
| 22. | 640 | 0.324 | 25.3... |  | 1.94 | 7... | 9.6... | 0.029... | -... |  |  | - | 1.98. |
| 20.... | 1,020 | 0.519 | $32.0 \ldots$ | 0.942. | 3.10 | 7... | $12.1 \ldots$ | 0.036... |  | -... |  | - | 3.15 . |
| 19. | 1,290 | 0.653 | 35.0... | 1.19... | 3.90 | 7... | 13.6... | 0.041... | -... |  |  | - | 3.98. |
| 18.... | 1,620 | 0.823 | 40.3... | 1.49... | 4.92 | 7... | 15.2 . | 0.046... | -... | -.... | -. | - | 5.01.. |
| 16..... | 2,580 | 1.310 | 50.8... | 2.38... | 7.81 | 7... | 19.2... | 0.058... | -... | -.... | -... | - | 7.97. |
| 14.... | 4,110 | 2.080 | $64.1 \ldots$ | 3.78... | 12.44 | 7... | 24.2... | 0.073... | 19... | 14.7... | 0.074... | - | 12.7 . |
| 12.... | 6,530 | 3.310 | 80.8... | $6.01 \ldots$ | 19.77 | 7... | 30.5... | 0.092... | 19... | 18.5... | 0.093... | 6.13... | 20.2.. |
| 10.... | 10,380 | 5.260 | 101.9... | 9.56... | 31.43 | 7... | $38.5 \ldots$ | 0.116... | 19... | 23.4... | 0.117... | 9.75... | 32.0.. |
| 9.... | 13,090 | 6.630 | 114.4... | 12.04... | 39.63 | 7... | 43.2... | 0.130... | 19... | 26.2... | 0.131... | 12.30... | 40.4.. |
| 8.... | 16,510 | 8.370 | 128.5... | 15.20... | 50.00 | $7 \ldots$ | 48.6... | 0.146... | 19... | 29.5... | 0.146... | 15.15.. | 51.0.. |
| 7..... | 20,820 | 10.550 | 144.3... | 19.16... | 63.03 | 7... | 54.5... | 0.164... | 19... | 33.1 ... | 0.166.. | 19.16... | 64.2.. |
| 6.... | 26,240 | 13.300 | 162.0... | 24.15... | 79.40... | 7... | 61.2... | 0.184... | 19... | 37.2... | 0.186... | 24.60... | 81.0.. |
| 5.... | 33,090 | 16.670 | 181.9... | 30.45... | 100.20 | 7... | 68.8... | 0.206... | 19... | 41.7... | 0.209... | $31.10 \ldots$ | 102.0.. |
| 4..... | 41,740 | 21.150 | 204.3... | 38.44... | 126.40 | 7... | 77.2... | 0.232 . . | 19... | 46.9... | 0.235... | $39.20 \ldots$ | 129.0.. |
| 3.... | 52,620 | 26.670 | 229.4. | 48.43... | 159.30 | 7... | 86.7... | 0.260... | 19... | 52.6... | 0.263... | 49.40... | 162.0.. |
| 2..... | 66,360 | 33.620 | 257.6 | 61.07... | 200.90 | 7. | 97.4 | 0.292 | 19... | 59.1 . | 0.296... | 62.30 | 205.0.. |
| 1.... | 83,690 | 44.210 | 289.3... | 77.03... | 253.30 | 19... | 66.4... | 0.332 .. | $37 \ldots$ | 47.6... | 0.333... | 78.60... | 258.0.. |
| 1/0.... | 105,600 | 53.490 | 324.9... | 97.15... | 319.60 | 19... | 74.5... | 0.373... | $37 .$. | 53.4 . | 0.374... | 99.10... | 326.0. |
| 2/0.... | 133,100 | 67.430 | 364.8... | 122.50... | 402.90 | 19... | 83.7... | 0.419... | $37 \ldots$ | 60.0... | 0.420... | 125.00... | 411.0.. |
| 3/0.... | 167,800 | 85.010 | 409.6 . | 154.40... | 507.90 | 19... | 94.0... | 0.470 . | 37. | 67.3. | 0.471... | 157.00... | 518.0.. |
| 4/0.... | 211,600 | 107.200 | 460.0... | 194.70... | 640.50 | 19... | 105.5... | 0.528... | $37 .$. | 75.6... | 0.529... | 199.00... | 653.0.. |
| 250. | -... | 127.000 | - ... | - ... | - | $37 \ldots$ | $82.2 \ldots$ | 0.575... | $61 \ldots$ | 64.0... | 0.576... | 235.00... | 772.0.. |
| 300. | -... | 152.000 | -... |  | - | $37 \ldots$ | 90.0... | 0.630... | 61... | 70.1... | 0.631... | 282.00... | 926.0.. |
| 350..... | -... | 177.000 | -... |  | - | 37... | 97.3... | $0.681 \ldots$ | $61 .$. | 75.7... | $0.681 \ldots$ | 329.00... | 1081.0.. |
| 400..... | -... | 203.000 | - | -... | - | 37... | 104.0... | 0.728... | $61 \ldots$ | 81.0... | 0.729... | 376.00... | 1235.0.. |
| 500..... | -... | 253.000 | -... | -... | - | 37... | 116.2... | 0.813... | $61 \ldots$ | 90.5... | 0.815... | 469.00... | 1544.0.. |
| 600. | -... | 304.000 | -... | -... | - | $61 \ldots$ | 99.2... | 0.893... | 91... | 81.2... | 0.893... | 563.00... | 1853.0. |
| 750. | -... | 380.000 | -... | -... | - | $61 \ldots$ | 110.9... | 0.998... | 91... | 90.8... | 0.999... | 704.00... | 2316.0.. |
| 1000. | -... | 507.000 | -... |  | - | 61... | 128.0... | 1.152... | 91... | 104.8... | 1.153.. | 939.00. | 3088.0.. |
| 1250. | -... | 633.000 | - | -... | - | 91... | 117.2 . | 1.289... | 127. | 99.2 . | 1.290.. | 1173.00 . | 3859.0.. |
| 1500.... | -... | 760.000 | -... | -... | - | 91... | 128.4... | 1.412... | 127... | 108.7... | 1.413... | 1408.00... | 4631.0.. |
| 1750. | - | 887.000 | - ... | -... | - | 127. | 117.4 | 1.526... | 169... | 101.8... | 1.527. | 1643.00... | 5403.0.. |
| 2000.... | -... | 1010.000 |  | -... | - | 127... | 125.5... | 1.632... | 169... | 108.8... | 1.632... | 1877.00... | 6175.0.. |
| 2500.... | - ... | 1263.000 | -... | -. | - | 127.. | 140.3... | 1.824... | 169... | 121.6... | 1.824... | 2370.00... | 7794.0.. |

### 12.5.4 Copper to Aluminium Conversion Tables

## Copper to Aluminium Conversion Tables

3-Conductor $75^{\circ} \mathrm{C}$ Copper or Aluminum Circuitry (Without Ground Wire)

| Amps | Copper |  | Aluminum |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Wire Size | Conduit Size | Wire Size | Conduit Size |
| 20 | \#14 | 1/2" | \#12 | 1/2" |
| 25 | \#12 | 1/2" | \#10 | $3 / 4^{\prime \prime}$ |
| 35 | \#10 | 3/4" | \#8 | $3 / 4^{\prime \prime}$ |
| 50 | \#8 | $3 / 4^{\prime \prime}$ | \#6 | $1^{\prime \prime}$ |
| 65 | \#6 | $1^{\prime \prime}$ | \#4 | $11 / 4^{\prime \prime}$ |
| 85 | \#4 | $1{ }^{\prime \prime}$ | \#2 | $11 / 4^{\prime \prime}$ |
| 100 | \#3 | $11 / 4^{\prime \prime}$ | \#1 | $11 / 4^{\prime \prime}$ |
| 115 | \#2 | $11 / 4^{\prime \prime}$ | 1/0 | $11 / 2^{\prime \prime}$ |
| 130 | \#1 | $11 / 4^{\prime \prime}$ | 2/0 | $2^{\prime \prime}$ |
| 150 | 1/0 | $11 / 2^{\prime \prime}$ | 3/0 | $2^{\prime \prime}$ |
| 175 | 2/0 | $11 / 2^{\prime \prime}$ | 4/0 | $21 / 2^{\prime \prime}$ |
| 200 | 3/0 | $2^{\prime \prime}$ | 250 | $21 / 2^{\prime \prime}$ |
| 230 | 4/0 | $2^{\prime \prime}$ | 300 | $21 / 2^{\prime \prime}$ |
| 255 | 250 | $21 / 2^{\prime \prime}$ | 400 | $3^{\prime \prime}$ |
| 285 | 300 | $21 / 2^{\prime \prime}$ | 500 | $3^{\prime \prime}$ |
| 310 | 350 | $21 / 2^{\prime \prime}$ | 500 | $3^{\prime \prime}$ |
| 335 | 400 | $3^{\prime \prime}$ | 600 | $31 / 2^{\prime \prime}$ |
| 380 | 500 | $3^{\prime \prime}$ | 750 | $4 \prime$ |
| 420 | 600 | $3^{\prime \prime}$ | (2) 300 | $4^{\prime}$ |
| 460 | 700 | $31 / 2^{\prime \prime}$ | (2) 350 | $5{ }^{\prime \prime}$ |
| 475 | 750 | $4^{\prime \prime}$ | (2) 400 | $5^{\prime \prime}$ |

4-Conductor $75^{\circ} \mathrm{C}$ Copper or Aluminum Circuitry (Without Ground Wire)

|  | Copper |  |  | Aluminum |  |
| :--- | :---: | :---: | :---: | :---: | :---: |
| Amps | Wire Size | Conduit Size |  | Wire Size |  |
| 20 | $\# 14$ | $1 / 2^{\prime \prime}$ | $\# 12$ | Conduit Size |  |
| 25 | $\# 12$ | $1 / 2^{\prime \prime}$ | $\# 10$ | $1 / 2^{\prime \prime}$ |  |
| 35 | $\# 10$ | $3 / 4^{\prime \prime}$ | $\# 8$ | $3 / 4^{\prime \prime}$ |  |
| 50 | $\# 8$ | $3 / 4^{\prime \prime}$ | $\# 6$ | $3 / 4^{\prime \prime}$ |  |
| 65 | $\# 6$ | $1^{\prime \prime}$ | $\# 4$ | $1^{\prime \prime}$ |  |
| 85 | $\# 4$ | $11 / 4^{\prime \prime}$ | $\# 2$ | $11 / 4^{\prime \prime}$ |  |
| 100 | $\# 3$ | $11 / 4^{\prime \prime}$ | $\# 1$ | $1 / 4^{\prime \prime}$ |  |
| 115 | $\# 2$ | $11 / 4^{\prime \prime}$ | $1 / 0$ | $11 / 2^{\prime \prime}$ |  |
| 130 | $\# 1$ | $11 / 2^{\prime \prime}$ | $2 / 0$ | $2^{\prime \prime \prime}$ |  |
| 150 | $1 / 0$ | $11 / 2^{\prime \prime}$ | $3 / 0$ | $2^{\prime \prime}$ |  |
| 175 | $2 / 0$ | $2^{\prime \prime}$ | $4 / 0$ | $2^{\prime \prime}$ |  |
| 200 | $3 / 0$ | $2 \prime$ | 250 | $21 / 2^{\prime \prime}$ |  |
| 230 | $4 / 0$ | $21 / 2^{\prime \prime}$ | 300 | $21 / 2^{\prime \prime}$ |  |
| 255 | 250 | $3^{\prime \prime}$ | 400 | $3^{\prime \prime}$ |  |
| 285 | 300 |  | 500 | $3^{\prime \prime}$ |  |


| Amps | Copper |  | Aluminum |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Wire Size | Conduit Size | Wire Size | Conduit Size |
| 310 | 350 | 3 " | 500 | $31 / 2^{\prime \prime}$ |
| 335 | 400 | $3^{\prime \prime}$ | 600 | 4 " |
| 380 | 500 | $31 / 2^{\prime \prime}$ | 750 | $4 \prime$ |
| 420 | 600 | 4 " | (2) 300 | $4 \prime$ |
| $460$ | 700 | $4 \prime$ | $\text { (2) } 350$ | 5" |
| 475 | 750 | $4^{\prime \prime}$ | (2) 400 | 5" |

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3-Conductor $75^{\circ} \mathrm{C}$ Copper or Aluminum Circuitry (With Ground Wire)

| Amps | Wire Size | Copper |  | Aluminum |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Conduit Size | Ground | Wire Size | Conduit Size | Ground |
| 20 | \#14 | 1/2" | \#16 | \#12 | 1/2" | \#12 |
| 25 | \#12 | 1/2" | \#14 | \#10 | $3 / 4^{\prime \prime}$ | \#12 |
| 35 | \#10 | $3 / 4^{\prime \prime}$ | \#12 | \#8 | $3 / 4^{\prime \prime}$ | \#10 |
| 50 | \#8 | $3 / 4{ }^{\prime \prime}$ | \#10 | \#6 | $1^{\prime \prime}$ | \#8 |
| 65 | \#6 | $1{ }^{\prime \prime}$ | \#8 | \#4 | $11 / 4^{\prime \prime}$ | \#6 |
| 85 | \#4 | 1 " | \#8 | \#2 | $11 / 4^{\prime \prime}$ | \#6 |
| 100 | \#3 | $11 / 4^{\prime \prime}$ | \#8 | \#1 | $11 / 4^{\prime \prime}$ | \#6 |
| 115 | \#2 | $11 / 4^{\prime \prime}$ | \#6 | 1/0 | $11 / 2^{\prime \prime}$ | \#4 |
| 130 | \#1 | $11 / 4^{\prime \prime}$ | \#6 | 2/0 | $2^{\prime \prime}$ | \#4 |
| 150 | 1/0 | $11 / 2^{\prime \prime}$ | \#6 | 3/0 | $2^{\prime \prime}$ | \#4 |
| 175 | 2/0 | $11 / 2^{\prime \prime}$ | \#6 | 4/0 | $21 / 2^{\prime \prime}$ | \#4 |
| 200 | 3/0 | $2^{\prime \prime}$ | \#6 | 250 | $21 / 2^{\prime \prime}$ | \#4 |
| 230 | 4/0 | $2^{\prime \prime}$ | \#4 | 300 | $21 / 2^{\prime \prime}$ | \#2 |
| 255 | 250 | $21 / 2^{\prime \prime}$ | \#4 | 400 | 3 " | \#2 |
| 285 | 300 | $21 / 2^{\prime \prime}$ | \#4 | 500 | $3 \prime \prime$ | \#1 |
| 310 | 350 | $21 / 2^{\prime \prime}$ | \#3 | 500 | 3 " | \#1 |
| 335 | 400 | 3 " | \#3 | 600 | $31 / 2^{\prime \prime}$ | \#1 |
| 380 | 500 | 3 " | \#3 | 750 | 4 " | \#1 |
| 420 | 600 | 3 " | \#2 | (2) 300 | 4 " | 1/0 |
| 460 | 700 | $31 / 2^{\prime \prime}$ | \#2 | (2) 350 | 5" | 1/0 |
| 475 | 750 | $4 \prime$ | \#2 | (2) 400 | 5" | 1/0 |

4-Conductor $75^{\circ} \mathrm{C}$ Copper or Aluminum Circuitry (With Ground Wire)

| Amps | Wire Size | Copper |  | Aluminum |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Conduit Size | Ground | Wire Size | Conduit Size | Ground |
| 20 | \#14 | 1/2" | \#16 | \#12 | 1/2" | \#12 |
| 25 | \#12 | 1/2" | \#14 | \#10 | $3 / 4^{\prime \prime}$ | \#12 |
| 35 | \#10 | 3/4" | \#12 | \#8 | $3 / 4{ }^{\prime \prime}$ | \#10 |

4-Conductor $75^{\circ} \mathrm{C}$ Copper or Aluminum Circuitry (With Ground Wire) - Cont'd

| Amps | Wire Size | Copper |  | Aluminum |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Conduit Size | Ground | Wire Size | Conduit Size | Ground |
| 50 | \#8 | 3/4" | \#10 | \#6 | $1^{\prime \prime}$ | \#8 |
| 65 | \#6 | $1^{\prime \prime}$ | \#8 | \#4 | 1 1/4" | \#6 |
| 85 | \#4 | $11 / 4^{\prime \prime}$ | \#8 | \#2 | $11 / 4^{\prime \prime}$ | \#6 |
| 100 | \#3 | $11 / 4^{\prime \prime}$ | \#8 | \#1 | $11 / 2^{\prime \prime}$ | \#6 |
| 115 | \#2 | $11 / 4^{\prime \prime}$ | \#6 | 1/0 | $2^{\prime \prime}$ | \#4 |
| 130 | \#1 | $11 / 2^{\prime \prime}$ | \#6 | 2/0 | $2^{\prime \prime}$ | \#4 |
| 150 | 1/0 | $11 / 2^{\prime \prime}$ | \#6 | 3/0 | $2^{\prime \prime}$ | \#4 |
| 175 | 2/0 | $2^{\prime \prime}$ | \#6 | 4/0 | $21 / 2^{\prime \prime}$ | \#4 |
| 200 | 3/0 | $2^{\prime \prime}$ | \#6 | 250 | $21 / 2^{\prime \prime}$ | \#4 |
| 230 | 4/0 | $21 / 2^{\prime \prime}$ | \#4 | 300 | 3 " | \#2 |
| 255 | 250 | $21 / 2^{\prime \prime}$ | \#4 | 400 | $3^{\prime \prime}$ | \#2 |
| 285 | 300 | $3^{\prime \prime}$ | \#4 | 500 | $31 / 2^{\prime \prime}$ | \#1 |
| 310 | 350 | $3^{\prime \prime}$ | \#3 | 500 | $31 / 2^{\prime \prime}$ | \#1 |
| 335 | 400 | $3^{\prime \prime}$ | \#3 | 600 | 4 " | \#1 |
| 380 | 500 | $31 / 2^{\prime \prime}$ | \#3 | 750 | $4 \prime$ | \#1 |
| 420 | 600 | $4{ }^{\prime \prime}$ | \#2 | (2) 300 | $4 \prime$ | 1/0 |
| 460 | 700 | $4 \prime$ | \#2 | (2) 350 | 5" | 1/0 |
| 475 | 750 | $4^{\prime \prime}$ | \#2 | (2) 400 | $5^{\prime \prime}$ | 1/0 |

### 12.5.5 Conduit inside Diameters and Electrical Conductor Areas-U.S. to Metric Conversion <br> Metric Converstions

1 meter $=39.37$ inches
1 centimeter $=0.39$ inch
1 millimeter $=0.039$ inch
1 inch $=0.025$ meter
1 inch $=2.564$ centimeters
1 inch $=25.641$ millimeters

| Conduit Inside Diameters |  |  |
| :--- | :---: | :---: |
| Trade Size | Inches | Millimeters |
| $1 / 2$ | 0.622 | 15.8 |
| $3 / 4$ | 0.824 | 20.9 |
| 1 | 1.049 | 26.6 |
| $11 / 4$ | 1.380 | 35.0 |
| $11 / 2$ | 1.610 | 40.9 |
| 2 | 2.067 | 52.5 |
| $21 / 2$ | 2.469 | 62.7 |
| 3 | 3.068 | 77.9 |
| $31 / 2$ | 3.548 | 90.1 |
| 4 | 4.026 | 102.3 |
| 5 | 5.047 | 128.2 |
| 6 | 6.065 | 154.1 |

Electrical Conductor Areas United States

| Size (AWG) | Cir Mills (area) | Sq MM (area) |
| :--- | :---: | :---: |
| 18 | 1,620 | 0.82 |
| 16 | 2,580 | 1.30 |
| 14 | 4,110 | 2.08 |
| 12 | 6,530 | 3.30 |
| 10 | 10,380 | 5.25 |
| 8 | 16,510 | 8.36 |
| 6 | 26,240 | 13.29 |
| 4 | 41,740 | 21.14 |
| 3 | 52,620 | 26.65 |
| 2 | 66,360 | 33.61 |
| 1 | 83,690 | 42.39 |
| $1 / 0$ | 10,600 | 53.49 |
| $2 / 0$ | 133,100 | 67.42 |
| $3 / 0$ | 167,800 | 85.00 |
| $4 / 0$ | 211,600 | 107.19 |
| 250 | 250,000 | 126.64 |
| 300 | 300,000 | 151.97 |
| 350 | 350,000 | 177.3 |
| 400 | 400,000 | 202.63 |
| 500 | 500,000 | 253.29 |
|  |  |  |


| Closest European Sizes |  |
| :---: | :---: |
| Size-Sq MM | Area-Cir Mils |
| 0.75 | 1,480 |
| 1.00 | 1,974 |
| 1.50 | 2,961 |
| 2.50 | 4,935 |
| 4.00 | 7,896 |
| 6.00 | 11,844 |
| 10.0 | 19,740 |
| 16.0 | 31,584 |
| 25.0 | 49,350 |
| - | 69,090 |
| 35.0 | 98,700 |
| 50.0 | - |
| - | 138,180 |
| 70.0 | 187,530 |
| 95.0 | 236,880 |
| 120.0 | 296,100 |
| 150.0 | 365,190 |
| - | 473,760 |

### 12.5.6 Conduit Weight Comparisons—Rigid, EMT, PVC

(All weights are per 100 feet of conduit)

| Size | Rigid Steel | IMC | Rigid <br> Aluminum | EMT | PVC Sch 40 | PVC Sch 80 | PVC Coated* <br> Rigid Steel |
| :--- | ---: | ---: | :--- | ---: | ---: | ---: | ---: |
| $1 / 2^{\prime \prime}$ | 80 | 60 | 28 | 29 | 16 | 21 | 87 |
| $3 / 4^{\prime \prime}$ | 108 | 82 | 37 | 45 | 22 | 29 | 115 |
| $1^{\prime \prime}$ | 160 | 116 | 55 | 65 | 33 | 42 | 166 |
| $11 / 4^{\prime \prime}$ | 208 | 150 | 72 | 96 | 46 | 61 | 217 |
| $11 / 2^{\prime \prime}$ | 254 | 182 | 89 | 111 | 56 | 71 | 262 |
| $2^{\prime \prime}$ | 344 | 242 | 119 | 141 | 74 | 98 | 367 |
| $21 / 2^{\prime \prime}$ | 550 | 401 | 188 | 215 | 117 | 149 | 557 |
| $3^{\prime \prime}$ | 710 | 493 | 246 | 260 | 153 | 200 | 724 |
| $31 / 2^{\prime \prime}$ | 855 | 573 | 296 | 365 | 185 | 246 | 917 |
| $4^{\prime \prime}$ | 1000 | 683 | 350 | 390 | 219 | 292 | 1056 |
| $5^{\prime \prime}$ | 1335 | - | 479 | - | 298 | 400 | 1535 |
| $6^{\prime \prime}$ | 1845 | - | 630 | - | 385 | 510 | 2025 |

*20 mil thickness.
Note: The above weights were taken from several manufacturers' catalogues, include the threaded coupling when applicable, and have been rounded off to the closest whole number.

### 12.5.7 Recommended Power and Ground Cable Sizes-By Power and Distance

To calculate the proper power and ground cable sizes, find the distance of the power cable along the top row. If your measurement is between two measurements, use the higher one. Next, find the total power the cable must support on the left. If your measurement is between two measurements, use the higher one. The size listed where your two measurements meet is the recommended cable size.

Recommended Cable Size by Power and Distance

| Total RMS Power (watts) | Distance |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 feet | 8 feet | 12 feet | 16 feet | 20 feet |
| 100 | 10 gauge | 10 gauge | 8 gauge | 8 gauge | 4 gauge |
| 200 | 10 gauge | 8 gauge | 8 gauge | 4 gauge | 4 gauge |
| 400 | 8 gauge | 8 gauge | 4 gauge | 4 gauge | 4 gauge |
| 600 | 8 gauge | 4 gauge | 4 gauge | 4 gauge | 4 gauge |
| 800 | 4 gauge | 4 gauge | 4 gauge | 2 gauge | 2 gauge |
| 1000 | 4 gauge | 4 gauge | 2 gauge | 2 gauge | 2 gauge |
| 1400 | 4 gauge | 2 gauge | 2 gauge | 2 gauge | 2 gauge |
| Current Draw by Power |  | Power \& Ground Cable Specs |  |  |  |
| Total RMS Power (watts) | Current Amps | Cable Size | Gauge | Current | perage (amps) |
| 100 | 16 |  |  |  |  |
| 200 | 32 |  |  |  |  |
| 400 | 64 |  |  |  |  |
| 600 | 96 |  |  |  |  |
| 800 | 128 |  |  |  |  |
| 1000 | 160 |  |  |  |  |
| 1200 | 172 |  |  |  |  |
| 1400 | 188 |  |  |  |  |


|  | Distance |  |  |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: |
| Total RMS Power (watts) | $\mathbf{4}$ feet | $\mathbf{8}$ feet | $\mathbf{1 2}$ feet | $\mathbf{1 6}$ feet | $\mathbf{2 0}$ feet |
| 600 | 96 | 8 |  | 100 |  |
| 800 | 128 | 10 |  | 60 |  |
| 1000 | 160 | 12 |  | 20 |  |
| 1200 | 172 | 14 |  | 15 |  |
| 1400 | 188 | 16 |  |  |  |

Note: These figures have been rounded off for easy reference. Ground cables should be the same size (or larger) as the power cable.

### 12.6.0 Types of Transformers

## Transformer types

Transformers can be constructed so that they are designed to perform a specific function. A basic understanding of the various types of transformers is necessary to understand the role transformers play in today's nuclear facilities.

EO 1.4 STATE the applications of each of the following types of transformers:
a. Distribution
b. Power
c. Control
d. Auto
e. Isolation
f. Instrument potential
g. Instrument current

## Types of Transformers

Transformers are constructed so that their characteristics match the application for which they are intended. The differences in construction may involve the size of the windings or the relationship between the primary and secondary windings. Transformer types are also designated by the function the transformer serves in a circuit, such as an isolation transformer.

## Distribution Transformer

Distribution transformers are generally used in electrical power distribution and transmission systems. This class of transformer has the highest power, or volt-ampere ratings, and the highest continuous voltage rating. The power rating is normally determined by the type of cooling methods the transformer may use. Some commonly used methods of cooling are by using oil or some other heat-conducting material. Ampere rating is increased in a distribution transformer by increasing the size of the primary and secondary windings; voltage ratings are increased by increasing the voltage rating of the insulation used in making the transformer.

## Power Transformer

Power transformers are used in electronic circuits and come in many different types and applications. Electronics or power transformers are sometimes considered to be those with ratings of 300 volt-amperes and below. These transformers normally provide power to the power supply of an electronic device, such as in power amplifiers in audio receivers.

Source: Integrated Publishing.

### 12.6.1 Dry-Type Transformers—KVA Ratings-Single- and Three-Phase

| Single Phase |  |  |  |  |
| :--- | :---: | :---: | :---: | :---: |
|  |  | Amperes |  |  |
|  |  |  |  |  |
| KVA Rating | $\mathbf{1 2 0 V}$ | $\mathbf{2 4 0 V}$ | $\mathbf{4 8 0 V}$ |  |
| 1 | 8.33 | 4.16 | 2.08 |  |
| 1.5 | 12.5 | 6.24 | 3.12 |  |
| 2 | 16.66 | 8.33 | 4.16 |  |
| 3 | 25 | 12.5 | 6.1 |  |
| 5 | 41 | 21 | 10.4 |  |
| 7.5 | 62 | 31 | 15.6 |  |
| 10 | 83 | 42 | 21 |  |
| 15 | 124 | 62 | 31 |  |
| 25 | 208 | 104 | 52 |  |
| 37.5 | 312 | 156 | 78 |  |
| 50 | 416 | 208 | 104 |  |
| 75 | 624 | 312 | 156 |  |
| 100 | 830 | 415 | 207 |  |
| 167 | 1390 | 695 | 348 |  |
| 200 | 1660 | 833 | 416 |  |
|  |  |  |  |  |

Three Phase

|  | Amperes |  |  |  |
| :--- | ---: | ---: | ---: | :---: |
| KVA Rating | $\mathbf{2 0 8 V}$ | $\mathbf{2 4 0 V}$ | $\mathbf{4 8 0 V}$ | $\mathbf{6 0 0 V}$ |
| 3 | 8.3 | 7.2 | 3.6 | 2.9 |
| 6 | 16.6 | 14.4 | 7.2 | 5.8 |
| 9 | 25.0 | 21.6 | 10.8 | 8.7 |
| $\mathbf{1 5}$ | 41.6 | 36 | 18 | 14.4 |
| 30 | 83.0 | 72 | 36 | 28.8 |
| 45 | 125 | 108 | 54 | 43 |
| 75 | 208 | 180 | 90 | 72 |
| 112.5 | 312 | 270 | 135 | 108 |
| 150 | 415 | 360 | 180 | 144 |
| 200 | 554 | 480 | 240 | 192 |
| 225 | 625 | 540 | 270 | 216 |
| 300 | 830 | 720 | 360 | 288 |
| 400 | 1110 | 960 | 480 | 384 |
| 500 | 1380 | 1200 | 600 | 487 |
| 750 | 2080 | 1800 | 900 | 720 |
|  |  |  |  |  |

### 12.7.0 Enclosure Types for All Locations

## Industry Standards

## Enclosure Types for All Locations

National Electrical Manufacturers Association (NEMA Standard 250) NEMA/EEC to IEC
$\left.\begin{array}{|ll|}\hline \text { TYPE } & \text { Intended Use and Description } \\ \hline \text { Type 1 } & \begin{array}{l}\text { General-purpose enclosures are suitable for general-purpose application indoors, where atmospheric conditions are } \\ \text { normal. These enclosures serve as protection against falling dust, but are not dust tight. }\end{array} \\ \hline \text { Type 2 } & \begin{array}{l}\text { Driptight (indoor) enclosures are similar to NEMA } 1 \text { enclosures, with the addition for drip shields, and are suitable for } \\ \text { application where condensation may be severe, such as that encountered in cooling rooms or laundries. }\end{array} \\ \hline \text { Type 3 } & \begin{array}{l}\text { Dust, rain-proof, and sleet-resistant enclosures provide proper protection against windblown dust and weather } \\ \text { hazards such as rain, sleet, or snow. They are suitable for applications outdoors on ship docks, canal locks, } \\ \text { construction work, and for applications in subways and tunnels; use indoors where dripping water is a problem. }\end{array} \\ \hline \text { Type 4 } & \begin{array}{l}\text { Dust, rain-proof, and sleet-resistant enclosures provide proper protection against falling dirt and weather hazards } \\ \text { such as rain, sleet, or snow. They are suitable for applications outdoors on ship docks, canal locks, construction } \\ \text { work, and for applications in subways and tunnels; use indoors where dripping water is a problem. }\end{array} \\ \hline \text { Type 4X } & \begin{array}{l}\text { Water-tight enclosures are suitable for dairies, breweries, etc., where the enclosure may be subjected to large } \\ \text { amounts of water from any angle. (They are not submersible.) }\end{array} \\ \hline \text { processing plants, dairies, refineries, and other industries where corrosion is prominent. }\end{array}\right]$

Type 12 Industrial use enclosures are oil tight. Hammond type 12 enclosures meet JIC standard and also satisfy requirements of NEMA.
The cover is held in place with screws, bolts, or other suitable fasteners, with a continuous gasket construction. The fastener parts are held captive when the door is opened. There are no holes through the enclosures for mounting or attaching controls inside the enclosure, and no conduit knock-outs or openings.
Type 13 Mounting feet, brackets, or other mounting means are provided. These enclosures are suitable for application to machine tools and other industrial processing machines where oil, coolants, water, filings, dust, or lint may enter, seep into, or infiltrate the enclosure through mounting holes, unused conduit knockouts, or holes used for mounting equipment with the enclosure.

The preceding descriptions are not intended to be complete representations of the National Electrical Manufacturers Association standards for enclosures.

## Underwriter Laboratories Inc. (UL 50 and UL 508)

TYPE Intended Use and DescriptionType 1 Indoor use primarily to provide protection against contact with the enclosed equipment and against limited amountof falling dirt.
Type 2 Indoor use to provide a degree of protection against limited amounts of falling water and dirt.
Type 3 Outdoor use to provide a degree of protection against windblown dust and windblown rain; undamaged by the formation of ice on the enclosure.
Type 3R Outdoor use to provide a degree of protection against falling rain; undamaged by the formation of ice on the enclosure.
Type 4 Either indoor or outdoor use to provide a degree of protection against falling rain, splashing water, and hose-directed water; undamaged by the formation of ice on the enclosure.
Type 4 X Same as type 4 except this one is corrosion resistant.
Type 6 Indoor or outdoor use to provide a degree of protection against entry of water during temporary submersion at a limited depth; undamaged by formation of ice on the enclosure.
Type 12 Indoor use to provide a degree of protection against dust, dirt, fiber flyings, dripping water, and external condensation of noncorrosive liquids.
Type 13 Indoor use to provide a degree of protection against lint, dust seepage, external condensation and spraying of water, oil, and noncorrosive liquids.

Note: Page numbers followed by $f$ indicate figures and $t$ indicate tables.

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[^0]:    1. See Federal Register for July 1, 1959. See also next to last paragraph of 2.2.5.
    2. See Federal Register for July 1, 1959.
[^1]:    1. By action of the 12th General Conference on Weights and Measures (1964), the liter is a special name for the cubic decimeter.
[^2]:    2. This section lists units of measurement that have traditionally been used in the United States. In keeping with the Omnibus Trade and Competitiveness Act of 1988, the ultimate objective is to make the International System of Units the primary measurement system used in the United States.
    3. Squares and cubes of customary but not of metric units are sometimes expressed by the use of abbreviations rather than symbols. For example, sq ft means square foot, and cu ft means cubic foot.
[^3]:    8. One international foot $=0.999998$ survey foot (exactly)

    One international mile $=0.999998$ survey mile $($ exactly $)$

[^4]:    9. One square survey foot $=1.000004$ square international feet

    One square survey mile $=1.000004$ square international miles

[^5]:    9. One square survey foot $=1.000004$ square international feet

    One square survey mile $=1.000004$ square international miles

[^6]:    By permission: www.metric-conversions.org

[^7]:    * Chart is provided as a guideline only. Jobsite variables can affect machine performance.

[^8]:    Source: Prof. K. Reddy, University of Illinois, Chicago

[^9]:    *This Division Office is to be filled in by the DOT-FHWA division using this form. The form can be accsessed on www.fhwa.dot.gov/con struction/reviews/revconc1.cfm

[^10]:    *With $3 / 8^{\prime \prime}$ mortar joints (bed and vertical).

[^11]:    $8^{\prime \prime} \times 21 / 4^{\prime \prime} \times 33 / 4^{\prime \prime}$ standard brick with $1 / 4^{\prime \prime}$ vertical mortar joints in running bond

[^12]:    *Common bond is also called American bond.
    Do not confuse common bond with everyday running bond.

[^13]:    Source: Pacificmetral.com

[^14]:    **Add $1 / 4^{*}$ for extended cross bars. Deduct $1 / 16^{\prime \prime}$ for $1 / 8^{*}$ bearing bars. Standard panel widths indicated with white numbers.

[^15]:    *Based on 27.429 bars/ft of grating width. Bearing bars $7 / 16^{\prime \prime}$ c.c. Add $3 \mathrm{lbs} / \mathrm{sq}$ ft for $7-S G-2.1 / 8^{\prime \prime}$ bearing bars available by inquiry. Note: Grating for spans to the left of the heavy line has a

[^16]:    ${ }^{1}$ Standard lengths are $6^{\prime}(183 \mathrm{~cm})$ and longer in multiples of $1^{\prime}(31 \mathrm{~cm})$.
    Note on Metrics: Metric equivalents are provided for surfaced (actual) sizes.

[^17]:    ${ }^{a}$ Per $1 \%$ change in moisture content, based on dimension at $10 \%$ moisture content and a straight-line relationship between moisture content at which shrinkage starts and total shrinkage. (Shrinkage assumed to start at $30 \%$ for all species except those indicated by footnote b.)
    ${ }^{b}$ Shrinkage assumed to start at $22 \%$ moisture content.

[^18]:    ${ }^{a}$ Results of tests on small, clear, straight-grained specimens. Property values were taken from world literature (not obtained from experiments conducted at the Forest Products Laboratory). Other species may be reported in the world literature, as well as additional data on many of these species. Some property values have been adjusted to $12 \%$ moisture content.
    ${ }^{b}$ AF is Africa; AM, America; AS, Asia.

[^19]:    ${ }^{\text {a }}$ Results of tests on small, clear, straight-grained specimens. Property values were taken from world literature (not obtained from experiments conducted at the Forest Products Laboratory). Other species may be reported in the world literature, as well as additional data on many of these species. Some property values have been adjusted to $12 \%$ moisture content.
    ${ }^{\text {b }}$ AF is Africa; AM, America; AS, Asia.

[^20]:    ${ }^{a}$ Sanded on two sides.

[^21]:    ${ }^{a}$ From NPA (1993). Particleboard made with phenol-formaldehyde-based resins does not emit significant quantities of formaldehyde. Therefore, such products and other particleboard products made with resin without formaldehyde are not subject to formaldehyde emission conformance testing.
    ${ }^{b}$ Panels designated as "exterior adhesive" must maintain 50\% MOR after ASTM D1037 accelerated aging.
    ${ }^{c} M O R=$ modulus of rupture; $M O E=$ modulus of elasticity. $N S=$ not specified. $1 \mathrm{MPa}=145 \mathrm{lb} / \mathrm{in}^{2} ; 1 \mathrm{~N}=0.22 \mathrm{lb}$.
    ${ }^{d} H=$ density $>800 \mathrm{~kg} / \mathrm{m}^{3}\left(>50 \mathrm{lb} / \mathrm{ft}^{3}\right), M=$ density 640 to $800 \mathrm{~kg} / \mathrm{m}^{3}\left(40\right.$ to $\left.50 \mathrm{lb} / \mathrm{ft}^{3}\right)$. LD $=$ density $<640 \mathrm{~kg} / \mathrm{m}^{3}\left(<40 \mathrm{lb} / \mathrm{ft}^{3}\right)$. Grade M-S refers to medium density; "special" grade added to standard after grades $\mathcal{M}-1, \mathcal{M}-2$, and $\mathcal{M}-3$. Grade $\mathcal{M}-S$ falls between M-1 M-2 in physical properties.

[^22]:    By permission: Glasgow Steel Nail, Glasgow, Scotland

[^23]:    *Shank diameter is measured on the smooth portion of the screw above the threads.
    ${ }^{\dagger}$ Root diameter is measured between the threads and does not include the thread height. Source: D.Lawless Hardware- hingeddummy.info.

[^24]:    ${ }^{a}$ Assignment of an adhesive type to only one structural/service environment category does not exclude certain adhesive formulations from falling into the next higher or lower category.
    ${ }^{b}$ Priming wood surfaces with hydroxymethylated resorcinol coupling agent improves resistance to delamination of epoxy, isocyanate, emulsion polymer/isocyanate, melamine and urea, phenolic, and resorcinolic adhesives in exterior service environment, particularly bonds to CCA-treated lumber.

[^25]:    * Production to meet FHWA on customer request.

[^26]:    Source: U.S. Department of Energy.

[^27]:    Source:www.maconline.org

[^28]:    ${ }^{(a)} 4$-cubic foot bags required to fill.
    ${ }^{(b)} A$ standard $8^{\prime \prime} \times 16^{\prime \prime}$ block equals 0.89 sq. ft .
    Multiply the number of blocks times 0.89 to calculate the total square footage needed.

[^29]:    Absorption Coefficients (a) of Bldg.Matls and Finishes- p. 2
    Source: Sengpielaudio Sound Engineering Studio,Germany.

[^30]:    ${ }^{(1)}$ Spaced $7^{\prime \prime}$ on ceiling; 8' on wall. Reduce by $50 \%$ for adhesive/nail-on application.
    ${ }^{(2)}$ Spaced $12^{\prime \prime}$ on ceiling: $16^{\prime \prime}$ on wall.
    ${ }^{(3)}$ Coverage figures shown here approximate the amount of joint compound needed to treat the flat joints, inside corners and outside corners using metal corner bead, in a typical room. Coverage can vary widely depending on factors such as condition of substrate, tools used, application methods, and other job factors.

[^31]:    * $100 \%$ efficiency for using electricity in your house to produce heat is a common marketing ploy by electric utility companies. It is misleading because you have to burn about 3 units of primary fuel to deliver 1 unit of electric energy to the house because of the thermal bottleneck in electricity generation. So $100 \%$ efficient use at your house is about $33 \%$ efficient in the use of the primary fuel.

[^32]:    1. Ground-Source Heat Pumps, An Efficient Choice for Residential and Commercial Use, J. Mark Hannifan, Joe E King, AIA, 1995.
[^33]:    Source: xgpower,Whitehouse, NJ.

[^34]:    By permission: The Okonite Company

